

7/21/2022

PURE ENERGY GROUP, LLC 400 DOMINION DRIVE STE 105 MORRISVILLE, NC 25760

Attn.: To Whom It May Concern

re job: SAM KWON RESIDENCE

43 DOONBEG DR,

FUQUAY-VARINA, NC 27526

The following calculations are for the structural engineering design of the photovoltaic panels and are valid only for the structural info referenced in the stamped plan set. The verification of such info is the responsibility of others.

After review, I certify that the roof structure has sufficient structural capacity for the applied PV loads.

All mounting equipment shall be designed and installed per manufacturer's approved installation specifications.

Design Criteria:

Code: 2018 NC Residential Code

ASCE 7-10

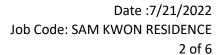
Live Load: 20 psf Ult Wind Speed: 115 mph

Exposure Cat: C

Ground Snow: 15 psf Min Roof Snow: NA

AHZ Consulting Engineers Inc. Professional Engineer projects@ahzengineers.com







Roof Properties:

	Roof 1	
Roof Type =	Shingle	
Roof Pitch (deg) =	18	
Mean Roof Height (ft) =	13	
Attachment Trib Width (ft) =	2.75	
Attachment Spacing (ft) =	4	
Framing Type =	Truss	
Framing Size =	2x4	
Framing OC Spacing (in.) =	24	
Section Thickness, b (in.) =	1.5	
Section Depth, d (in.) =	3.5	
Section Modulus, Sx (in.^3) =	3.1	
Moment of Inertia, Ix (in.^4) =	5.4	
Framing Span (ft) =	9	
Deflection Limit D+L (in.) =	1.8	
Deflection Limit S or W (in.) =	1.20	
Attachments Pattern =	Stacked	
Framing Upgrade =	Adequate	
Sister Size =	NA	
Wood Species =	DF #2	
Wood Fb (psi) =	900	
Wood Fv (psi) =	180	
Wood E (psi) =	1600000	
C_D (Wind) =	1.6	
C_D (Snow) =	1.15	
C _{LS} =	1.15	
$C_M = C_t = C_L = C_i =$	1.0	
C _F =	1.5	
C _{fu} =	1.00	
C _r =	1.15	
F'b_wind (psi) =	2857	
F'b_snow (psi) =	2053	
F'v wind (psi) =	288	
F'v_snow (psi) =	207	
M_allowable_wind (lb-ft) =	729	
M_allowable_snow (lb-ft) =	524	
V_allowable_wind (lbs) =	1008	
V_allowable_snow (lbs) =	725	
V_allowable_sllow (lbs) = E' (psi) =		
E (psi) =	1000000	

Date:7/21/2022 Job Code: SAM KWON RESIDENCE

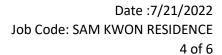




Load Calculation:

Load Calculation.		
Dead Load Calculations:		
Panels Dead Load (psf) =	3.0	
. ,	Roof 1	
Roofing Weight (psf) =	3.0	
Decking Weight (psf) =	2.0	
Framing Weight (psf) =	0.6	
Misc. Additional Weight (psf) =	1.0	
Existing Dead Load (psf) =	6.6	
Total Dead Load (psf) =	9.6	
Crow Lood Coloulations		
Snow Load Calculations: Ground Snow Load, pg (pcf) =	15	
Ground Snow Load, pg (psf) =	NA	
Min Flat Snow, pf_min (psf) = Min Sloped Snow, ps_min (psf) =	NA NA	
	1.0	
Snow Importance Factor, Ic =	0.9	
Exposure Factor, Ce =	Roof 1	
 Thermal Factor, Ct =	1.2	
Flat Roof Snow, pf (psf) =	11.34	
Slope Factore, Cs =	1.00	
Sloped Roof Snow, ps (psf) =	11	
stoped floor strong ps (ps)		
Wind Load Calculations:		
Ultimate Wind Speed (mph) =	115	
Directionality Factor, kd =	0.85	
Topographic Factor, kzt =	1.0	
	Roof 1	
Velocity Press Exp Factor, kz =	0.85	
Solar Equalization Factor, γa =	1.00	
External Pressure Up, GCp_1 =	-0.85	
External Pressure Up, GCp_2 =	-1.55	
External Pressure Up, GCp_3 =	-2.45	
External Pressure Down, GCp =	0.45	
Design Pressure Up, p_1 (psf) =	-20.8	
Design Pressure Up, p_2 (psf) =	-37.9	
Design Pressure Up, p_3 (psf) =	-59.9	
Design Pressure Down, p (psf) =	16.0	

Hardware Checks:





Lag Screw Checks:

	Roof 1	
Ref. Widthrawal Value, W (lb/in) =	266	
$(C_{M} = C_{t} = C_{eg} = 1.0) C_{D} =$	1.6	
Adjusted Widthrawal Value, W' (lb/in) =	426	
Lag Penetration, p (in.) =	2.5	
Allowable Widthrawal Force, W'p (lbs) =	1064	
Applied Uplift Force (lbs) =	-231	
Uplift DCR =	0.22	
Ref. Lateral Value, Z (lbs) =	270	
$(C_{M} = C_{t} = C_{\Delta} = C_{eg} = 1.0) C_{D} =$	1.15	
Adjusted Lateral Value, Z' (lbs) =	311	
Applied Laeral Force (lbs) =	49	
Angle of Resultant Force, α (deg) =	78	
ljusted Interaction Lateral Value, Z'_{α} (lbs) =	964	
Lateral DCR =	0.05	

Roof Framing Checks:

Force Checks:

	Roof 1
LC1: D+S	
Applied Moment (lb-ft) =	476
Applied Shear (lbs) =	318
Allowable Moment (lb-ft) =	524
Allowable Shear (lbs) =	725
Moment DCR =	0.91
Shear DCR =	0.44
LC2: D+0.6W	
Applied Moment (lb-ft) =	429
Applied Shear (lbs) =	286
Allowable Moment (lb-ft) =	729
Allowable Shear (lbs) =	1008
Moment DCR =	0.59
Shear DCR =	0.28
LC3: D+0.75(S+0.6W)	
Applied Moment (lb-ft) =	594
Applied Shear (lbs) =	396
Allowable Moment (lb-ft) =	729
Allowable Shear (lbs) =	1008



Date:7/21/2022 Job Code: SAM KWON RESIDENCE

5 of 6

Moment DCR =	0.81
Shear DCR =	0.39
LC4: 0.6D+0.6W	
Applied Moment (lb-ft) =	234
Applied Shear (lbs) =	156
Allowable Moment (lb-ft) =	729
Allowable Shear (lbs) =	1008
Moment DCR =	0.32
Shear DCR =	0.15

<u>Deflection Checks (Service Level):</u>

		Roof 1	
LC1: D+L	-		
	Deflection (in.) =	0.13	
	Deflection Limit (in.) =	2.07	
	Deflection DCR =	0.06	
LC2: S			
	Deflection (in.) =	0.16	
	Deflection Limit (in.) =	1.38	
	Deflection DCR =	0.11	
LC3: W (Down)			
	Deflection (in.) =	0.09	
	Deflection Limit (in.) =	1.38	
	Deflection DCR =	0.07	
LC4: W (Up)			
	Deflection (in.) =	-0.12	
	Deflection Limit (in.) =	1.38	
	Deflection DCR =	0.09	

Seismic Check:

Existing Weight:

Wall Weight (psf) =	17
Tributary Wall Area (ft ²) =	1000
Total Wall Weight (lbs) =	17000
Roof Weight (psf) =	7
Roof Area (ft ²) =	2400
Total Roof Weight (lbs) =	15844
Total Existing Weight (lbs) =	32844

Additional PV Weight:

PV Panel Weight (lbs) = 54



CONSULTING ENGINEERS INC.

Date :7/21/2022 Job Code: SAM KWON RESIDENCE

6 of 6

Number of Panels = 14

Total Additional PV Weight (lbs) = 762

Weight Increase:

(Existing W + Additional W)/(Existing W) = 1.02

The increase in weight as a result of the solar system is less than 10% of the existing structure and therefore no further seismic analysis is required.

Limits of Scope of Work and Liability:

Existing structure is assumed to have been designed and constructed following appropriate codes at time of erection, and assumed to have appropriate permits. The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgements. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. All PV modules, racking, and mounting equipment shall be designed and installed per manufacturer's approved installation specifications. The Engineer of Record and the engineering consulting firm assume no responsibility for misuse or improper installation. This analysis is not stamped for water leakage. Framing was determined based on information in provided plans and/or photos, along with engineering judgement. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction.