

STRUCTURAL CERTIFICATION REPORT*Roof-mounted Solar Panels*

April 16, 2024

To: Poly Energy
174 Corley Mill Rd
Lexington, SC 29072**Re: Marie Whitaker**
105 Wrench St
Erwin, NC 28339
AHJ: **Erwin (City), NC**

Poly Energy proposes to install new roof-mounted solar panels at this residence and asked *Right Angle Engineering* to review the existing structure for suitability. This letter summarizes the methods that were used to survey, evaluate, and certify the existing roof framing and the attachment of the new solar panels to it.

STRUCTURAL DESIGN

Building Code: North Carolina Residential Code 2018
Design Standards: ASCE 7-16
Snow: Ground: $p_g = 10.0$ psf | Flat Roof: $p_f = 11.93$ psf | Sloped Roof: $p_s = 8.75$ psf
Wind: Ultimate Wind Speed = 119.0 mph | Exposure = C
Seismic: Risk Category = 2 | Seismic Design Category = B | Site Class = D

STRUCTURE

Field Technicians from *Poly Energy* visited the site and observed the existing structure :

| Array Name | Panel Quantity | Roof Framing | Material | Pitch |
|------------|----------------|-----------------------------------|------------------|-------|
| Array 1 | 6 | Pre-Manufactured Truss 24" o.c. | Asphalt Shingles | 26° |

ANCHORAGE

The solar panel anchorage shall be installed according to the manufacturer's most current installation manual. Anchorage shall be staggered to distribute the load evenly to adjacent roof members. The solar panels should be mounted parallel (max 10 inches) to the roof surface.

| Array Name | Connection Type | Fastener | Max Anchorage Spacing |
|------------|-----------------|---------------------------------------------------------|-----------------------|
| Array 1 | Comp Mount | 5/16" lag screw (2.5" embedment) into roof substructure | 48" |

Installation Instructions

Solar panels and the equipment shall be installed per the manufacturer's installation specifications. Improper installation will void this certification. Deviations from the approved structural plans (including equipment substitutions) are not allowed without written approval from Right Angle Engineering. Prior to installation, the installer should:

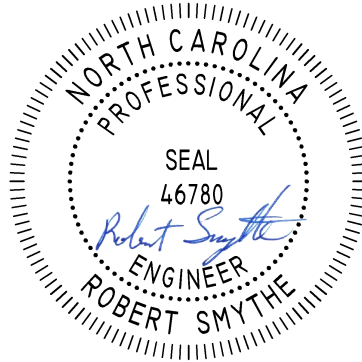
- Confirm that the existing structure matches the information provided in the structural survey, the approved installation plans and this certification.
- Identify discrepancies between this certification and the approved installation plans. If found, then this certification shall govern.
- Identify structural elements that are dangerous (cracked, broken, excessive sag, signs of overstress, rot, decay, fire, water). If found, installation shall cease until those elements are adequately abated and made to comply with the referenced building code.
- Verify that both the existing structure and the solar addition has been permitted through the AHJ.

STRUCTURAL CERTIFICATION

I certify the addition of solar panels on the roof of this structure does not cause the structure to become unsafe or make it generally less compliant with the life-safety requirements of the referenced building code. Based on the evaluation methods described below, for the loads that exist at this site, the existing framing will safely support the new solar panels if they are installed and attached correctly. Electrical design is not included in this certification.

| Array Name | Certification Method | Retrofits |
|------------|-----------------------------------------------------------------|---------------|
| Array 1 | Prescriptive method North Carolina Existing Building code 807.4 | None required |

Regards,



04/16/2024

Robert D. Smythe, P.E.
Right Angle Engineering

Job Details

| Roof Snow Load - ASCE 7-16 | |
|---------------------------------------------------------------------------------|-----------|
| Ground Snow Load (p_g) <i>Section 7.2</i> | 10.0 psf |
| Exposure Factor (C_e) <i>Table 7.3-1</i> | 0.9 |
| Thermal Factor (C_t) <i>Table 7.3-2</i> | 1.1 |
| Importance Factor (I_s) <i>Table 1.5-2</i> | 1 |
| Flat Roof Snow Load (p_f) <i>Equation 7.3-1</i> | 11.93 psf |
| Non-Slippery Surface Slope Factor (C_s) <i>Figure 7.4-1</i> | 1 |
| Slippery Surface Slope Factor (C_s) <i>Figure 7.4-1</i> | 0.73 |
| Roof Snow Load <i>Equation 7.4-1</i> | 11.93 psf |
| Reduced Snow Load (Slippery Surface) <i>Equation 7.4-1</i> | 8.75 psf |

| Design Criteria | |
|--------------------------------------------------------------------------|------------|
| Wind Speed (V_{ult}) <i>Local Design Criteria</i> | 119.0mph |
| Exposure Category | C |
| Risk Category | 2 |
| Mean Roof Height | 20 ft |
| Roof Type | Gable Roof |
| Building Type | Enclosed |

| Roof Live Load | |
|----------------------------------------------------------------|---------|
| Existing Roof Live Load <i>ASCE 7-16 Table 4.3-1</i> | 20 psf |
| Roof Live Load with Solar Panels | 0.0 psf |

| ¹ Roof Dead Load | | | |
|-------------------------------|---------|---------------------------------|---------|
| Asphalt Shingles | 2.0 psf | No Drywall | 0.0 psf |
| 5/8" Plywood Sheathing | 2.0 psf | Solar Panel Array | 2.8 psf |
| Roof Framing | 4 psf | Dead Load Without Panels | 9.2 psf |
| Insulation | 1.2 psf | | |

¹Roof Dead Load is taken from the worst case scenario dead load from all arrays of the job in order to provide a more conservative evaluation.

Array 1

| Array Details | | GCP Zones | | | |
|----------------------------------------------------------------------------------------------------------|----------------------------|----------------------------------------------------------------------------------------------------|--------------|-------|--------|
| | | 1/2e | 2n/2r /3e | 3r | |
| Roof Framing | Pre-Manufactured Truss | GC_p <i>Figure 30.3-(2A-5B)</i> | -1.5 | -2.5 | -3.6 |
| Spacing | 24.0" | Design Pressure Up [psf] <i>Equation 29.4-7 $\gamma_a=0.75 \gamma_E=1.0$,</i> | -31.1 | -51.8 | -74.6 |
| Beam Span | 22.0' | Factored Design Pressure Up [psf] <i>ASD LC (.6D + .6W)</i> | -17.1 | -29.6 | -43.3 |
| Roof Pitch | 26° | Exposed Design Pressure Up [psf] <i>$\gamma_a=0.75 \gamma_E=1.5$,</i> | -46.6 | -77.7 | -111.9 |
| Solar Rails | Pegasus Rail | Design Pressure Down [psf] | 10.4 | 10.4 | 10.4 |
| Module Type | Q.PEAK DUO BLK ML-G10+ 405 | Tributary Area [ft²] | 37.5 | 22.5 | 15.4 |
| Module Wind rating [psf] <i>Safety Factor = 1.5</i> | 55.69 | Maximum Connection Spacing [in] | 146 | 88 | 60 |
| Panel Quantity | 6 | Maximum Rail Span [in] | 67 | 67 | 61 |
| Panel Array Area | 126.81 ft ² | Maximum Rail Cantilever [in] | 16 | 16 | 16 |
| Panel Orientation | Portrait | Design Connection Spacing [in] | 48 | 48 | 48 |
| Lag Screw Embedment | 2.5" | Deign Connection Spacing (exposed) [in] | 67 | 48 | 24 |
| Roof Attachment Type | Comp Mount 5/16" lag screw | | | | |
| Shear Capacity <i>NDS 2015 Table 12K</i> | 190.0 lbs | | | | |
| Pullout Capacity <i>NDS 2015 Table 12K</i> | 665.0 lbs | | | | |
| Velocity Pressure <i>Equation 26.10-1 ($K_z=0.9, K_{ht}=1, K_d=0.85, K_e=0.99$)</i> | 27.61 psf | | | | |

| Prescriptive Method: North Carolina Existing Building Code 807.4 | |
|------------------------------------------------------------------|------------|
| Total load on member without solar | 1372.4 lbs |
| Total load on member with solar | 1372.4 lbs |
| Percentage of total design load on member with solar | 1.0% |

The 2018 North Carolina Existing Building section 807.4 indicates that alterations to an existing building that results in less than a 10.0% increase in the total stress may be performed without a structural evaluation of the existing building. As demonstrated in the above calculations, the additional weight of the solar panels will be less than 10.0% increase in the gravity loading and therefore stress on the existing roof framing. Load case before and load case after solar panels have been added have both been considered.