

Date:09/2/2025

# **Post Install Letter**

Project Name: Mai Lee

Project Address: 335 GWENDOLYN WAY, Fuquay-Varina, NC, 27526

Contractor: Lifetime Energy

Attn: Whom it May Concern

Pursuant to your request, we have reviewed the installation of the above-referenced solar panel system. As you are aware, this office initially prepared a structural assessment, dated June 23, 2025. This install was inspected and found to be in compliance with the lay out plan as specified in our report, product installation criteria, and the requirements of the current building codes. The installation is in compliance with the North Carolina Residential Code Book, professional engineering assessment and judgment and covers this dwellings assessment for solar panel connections and support only. We have determined that the equipment will not create a negative impact on the building's structural design, including any additional loads imposed (dead, snow, wind).

This letter pertains only to the panel support attachments to the roof framing and not the engineered photovoltaic panel products, components, or electrical related installation connections. Should you have any questions regarding the above or if you require additional information on dont hesitate to contact me.

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Exp. Date: 12/31/2025 Date Certified and Signed: 09/2/2025



## **Solar Module Details**

Module Type	JA SOLAR
Module Quantity	51
Module Model Number	405 watt

## **Design Criteria**

## Code: 2018 North Carolina Residential Code ASCE 7-16

Live Load (psf)	20
Ult Wind Speed (mph)	145
Exposure Cat	С
Ground Snow (psf)	10

# **Structure Geometry**

	Eave Height, he (ft)	15 to 20ft
1	Pitch of main roof (deg)	20
1	Building Length, L (ft)	80
1	Building Width, B (ft)	50
1	Roof Area (Module Area) ft2	4000.00
;	Standoff(i.e., Roof Mount) Spacing Feet	4`

NOTE: attachments should be installed in a staggered configuration to properly destributor loading.

# **Roof Properties**

Roof Geometry type	Gable Roof
Roof Type	Truss
Roof Pitch (deg)	20



		<u>.</u>
	Roofing Type	Comp Shingles
	Sheathing Type	1/2" OSB Board
	Wood species	No. 2, Douglas Fir-Larch
	Wood Fb (psf)	900
	Wood Fv (psf)	180
	Wood E (psf)	1600000
	Purlin C/C Spacing (in)	12
	Rafter C/C. Spacing (in.)	24
Purlin		
-	Section Thickness, b (in.)	2
	Section Depth, d (in.)	4
Rafter		
	Section Thickness, b (in.)	2
	Section Depth, d (in.)	4
	Maximum Rafter Span (ft)	7
Factors		
	Cd(wind)	1.60
	Cd(Snow)	1.60
	CLS	1.15
	СМ	1
	Ct	1
	CL	0.75
	CF	1.5
	Cfu	1
	Cv	1
	Cr	1
	M allowable_wind	413.44
	M allowable_snow	297.16



# Dead Load(psf)

	Comp Shingles	3.00 psf
	1/2" OSB Board	2.00 psf
	Insulation	2.00 psf
	Total Roof 1DL	7 psf
	No. 2, Douglas Fir-Larch	31.00 lb/ft <sup>3</sup>
		'
	Solar Panel DL	3.00 psf
		•
		Roof 1
	Roof_Dist_DL	7.00 psf
	M_Roof_Dist_DL	3314.61
	Def_Roof_Dist_DL	1.97
		•
	PV_uni_Dist_DL	3.00 psf
	M_PV_uni_Dist_DL	10.54
	Def_PV_uni_Dist_DL	0.01
		•
	Total_Uni_DL	11.00 psf
	M_Total_DL	3325.15
	Def_Total_DL	1.98
Snow Load(psf)		
	Ground Snow Load, pg	10
	Importance Factor, Ic	1
	Thermal Factor, Ct	1
	Exposure Factor, Ce	1
	Flat roof snow, pF	10
	Slope Factor, Cs	1
	Sloped Row Snow, ps	10
		1
	Uni_Dist_S	10.00
	 M_uni_Dist_s	61.25
	Def_uni_Dist_S	0.04
	_	1



#### Notation

- $A = \text{Effective wind area, in } \text{ft}^2 \text{ (m}^2\text{)}.$
- $A_n$  = Normalized wind area, non-dimensional.
- $d_1$  = For rooftop solar array, horizontal distance orthogonal to the panel edge to an adjacent panel or the building edge, ignoring any rooftop equipment in Fig. 29.4-7, in ft (m).
- $d_2$  = For rooftop solar arrays, horizontal distance from the edge of one panel to the nearest edge in the next row in Fig. 29.4-7, in ft (m).
- h = Mean roof height of a building except that eave height shall be used for roof angle  $\theta$  less than or equal to  $10^{\circ}$ , in ft (m).
- $h_1$  = Height of the gap between the panels and the roof surface, in ft (m).
- $h_2$  = Height of a solar panel above the roof at the upper edge of the panel, in ft (m).
- $h_{pt}$  = Mean parapet height above the adjacent roof surface for use with Eq. (29.4-5), in ft (m).
- $\dot{L}_p$  = Panel chord length.
- $W_L$  = Width of a building on its longest side in Fig. 29.4-7, in ft (m).
- $W_S$  = Width of a building on its shortest side in Fig. 29.4-7, in ft (m).
- $\gamma_E$  = Array edge factor as defined in Section 29.4.4.
- $\theta$  = Angle of plane of roof from horizontal, in degrees.
- $\omega$  = Angle that the solar panel makes with the roof surface in Fig. 29.4-7, in degrees.

#### Notes

- 1.  $(GC_{\rm m})$  acts toward (+) and away (-) from the top surface of the panels.
- 2. Linear interpolation is allowed for  $\omega$  between 5° and 15°.
- 3.  $A_n = (1,000/[\text{max}(L_b,15)^2]\text{A}$ , where A is the effective wind area of the structural element of the solar panel being considered, and  $L_b$  is the minimum of  $0.4(h\text{W}_\text{L})^{0.5}$  or h or  $W_s$  in ft (m).

FIGURE 29.4-7 (Continued). Design Wind Loads (All Heights): Rooftop Solar Panels for Enclosed and Partially Enclosed Buildings, Roof  $\theta \le 7^\circ$ 

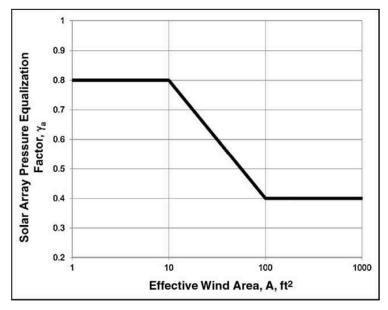


FIGURE 29.4-8 Solar Panel Pressure Equalization Factor,  $\gamma_a$ , for Enclosed and Partially Enclosed Buildings of All Heights

The roof shall be designed for both of the following:

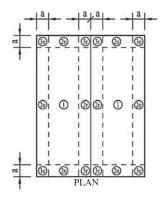
- The case where solar panels are present. Wind loads acting on solar collectors in accordance with this section shall be applied simultaneously with roof wind loads specified in other sections acting on areas of the roof not covered by the plan projection of solar collectors. For this case, roof wind loads specified in other sections need not be applied on
- areas of the roof covered by the plan projection of solar collectors.
- 2. Case where the solar panels have been removed.

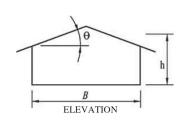
#### 29.5 PARAPETS

Wind loads on parapets are specified in Section 27.3.5 for buildings of all heights designed using the Directional Procedure



## Diagrams

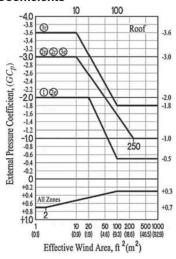


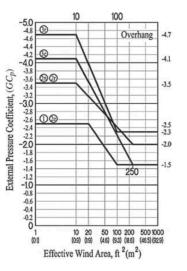


#### Notation

- a = 10% of least horizontal dimension or 0.4h, whichever is smaller, but not less than either 4% of least horizontal dimension or 3 ft (0.9 m). If an overhang exists, the edge distance shall be measured from the outside edge of the overhang. The horizontal dimensions used to compute the edge distance shall not include any overhang distances.
- B = Horizontal dimension of building measured normal to wind direction, in ft (m).
- $h = \text{Mean roof height, in ft (m), except that eave height shall be used for } \theta \le 10^{\circ}.$
- $\theta$  = Angle of plane of roof from horizontal, in degrees.

#### External Pressure Coefficients





## Notes

- 1. Vertical scale denotes  $(GC_P)$  to be used with  $q_h$ .
- 2. Horizontal scale denotes effective wind area, in  $ft^2$  (m<sup>2</sup>).
- 3. Plus and minus signs signify pressures acting toward and away from the surfaces, respectively.
- 4. Each component shall be designed for maximum positive and negative pressures.
- 5. Values of  $(GC_p)$  for roof overhangs include pressure contributions from both upper and lower surfaces.
- 6. If overhangs exist, the lesser horizontal dimension of the building shall not include any overhang dimension, but the edge distance, *a*, shall be measured from the outside edge of the overhang.

FIGURE 30.3-2B Components and Cladding [ $h \le 60$  ft ( $h \le 18.3$  m)]: External Pressure Coefficients, ( $GC_p$ ), for Enclosed and Partially Enclosed Buildings—Gable Roofs,  $7^{\circ} < \theta \le 20^{\circ}$ 



# **Wind Load**

 Ultimate Wind Speed	145
Directionality Factor,kd	0.85
Topographic factor	1.00
Velocity pressure exposure factor,kz	0.88
Ground Elevation Factor,ke	1.62
Side Wall Width	50
Median Roof Height	129.36
Velocity pressure,qz	40.26
External Pressure Up,GCp_1	-0.7
External Pressure Up,GCp_2	-0.6
External Pressure Up,GCp_3	-1.3
External Pressure Down,GCp	-0.18
Design Pressure Up,p_1	-23.56
Design Pressure Up,p_2	-20.19
Design Pressure Up,p_3	-43.75
Design Pressure Up,p	-6.06
Uni_Dist_W_up	-43.75
M_uni_Dist_W_up	-267.97
Def_uni_Dist_W_up	-0.16
Uni_Dist_W_down	-6.06
M_uni_Dist_W_down	-37.12
Def_uni_Dist_W_down	-0.02

# Lag Screw Uplift Check (ASD)

5/16" Lag Screw Withdrawl value	205.00 lb/in
Lag Screw Penetration	2.5 inches
Roof1 0.6D+0.6W(up z1)	2258.736 > 512.5
Roof1 0.6D+0.6W(up z2)	2260.758 > 512.5



## Framing Check (ASD):

Roof1 uni 1.0D+0.6W	3325.15 > 413.44
Lag Screw Penetration	3371.09 > 413.44
Roof1 0.6D+0.6W(up z1)	3386.40 > 297.16
Roof1 0.6D+0.6W(up z2)	1834.31 > 413.44

#### Seismic Check:

Wood	5.00 psf	
2x4 Studs @ 16"	2.00 psf	
Gypsum	3.00 psf	
Misc(insulatioon,etc)	2.00 psf	
Total wall DL	12.00 psf	
Total Wall Area (Approx)	2590.00 ft <sup>2</sup>	
Total Wall W	31080 lbs	
Total Roof DL	7.50 psf	
Total Roof Area (Approx)	1276.93 ft <sup>2</sup>	
Total Roof W	9576.975 lbs	
PV Panel W	3.00 psf	
Area of panel	1020.00 ft <sup>2</sup>	
Total Roof W	1421.22 lbs	
% increase=(Wadditional)/Wexisting	3.73%	ок

The increase in weight as a result of the solar system is less than 10% of the existing structure and therefore no further seismic analysis is required.

## **Limits of Scope of Work and Liability**

Existing structure is assumed to have been designed and constructed following appropriate codes at time of erection, and assumed to have appropriate permits. The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgements. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. All PV modules, racking, and mounting equipment shall be designed and installed per manufacturer's approved installation specifications. The Engineer of Record and Engineerinc assume no responsibility for misuse or improper installation. This analysis is not stamped for water leakage. Framing was determined based on information in provided plans and/or photos, along with engineering judgement. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction. Prior to the commencement of work, the contractor shall verify the existing roof and framing conditions. Notify Engineerinc and the engineer of record of any Discrepancies prior to starting construction. Prior to the commencement of work, the contractor shall inspect framing for any damage such as water damage, cracked framing, etc. and notify the E.O.R. if any issues are found. These plans/calculations are estamped for structural code compliance of the roof framing supporting the proposed PV installation reference only. These plans/calculations are not stam