



July 31, 2023

To: Blue Raven Solar  
1403 North Research Way, Building J  
Orem, UT. 84097

Subject: Certification Letter  
Mallick Residence  
486 Silver Maple Dr  
Fuquay-Varina, NC. 27526

To Whom It May Concern,

A jobsite observation of the condition of the existing framing system was performed by an audit team of Blue Raven Solar. All review is based on these observations and the design criteria listed below and only deemed valid if provided information is true and accurate.

On the above referenced project, the roof structural framing has been reviewed for additional loading due to the installation of the solar PV addition to the roof. The structural review only applies to the section of the roof that is directly supporting the solar PV system and its supporting elements. The observed roof framing is described below. If field conditions differ, contractor to notify engineer prior to starting construction.

The roof structure of (MP1) consists of composition shingle on roof plywood that is supported by pre-manufactured trusses that are spaced at @ 24"o.c.. The top chords, sloped at 27 degrees, are 2x4 sections, the bottom chords are 2x4 sections and the web members are 2x4 sections. The truss members are connected by steel gusset plates. The max unsupported projected horizontal top chord span is approximately 7'-3".

The existing roof framing system of (MP1) is judged to be adequate to withstand the loading imposed by the installation of the solar panels. No reinforcement is necessary.

The spacing of the solar standoffs should be kept at 72" o.c. for landscape and 48" o.c. for portrait orientation, with a staggered pattern to ensure proper distribution of loads.

The scope of this report is strictly limited to an evaluation of the fastener attachment, underlying framing and supporting structure only. The attachment's to the existing structure are required to be in a staggered pattern to ensure proper distribution of loading. All panels, racking and hardware shall be installed per manufacturer specifications and within specified design limitations. All waterproofing shall be provided by the manufacturer.

Note: Seismic check is not required since  $S_s < .4g$  and Seismic Design Category (SDC) < B

Design Criteria:

- Applicable Codes = 2018 North Carolina State Building Code (NCSBC), ASCE 7-10
- Roof Dead Load = 7 psf (MP1)
- Roof Live Load = 20 psf
- Wind Speed = 115 mph (Vult), Exposure C, Risk Category II
- Ground Snow Load = 15 psf - Roof Snow Load = 10.5 psf
- Attachment: 1 - 5/16 dia. lag screw with 2.5 inch min. embedment depth, at spacing shown above.

Please contact me with any further questions or concerns regarding this project.

Sincerely,

John Calvert, P.E.  
Project Engineer



Digitally signed by  
John A. Calvert  
Date: 2023.07.31  
10:40:07 -06'00'



**Gravity Loading**

Roof Snow Load Calculations		
$p_g$ = Ground Snow Load =	15 psf	
$p_f = 0.7 C_e C_t I p_g$		(ASCE7 - Eq 7-1)
$C_e$ = Exposure Factor =	1	(ASCE7 - Table 7-2)
$C_t$ = Thermal Factor =	1	(ASCE7 - Table 7-3)
$I$ = Importance Factor =	1	
$p_f$ = Flat Roof Snow Load =	10.5 psf	
$p_s = C_s p_f$		(ASCE7 - Eq 7-2)
$C_s$ = Slope Factor =	1	
<b><math>p_s</math> = Sloped Roof Snow Load =</b>	<b>10.5 psf</b>	

PV Dead Load = 3 psf (Per Blue Raven Solar)	
DL Adjusted to 27 Degree Slope	3.37 psf
PV System Weight	
Weight of PV System (Per Blue Raven Solar)	3.0 psf
X Standoff Spacing =	4.00 ft
Y Standoff Spacing =	6.08 ft
Standoff Tributary Area =	24.33 sft
<b>Point Loads of Standoffs</b>	<b>73 lb</b>
Note: PV standoffs are staggered to ensure proper distribution of loading	

Roof Live Load = 20 psf	
Note: Roof live load is removed in area's covered by PV array.	

Roof Dead Load (MP1)		
Composition Shingle	4.00	
Roof Plywood	2.00	
2x4 Top Chords @ 24"o.c.	0.73	
Vaulted Ceiling	0.00	(Ceiling Not Vaulted)
Miscellaneous	0.27	
<b>Total Roof DL (MP1)</b>	<b>7.0 psf</b>	
DL Adjusted to 27 Degree Slope	7.9 psf	



### Wind Calculations

#### Per ASCE 7-10 Components and Cladding

Input Variables	
Wind Speed	115 mph
Exposure Category	C
Roof Shape	Hip/Gable
Roof Slope	27 degrees
Mean Roof Height	20 ft
Effective Wind Area	21.3 ft

Design Wind Pressure Calculations	
Wind Pressure $P = qh * G * C_n$	
$qh = 0.00256 * K_z * K_{zt} * K_d * V^2$	(Eq. 30.3-1)
$K_z$ (Exposure Coefficient) = 0.9	(Table 30.3-1)
$K_{zt}$ (topographic factor) = 1	(Fig. 26.8-1)
$K_d$ (Wind Directionality Factor) = 0.85	(Table 26.6-1)
$V$ (Design Wind Speed) = 115 mph	(Fig. 26.5-1A)
Risk Category = II	(Table 1.5-1)
$qh = 25.90$	
$0.6 * qh = 15.54$	

Standoff Uplift Calculations-Portrait				
	Zone 1	Zone 2	Zone 3	Positive
$G_{Cp} =$	-0.85	-1.52	-2.42	0.43
Uplift Pressure =	-13.20 psf	-23.67 psf	-37.59 psf	11.1 psf
X Standoff Spacing =	4.00	4.00	2.67	
Y Standoff Spacing =	6.08	3.041666667	3.041666667	
Tributary Area =	24.33	12.17	8.11	
Dead Load on Attachment =	73.00	36.50	24.33	
Footing Uplift (0.6D+0.6W) =	-277 lb	-266 lb	-290 lb	

Standoff Uplift Calculations-Landscape				
	Zone 1	Zone 2	Zone 3	Positive
$G_{Cp} =$	-0.85	-1.52	-2.42	0.43
Uplift Pressure =	-13.20 psf	-23.67 psf	-37.59 psf	10.0 psf
X Standoff Spacing =	6.00	6.00	4.00	
Y Standoff Spacing =	3.50	1.75	1.75	
Tributary Area =	21.00	10.50	7.00	
Dead Load on Attachment =	63.00	31.50	21.00	
Footing Uplift (0.6D+0.6W) =	-239 lb	-230 lb	-251 lb	

Standoff Uplift Check	
Maximum Design Uplift =	-290 lb
Standoff Uplift Capacity =	450 lb
450 lb capacity > 290 lb demand	Therefore, OK

Fastener Capacity Check	
Fastener =	1 - 5/16" dia. lag
Number of Fasteners =	1
Embedment Depth =	2.5
Pullout Capacity Per Inch =	250 lb
Fastener Capacity =	625 lb
w/ F.S. of 1.5 & DOL of 1.6 =	667 lb
667.2 lb capacity > 290 lb demand	Therefore, OK

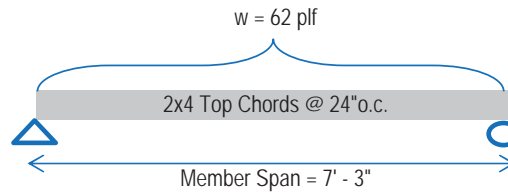


**Framing Check**  
**(MP1)**

PASS

Dead Load 7.9 psf  
 PV Load 3.4 psf  
 Live Load 20.0 psf

Governing Load Combo = DL + LL  
 Total Load 31.2 psf



Member Properties				
Member Size	S (in <sup>3</sup> )	I (in <sup>4</sup> )	Lumber Sp/Gr	Member Spacing
2x4	3.06	5.36	DF#2	@ 24"o.c.

Check Bending Stress								
Fb (psi) =	f <sub>b</sub>	x	C <sub>d</sub>	x	C <sub>f</sub>	x	C <sub>r</sub>	(NDS Table 4.3.1)
	900	x	1.25	x	1.5	x	1.15	
Allowed Bending Stress = 1940.6 psi								

Maximum Moment =  $(wL^2) / 8$   
 = 410.2932 ft#  
 = 4923.518 in#

Actual Bending Stress = (Maximum Moment) / S  
 = 1607.7 psi

Allowed > Actual -- 82.9% Stressed -- Therefore, OK

Check Deflection		
Allowed Deflection (Total Load) =	$L/180$	(E = 1600000 psi Per NDS)
	= 0.483 in	
Deflection Criteria Based on =	Continuous Span	
Actual Deflection (Total Load) =	$(wL^4) / (185E*I)$	
	= 0.188 in	
	= L/463 > L/180	Therefore OK
Allowed Deflection (Live Load) =	$L/240$	
	0.362 in	
Actual Deflection (Live Load) =	$(wL^4) / (185E*I)$	
	0.121 in	
	L/720 > L/240	Therefore OK

Check Shear		
Member Area = 5.3 in <sup>2</sup>	F <sub>v</sub> (psi) = 180 psi	(NDS Table 4A)
Allowed Shear = F <sub>v</sub> * A = 945 lb	Max Shear (V) = w * L / 2 =	226 lb

Allowed > Actual -- 24% Stressed -- Therefore, OK