

10/17/2023

Palmetto Solar

997 Morrison Dr, Ste 200, Charleston, SC, 29403



Attn.: To Whom It May Concern

Job: Eduardo Carranza

Project Address: 94 Mary Robertson Street, Dunn, NC, 28334

The following calculations are for the structural engineering design of the photovoltaic panels and are valid only for the structural info referenced in the stamped plan set. I certify that the roof structure has sufficient structural capacity for the applied PV loads. All mounting equipment shall be designed and installed per manufacturer's approved installation specifications.

Design Criteria

Code: 2018 NCSBC, IBC 2018, ASCE 7-16

Live Load: 20 psf

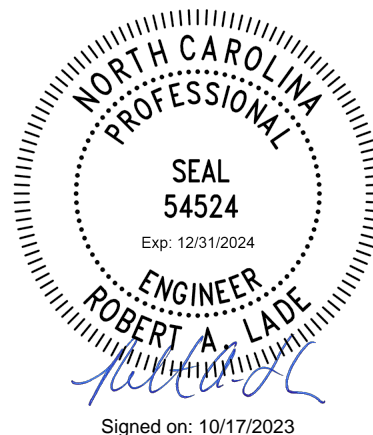
Ult Wind Speed: 120.0 mph

Exposure Cat: C

Ground Snow: 20.0 psf

Min Snow Roof: N/A

Current Renewables Engineering Inc.
Professional Engineer
info@currentrenewableseng.com



Roof Properties:

	Check 1
Roof Type:	Shingle
Roof Pitch (deg):	18
Mean Roof Height (ft):	23.00
Attachment Trib Width (ft):	3.25
Attachment Spacing (ft):	4.00
Framing Type:	Rafter
Framing Size:	2x4
Framing OC Spacing (in):	24.00
Section Thickness, b (in):	1.50
Section Depth, d (in):	3.50
Section Modulus, S _x (in ³):	3.062
Moment of Inertia, I _x (in ⁴):	5.359
Unsupported Span (ft):	10.00
Upper Chord Length (ft):	21.00
Deflection Limit D+L (in):	4.200
Deflection Limit S or W (in):	2.800
Framing Upgrade:	No
Sister Size:	N/A
Wood Species:	DF
Wood F _b (psi):	900.00
Wood F _v (psi):	180.00
Wood E (psi):	1,600,000.00
C _D (Wind):	1.60
C _D (Snow):	1.15
C _{LS} :	1.00
C _M = C _t = C _L = C _i :	1.00
C _F :	1.50
C _{fu} :	1.00
C _r :	1.15
F ['] b Wind (psi):	2,484.00
F ['] b Snow (psi):	1,785.37
F ['] v Wind (psi):	288.00
F ['] v Snow (psi):	207.00
Moment Allowable Wind (lb-ft):	633.94
Moment Allowable Snow (lb-ft):	455.64
V Allowable Wind (lbs):	1,008.00
V Allowable Snow (lbs):	724.50
E' (psi):	1,600,000

Load Calculations:**Dead Load Calculations:**

	Check 1
Panel Dead Load (psf):	3.00
Roofing Weight (psf):	3.00
Decking Weight (psf):	2.00
Framing Weight (psf):	0.60
Misc. Additional Weight (psf):	1.00
Existing Dead Load (psf):	6.60
Total Dead Load (psf):	9.60

Wind Load Calculations:

	Check 1
Ultimate Wind Speed (mph):	120.00
Directionality Factor, kd:	0.85
Topographic Factor, kzt:	1.00
Velocity Press Exp Factor, kz:	0.93
Velocity Pressure, qz (psf):	28.91
External Pressure Up, GCp ₁ :	-2.00
External Pressure Up, GCp ₂ :	-2.57
External Pressure Up, GCp ₃ :	-3.06
External Pressure Down, GCp:	0.46
Design Pressure Up, p ₁ :	-39.90
Design Pressure Up, p ₂ :	-51.26
Design Pressure Up, p ₃ :	-61.01
Design Pressure Down, p (psf):	16.00

Snow Load Calculations:

	Check 1
Ground Snow Load, pg (psf):	20.00
Min Flat Snow, pf_min (psf):	0.00
Min Sloped Snow, ps_min (psf):	0.00
Snow Importance Factor, Ic:	1.00
Exposure Factor, Ce:	0.90
Thermal Factor, Ct:	1.10
Flat Roof Snow, pf (psf):	13.86
Slope Factor, Cs:	0.87
Sloped Roof Snow, ps (psf):	12.01

Hardware Checks:**Lag Screw Check:**

	Check 1
Ref. Withdrawal Value, W (lb/in):	266.00
($C_m = C_t = C_{eg} = 1.0$) C_D :	1.60
Adjusted Withdrawal Value, W' (lb/in):	425.60
Penetration, p (in.):	2.50
Allowable Withdrawal Force, W'p (lbs):	1,064.00
Applied Uplift Force (lbs):	-246.94
Uplift DCR:	0.232
Ref. Lateral Value, Z (lbs):	270.00
($C_m = C_t = C_{\delta} = C_{eg} = 1.0$) C_D :	1.15
Adjusted Lateral Value, Z' (lbs):	310.50
Applied Lateral Force (lbs):	60.31
Angle of Resultant Force, alpha (deg):	76.28
Adjusted Interaction Lateral Value, Z'alpha (lbs):	936.14
Lateral DCR:	0.064

Roof Framing Checks:**Force Checks****LC1: D+S****Check 1**

Applied Moment (lb-ft):	449.5
Applied Shear (lbs):	259.3
Allowable Moment (lb-ft):	455.6
Allowable Shear (lbs):	724.5
Moment DCR:	0.986
Shear DCR:	0.358

LC2: D+0.6W**Check 1**

Applied Moment (lb-ft):	399.3
Applied Shear (lbs):	230.3
Allowable Moment (lb-ft):	633.9
Allowable Shear (lbs):	1,008.0
Moment DCR:	0.630
Shear DCR:	0.229

LC3: D+0.75(S+0.6W)**Check 1**

Applied Moment (lb-ft):	536.8
Applied Shear (lbs):	309.6
Allowable Moment (lb-ft):	633.9
Allowable Shear (lbs):	1,008.0
Moment DCR:	0.847
Shear DCR:	0.307

LC4: 0.6D+0.6W**Check 1**

Applied Moment (lb-ft):	415.5
Applied Shear (lbs):	239.7
Allowable Moment (lb-ft):	633.9
Allowable Shear (lbs):	1,008.0
Moment DCR:	0.655
Shear DCR:	0.238

Deflection Checks (Service Level):**LC1: D+L**

	Check 1
Deflection (in.):	1.069
Deflection Limit (in.):	4.200
Deflection DCR:	0.255

LC2: S

	Check 1
Deflection (in.):	0.322
Deflection Limit (in.):	2.800
Deflection DCR:	0.115

LC3: W (Down)

	Check 1
Deflection (in.):	0.180
Deflection Limit (in.):	2.800
Deflection DCR:	0.064

LC4: W (Up)

	Check 1
Deflection (in.):	0.449
Deflection Limit (in.):	2.800
Deflection DCR:	0.160

Seismic Check:

Existing Weight:

Wall Weight (psf):	17.00
Tributary Wall Area (ft ²):	2,880.00
Total Wall Weight (lbs):	48,960.00
Roof Weight (psf):	6.60
Roof Area (ft ²):	2,303.00
Total Roof Weight (lbs):	15,203.40
Total Existing Weight (lbs):	64,163.40

Additional PV Weight:

PV Panel Weight (lbs):	64.35
Number of Panels:	15
Total Additional PV Weight (lbs):	965.25

Weight Increase:

$(\text{Existing W} + \text{Additional W}) \div (\text{Existing W}) = 101.50\%$

The increase in weight as a result of the solar system is less than 10% of the existing structure. Therefore, no further seismic analysis is required.

Limits of Scope of Work and Liability:

Existing structure is assumed to have been designed and constructed following appropriate codes at time of erection, and assumed to have appropriate permits. The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgements. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. All PV modules, racking, and mounting equipment shall be designed and installed per manufacturer's approved installation specifications. The Engineer of Record and the engineering consulting firm assume no responsibility for misuse or improper installation. This analysis is not stamped for water leakage. Framing was determined based on information in provided plans and/or photos, along with engineering judgement. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction.