

08-31-2023

Palmetto Solar

997 Morrison Dr, Ste 200, Charleston, SC 29403

Attn.: To Whom It May Concern

re job: Mikale Bohlmann

207 Barnsley Rd, Angier, NC 27501, USA

The following calculations are for the structural engineering design of the photovoltaic panels and are valid only for the structural info referenced in the stamped plan set.

I certify that the roof structure has sufficient structural capacity for the applied PV loads.

All mounting equipment shall be designed and installed per manufacturer's approved installation specifications.

**Design Criteria:**

Code: 2018 NCSBC, IBC 2018, ASCE 7-16,

Live Load: 20 psf

Ult Wind Speed: 117 mph

Exposure Cat: C

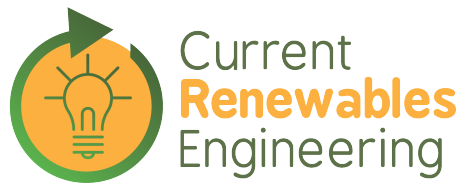
Ground Snow: 20 psf

Min Snow Roof: N/A

Current Renewables Engineering Inc.  
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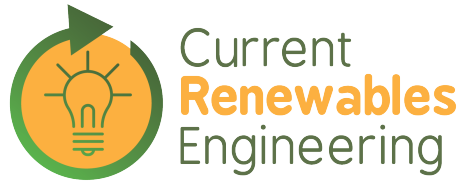


**Roof Properties:**

Roof 1

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Roof Type = Shingle  
Roof Pitch (deg) = 27.0  
Mean Root Height (ft) = 13.0  
Attachment Trib Width (ft) = 3.3  
Attachment Spacing (ft) = 4.0  
Framing Type = Rafter  
Framing Size = 2x4  
Framing OC Spacing (in.) = 24.0  
Section Thickness, b (in) = 1.5  
Section Depth, d (in) = 3.5  
Section Modulus,  $S_x$  (in<sup>3</sup>) = 3.062  
Moment of Inertia,  $I_x$  (in<sup>4</sup>) = 5.359  
Unsupported Span (ft) = 9.5  
Upper Chord Length (ft) = 20.0  
Deflection Limit D+L (in) = 4.0  
Deflection Limit S or W (in) = 2.667  
Framing Upgrade = No  
Sister Size = NA  
Wood Species = DF  
Wood Fb (psi) = 900.0  
Wood Fv (psi) = 180.0  
Wood E (psi) = 1600000.0  
 $C_D$  (wind) = 1.6  
 $C_d$  (snow) = 1.15  
 $C_{LS}$  = 1.0  
 $C_M = C_t = C_L = C_i$  = 1.0  
 $C_F$  = 1.5  
 $C_{fu}$  = 1.0  
 $C_r$  = 1.15  
F'b wind (psi) = 2484.0  
F'b snow (psi) = 1785.37  
F'v wind (psi) = 288.0  
F'v snow (psi) = 207.0  
M allowable wind (lb-ft) = 633.94  
M allowable snow (lb-ft) = 455.64  
V allowable wind (lbs) = 1008.0  
V allowable snow (lbs) = 724.5  
E' (psi) = 1600000.0



**Load Calculation:**

**Dead Load Calculations:**

Roof 1

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Panels Dead Load (psf) = 3.0  
Roofing Weight (psf) = 3.0  
Decking Weight (psf) = 2.0  
Framing Weight (psf) = 0.602  
Misc. Additional Weight (psf) = 1.0  
Existing Dead Load (psf) = 6.602  
Total Dead Load (psf) = 9.602

**Wind Load Calculations:**

Ultimate Wind Speed (mph) = 117.441  
Directionality Factor,  $k_d$  = 0.85  
Topographic Factor,  $k_{zt}$  = 1.0  
Velocity Press Exp Factor,  $k_z$  = 0.849  
Velocity Pressure,  $q_z$  (psf) = 25.477  
External Pressure Up,  $GCp\_1$  = -1.5  
External Pressure Up,  $GCp\_2$  = -2.167  
External Pressure Up,  $GCp\_3$  = -2.453  
External Pressure Down,  $GCp$  = 0.465  
Design Pressure Up,  $p\_1$  (psf) = -26.369  
Design Pressure Up,  $p\_2$  (psf) = -38.098  
Design Pressure Up,  $p\_3$  (psf) = -43.122  
Design Pressure Down,  $p$  (psf) = 16.0

**Snow Load Calculations:**

Ground Snow Load,  $p_g$  (psf) = 20.0  
Min Flat Snow,  $pf\_min$  (psf) = 0.0  
Sloped Snow,  $ps\_min$  (psf) = 0.0  
Snow Importance Factor,  $I_c$  = 1.0  
Exposure Factor,  $C_e$  = 0.9  
Thermal Factor,  $C_t$  = 1.1  
Flat Roof Snow,  $pf$  (psf) = 13.86  
Slope Factor,  $C_s$  = 1.0  
Sloped Roof Snow,  $ps$  (psf) = 13.86

**Lag Screw Checks:**

## Roof 1

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Ref. Withdrawal Value,  $W$  (lb/in) = 266.0  
 ( $C_m = C_t = C_{eg} = 1.0$ ) CD = 1.6  
 Adjusted Withdrawal Value,  $W'$  (lb/in) = 425.6  
 Lag Penetration,  $p$  (in.) = 2.5  
 Allowable Withdrawal Force,  $W'p$  (lbs) = 1064.0  
 Applied Uplift Force (lbs) = -159.455  
 Uplift DCR = 0.15  
 Ref. Lateral Value,  $Z$  (lbs) = 266.0  
 ( $C_m = C_t = C_{\Delta} = C_{eg} = 1.0$ ) CD = 1.15  
 Adjusted Lateral Value,  $Z'$  (lbs) = 310.5  
 Applied Lateral Force (lbs) = 101.036  
 Angle of Resultant Force,  $\alpha$  (deg) = 1.006  
 Adjusted Interaction Lateral Value,  $Z'\alpha$  (lbs) = 627.654  
 Lateral DCR = 0.161



**Roof Framing Checks:**

**Force Checks:**

LC1: D+S

Roof 1

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Applied Moment (lb-ft) = 439.0  
Applied Shear (lbs) = 267.0  
Allowable Moment (lb-ft) = 456.0  
Allowable Shear (lbs) = 724.0  
Moment DCR = 0.964  
Shear DCR = 0.369

LC2: D+0.6W

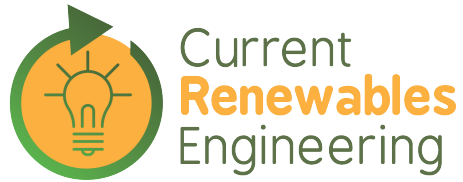
Applied Moment (lb-ft) = 360.0  
Applied Shear (lbs) = 219.0  
Allowable Moment (lb-ft) = 634.0  
Allowable Shear (lbs) = 1008.0  
Moment DCR = 0.567  
Shear DCR = 0.217

LC3: D+0.75(S+0.6W)

Applied Moment (lb-ft) = 509.0  
Applied Shear (lbs) = 310.0  
Allowable Moment (lb-ft) = 634.0  
Allowable Shear (lbs) = 1008.0  
Moment DCR = 0.803  
Shear DCR = 0.307

LC4: 0.6D+0.6W

Applied Moment (lb-ft) = 288.0  
Applied Shear (lbs) = 175.0  
Allowable Moment (lb-ft) = 634.0  
Allowable Shear (lbs) = 1008.0  
Moment DCR = 0.454  
Shear DCR = 0.174



**Deflection Checks (Service Level):**

LC1: D+L

Deflection (in.) = 0.803  
Deflection Limit (in.) = 4.0  
Deflection DCR = 0.201

LC2: S

Deflection (in.) = 0.303  
Deflection Limit (in.) = 2.667  
Deflection DCR = 0.114

LC3: W (Down)

Deflection (in.) = 0.147  
Deflection Limit (in.) = 2.667  
Deflection DCR = 0.055

LC4: W (Up)

Deflection (in.) = 0.242  
Deflection Limit (in.) = 2.667  
Deflection DCR = 0.091



**Seismic Check:**

**Existing Weight:**

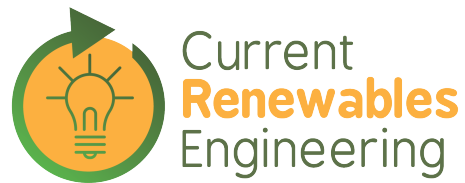
Wall Weight (psf) = 17.0  
Tributary Wall Area (ft<sup>2</sup>) = 880.0  
Total Wall Weight (lbs) = 14960.0  
Roof Weight (psf) = 6.602  
Roof Area (ft<sup>2</sup>) = 1920.0  
Total Roof Weight (lbs) = 12675.0  
**Total Existing Weight (lbs) = 27635.0**

**Total Additional PV Weight (lbs) = 1158.3**

**Weight Increase:**

$$\frac{\text{(Existing W + Additional W)}}{\text{(Existing W)}} = 1.042$$

The increase in weight as a result of the solar system is less than 10% of the existing structure and therefore no further seismic analysis is required.



**Limits of Scope of Work and Liability:**

The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgments. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. This analysis is not stamped for water leakage. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction.