

910-893-7525 www.harnett.org

PERMIT NUMBER ERES2505-0008

JOB ADDRESS: 466 WOODWIND DR	PERMIT SUBTYPE: RESIDENTIAL SOLAR PANELS		PARCEL NO: 0534-51-4734.000
DESCRIPTION: 26 roof mounted solar panels	DATE ISSUED: 5/20/2025	DATE EXPIRED:	
PLAN NAME:	ZONING DISTRICT: RA-20R - 0.35 acres (100.0%)		
APPLICANT: Top Tier Solar Solutions, LLC		PHONE: ((855)997-1213

APPLICANT: Top Tier Solar Solutions, LLC	PHONE: (855)997-1213
1530 Center Park Dr. Charlotte, NC 28217	EMAIL: nc@toptiersolarsolutions.com
CONTRACTOR: Top Tier Solar Solutions, LLC	PHONE: (855)997-1213
1530 Center Park Dr. Charlotte, NC 28217	EMAIL: nc@toptiersolarsolutions.com
OWNER: HARRISON SHIRLEY M	PHONE:
466 WOODWIND DR SPRING LAKE, NC 28390 SPRING LAKE, NC 28390-0000	EMAIL:

REQUIRED INSPECTIONS			
INSPECTION TYPE	APPROVAL	DATE	COMMENTS
FINAL**			
ROUGH IN			

Top Tier Solar Solutions Contractor Address: 1530 Center Park Dr #2911, Charlotte, NC 28217

May 09, 2025

Subject: Proposed Solar Panel Installation

Shirley Harrison Residence, 466 Woodwind Dr, Spring Lake, NC

DC System Size: 10.530 kW PV Letters Job #004-22515

To Whom it May Concern,

We have reviewed information, provided by our client, related to the proposed solar panel installation at the above-referenced address. The purpose of the review was to determine if the existing roof is structurally adequate for the proposed installation. Based on our review and analysis of the given information, and in accordance with governing building codes, I certify that the capacity of the structural roof framing that directly supports the additional gravity loading due to the solar panel supports and modules had been reviewed and determined to meet or exceed the requirements in accordance with the Design Criteria.

Design Parameter Summary

Governing Building Code: 2018 North Carolina Residential Code

Risk Category: II Wind Exposure: C

Design Wind Speed: 120 mph Ground Snow Load: 15 psf

Roof Information

Roof Structure: 2x4 @ 24" O.C. (Assumed)

Roofing Material: Metal Roof Roof Slope: 18, 36 degrees

Roof Connection Details

S-5! ProteaBracket to existing metal roof, at 45" O.C. max

Stagger attachments to avoid overloading any individual truss top chord.

Engineering Analysis

The proposed installation - including weight of panels, racking, mounts, and inverters where applicable - will be approximately 3 psf. In the areas where panels are installed, roof live loads will not be present. The reduction of roof live load is adequate to fully or partially compensate for the addition of the panel installation. Because the member forces in the area of the solar panels are not increased by more than 5%, and so per provisions in the adopted building codes, the structure need not be altered for gravity loading.

The proposed installation will be 6" max. above the roof surface (flush mounted) and parallel to the roof surface. Therefore, any increase in wind loading on the building structure from the solar panel installation is expected to be negligible. Wind is the governing lateral load case. Because the increase in lateral loading is not increased by more than 10%, per provisions in the adopted building codes, the structure need not be altered for lateral loading.

Wind uplift on the panels has been calculated in accordance with the relevant provisions of ASCE 7-10. This loading has been used to verify the adequacy of the connection specified above. Connection locations should be in accordance with design drawings.

IronRidge XR10 rails will support the modules and will fasten to the roof structure with S-5! ProteaBracket along the rail.

Conclusion

The roof structure need not be altered for either gravity loading (including snow) or lateral loading (including wind). Therefore, the existing structure is permitted to remain unaltered. Connections to the roof must be made per the "Roof Connection Details" section above. Copies of all relevant calculations are enclosed.

Limitations and Disclaimers

Electrical design is excluded from this analysis. Waterproofing is the sole responsibility of the installer and is also excluded from this analysis. Solar panels must be installed per manufacturer specifications. Structural design and analysis of the adequacy of solar panels, racks, mounts, and other components is performed by each component's respective manufacturer; the undersigned makes no statement of opinion regarding such components. This letter and the opinions expressed herein are rendered solely for the benefit of the permitting authority (city or county building department) and your office, and may not be utilized or relied on by any other party.

If you have any questions or concerns, please contact us at (208)-994-1680, or by email at Projects@pvletters.com.

Sincerely,

P.E.

P.E.

OFESS

OFESS Trevor A. Jones, P.E.

5/9/2025



Standard Loading Comparison (Roof 1)

This calculation justifies the additional solar load by comparing existing to proposed gravity loads in the location of the solar panels.

	Without Solar	With Solar	
Dead Load			
Metal Roof	3	3	psf
1/4" Plywood	1	1	psf
Framing	4	4	psf
Insulation	1	1	psf
1/2" Gypsum Ceiling	2	2	psf
M,E, & Misc	1.5	1.5	psf
Solar Panel	0	3	psf
Total Dead Load	12.5	15.5	psf
Snow Load			
Ground Snow Load, $P_{\rm g}$	15	5	psf
Exposure Factor, C _e	1.00		
Thermal Factor, C _t	1.1		
Importance Factor, I _s	1		
Flat Roof Snow Load	12		ASCE 7 Eqn. 7.3-1 or jurisdiction min.
Slope	36	<u> </u>	degrees
Unobstructed Slippery Surface?	No	No	
Slope Factor, C _s	1.00	1.00	
Sloped Roof Snow Load	11.6	11.6	psf
Live Load			
Roof Live Load	20	0	psf
Load Combination			
D + Lr	32.5	15.5	psf
D + S	24.1	27.1	psf
Max. Load	32.5	27.1	psf
% of original		83.23%	_
D 14	D 41 4 4 1 6	•	1 (1 1 (1

Result: Because the total forces are decreased, per the relevant code provisions stated in the body of the letter, the existing roof structure is permitted to remain unaltered.



Bracket Connection Calculation (per ASCE 7-10) (Roof 1)

This calculation justifies the connection of the solar panels to existing roof members, by showing the connection capacity is equal to or greater than the uplift force demands.

Connection Demand

Spacing perpendicular to rail, in Roof Angle, degrees
Roof Layout
Wind Speed, mph
Exposure Coefficient, K_z
Topographic Factor, K_{zt}
Directionality Factor, K_d
Elevation Factor, K_e
Velocity Pressure q_z, psf
Prying Coefficient

34
36
Gable
120
0.85
1.00
0.85
1.00
26.5
1

Zones:

<u>1</u>	<u>2</u>	<u>3</u>
45	45	45
1.00	2.00	2.00
No	No	No
10.6	10.6	10.6
0.79	0.79	0.79
20.9	41.9	41.9
133.1	266.1	266.1
19.1	19.1	19.1
114.0	247.1	247.1

Spacing parallel to rail, in

 $GC_p(max)$

Exposed Panels? ($\gamma_E = 1.5$)

Effective Wind Area on each con., ft²

Pressure Equalization Factor, γ_a

Uplift Force, psf

Max. Uplift Force / Connection (0.6 WL), lbs

Solar Dead Load (0.6 DL). Lbs

Max. Uplift Force (0.6 WL - 0.6 DL), lbs

Connection Capacity

Connection Type
Ultimate Capacity, lbs
Factor of Safety
Total Capacity, lbs

S-5! ProteaBracket

817		
2.50		
326.8		

Compare ASD Factored Demand to Capacity

Demand, lbs Capacity, lbs 247.1 326.8

Result

Capacity exceeds demands. Therefore, connection passes.



Standard Loading Comparison (Roof 2)

This calculation justifies the additional solar load by comparing existing to proposed gravity loads in the location of the solar panels.

With Solar

Without Solar

	Williout Solai	With Solar	
Dead Load			
Metal Roof	3	3	psf
1/4" Plywood	1	1	psf
Framing	4	4	psf
Insulation	1	1	psf
1/2" Gypsum Ceiling	2	2	psf
M,E, & Misc	1.5	1.5	psf
Solar Panel	0	3	psf
Total Dead Load	12.5	15.5	psf
Snow Load			
Ground Snow Load, P_g	15	5	psf
Exposure Factor, C _e	1.0	00	
Thermal Factor, C _t	1.1		
Importance Factor, I _s	1		7
Flat Roof Snow Load	12		ASCE 7 Eqn. 7.3-1 or jurisdiction min.
Slope	18	3	degrees
Unobstructed Slippery Surface?	No	No	
Slope Factor, C _s	1.00	1.00	
Sloped Roof Snow Load	11.6	11.6	psf
Live Load			
Roof Live Load	20	0	psf
Load Combination			
D + Lr	32.5	15.5	psf
D + S	24.1	27.1	psf
Max. Load	32.5	27.1	psf
% of original		83.23%	

Result: Because the total forces are decreased, per the relevant code provisions stated in the body of the letter, the existing roof structure is permitted to remain unaltered.



Bracket Connection Calculation (per ASCE 7-10) (Roof 2)

This calculation justifies the connection of the solar panels to existing roof members, by showing the connection capacity is equal to or greater than the uplift force demands.

Connection Demand

Spacing perpendicular to rail, in Roof Angle, degrees Roof Layout Wind Speed, mph Exposure Coefficient, K_z Topographic Factor, K_{zt} Directionality Factor, K_d Elevation Factor, K_e Velocity Pressure q_z, psf **Prying Coefficient**

34
18
Gable
120
0.85
1.00
0.85
1.00
26.5
1

Zones:

Spacing parallel to rail, in
GC_p (max)
Exposed Panels? ($\gamma_E = 1.5$)
Effective Wind Area on each con., ft ²
Pressure Equalization Factor, γ_a
Uplift Force, psf
Max. Uplift Force / Connection (0.6 WL), lbs

Solar Dead Load (0.6 DL). Lbs
Max. Uplift Force (0.6 WL - 0.6 DL), lbs

<u>l</u>	<u>2</u>	<u>3</u>
45	45	37.5
0.90	2.20	2.60
No	No	No
10.6	10.6	8.8
0.79	0.79	0.80
18.8	46.1	55.1
119.8	292.7	292.0
19.1	19.1	15.9
100.7	273.7	276.1

Connection Capacity

Connection Type Ultimate Capacity, lbs Factor of Safety Total Capacity, lbs

817
2.50
326.8

Compare ASD Factored Demand to Capacity

Demand, lbs Capacity, lbs

Result

276.1	
326.8	

Capacity exceeds demands. Therefore, connection passes.



June 30, 2025

Subject: Shirley Harrison Solar Panel Installation

466 Woodwind Dr, Spring Lake, NC

Contractor Name: Top Tier Solar Solutions

Contractor Address: 1530 Center Park Dr #2911, Charlotte, NC

To Whom It May Concern,

This letter is submitted on behalf of my client, EnergyScape Renewables.

I am a North Carolina registered Professional Engineer. A field inspection of the installation has been performed by a person under my direct supervisory control. I hereby affirm the following:

- 1. The PV equipment's structural installation has been designed and inspected,
- 2. The equipment will not create a negative impact on the building's structural design, including any additional loads imposed (dead, snow, wind), and
- 3. The installation is in compliance with the North Carolina Residential Code.

Limitations and Disclaimers

Electrical design is excluded from this analysis. Structural design and analysis of the adequacy of solar panels, racks, mounts, rails, and other components is performed by each component's respective manufacturer. This letter and the opinions expressed herein are rendered solely for the benefit of the permitting authority (city or county building department) and my client's office and may not be utilized or relied on by any other party.

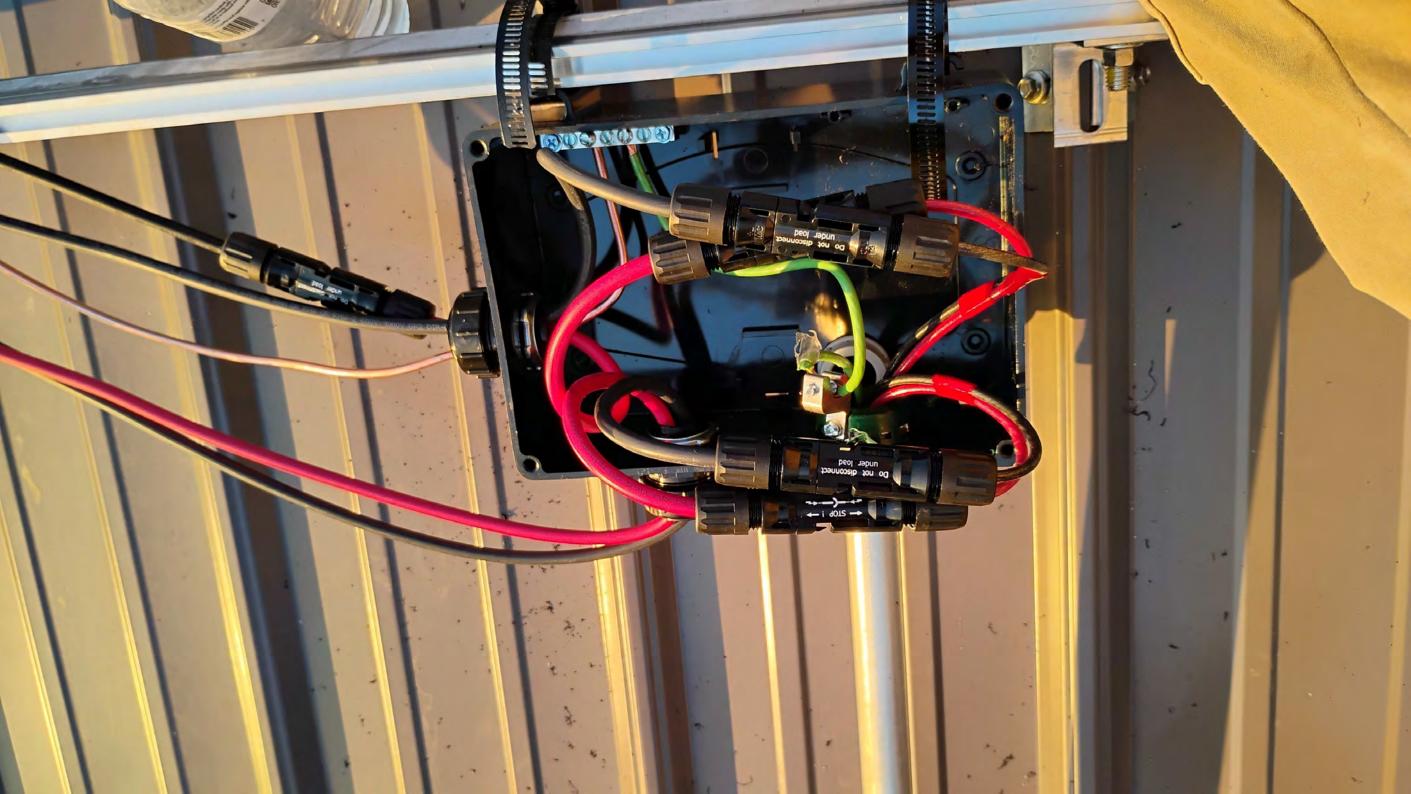
Sincerely,

Trevor Jones, P.E.



June 30, 2025





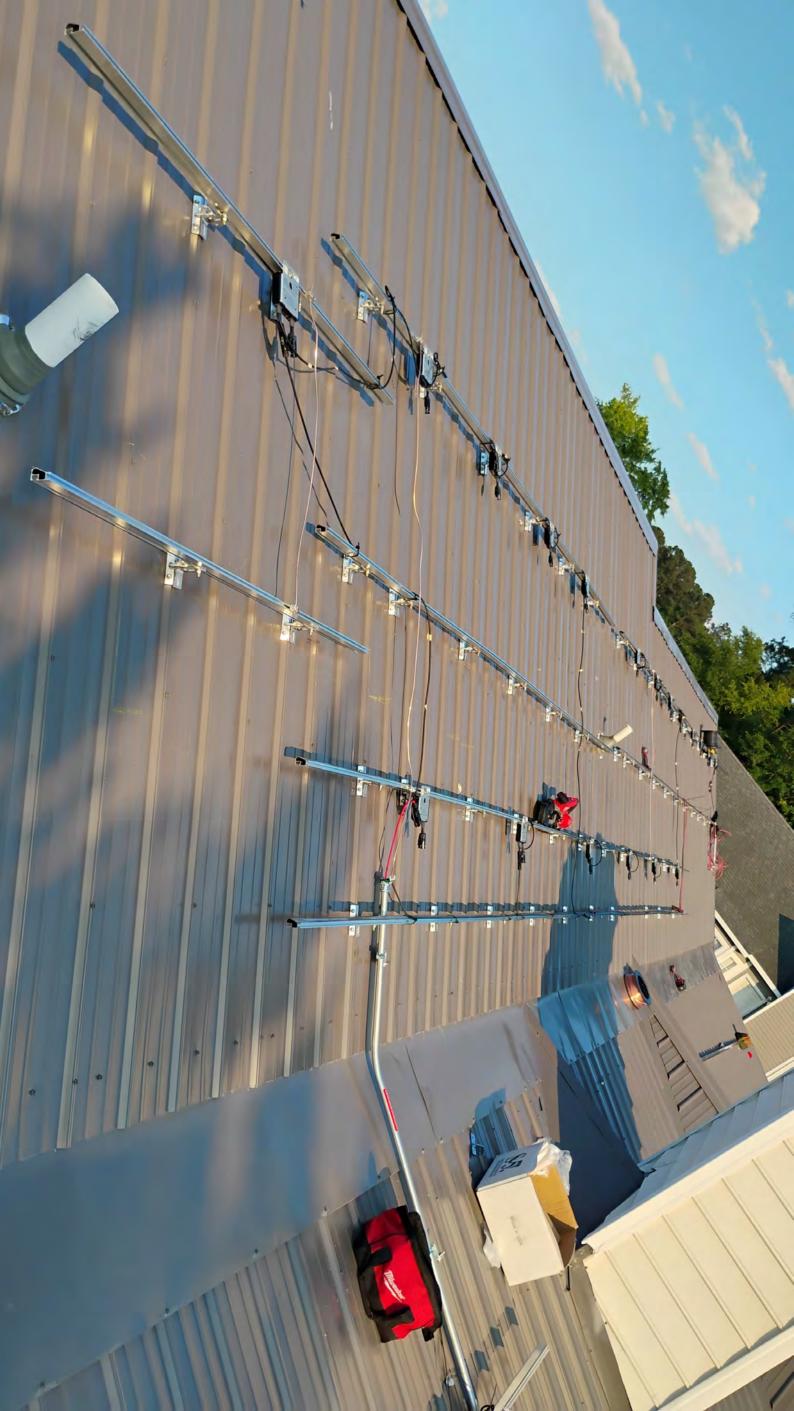






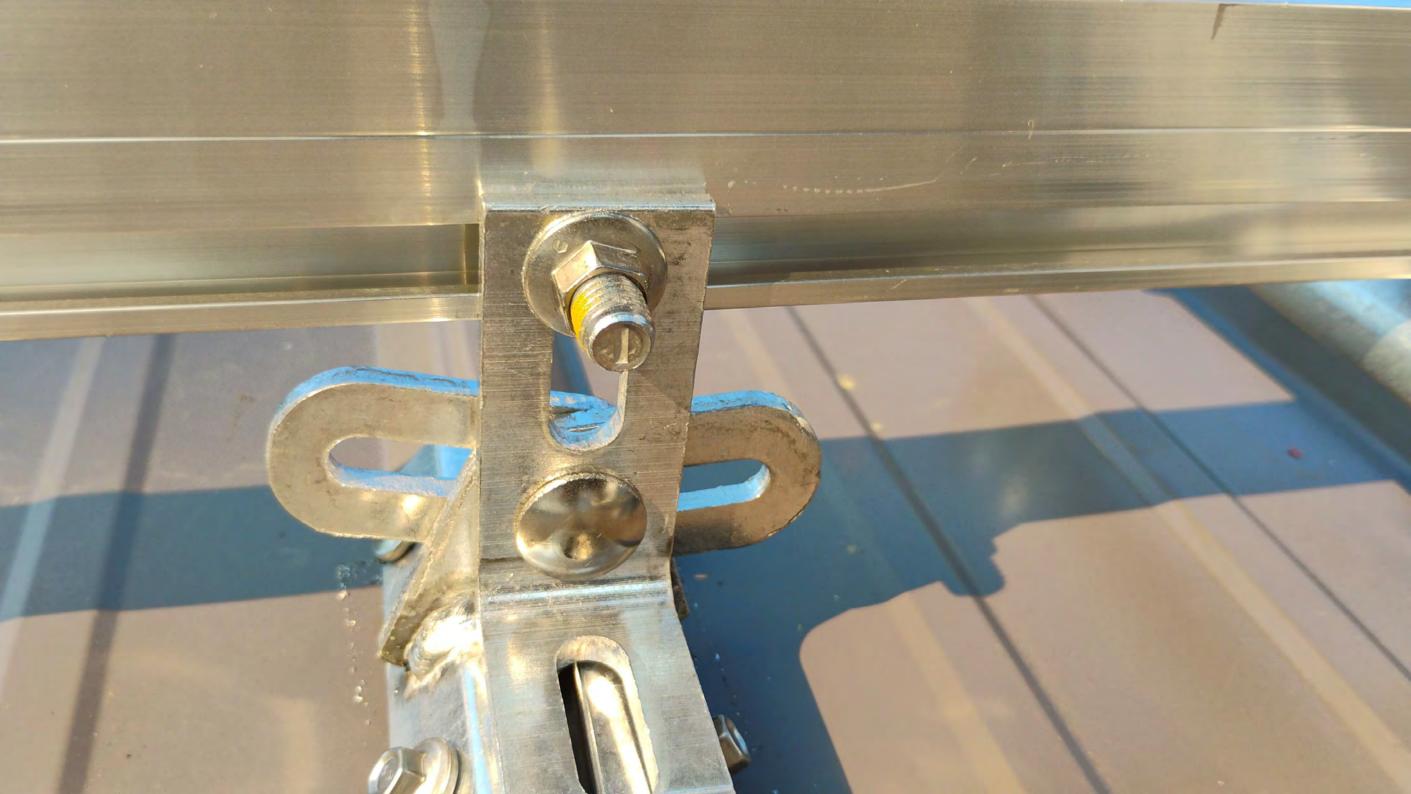






















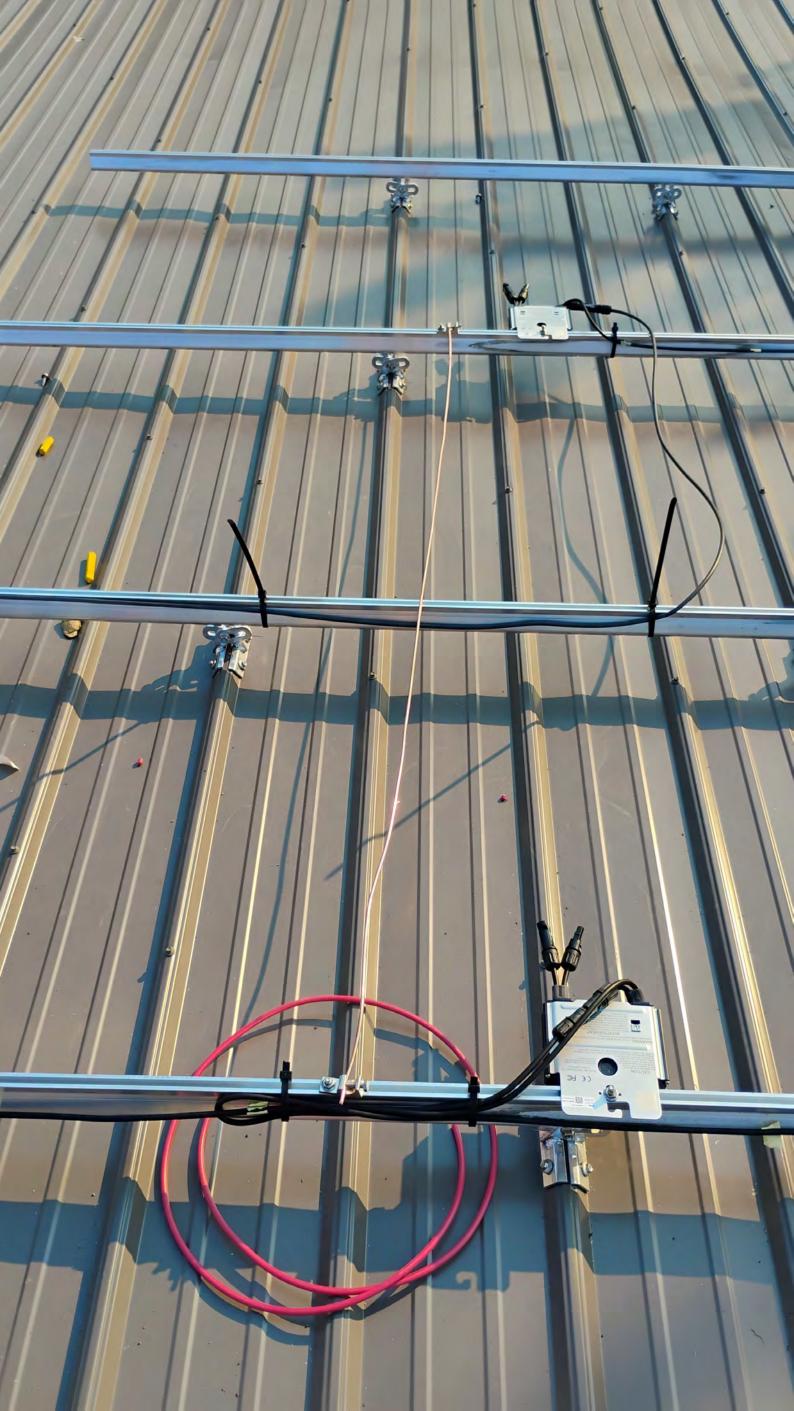














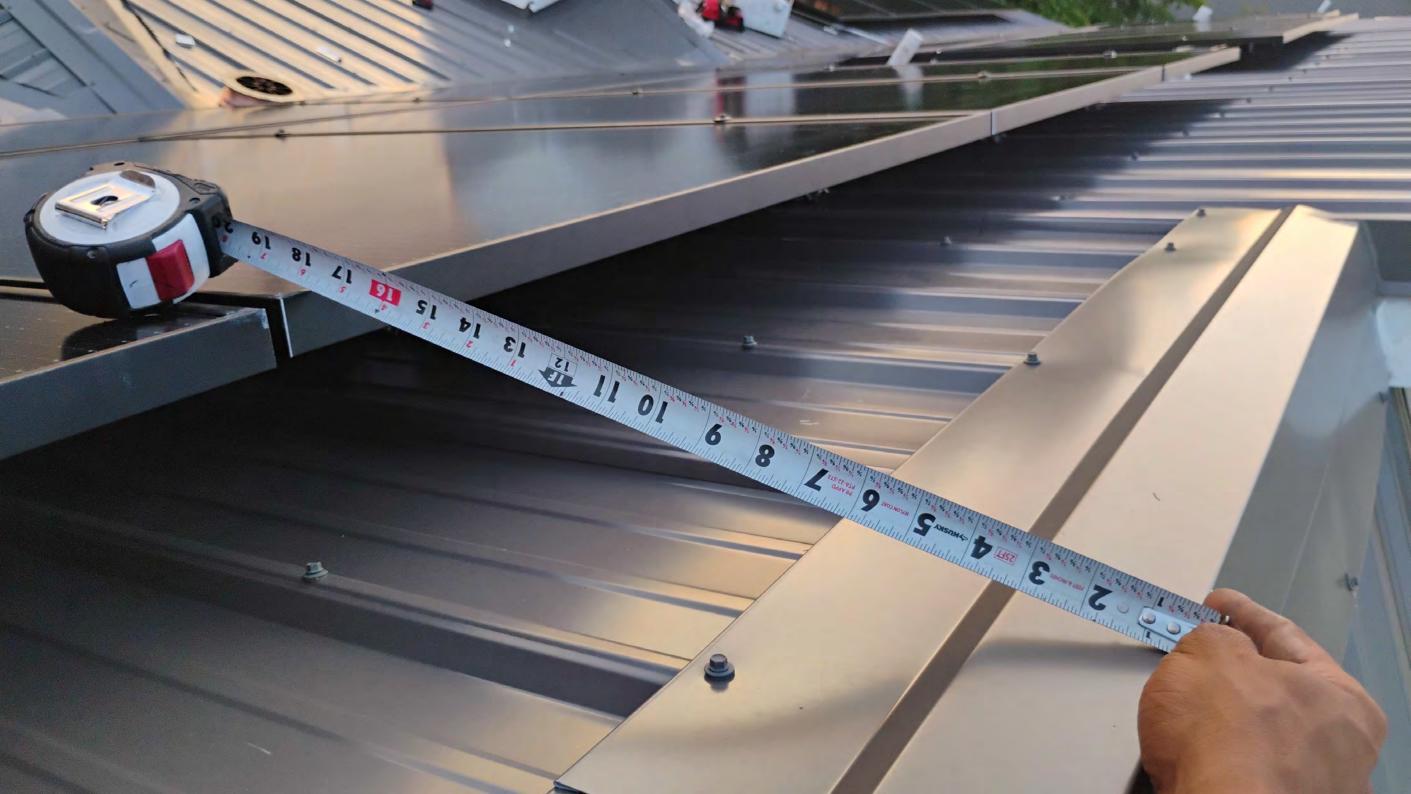
















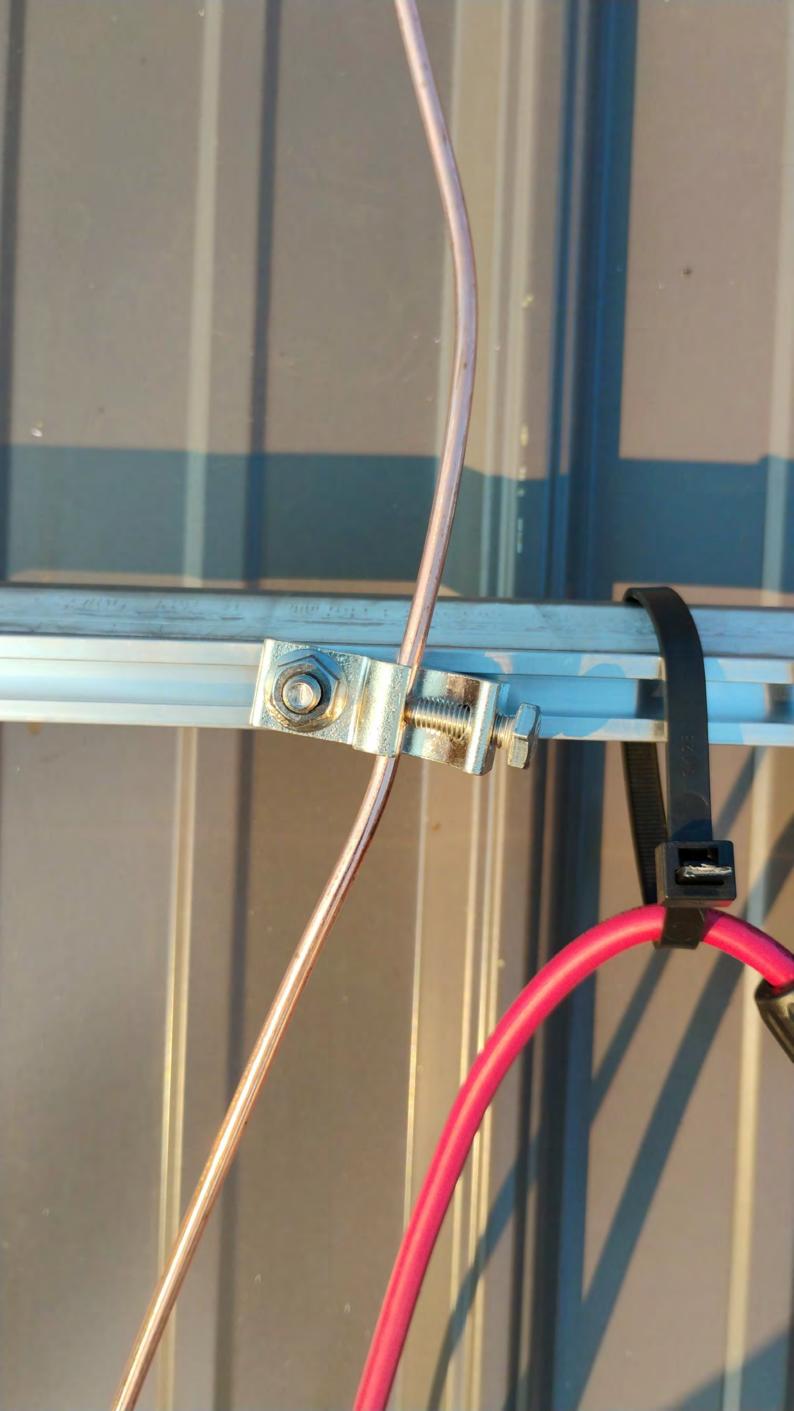














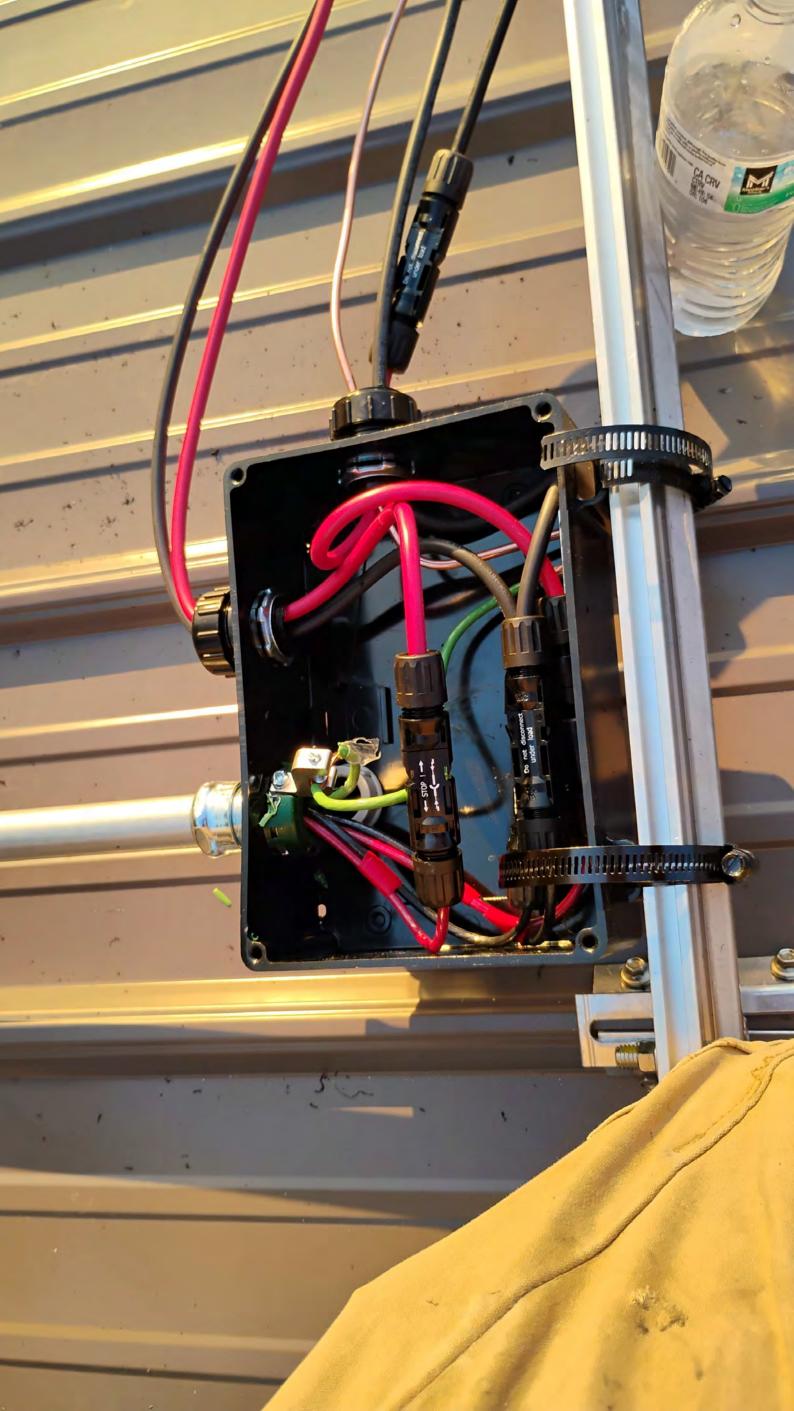








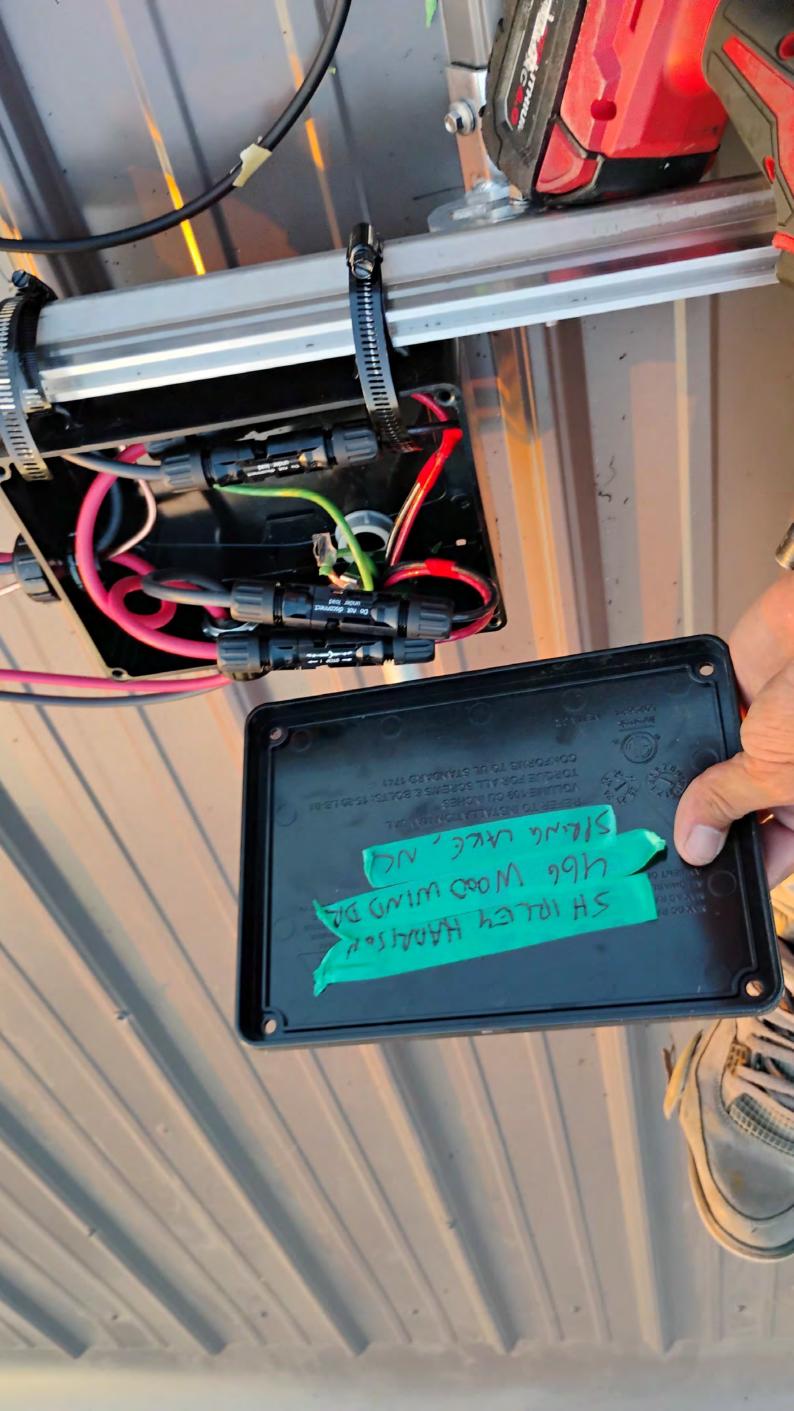




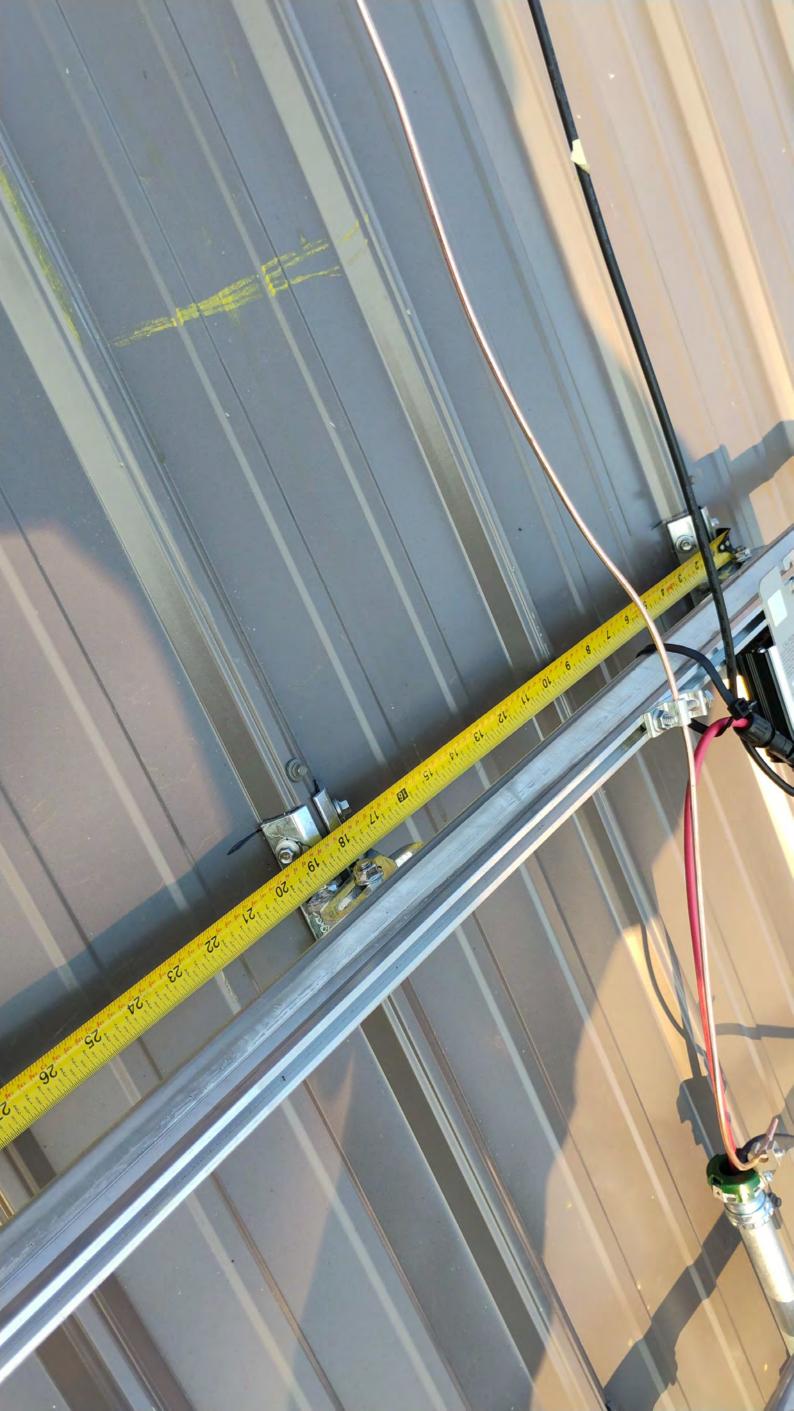






















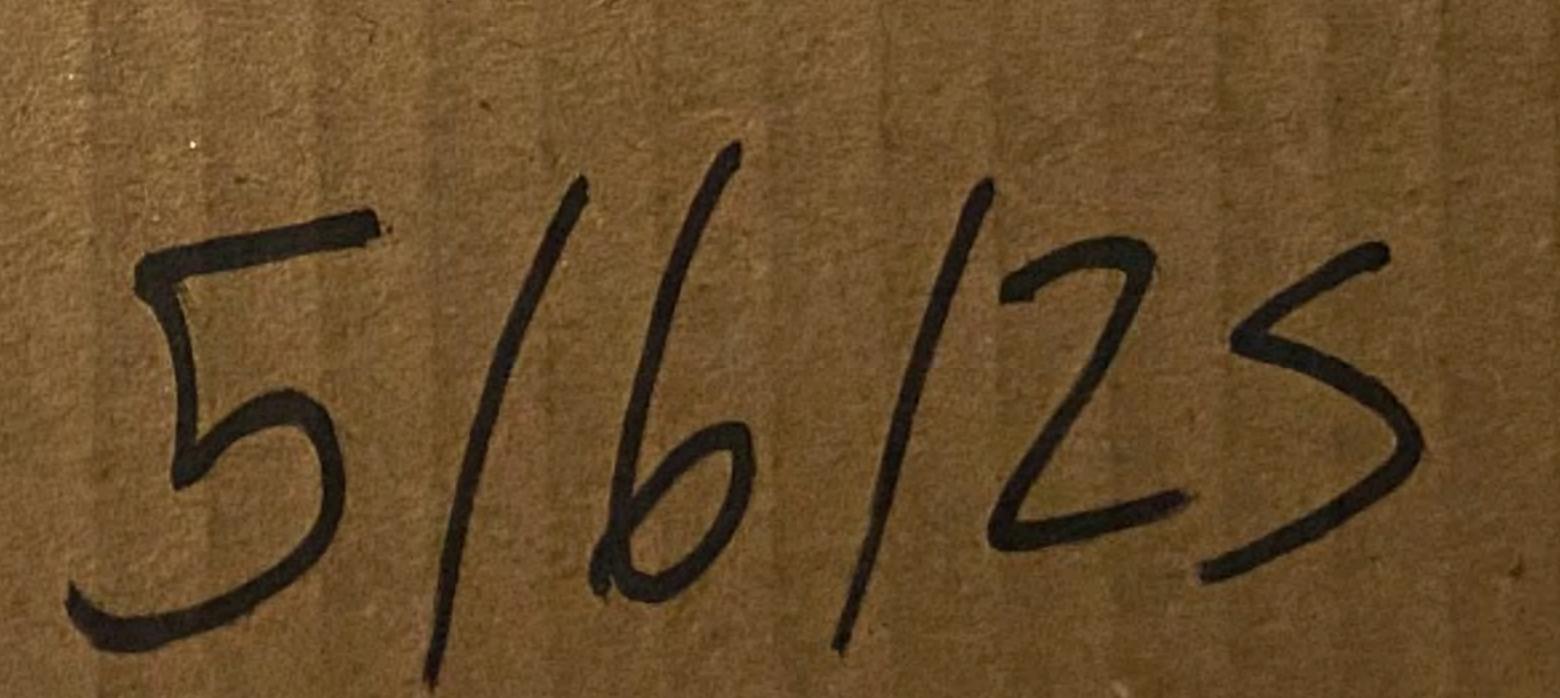






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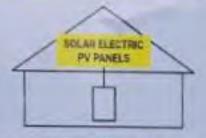
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A DANGER

A PELIGRO



SOLAR PV SYSTEM **EQUIPPED WITH** RAPID SHUTDOWN



TURN RAPID SHUTDOWN SWITCH TO THE "OFF" POSITION TO SHUT DOWN PV SYSTEM AND REDUCE SHOCK HAZARD IN THE ARRAY

AC DISCONNECT

PHOTOVOLTAIC SYSTEM POWER SOURCE

RATED AC **OUTPUT CURRENT** NOMINAL OPERATING AC VOLTAGE

240 VOLTS

♠ WARNING

POWER SOURCE OUTPUT CONNECTION DO NOT RELOCATE THIS OVERCURRENT DEVICE

WARNING

ELECTRIC SHOCK HAZARD

TERMINALS ON THE LINE AND LOAD SIDES MAY BE ENERGIZED IN THE OPEN POSITION

CAUTION:

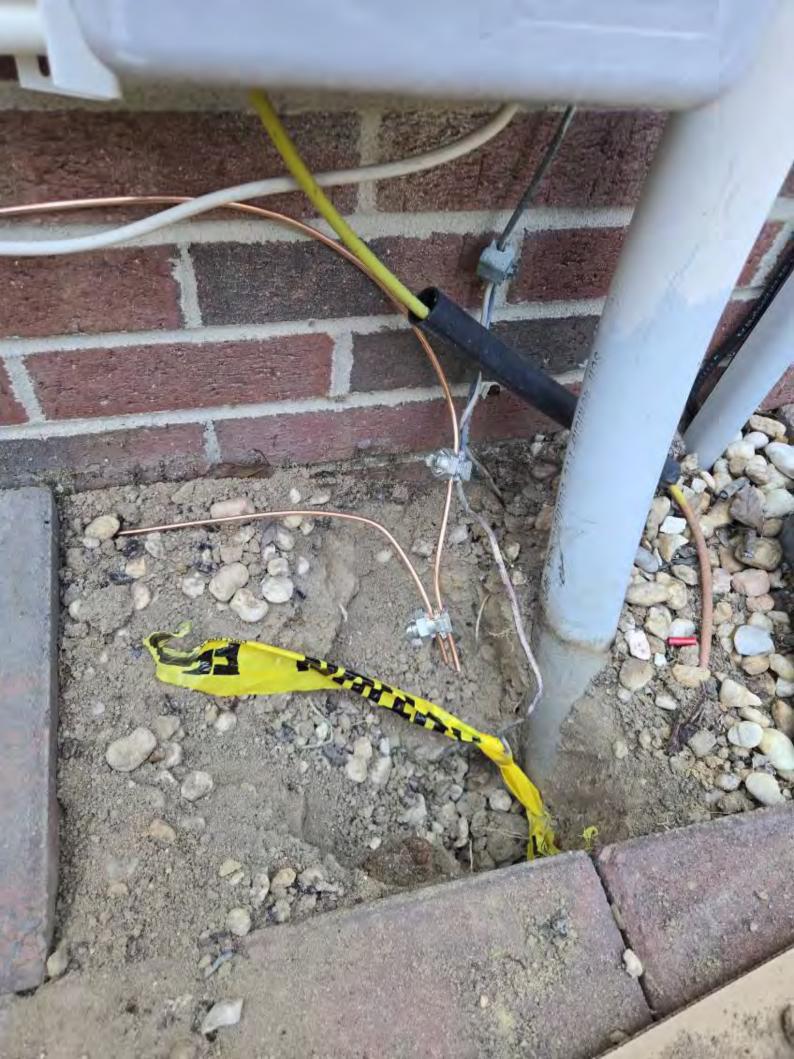
MULTIPLE SOURCES OF POWER THE SERVICE DISCONNECT IS LOCATED INSIDE OPPOSITE THE LITERTY METER





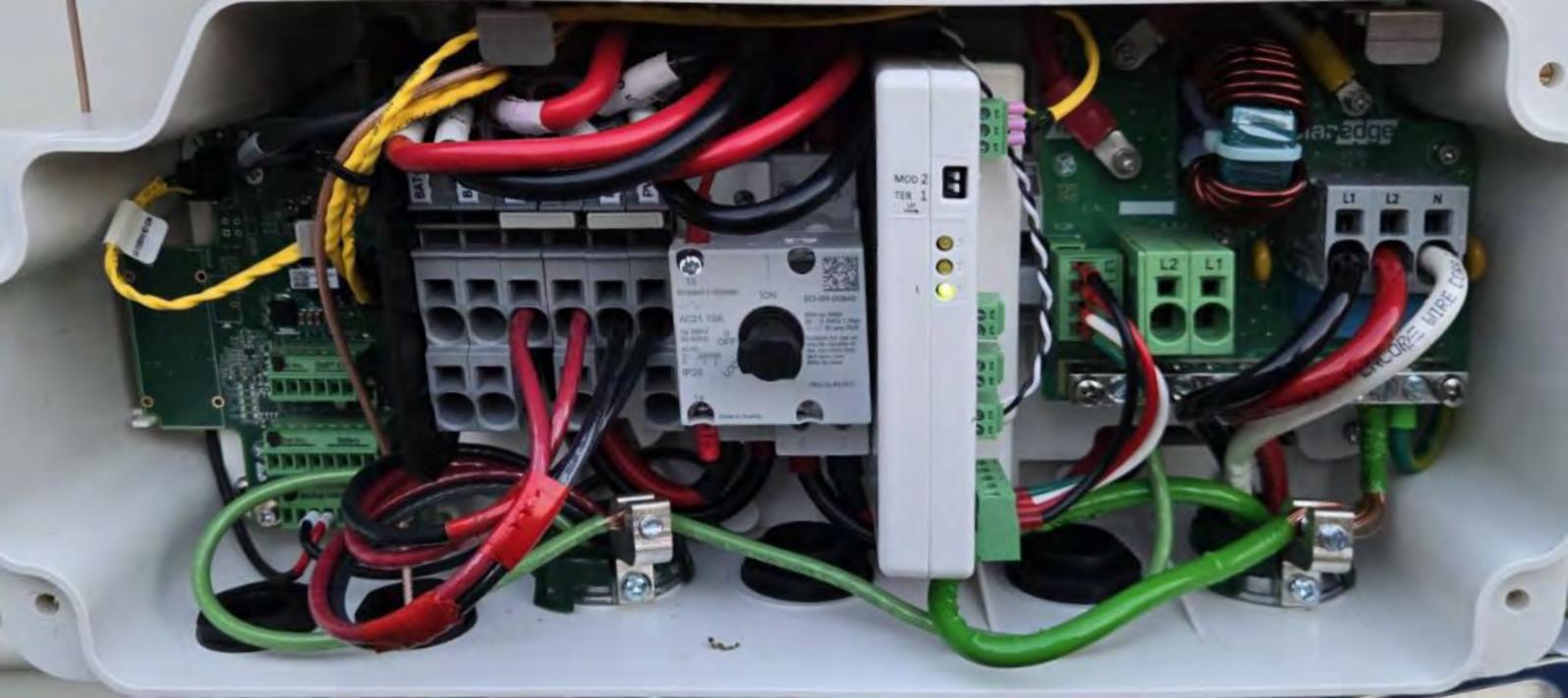














AC TRMS CLAMP METER

