

STRUCTURAL CALCULATIONS FOR PV INSTALLATION

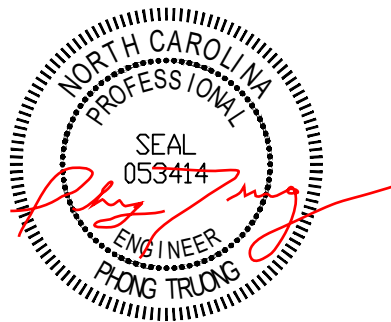
Prepared for



Solar-Roof-Check

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USER:	Kevin Nguyen
COMPANY NAME:	Solar-Roof-Check
SRC JOB ID:	35092
JOB REPORT DATE:	2022-07-21
JOB NUMBER:	123
JOB NAME:	Petri Brand
JOB ADDRESS:	111 Clearwater Harbor Sanford, NC 27332



Program Version: 2020-05-08:7

Contact: Support@Solar-Roof-Check.com

Phone: 844-783-5483

USER INPUT:

1. Ceiling Type		:= 1/2 gyp. Bd.
2. Collar Tie Spacing (ft)		:= 0
3. Roof Coverage %		:= 24.51
4. Roof Framing Size		:= 2x4@24
5. Ground Snow (psf)		:= 10
6. Vertical Snow Load on slope (psf)		:= 9.4
7. Lag Screw Diameter (in)	:= d	:= 5/16
8. Lag Screw Embedment (in)	:= e	:= 2.5
9. Overall Span (ft)		:= 30
10. PV Weight (psf)	:= DL_{PV}	:= 3.00
11. Rafter Sloped Span (ft)	:= L_r	:= 7.5
12. Rail System		:= RailLess
13. Roofing Type		:= Comp. Shingle
14. Roof Mean Height (ft)	:= h	:= 25
15. Roof Slope (degrees)	:= θ	:= 20
16. Roof Structure Type		:= Truss
17. Sloped Ceiling?		:= No
18. S _{fh} = RF Mount Horizontal Spacing (ft)		:= 4.00
19. S _{fv} = RF Mount Vertical Spacing (ft)		:= 3.42
20. RF Mount Staggered		:= Yes
21. Wind Exposure		:= C
22. Wind Speed (mph)	:= V	:= 120
23. L _{PV} =PV Length (in)		:= 74
24. W _{PV} = PV Width (in)		:= 41
25. Gable, Hip, or Flat/Monoslope		:= Gable Roof
26. Roof Overhangs		:= Yes
27. S _r = Rafter Spacing (ft)		:= 2.00
28. PV Orientation		:= Landscape
29. Elevation Above Sea Level (ft)	:= Elev	:= 398

FORMULA SYMBOLS:

TA_{fp}	:=	Tributary Area to RF Mounts with Portrait PV Orientation (sf)
TA_{fl}	:=	Tributary Area to RF Mounts with Landscape PV Orientation (sf)
TA_r	:=	Tributary Area to Rafters (sf)
γ_{af}	:=	Pressure Equalization Factor for RF Mounts with Portrait PV Orientation
γ_{al}	:=	Pressure Equalization Factor for RF Mounts with Landscape PV Orientation
γ_{ar}	:=	Pressure Equalization Factor for Rafters
P_{1fp}	:=	Design Wind Pressure to RF Mounts with Portrait PV Orientation – Zone 1 (psf)
P_{2fp}	:=	Design Wind Pressure to RF Mounts with Portrait PV Orientation – Zone 2 (psf)
P_{1fl}	:=	Design Wind Pressure to RF Mounts with Landscape PV Orientation – Zone 1 (psf)
P_{2fl}	:=	Design Wind Pressure to RF Mounts with Landscape PV Orientation – Zone 2 (psf)
P_{rup}	:=	Design Wind Pressure Up to Rafters (psf)
P_{rdn}	:=	Design Wind Pressure Down to Rafters (psf)
GC_{p1}	:=	External Pressure Coefficient for RF Mounts – Zone 1 (depends on TA Fastener)
GC_{p2}	:=	External Pressure Coefficient for RF Mounts – Zone 2 (depends on TA Fastener)
GC_{pup}	:=	External Pressure Coefficient, Wind Up for Rafters (depends on TA_r)
GC_{pdn}	:=	External Pressure Coefficient, Wind Down for Rafters (depends on TA_r)
S	:=	Snow on Roof (psf)
S_{fh}	:=	RF Mount Horizontal Spacing (ft)
S_{fv}	:=	RF Mount Vertical Spacing (ft)
S_r	:=	Rafter Spacing (ft)
L_{pv}	:=	PV Length (ft)
W_{pv}	:=	PV Width (ft)
L_r	:=	Span of Rafter (ft)
γ_E	:=	Edge Array Factor

Dead Loads (DL): = psf

Roofing		:= 2.20
Ply		:= 1.50
Rafter		:= 0.65
Miscellaneous (Misc)		:= 1.00
Clg. Joists		:= 0.65
Insulation (Insul)		:= 0.50
Ceiling		:= 2.20
<u>Flat Ceiling</u>	$DL_{tc} = \text{Roofing} + \text{Ply} + \text{Rafter} + \text{Misc (psf)}$:= 5.35
	$DL_{bc} = \text{Clg.joists} + \text{Insul.} + \text{Ceiling (psf)}$:= 3.35
	$wDL_{tc} = DL_{tc} \times Sr \text{ (plf)}$:= 10.70
<u>Sloped Ceiling</u>	$DL_{tc} = DL_{tc} + \text{Insul.} + \text{Ceiling (psf)}$:= N/A
	$wDL_{tc} = DL_{tc} \times Sr \text{ (plf)}$:= N/A

DESIGN CRITERIA	ASCE REFERENCE		
Risk Category II	Table 1.5-1		
Importance Factor	Table 1.5-2	I	:= 1.00
Basic Wind Speed	Fig. 26.5-1b	V	:= 120
Wind Directionality	Table 26.6-1	K _d	:= 0.85
Exposure Category :	Sect. 26.7.3		:= C
Topographic Factor	Sect. 26.8	K _{zt}	:= 1.00
Elevation above Sea Level	Table 26.9-1	K _e	:= 0.99
Velocity Pressure Exposure Coeff.	Table 26.10-1	K _z	:= 0.94
Velocity Pressure	Eq. 26.10-1	$q_h = .00256 \times K_z \times K_{zt} \times K_d \times K_e \times V^2$:= 29.03
Edge Array Factor	Sect. 29.4.4	$\gamma_E \text{ (Zone 1)} = 1.0 \quad \gamma_E \text{ (Zone2)} = 1.5$	
PV Pressure Equalization Factor	Fig. 29.4-8	γ_a	:= 0.70

RF MOUNT- Wind UP Pressure (Note: Divide TA by 2 for 2 Rail System)

Portrait Orientation:

ASCE REFERENCE		$TA_{fl} = S_{fh} \times L_{pv}$:=	N/A
Fig. 29.4-8		γ_a	:=	N/A
Fig. 30.3-2*	Zone 1:	GC_{p1}	:=	N/A
Eq. 29.4-7		$P_{1f} = q_h \times GC_{p1} \times \gamma_E \times \gamma_a$:=	N/A
Fig. 30.3-2*	Zone 2e:	GC_{p2e}	:=	N/A
Eq. 29.4-7		$P_{2ef} = q_h \times GC_{p2e} \times \gamma_E \times \gamma_a$:=	N/A
Fig. 30.3-2*	Zone 2r:	GC_{p2r}	:=	N/A
Eq. 29.4-7		$P_{2rf} = q_h \times GC_{p2r} \times \gamma_E \times \gamma_a$:=	N/A

Landscape Orientation:

ASCE REFERENCE		$TA_{fl} = S_{fh} \times W_{pv}$:=	13.67
Fig. 29.4-8		γ_a	:=	0.70
Fig. 30.3-2*	Zone 1:	GC_{p1}	:=	2.00
Eq. 29.4-7		$P_{1f} = q_h \times GC_{p1} \times \gamma_E \times \gamma_a$:=	40.90
Fig. 30.3-2*	Zone 2e:	GC_{p2e}	:=	2.50
Eq. 29.4-7		$P_{2ef} = q_h \times GC_{p2e} \times \gamma_E \times \gamma_a$:=	76.69
Fig. 30.3-2*	Zone 2r:	GC_{p2r}	:=	2.81
Eq. 29.4-7		$P_{2rf} = q_h \times GC_{p2r} \times \gamma_E \times \gamma_a$:=	86.08

***NOTE:** For Flat and Monoslope Roofs, refer to Figures 30.3-5A and 30.3-5B

RAFTERS- Wind UP Pressure

ASCE REFERENCE		$TA_r = S_{fh} \times L_r$:= 30.00
Fig. 29.4-8		γ_{ar}	:= 0.54
Fig. 30.3-2*	Zone 1:	GC_{pup}	:= 1.62
Eq. 29.4-7		$P_{rup} = q_h \times GC_{pup} \times \gamma_E \times \gamma_a$:= 25.51

RAFTERS- Wind DOWN Pressure

ASCE REFERENCE		$TA_r = S_{fh} \times L_r$:= 30.00
Fig. 29. 4-8		γ_{ar}	:= 0.54
Fig 30. 3-2*	Zone 1:	GC_{pdn}	:= 0.42
Eq. 29.4-7		$P_{rdn} = q_h \times GC_{pdn} \times \gamma_E \times \gamma_a$:= 16.00

LOAD COMB. #1: WIND UPLIFT ON RF MOUNTS (0.6DL - 0.6 Wind Up) (Cd = 1.6)

$$t = \text{Withdrawal Capacity (lb/inch)} \quad t := 235.00$$

$$W = \text{Total Withdrawal Capacity} \quad W := C_d \times t \times e \quad W := 940.00$$

ZONE 1	$P := TA_f \times (0.6 \times P_{1f} - 0.6 \times DL_{pV} \times \cos(\theta \times \text{deg}))$:= 312.29
	$\% = W \times 100/P_{1f}$:= 301.0%

Code Compliant if % is equal to or more than 100%

ZONE 2e	$P := TA_f \times (0.6 \times P_{2ef} - 0.6 \times DL_{pV} \times \cos(\theta \times \text{deg}))$:= 605.78
	$\% = W \times 100/P_{2ef}$:= 155.2%

Code Compliant if % is equal to or more than 100%

ZONE 2r	$P := TA_f \times (0.6 \times P_{2rf} - 0.6 \times DL_{pV} \times \cos(\theta \times \text{deg}))$:= 682.73
	$\% = W \times 100/P_{2rf}$:= 137.7%

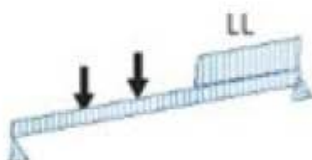
Code Compliant if % is equal to or more than 100%

LOAD COMB. # 2: On Rafters DL RF + DL SOLAR + RF LL (CD=1.25) with LL = 20 psf

$$W := LL \times S_r \text{ plf} \quad (\text{plf}) \quad := 39.26 \quad P_{sp} := TA_f \times DL_{PV} \quad := 41.00$$

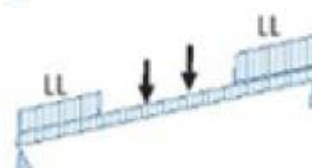
$$c := \frac{(L - 5.5)}{2} \quad (\text{ft}) \quad := \text{N/A} \quad M_{DL} := \left(WDL_{tc} \times \frac{L^2}{8} + P_{sp} \times \frac{L}{a} \right) \times \cos(\theta) \quad := 134.91$$

Spans < 10.0 ft



$$M_{LL} := \frac{\left(\left(W \times \frac{3}{2 \times L} \right) \times (2 \times L - 3) \right)^2 \times \cos(\theta)}{2 \times W} \quad := 106.26$$

Spans => 10.0 ft



$$M_{LL} := \frac{\left((W \times c \times (2 \times L - c) + W \times c^2) \right)^2 \times \cos(\theta)}{2 \times L \times 2 \times W} \quad := \text{N/A}$$

$$M_2 := M_{DL} + M_{LL} \quad (\text{lb-ft}) \quad := 241.17 \quad S_r := M_2 \times \frac{12}{Fb \times Cd_{LL} \times Cf_x \times C_r \times C_{LS}} \quad := 1.34$$

$$\% := \frac{S_x}{S_r} \times 100 \quad \ll \text{ If equal to or more than 100\% Code Compliant, OK!} \quad := 228.0\%$$

LOAD COMB. #3: On Rafters, Zone 1 DL Rf + DL Solar + Wind Down (Cd = 1.6)

$$P_3 := TA_f \times (0.6 \times P_{rdn} + DL_{PV} \times \cos \theta) \quad := 169.73$$

$$M_3 := \left(WDL_{tc} \times \frac{L^2}{8} \right) \times \cos(\theta \times \text{deg}) + P_3 \times \frac{L}{a} \quad := 353.58$$

$$S_r := M_3 \times \frac{12}{Fb \times Cd_{Wind} \times Cf_x \times C_r \times C_{LS}} \quad := 1.54$$

$$\% := \frac{S_x \times 100}{S_r} \quad \ll \text{ If equal to or more than 100\% Code Compliant, OK!} \quad := 199.1\%$$

LOAD COMB. #4: On Rafters DL Rf + DL Solar + Snow (Cd=1.15)

$$S := \text{Sloped Roof Snow Load (psf)} \quad := 9.40$$

$$P_4 := TA_f \times (DL_{PV} \times S) \quad := 169.47$$

$$M_4 := \left(WDL_{tc} \times \frac{L^2}{8} + P_4 \times \frac{L}{a} \right) \times \cos(\theta) \quad := 336.11$$

$$S_r := M_4 \times \frac{12}{Fb \times Cd_{Snow} \times Cf_x \times C_r \times C_{LS}} \quad := 2.03$$

$$\% := \frac{S_x \times 100}{S_r} \quad \ll \text{If equal to or more than 100\% code compliant, ok!} \quad := 150.5\%$$

LOAD COMB. #5: On Rafters, Zone 1 DL Rf + DL Solar + .75Wind + .75Snow (Cd=1.6)

$$S := \text{Sloped Roof Snow Load (psf)} \quad := 9.40$$

$$P_5 := TA_f \times (DL_{PV} + .75 \times S) \times \cos\theta + TA_f \times 0.75 \times 0.6 \times p_{rdn} \quad := 227.47$$

$$M_5 := \left(WDL_{tc} \times \frac{L^2}{8} + P_5 \times \frac{L}{a} \right) \times \cos(\theta) \quad := 449.81$$

$$S_r := M_5 \times \frac{12}{Fb \times Cd_{Wind} \times Cf_x \times C_r \times C_{LS}} \quad := 1.96$$

$$\% := \frac{S_x \times 100}{S_r} \quad \ll \text{If equal to or more than 100\% Code Compliant, OK!} \quad := 156.5\%$$

LOADING COMB. #6: CHECK SEISMIC LOADING: C=COVERAGE ON ROOF

$$\text{Existing DL} := DL_{Rf} + \text{Walls} \quad \text{Walls} := 5.5 \text{ psf} \quad := 14.20$$

$$\text{Proposed DL} := \text{Existing DL} + DL_{Solar} \times C \quad := 14.94$$

$$\text{Seismic Increase} := 100 \times \left(\frac{\text{Proposed DL}}{\text{Existing DL}} \right) - 100 \quad \ll \text{If equal to or more than 10\% Code Compliant, OK!} \quad := 5.2\%$$

SEISMIC SUMMARY: EXISTING BUILDING PROVISIONSSection 11B.3- Exceptions:

1. The addition complies with the requirements for new structures.
2. The addition does not increase the seismic force by more than 10%.
3. The addition does not decrease the seismic resistance of any structural element.

LOAD COMB. #7: On Rafters, ZONE 1 (0.6)(DL RF + DL SOLAR) - WIND UP (CD=1.6)

$$P_7 := TA_f \times (0.6 \times p_{rup} - 0.6 \times DL_{PV} \times \cos\theta) \quad := \quad 186.03$$

$$M_7 := \left(P_7 \times \frac{L}{a} \right) - wDL_{tc} \times \frac{L^2}{8} \times \cos(\theta \times \text{deg}) \quad := \quad 267.64$$

$$S_r := M_7 \times \frac{12}{Fb \times Cd_{Wind} \times Cf_x \times C_r \times C_{LS}} \quad := \quad 1.16$$

$$\% := \frac{S_x \times 100}{S_r} \quad \ll \text{IF EQUAL TO OR MORE THAN 100\% CODE COMPLIANT, OK!} \quad := \quad 263.0\%$$

LIMITS OF SCOPE OF WORK AND LIABILITY

Note:

1. Prior to commencement of work, the Solar Installer shall verify that the roof framing sizes, spacing, and spans (between supports), are as noted in these documents. The Engineer of Record must be notified if any discrepancies are discovered, before proceeding.
2. These documents are Stamped for Structural Code compliance of the roof members that support the PV solar system only.
3. These documents are not stamped for rain water leakage prevention.
4. As a precaution, old or wet snow should be removed from the roof, if the snow builds up to 18" or more.
5. Existing deficiencies which are unknown and not observable due to their being concealed inside walls or sandwiched behind gypsum board ceilings at the time of inspection are not included in the scope of work. These calculations are only for the roof framing which supports the new PV modules. These calculations do not include a complete lateral analysis of the building, nor a prediction of the life expectancy of the existing building.

Appendix

Table 3 represents the maximum Moment ($M = PL/a$) resulting from point loads (RF Mounts), for any spans (L) listed.

$$a = 4.50$$

TABLE 3

The Moment Factor "a" for a 2 Rail System

Length	Staggered		Unstaggered	
	Portrait	Landscape	Portrait	Landscape
L = 4'	4.00	4.00	4.00	2.91
L = 5'	4.00	3.50	3.72	2.50
L = 6'	4.00	3.00	3.43	2.09
L = 7'	3.60	2.84	2.95	1.83
L = 8'	3.20	2.67	2.46	1.56
L = 9'	3.03	2.59	2.34	1.44
L = 10'	2.86	2.50	2.22	1.31
L = 11'	2.77	2.25	2.04	1.19
L = 12'	2.67	2.00	1.85	1.07
L = 13'	2.61	1.88	1.74	0.90
L = 14'	2.55	1.75	1.62	0.92
L = 15'	2.51	1.68	1.50	0.86
L = 16'	2.46	1.60	1.38	0.81
L = 17'	2.43	1.45	1.31	0.76
L = 18'	2.40	1.29	1.23	0.72
L = 19'	2.38	1.24	1.17	0.68
L = 20'	2.35	1.18	1.11	0.65
L = 21'	2.22	1.14	1.06	0.62
L = 22'	2.09	1.10	1.00	0.59

The Moment Factor "a" for a Railless or Shared Rail System

Length	Staggered		Unstaggered	
	Portrait	Landscape	Portrait	Landscape
L = 4'	5.00	5.00	5.00	5.00
L = 5'	5.00	5.00	5.00	5.00
L = 6'	5.00	5.00	5.00	5.00
L = 7'	4.50	4.50	4.50	4.19
L = 8'	4.00	4.00	4.00	3.37
L = 9'	4.00	4.00	4.00	3.12
L = 10'	4.00	4.00	4.00	2.86
L = 11'	4.00	4.00	3.70	2.48
L = 12'	4.00	4.00	3.39	2.09
L = 13'	4.00	3.87	3.35	1.98
L = 14'	4.00	3.73	3.30	1.87
L = 15'	4.00	3.32	2.88	1.78
L = 16'	4.00	2.90	2.46	1.68
L = 17'	4.00	2.74	2.36	1.57
L = 18'	4.00	2.57	2.25	1.45
L = 19'	4.00	2.46	2.18	1.38
L = 20'	4.00	2.35	2.11	1.30
L = 21'	4.00	2.28	2.06	1.25
L = 22'	4.00	2.20	2.00	1.20

REFERENCES: LATEST EDITION

<u>Duration Factors</u>	<u>Section Modules</u>		<u>Size Form Factor</u>	
$C_{dwind} := 1.6$	$S_x := 3.06$	$S_{2 \times 12} := 31.640$	$C_{f_{2 \times 2}} := 1.5$	$C_{f_{4 \times 4}} := 1.5$
$C_{dsnow} := 1.15$	$S_{2 \times 2} := 0.563$	$S_{4 \times 4} := 7.150$	$C_{f_{2 \times 4}} := 1.5$	$C_{f_{4 \times 6}} := 1.3$
$C_{dDL} := 0.9$	$S_{2 \times 4} := 3.063$	$S_{4 \times 6} := 17.650$	$C_{f_{2 \times 6}} := 1.3$	$C_{f_{4 \times 8}} := 1.3$
$C_{dLL} := 1.25$	$S_{2 \times 6} := 7.563$	$S_{4 \times 8} := 30.660$	$C_{f_{2 \times 8}} := 1.2$	$C_{f_{4 \times 10}} := 1.2$
$C_{LS} := 1.00$	$S_{2 \times 8} := 13.140$	$S_{4 \times 10} := 49.900$	$C_{f_{2 \times 10}} := 1.1$	$C_{f_{4 \times 12}} := 1.1$
$F_b := 1000$	$S_{2 \times 10} := 21.390$	$S_{4 \times 12} := 73.800$	$C_{f_{2 \times 12}} := 1.0$	