BLUE RAVEN SOLAR, LLC.
Firm License No. D-0449
1403 North Research Way , Bldg. J
Orem, UT 84097

June 9, 2025

To: Blue Raven Solar
1403 North Research Way, Building J
Orem, UT. 84097

Subject: Certification Letter
Gbeddy Residence
963 Micahs Way N
Spring Lake, NC. 28390

To Whom It May Concern,

A jobsite observation of the condition of the existing framing system was performed by an audit team of Blue Raven Solar. All review is based on these observations and the design criteria listed below and only deemed valid if provided information is true and accurate.

On the above referenced project, the roof structural framing has been reviewed for additional loading due to the installation of the solar PV addition to the roof. The structural review only applies to the section of the roof that is directly supporting the solar PV system and its supporting elements. The observed roof framing is described below. If field conditions differ, contractor to notify engineer prior to starting construction.

The roof structures of (MP1&2) consist of composition shingle on roof plywood that is supported by pre-manufactured trusses that are spaced at @ 24"o.c.. The top chords, sloped at 45 degrees, are 2x4 sections, the bottom chords are 2x4 sections and the web members are 2x4 sections. The truss members are connected by steel gusset plates. The max unsupported projected horizontal top chord span is approximately 7'-0''.

The existing roof framing systems of (MP1&2) are judged to be adequate to withstand the loading imposed by the installation of the solar panels. No reinforcement is necessary.

The spacing of the solar standoffs for (MP1) should be kept at 72" o.c. for landscape and 48" o.c. for portrait orientation, with a staggered pattern to ensure proper distribution of loads.The spacing of the solar standoffs for (MP2) should be kept at 32" o.c. for landscape and 24" o.c. for portrait orientation, with a staggered pattern to ensure proper distribution of loads.

The scope of this report is strictly limited to an evaluation of the fastener attachment, underlying framing and supporting structure only. The attachment's to the existing structure are required to be in a staggered pattern to ensure proper distribution of loading. All panels, racking and hardware shall be installed per manufacturer specifications and within specified design limitations. All waterproofing shall be provided by the manufacturer.

Note: Seismic check is not required since Ss<.4g and Seismic Design Category (SDC) < B

Design Criteria:

- Applicable Codes = 2018 North Carolina State Building Code (NCSBC), ASCE 7-1C
- Roof Dead Load = 7 psf (MP1&2)
- Roof Live Load = 20 psf
- Wind Speed = 115 mph (Vult), Exposure C, Risk Category II
- Ground Snow Load = 15 psf - Roof Snow Load = 10.5 psf
- Attachment: 2 - #14 Wood Screws with 2.5 inch min. embedment depth, at spacing shown above.
- Attachment: 6 - #14 Wood Screws into wood decking, at spacing shown above.

Digitally signed by John A. Calvert
Date: 2025.06.09 10:26:48 -06'00'

Please contact me with any further questions or concerns regarding this project.

Sincerely,

John Calvert, P.E.
Project Engineer

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Gravity Loading

Roof Snow Load Calculations		
p_g = Ground Snow Load =	15 psf	
$p_f = 0.7\ C_e\ C_t\ I\ p_g$		(ASCE7 - Eq 7-1)
C_e = Exposure Factor =	1	(ASCE7 - Table 7-2)
C_t = Thermal Factor =	1	(ASCE7 - Table 7-3)
I = Importance Factor =	1	
p_f = Flat Roof Snow Load =	10.5 psf	
$p_s = C_s p_f$		(ASCE7 - Eq 7-2)
Cs = Slope Factor =	1	
p_s = Sloped Roof Snow Load =	**10.5 psf**	

PV Dead Load = 3 psf (Per Blue Raven Solar)	
DL Adjusted to 45 Degree Slope	4.24 psf
PV System Weight	
Weight of PV System (Per Blue Raven Solar)	3.0 psf
X Standoff Spacing =	4.00 ft
Y Standoff Spacing =	3.04 ft
Standoff Tributary Area =	12.17 sft
Point Loads of Standoffs	**37 lb**

Note: PV standoffs are staggered to ensure proper distribution of loading

Roof Live Load = 20 psf

Note: Roof live load is removed in area's covered by PV array.

Roof Dead Load (MP1&2)		
Composition Shingle	4.00	
Roof Plywood	2.00	
2x4 Top Chords @ 24"o.c.	0.73	
Vaulted Ceiling	0.00	(Ceiling Not Vaulted)
Miscellaneous	0.27	
Total Roof DL (MP1&2)	**7.0 psf**	
DL Adjusted to 45 Degree Slope	9.9 psf	

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Wind Calculations

Per ASCE 7-10 Components and Cladding

Input Variables	
Wind Speed	115 mph
Exposure Category	C
Roof Shape	Hip/Gable
Roof Slope	45 degrees
Mean Roof Height	20 ft
Effective Wind Area	21.3 ft

Design Wind Pressure Calculations		
Wind Pressure P = qh*G*Cn		
qh = 0.00256 * Kz * Kzt * Kd * V^2		(Eq. 30.3-1)
Kz (Exposure Coefficient) =	0.9	(Table 30.3-1)
Kzt (topographic factor) =	1	(Fig. 26.8-1)
Kd (Wind Directionality Factor) =	0.85	(Table 26.6-1)
V (Design Wind Speed) =	115 mph	(Fig. 26.5-1A)
Risk Category =	II	(Table 1.5-1)
qh =	25.90	
0.6 * qh =	15.54	

Standoff Uplift Calculations-Portrait					
	Zone 1	Zone 2	Zone 3	Positive	
GCp =	-0.94	-1.15	-1.15	0.86	(Fig. 30.4-1)
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	22.4 psf	
X Standoff Spacing =	4.00	4.00	2.67		
Y Standoff Spacing =	3.04	3.041666667	3.04166667		
Tributary Area =	12.17	12.17	8.11		
Dead Load on Attachment=	36.50	36.50	24.33		
Footing Uplift (0.6D+0.6W)=	**-155 lb**	**-195 lb**	**-130 lb**		

Standoff Uplift Calculations-Landscape					
	Zone 1	Zone 2	Zone 3	Positive	
GCp =	-0.94	-1.15	-1.15	0.86	(Fig. 30.4-1)
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	10.5 psf	
X Standoff Spacing =	6.00	6.00	4.00		
Y Standoff Spacing =	1.75	1.75	1.75		
Tributary Area =	10.50	10.50	7.00		
Dead Load on Attachment=	31.50	31.50	21.00		
Footing Uplift (0.6D+0.6W) =	**-134 lb**	**-168 lb**	**-112 lb**		

Standoff Uplift Check	
Maximum Design Uplift =	-195 lb
Standoff Uplift Capacity =	400 lb
400 lb capacity > 195 lb demand	**Therefore, OK**

Fastener Capacity Check	
Fastener =	2 - #14 Wood Screws
Number of Fasteners =	2
Embedment Depth =	1.75
Pullout Capacity Per Inch =	150 lb
Fastener Capacity =	525 lb
w/ F.S. of 1.5 & DOL of 1.6=	560 lb
560 lb capacity > 195 lb demand	**Therefore, OK**

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Wind Calculations

Per ASCE 7-10 Components and Cladding

Input Variables	
Wind Speed	115 mph
Exposure Category	C
Roof Shape	Hip/Gable
Roof Slope	45 degrees
Mean Roof Height	20 ft
Effective Wind Area	21.3 ft

Design Wind Pressure Calculations		
Wind Pressure P = qh*G*Cn		
qh = 0.00256 * Kz * Kzt * Kd * V^2		(Eq. 30.3-1)
Kz (Exposure Coefficient) =	0.9	(Table 30.3-1)
Kzt (topographic factor) =	1	(Fig. 26.8-1)
Kd (Wind Directionality Factor) =	0.85	(Table 26.6-1)
V (Design Wind Speed) =	115 mph	(Fig. 26.5-1A)
Risk Category =	II	(Table 1.5-1)
qh =	25.90	
0.6 * qh =	15.54	

Standoff Uplift Calculations-Portrait					
	Zone 1	Zone 2	Zone 3	Positive	
GCp =	-0.94	-1.15	-1.15	0.86	(Fig. 30.4-1)
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	22.4 psf	
X Standoff Spacing =	2.00	2.00	1.33		
Y Standoff Spacing =	3.04	3.041666667	3.04166667		
Tributary Area =	6.08	6.08	4.05		
Dead Load on Attachment=	18.25	18.25	12.14		
Footing Uplift (0.6D+0.6W)=	**-78 lb**	**-97 lb**	**-65 lb**		

Standoff Uplift Calculations-Landscape					
	Zone 1	Zone 2	Zone 3	Positive	
GCp =	-0.94	-1.15	-1.15	0.86	(Fig. 30.4-1)
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	10.0 psf	(Minimum)
X Standoff Spacing =	2.67	2.67	1.78		
Y Standoff Spacing =	1.75	1.75	1.75		
Tributary Area =	4.67	4.67	3.12		
Dead Load on Attachment=	14.02	14.02	9.35		
Footing Uplift (0.6D+0.6W) =	**-60 lb**	**-75 lb**	**-50 lb**		

Standoff Uplift Check	
Maximum Design Uplift =	-97 lb
Standoff Uplift Capacity =	159 lb
159 lb capacity > 97 lb demand	**Therefore, OK**

Fastener Capacity Check	
Fastener =	6 - #14 Wood Screws
Number of Fasteners =	6
Embedment Depth =	0.5
Pullout Capacity Per Screw =	177 lb
Fastener Capacity =	1062 lb
w/ F.S. of 4 & DOL of 1.6=	426 lb
425.6 lb capacity > 97 lb demand	**Therefore, OK**

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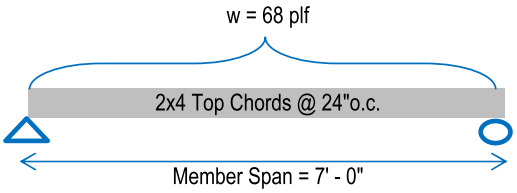
Framing Check
(MP1&2) **PASS**

Dead Load	9.9 psf
PV Load	4.2 psf
Live Load	20.0 psf
Governing Load Combo = DL + LL	
Total Load	**34.1 psf**



Member Properties

Member Size	S (in^3)	I (in^4)	Lumber Sp/Gr	Member Spacing
2x4	3.06	5.36	DF#2	@ 24"o.c.

Check Bending Stress

Fb (psi) =	f'b	x	Cd	x	Cf	x	Cr	(NDS Table 4.3.1)
	900	x	1.25	x	1.5	x	1.15	

Allowed Bending Stress = 1940.6 psi

Maximum Moment = (wL^2) / 8
= 418.2412 ft#
= 5018.894 in#

Actual Bending Stress = (Maximum Moment) / S
= 1638.9 psi

Allowed > Actual -- 84.5% Stressed -- Therefore, OK

Check Deflection

Allowed Deflection (Total Load) = L/180 (E = 1600000 psi Per NDS)
= 0.466 in

Deflection Criteria Based on = Continuous Span

Actual Deflection (Total Load) = (w*L^4) / (185*E*I)
= 0.179 in
= L/470 > L/180 **Therefore OK**

Allowed Deflection (Live Load) = L/240
0.35 in

Actual Deflection (Live Load) = (w*L^4) / (185*E*I)
0.105 in
L/800 > L/240 **Therefore OK**

Check Shear

Member Area = 5.3 in^2 Fv (psi) = 180 psi (NDS Table 4A)

Allowed Shear = Fv * A = 945 lb Max Shear (V) = w * L / 2 = 239 lb

Allowed > Actual -- 25.3% Stressed -- Therefore, OK