



BLUE RAVEN SOLAR, LLC.
Firm License No. D-0369
1403 North Research Way , Bldg. J
Orem, UT 84097

September 16, 2021

To: Blue Raven Solar
1220 S. 630 E. Ste. 430
American Fork, UT. 84003

Subject: Certification Letter
Smith Residence
111 Alban Row
Fuquay-Varina, NC. 27526

To Whom It May Concern,

A jobsite observation of the condition of the existing framing system was performed by an audit team of Blue Raven Solar. All attached structural calculations are based on these observations and the design criteria listed below.

On the above referenced project, the roof structural framing has been reviewed for additional loading due to the installation of the solar PV addition to the roof. The structural review, including the plans and calculations only apply to the section of the roof that is directly supporting the solar PV system and its supporting elements. The observed roof framing is described below.

The roof structure of (MP1) consists of composition shingle on roof plywood that is supported by 2x8 rafters @ 16" o.c.. The rafters support a vaulted ceiling and have a max projected horizontal span of 14'-0", with a slope of 44 degrees. The rafters are supported at the ridge by a ridge board and at the eave by a load bearing wall.

The existing roof framing system of (MP1) is judged to be adequate to withstand the loading imposed by the installation of the solar panels. No reinforcement is necessary.

The spacing of the solar standoffs should be kept at 64" o.c. for landscape and 48" o.c. for portrait orientation, with a staggered pattern to ensure proper distribution of loads.

The scope of this report is strictly limited to an evaluation of the fastener attachment, underlying framing and supporting structure only. The attachment's to the existing structure are required to be in a staggered pattern to ensure proper distribution of loading. All panels, racking and hardware shall be installed per manufacturer specifications and within specified design limitations. All waterproofing shall be provided by the manufacturer.

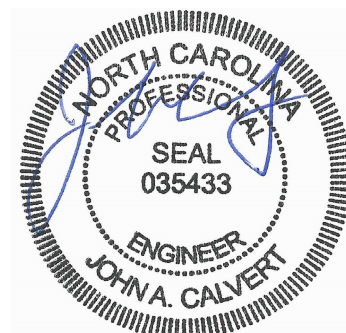
Design Criteria:

- Applicable Codes = 2018 North Carolina State Building Code (NCSBC), ASCE7-10, and NDS-12
- Roof Dead Load = 13 psf (MP1)
- Roof Live Load = 20 psf
- Wind Speed = 115 mph, Exposure C
- Ground Snow Load = 15 psf - Roof Snow Load = 10.5 psf
- Attachments: (1) 5/16" dia lag screw with 2.5" min embedment depth, at spacing shown above.

Please contact me with any further questions or concerns regarding this project.

Sincerely,

John Calvert, P.E.
Project Engineer





Wind Calculations

Per ASCE7-10 Components and Cladding

Input Variables	
Wind Speed	115 mph
Exposure Category	C
Roof Shape	Gable/Hip
Roof Slope	44 degrees
Mean Roof Height	20 ft
Effective Wind Area	21.3 ft

Design Wind Pressure Calculations	
Wind Pressure $P = qh * G * C_n$	
$qh = 0.00256 * K_z * K_{zt} * K_d * V^2$	(Eq. 30.3-1)
K_z (Exposure Coefficient) = 0.9	(Table 30.3-1)
K_{zt} (topographic factor) = 1	(Fig. 26.8-1)
K_d (Wind Directionality Factor) = 0.85	(Table 26.6-1)
V (Design Wind Speed) = 115 mph	(Fig. 26.5-1A)
Risk Category = II	(Table 1.5-1)
$qh = 25.90$	
$0.6 * qh = 15.54$	

Standoff Uplift Calculations-Portrait				
	Zone 1	Zone 2	Zone 3	Positive
$G C_p =$	-0.94	-1.15	-1.15	0.86
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	22.4 psf
X Standoff Spacing =	4.00	4.00	2.67	
Y Standoff Spacing =	6.08	3.041666667	3.041666667	
Tributary Area =	24.33	12.17	8.11	
Dead Load on Attachment =	73.00	36.50	24.33	
Footing Uplift (0.6D+0.6W) =	-310 lb	-195 lb	-130 lb	

Standoff Uplift Calculations-Landscape				
	Zone 1	Zone 2	Zone 3	Positive
$G C_p =$	-0.94	-1.15	-1.15	0.86
Uplift Pressure =	-14.55 psf	-17.80 psf	-17.80 psf	10.5 psf
X Standoff Spacing =	5.33	5.33	3.56	
Y Standoff Spacing =	3.50	1.75	1.75	
Tributary Area =	18.67	9.33	6.22	
Dead Load on Attachment =	56.00	28.00	18.67	
Footing Uplift (0.6D+0.6W) =	-238 lb	-149 lb	-100 lb	

Standoff Uplift Check	
Maximum Design Uplift =	-310 lb
Standoff Uplift Capacity =	450 lb
450 lb capacity > 310 lb demand	Therefore, OK

Fastener Capacity Check	
Fastener =	1 - 5/16" dia Lag
Number of Fasteners =	1
Embedment Depth =	2.5
Pullout Capacity Per Inch =	250 lb
Fastener Capacity =	625 lb
w/ F.S. of 1.5 & DOL of 1.6 =	667 lb
667.2 lb capacity > 310 lb demand	Therefore, OK



Gravity Loading

Roof Snow Load Calculations		
p_g = Ground Snow Load =	15 psf	
$p_f = 0.7 C_e C_t I p_g$		(ASCE7 - Eq 7-1)
C_e = Exposure Factor =	1	(ASCE7 - Table 7-2)
C_t = Thermal Factor =	1	(ASCE7 - Table 7-3)
I = Importance Factor =	1	
p_f = Flat Roof Snow Load =	10.5 psf	
$p_s = C_s p_f$		(ASCE7 - Eq 7-2)
C_s = Slope Factor =	1	
p_s = Sloped Roof Snow Load =	10.5 psf	

PV Dead Load = 3 psf (Per Blue Raven Solar)	
PV System Weight	
Weight of PV System (Per Blue Raven Solar)	3.0 psf
X Standoff Spacing =	4.00 ft
Y Standoff Spacing =	6.08 ft
Standoff Tributary Area =	24.33 sft
Point Loads of Standoffs	73 lb

Note: PV standoffs are staggered to ensure proper distribution of loading

Roof Live Load = 20 psf	
Note: Roof live load is removed in area's covered by PV array.	

Roof Dead Load (MP1)	
Composition Shingle	4.00
Roof Plywood	2.00
2x8 Rafters @ 16"o.c.	2.27
Vaulted Ceiling	4.00
Miscellaneous	0.73
Total Roof DL (MP1)	13.0 psf
DL Adjusted to 44 Degree Slope	18.1 psf

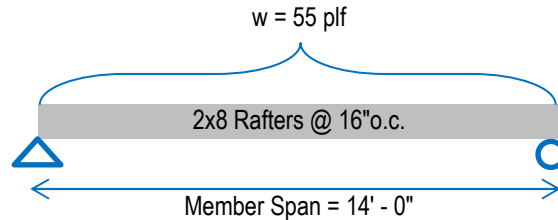


Framing Check

(MP1)

PASS

Dead Load 18.1 psf
 PV Load 3.0 psf
 Live Load 20.0 psf



Governing Load Combo = DL + LL
Total Load 41.1 psf

Member Properties

Member Size	S (in ³)	I (in ⁴)	Lumber Sp/Gr	Member Spacing
2x8	13.14	47.63	DF#2	@ 16"o.c.

Check Bending Stress

$$F_b \text{ (psi)} = f_b \times C_d \times C_f \times C_r \quad \text{(NDS Table 4.3.1)}$$

$$900 \times 1.25 \times 1.2 \times 1.15$$

Allowed Bending Stress = 1552.5 psi

Maximum Moment = $(wL^2) / 8$
 = 1341.689 ft#
 = 16100.27 in#

Actual Bending Stress = (Maximum Moment) / S
 = 1225.3 psi

Allowed > Actual -- 79% Stressed -- Therefore, OK

Check Deflection

Allowed Deflection (Total Load) = $L/180$ (E = 1600000 psi Per NDS)
 = 0.933 in

Deflection Criteria Based on = Continuous Span

Actual Deflection (Total Load) = $(wL^4) / (185 \times E \times I)$
 = 0.258 in
 $L/652 > L/180$ **Therefore OK**

Allowed Deflection (Live Load) = $L/240$
 0.7 in

Actual Deflection (Live Load) = $(wL^4) / (185 \times E \times I)$
 0.126 in
 $L/1334 > L/240$ **Therefore OK**

Check Shear

Member Area = 10.9 in² F_v (psi) = 180 psi (NDS Table 4A)

Allowed Shear = F_v * A = 1958 lb Max Shear (V) = w * L / 2 = 383 lb

Allowed > Actual -- 19.6% Stressed -- Therefore, OK