



1/21/2025

CAROLINA CONNECTIONS
422 HUFFMAN MILL ROAD, SUITE 105
BURLINGTON, NC 27215

Attn.: To Whom It May Concern

re job: MARK CIOLEK RESIDENCE
15 PAPER BIRCH WAY,
FUQUAY VARINA, NC 27526

The following calculations are for the structural engineering design of the photovoltaic panels and are valid only for the structural info referenced in the stamped plan set. The verification of such info is the responsibility of others.

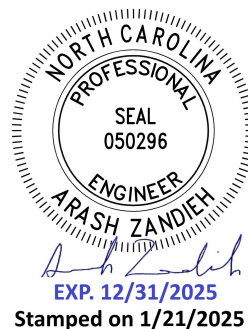
After review, I certify that the roof structure has sufficient structural capacity for the proposed 26 PV modules.

All mounting equipment shall be designed and installed per manufacturer's approved installation specifications.

Design Criteria:

| | | | |
|-----------------|-----------------------|-----|-------------------|
| Code: | 2018 NC Building Code | | |
| | ASCE 7-10 | | |
| Live Load: | 20 | psf | |
| Ult Wind Speed: | 116 | mph | |
| Exposure Cat: | B | | |
| Ground Snow: | 15 | psf | Min Roof Snow: NA |

AHZ Consulting Engineers Inc.
Professional Engineer
projects@ahzengineers.com





Roof Properties:

| | Roof 1 |
|---|-----------------|
| Roof Type = | Shingle |
| Roof Pitch (deg) = | 28 |
| Mean Roof Height (ft) = | 23 |
| Attachment Trib Width (ft) = | 2.75 |
| Attachment Spacing (ft) = | 4 |
| Framing Type = | Truss |
| Framing Size = | 2x4 |
| Framing OC Spacing (in.) = | 24 |
| Section Thickness, b (in.) = | 1.5 |
| Section Depth, d (in.) = | 3.5 |
| Section Modulus, S _x (in. ³) = | 3.1 |
| Moment of Inertia, I _x (in. ⁴) = | 5.4 |
| Framing Span (ft) = | 8 |
| Deflection Limit D+L (in.) = | 1.6 |
| Deflection Limit S or W (in.) = | 1.07 |
| Attachments Pattern = | Fully Staggered |
| Framing Upgrade = | Adequate |
| Sister Size = | NA |
| Wood Species = | DF #2 |
| Wood F _b (psi) = | 900 |
| Wood F _v (psi) = | 180 |
| Wood E (psi) = | 1600000 |
| C _D (Wind) = | 1.6 |
| C _D (Snow) = | 1.15 |
| C _{LS} = | 1.15 |
| C _M = C _t = C _L = C _i = | 1.0 |
| C _F = | 1.5 |
| C _{fu} = | 1.00 |
| C _r = | 1.15 |
| F' _b _wind (psi) = | 2857 |
| F' _b _snow (psi) = | 2053 |
| F' _v _wind (psi) = | 288 |
| F' _v _snow (psi) = | 207 |
| M_allowable_wind (lb-ft) = | 729 |
| M_allowable_snow (lb-ft) = | 524 |
| V_allowable_wind (lbs) = | 1008 |
| V_allowable_snow (lbs) = | 725 |



E' (psi) = 1600000

Load Calculation:

Dead Load Calculations:

| | |
|---------------------------------|-----|
| Panels Dead Load (psf) = | 3.0 |
| Roof 1 | |
| Roofing Weight (psf) = | 3.0 |
| Decking Weight (psf) = | 2.0 |
| Framing Weight (psf) = | 0.6 |
| Misc. Additional Weight (psf) = | 1.0 |
| Existing Dead Load (psf) = | 6.6 |
| Total Dead Load (psf) = | 9.6 |

Snow Load Calculations:

| | |
|---------------------------------|-------|
| Ground Snow Load, pg (psf) = | 15 |
| Min Flat Snow, pf_min (psf) = | NA |
| Min Sloped Snow, ps_min (psf) = | NA |
| Snow Importance Factor, Ic = | 1.0 |
| Exposure Factor, Ce = | 0.9 |
| Roof 1 | |
| Thermal Factor, Ct = | 1.2 |
| Flat Roof Snow, pf (psf) = | 11.34 |
| Slope Factor, Cs = | 1.00 |
| Sloped Roof Snow, ps (psf) = | 11 |

Wind Load Calculations:

| | |
|---------------------------------|-------|
| Ultimate Wind Speed (mph) = | 116 |
| Directionality Factor, kd = | 0.85 |
| Topographic Factor, kzt = | 1.0 |
| Roof 1 | |
| Velocity Press Exp Factor, kz = | 0.70 |
| Solar Equalization Factor, ya = | 1.00 |
| External Pressure Up, GCp_1 = | -0.95 |
| External Pressure Up, GCp_2 = | -1.15 |
| External Pressure Up, GCp_3 = | -1.15 |
| External Pressure Down, GCp = | 0.85 |
| Design Pressure Up, p_1 (psf) = | -19.5 |
| Design Pressure Up, p_2 (psf) = | -23.6 |
| Design Pressure Up, p_3 (psf) = | -23.6 |
| Design Pressure Down, p (psf) = | 17.4 |



Hardware Checks:

Attachment Checks:

| | Roof 1 |
|-------------------------------|---------------|
| Attachment Type = | QM HUG |
| Allowable Up Force (lbs) = | 1000 |
| | 2000 |
| Allowable Side Force (lbs) = | 240 |
| Applied Uplift Force (lbs) = | -111 |
| Uplift DCR = | 0.11 |
| Applied Down Force (lbs) = | 198 |
| Down DCR = | 0.10 |
| Applied Lateral Force (lbs) = | 74 |
| Lateral DCR = | 0.31 |

Roof Framing Checks:

Force Checks:

| | Roof 1 |
|----------------------------|---------------|
| LC1: D+S | |
| Applied Moment (lb-ft) = | 223 |
| Applied Shear (lbs) = | 168 |
| Allowable Moment (lb-ft) = | 524 |
| Allowable Shear (lbs) = | 725 |
| Moment DCR = | 0.43 |
| Shear DCR = | 0.23 |
| LC2: D+0.6W | |
| Applied Moment (lb-ft) = | 214 |
| Applied Shear (lbs) = | 161 |
| Allowable Moment (lb-ft) = | 729 |
| Allowable Shear (lbs) = | 1008 |
| Moment DCR = | 0.29 |
| Shear DCR = | 0.16 |
| LC3: D+0.75(S+0.6W) | |
| Applied Moment (lb-ft) = | 277 |
| Applied Shear (lbs) = | 208 |
| Allowable Moment (lb-ft) = | 729 |
| Allowable Shear (lbs) = | 1008 |
| Moment DCR = | 0.38 |
| Shear DCR = | 0.21 |
| LC4: 0.6D+0.6W | |



| | |
|----------------------------|-------------|
| Applied Moment (lb-ft) = | 63 |
| Applied Shear (lbs) = | 47 |
| Allowable Moment (lb-ft) = | 729 |
| Allowable Shear (lbs) = | 1008 |
| Moment DCR = | 0.09 |
| Shear DCR = | 0.05 |

Deflection Checks (Service Level):

Roof 1

LC1: D+L

| | |
|--------------------------|-------------|
| Deflection (in.) = | 0.06 |
| Deflection Limit (in.) = | 1.84 |
| Deflection DCR = | 0.03 |

LC2: S

| | |
|--------------------------|-------------|
| Deflection (in.) = | 0.05 |
| Deflection Limit (in.) = | 1.23 |
| Deflection DCR = | 0.04 |

LC3: W (Down)

| | |
|--------------------------|-------------|
| Deflection (in.) = | 0.03 |
| Deflection Limit (in.) = | 1.23 |
| Deflection DCR = | 0.03 |

LC4: W (Up)

| | |
|--------------------------|-------------|
| Deflection (in.) = | -0.04 |
| Deflection Limit (in.) = | 1.23 |
| Deflection DCR = | 0.03 |

Seismic Check:

Existing Weight:

| | |
|--|--------------|
| Wall Weight (psf) = | 17 |
| Tributary Wall Area (ft ²) = | 3000 |
| Total Wall Weight (lbs) = | 51000 |
| Roof Weight (psf) = | 7 |
| Roof Area (ft ²) = | 2400 |
| Total Roof Weight (lbs) = | 15844 |
| Total Existing Weight (lbs) = | 66844 |

Additional PV Weight:

| | |
|---|-------------|
| PV Panel Weight (lbs) = | 54 |
| Number of Panels = | 26 |
| Total Additional PV Weight (lbs) = | 1416 |



Weight Increase:

$$(\text{Existing W} + \text{Additional W})/(\text{Existing W}) = 1.02$$

The increase in weight as a result of the solar system is less than 10% of the existing structure and therefore no further seismic analysis is required.

Limits of Scope of Work and Liability:

Existing structure is assumed to have been designed and constructed following appropriate codes at time of erection, and assumed to have appropriate permits. The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgements. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. All PV modules, racking, and mounting equipment shall be designed and installed per manufacturer's approved installation specifications. The Engineer of Record and the engineering consulting firm assume no responsibility for misuse or improper installation. This analysis is not stamped for water leakage. Framing was determined based on information in provided plans and/or photos, along with engineering judgement. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction.