November 12, 2021

BES Project Number: 4641 Crystal Johnson

Power Home Solar, LLC
919 N Main St.
Mooresville, NC 28115

Project Location: Crystal Johnson: 465 Kinsman Ct., Fuquay-Varina, NC 27526
Solar Array Installation

To Whom It May Concern:

Per your request, BES has reviewed the existing structure at the above referenced location. The purpose of this review was to determine the adequacy of the existing structure to support the proposed installation of solar panels on the roof as shown on the attached panel layout plan.

Based upon our review, we certify that existing roof structure will adequately support with the following: Racking and attachment mounting connection: (1) 5/16" lag screw w/ min. 2.5" embedment into framing at max 48" o/c along rails (2) rails per row of panels, evenly spaced; panel length perpendicular to the rails not to exceed 67 in. Solar module mounting hardware design is by the manufacturer.

Limitations: Installation of the solar panels must be performed in accordance with manufacturer recommendations. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. The contractor must notify BES should any damage, deterioration or discrepancies between the as-built condition of the structure and the condition described in this letter be found. Connections to existing roof framing must be staggered, except at array ends, so as to not overload any existing structural member. The design of the solar panel racking (mounts, rails, etc.) is the responsibility of the manufacturer. Waterproofing around the roof penetrations is the responsibility of others. BES assumes no responsibility for improper installation of the solar array. Existing structure meets or exceeds standard building practices with current building code with assumed single layer asphalt shingles.

Sincerely,

NORTH CAROLINA
PROFESSIONAL
SEAL
43963
JERMEY L. BOWERS
ENGINEER
11/12/2021

Jermey Bowers M.E., P.E.
Principal Engineer

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Bowers Engineering Services
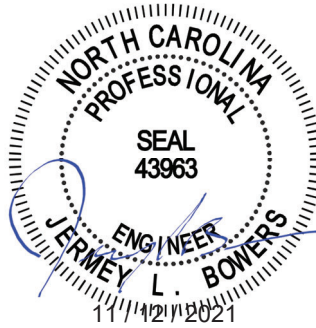121 S. Main ST
Auburn, IN
(260) 333-0900

Structural Analysis

Location

465 Kinsman Ct
Fuquay-Varina, NC 27526

Roof Mount Solar

11/12/2021

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Project: 4641   Rev: -

STRUCTURAL SUMMARY

CODE SPEC

WIND

IBC 2015   Speed: 115   MPH
ASCE 7-10   Exp.:   C

Risk Cat: II

Wind Load - uplift

		Max lb
Zone 1	-21.88 psf	-147 lb
Zone 2	-40.98 psf	-275.36 lb
Zone 3	-62.85 psf	-422.36 lb
Max trib	11.20 ft2	

Max loading at connection

Negitive   -422.36 lb/fastener

Connection (Pull Out)

Lag screw	5/16 in			
Cd	1.60 Table 2.3.2			
embedment	2.5 in			
Nominal CapacityPrying	205.00 lbs G=0.42			
Max capacity (lbs)	*533.00*	>	*422.36*	*OK*

Note:

* Lag screws to be diameter 5/16x2.5inches long.

* All fasteners need to be placed at roof rafters.

Cust Name:	Bowers Engineering Services	Subject:	Roof Mount		
Job Number:	4641	Originator:	0	Checker:	

Input Data:

Wind Speed, V =	115	mph (Wind Map, Figure 26.5-1A-C)
Bldg. Classification =	II	(Table 1-1 Occupancy Category)
Exposure Category =	C	(Sect. 26.7)
Ridge Height, hr =	17.00	ft. (hr >= he)
Eave Height, he =	10.00	ft. (he <= hr)
Building Width =	28.00	ft. (Normal to Building Ridge)
Building Length =	72.00	ft. (Parallel to Building Ridge)
Roof Type =	Gable	(Gable or Monoslope)
Topo. Factor, Kzt =	1.00	(Sect. 26.8 & Figure 26.8-1)
Direct. Factor, Kd =	0.85	(Table 26.6)
Enclosed? (Y/N)	Y	(Sect. 28.6-1 & Figure 26.11-1)
Hurricane Region?	N	
Component Name =	Decking	(Purlin, Joist, Decking, or Fastener)
Effective Area, Ae =	11.1	ft.^2 (Area Tributary to C&C)
Overhangs? (Y/N)	N	(if used, overhangs on all sides)

Plan

Elevation
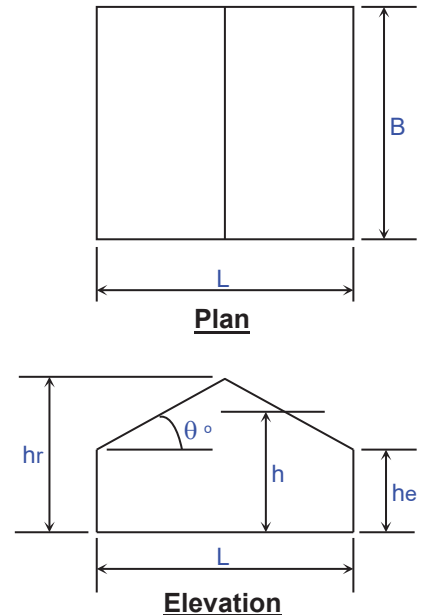
Resulting Parameters and Coefficients:

Roof Angle, θ =	26.57	deg.
Mean Roof Ht., h =	13.50	ft. (h = (hr+he)/2, for roof angle >10 deg.)

Roof External Pressure Coefficients, GCp:

GCp Zone 1-3 Pos. =	0.49	(Fig. 30.4-2A, 30.4-2B, and 30.4-2C)
GCp Zone 1 Neg. =	-0.90	(Fig. 30.4-2A, 30.4-2B, and 30.4-2C)
GCp Zone 2 Neg. =	-1.68	(Fig. 30.4-2A, 30.4-2B, and 30.4-2C)
GCp Zone 3 Neg. =	-2.57	(Fig. 30.4-2A, 30.4-2B, and 30.4-2C)

Positive & Negative Internal Pressure Coefficients, GCpi (Figure 26.11-1):

+GCpi Coef. =	0.00	(positive internal pressure)
-GCpi Coef. =	0.00	(negative internal pressure)

If z <= 15 then: Kz = 2.01*(15/zg)^(2/α) , If z > 15 then: Kz = 2.01*(z/zg)^(2/α) (Table 30.3-1)

α =	9.50	(Table 26.9-1)
zg =	900	(Table 26.9-1)
Kh =	0.85	(Kh = Kz evaluated at z = h)

Velocity Pressure: qz = 0.00256*Kz*Kzt*Kd*V^2 (Sect. 30.3.2, Eq. 30.3-1)

qh =	24.43	psf qh = 0.00256*Kh*Kzt*Kd*V^2 (qz evaluated at z = h)

Design Net External Wind Pressures (Sect. 30.4 & 30.6):
For h <= 60 ft.: p = qh*((GCp) - (+/-GCpi)) (psf)
For h > 60 ft.: p = q*(GCp) - qi*(+/-GCpi) (psf)
   where: q = qh for roof
        qi = qh for roof (conservatively assumed per Sect. 30.6)

Wind Load Tabulation for Roof Components & Cladding							
Component	z	Kh	qh	p = Net Design Pressures (psf)			
	(ft.)		(psf)	Zone 1,2,3 (+)	Zone 1 (-)	Zone 2 (-)	Zone 3 (-)
Decking	0	0.85	24.43	11.99	-21.88	-40.98	-62.85
	15.00	0.85	24.43	11.99	-21.88	-40.98	-62.85
For z = hr:	17.00	0.85	24.43	11.99	-21.88	-40.98	-62.85
For z = he:	10.00	0.85	24.43	11.99	-21.88	-40.98	-62.85
For z = h:	13.50	0.85	24.43	11.99	-21.88	-40.98	-62.85

Notes: 1. (+) and (-) signs signify wind pressures acting toward & away from respective surfaces.
   2. Width of Zone 2 (edge), 'a' = 3.00 ft.
   3. Width of Zone 3 (corner), 'a' = 3.00 ft.
    4. For monoslope roofs with θ <= 3 degrees, use Fig. 30.4-2A for 'GCp' values with 'qh'.
    5. For buildings with h > 60' and θ > 10 degrees, use Fig. 30.6-1 for 'GCpi' values with 'qh'.
    6. For all buildings with overhangs, use Fig. 30.4-2B for 'GCp' values per Sect. 30.10.
    7. If a parapet >= 3' in height is provided around perimeter of roof with θ <= 10 degrees,
       Zone 3 shall be treated as Zone 2.
    8. Per Code Section 30.2.2, the minimum wind load for C&C shall not be less than 16 psf.
    9. References : a. ASCE 7-02, "Minimum Design Loads for Buildings and Other Structures".
            b. "Guide to the Use of the Wind Load Provisions of ASCE 7-02"
              by: Kishor C. Mehta and James M. Delahay (2004).

Roof Components and Cladding:

| θ <= 7 deg. | 7 deg. < θ <= 27 deg. | 27 deg. < θ <= 45 deg. |
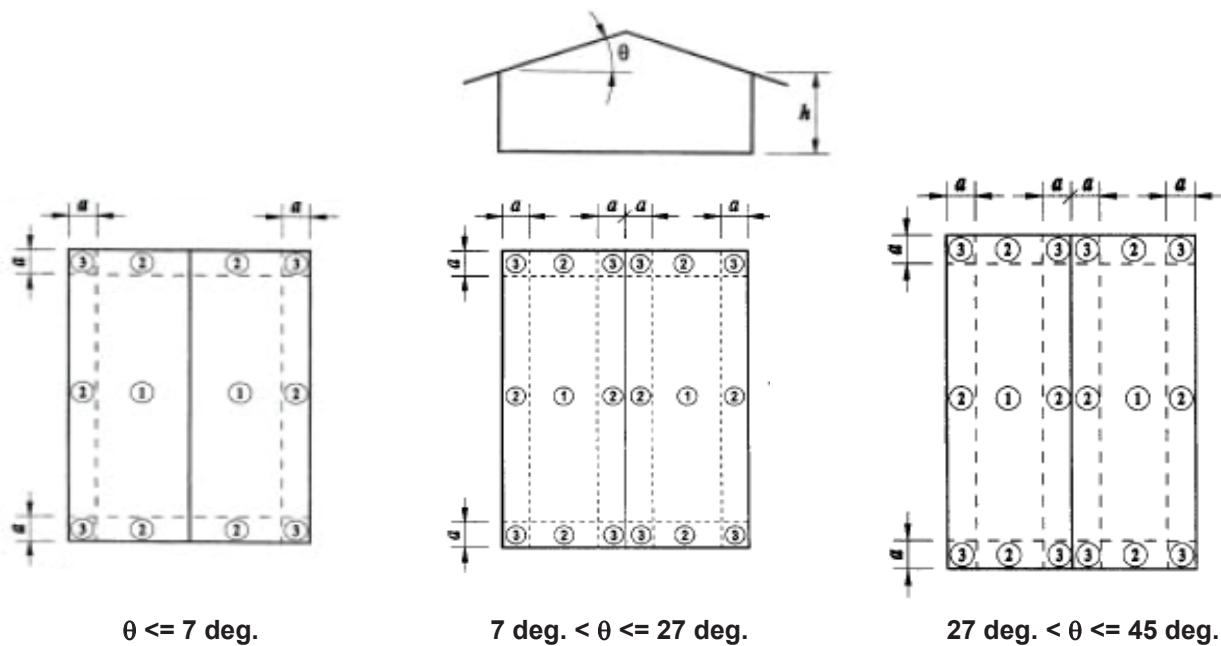
Roof Zones for Buildings with h <= 60 ft.
(for Gable Roofs <= 45° and Monoslope Roofs <= 3°)

ROOF PLAN
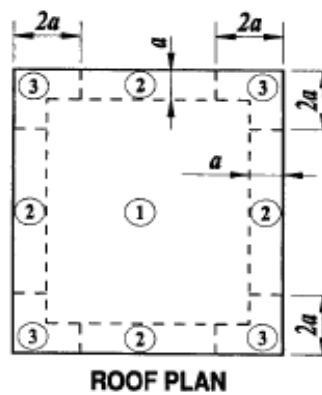
Roof Zones for Buildings with h > 60 ft.
(for Gable Roofs <= 10° and Monoslope Roofs <= 3°)

Frame Design

General Info

Cust. Name:	Bowers Engineering Services		Subject:	Roof Mount		Checker:
Job Number:	4641		Originator:			
Date:	11/12/21					
Address	465 Kinsman Ct	Rev:	-			
City, State:	Fuquay-Varina, NC 27526					

Roof Rafter

Roof Loads

Rafter Size=	2x4	**SYP #1**	Dead Load=	8.00	psf
			Live Load=	20	psf
Trib. Area=	2	ft	Snow Load=	15	psf
Rafter length=	7.5	ft			

Load Combination

			D+0.75L+0.75S	34.3	psf
w=	68.50	plf			
M=	481.64	lb-ft			
			EI=	8.6E+06	ib-in
			S_x =	3.0625	

Dead Load Roof Materials

Sheathing=	2	psf	C_M=	1.0	
Aspahlt Shingles=	3	psf	C_r=	1.2	
Insulation=	0.50	psf	C_D=	1.15	
Solar Panels=	2.50	psf	C_F=	1.3	
Assume Light-frame wood roof			C_L=	1.0	
			Fb=	1500	psi

$$fb = \frac{M * 12}{Sx}$$

$$F'b = Fb * Cd * Cr * Cf * Cm * Cl$$

fb=	1887.24	psi	
F'b=	2578.88	psi	**OK**

$$\frac{5wl^4}{384EI}$$

ΔL=	0.33	in			
ΔS=	0.25	in			
ΔD+L=	0.47	in			
				l/120	
$\Delta_{allow\ in}$=	0.47	<		0.75	**OK**
				l/180	
$\Delta_{allow\ in}$=	0.33	<		0.50	**OK**

2x4	*@*	***2 o/c*** **OK**

Assume rafters are fully braced