



10/5/2021

PURE ENERGY GROUP, LLC
3661 SUNSET AVE #617
ROCKY MOUNT, NC 27804

Attn.: To Whom It May Concern

re job: BRITTANY AND JONATHAN CHAPMAN RESIDENCE
38 DARLEY CT,
LILLINGTON, NC 27546

The following calculations are for the structural engineering design of the photovoltaic panels and are valid only for the structural info referenced in the stamped plan set. The verification of such info is the responsibility of others.

After review, I certify that the roof structure has sufficient structural capacity for the applied PV loads.

All mounting equipment shall be designed and installed per manufacturer's approved installation specifications.

Design Criteria:

Code:	2018 NC Building Code		
	ASCE 7-10		
Live Load:	20	psf	
Ult Wind Speed:	117	mph	
Exposure Cat:	B		
Ground Snow:	10	psf	Min Roof Snow: NA

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Roof Properties:

	Roof 1	Roof 2
Roof Type =	Shingle	Shingle
Roof Pitch (deg) =	34	23
Mean Roof Height (ft) =	23	23
Attachment Trib Width (ft) =	2.75	2.75
Attachment Spacing (ft) =	4	4
Framing Type =	Truss	Truss
Framing Size =	2x4	2x4
Framing OC Spacing (in.) =	24	24
Section Thickness, b (in.) =	1.5	1.5
Section Depth, d (in.) =	3.5	3.5
Section Modulus, S _x (in. ³) =	3.1	3.1
Moment of Inertia, I _x (in. ⁴) =	5.4	5.4
Framing Span (ft) =	8	8
Deflection Limit D+L (in.) =	1.6	1.6
Deflection Limit S or W (in.) =	1.07	1.07
Attachments Pattern =	Fully Staggered	Fully Staggered
Framing Upgrade =	Adequate	Adequate
Sister Size =	NA	NA
Wood Species =	DF #2	DF #2
Wood Fb (psi) =	900	900
Wood Fv (psi) =	180	180
Wood E (psi) =	1600000	1600000
C _D (Wind) =	1.6	1.6
C _D (Snow) =	1.15	1.15
C _{LS} =	1.15	1.15
C _M = C _t = C _L = C _i =	1.0	1.0
C _F =	1.5	1.5
C _{fu} =	1.00	1.00
C _r =	1.15	1.15
F' _{b_wind} (psi) =	2857	2857
F' _{b_snow} (psi) =	2053	2053
F' _{v_wind} (psi) =	288	288
F' _{v_snow} (psi) =	207	207
M _{allowable_wind} (lb-ft) =	729	729
M _{allowable_snow} (lb-ft) =	524	524
V _{allowable_wind} (lbs) =	1008	1008
V _{allowable_snow} (lbs) =	725	725
E' (psi) =	1600000	1600000

Load Calculation:

Dead Load Calculations:

Panels Dead Load (psf) =	3.0		
		Roof 1	Roof 2
Roofing Weight (psf) =	3.0	3.0	3.0
Decking Weight (psf) =	2.0	2.0	2.0
Framing Weight (psf) =	0.6	0.6	0.6
Misc. Additional Weight (psf) =	1.0	1.0	1.0
Existing Dead Load (psf) =	6.6	6.6	6.6
Total Dead Load (psf) =	9.6	9.6	9.6

Snow Load Calculations:

Ground Snow Load, p_g (psf) =	10		
Min Flat Snow, p_{f_min} (psf) =	NA		
Min Sloped Snow, p_{s_min} (psf) =	NA		
Snow Importance Factor, I_c =	1.0		
Exposure Factor, C_e =	0.9		
		Roof 1	Roof 2
Thermal Factor, C_t =	1.2	1.2	1.2
Flat Roof Snow, p_f (psf) =	7.56	7.56	7.56
Slope Factor, C_s =	1.00	1.00	1.00
Sloped Roof Snow, p_s (psf) =	8	8	8

Wind Load Calculations:

Ultimate Wind Speed (mph) =	117		
Directionality Factor, k_d =	0.85		
Topographic Factor, k_{zt} =	1.0		
		Roof 1	Roof 2
Velocity Press Exp Factor, k_z =	0.70	0.70	0.70
Solar Equalization Factor, γ_a =	1.00	1.00	1.00
External Pressure Up, G_{Cp_1} =	-0.95	-0.85	-0.85
External Pressure Up, G_{Cp_2} =	-1.15	-1.55	-1.55
External Pressure Up, G_{Cp_3} =	-1.15	-2.45	-2.45
External Pressure Down, G_{Cp} =	0.85	0.45	0.45
Design Pressure Up, p_{-1} (psf) =	-19.8	-17.7	-17.7
Design Pressure Up, p_{-2} (psf) =	-24.0	-32.3	-32.3
Design Pressure Up, p_{-3} (psf) =	-24.0	-51.1	-51.1
Design Pressure Down, p (psf) =	17.7	16.0	16.0

Hardware Checks:

Lag Screw Checks:

	Roof 1	Roof 2
Ref. Withdrawal Value, W (lb/in) =	266	266
($C_M = C_t = C_{eg} = 1.0$) C_D =	1.6	1.6
Adjusted Withdrawal Value, W' (lb/in) =	426	426
Lag Penetration, p (in.) =	2.5	2.5
Allowable Withdrawal Force, W'p (lbs) =	1064	1064
Applied Uplift Force (lbs) =	-142	-195
Uplift DCR =	0.13	0.18
Ref. Lateral Value, Z (lbs) =	270	270
($C_M = C_t = C_{\Delta} = C_{eg} = 1.0$) C_D =	1.15	1.15
Adjusted Lateral Value, Z' (lbs) =	311	311
Applied Lateral Force (lbs) =	65	45
Angle of Resultant Force, α (deg) =	65	77
Adjusted Interaction Lateral Value, Z' $_{\alpha}$ (lbs) =	749	946
Lateral DCR =	0.09	0.05

Roof Framing Checks:

Force Checks:

	Roof 1	Roof 2
LC1: D+S		
Applied Moment (lb-ft) =	183	183
Applied Shear (lbs) =	137	137
Allowable Moment (lb-ft) =	524	524
Allowable Shear (lbs) =	725	725
Moment DCR =	0.35	0.35
Shear DCR =	0.19	0.19
LC2: D+0.6W		
Applied Moment (lb-ft) =	216	205
Applied Shear (lbs) =	162	154
Allowable Moment (lb-ft) =	729	729
Allowable Shear (lbs) =	1008	1008
Moment DCR =	0.30	0.28
Shear DCR =	0.16	0.15
LC3: D+0.75(S+0.6W)		
Applied Moment (lb-ft) =	248	240
Applied Shear (lbs) =	186	180
Allowable Moment (lb-ft) =	729	729



Allowable Shear (lbs) =	1008	1008
Moment DCR =	0.34	0.33
Shear DCR =	0.18	0.18
LC4: 0.6D+0.6W		
Applied Moment (lb-ft) =	65	52
Applied Shear (lbs) =	49	39
Allowable Moment (lb-ft) =	729	729
Allowable Shear (lbs) =	1008	1008
Moment DCR =	0.09	0.07
Shear DCR =	0.05	0.04

Deflection Checks (Service Level):

	Roof 1	Roof 2
LC1: D+L		
Deflection (in.) =	0.06	0.06
Deflection Limit (in.) =	1.84	1.84
Deflection DCR =	0.03	0.03
LC2: S		
Deflection (in.) =	0.03	0.03
Deflection Limit (in.) =	1.23	1.23
Deflection DCR =	0.03	0.03
LC3: W (Down)		
Deflection (in.) =	0.03	0.03
Deflection Limit (in.) =	1.23	1.23
Deflection DCR =	0.03	0.02
LC4: W (Up)		
Deflection (in.) =	-0.04	-0.03
Deflection Limit (in.) =	1.23	1.23
Deflection DCR =	0.03	0.03

Seismic Check:

Existing Weight:

Wall Weight (psf) =	17
Tributary Wall Area (ft ²) =	3000
Total Wall Weight (lbs) =	51000
Roof Weight (psf) =	7
Roof Area (ft ²) =	2400
Total Roof Weight (lbs) =	15844
Total Existing Weight (lbs) =	66844

**Additional PV Weight:**

PV Panel Weight (lbs) =	54
Number of Panels =	29
Total Additional PV Weight (lbs) =	1579

Weight Increase:

$$(\text{Existing W} + \text{Additional W}) / (\text{Existing W}) = 102\%$$

The increase in weight as a result of the solar system is less than 10% of the existing structure and therefore no further seismic analysis is required.

Limits of Scope of Work and Liability:

Existing structure is assumed to have been designed and constructed following appropriate codes at time of erection, and assumed to have appropriate permits. The calculations produced are only for the roof framing supporting the proposed PV installation referenced in the stamped planset and were completed according to generally recognized structural analysis standards and procedures, professional engineering and design experience, opinions and judgements. Existing deficiencies which are unknown or were not observable during time of inspection are not included in this scope of work. All PV modules, racking, and mounting equipment shall be designed and installed per manufacturer's approved installation specifications. The Engineer of Record and the engineering consulting firm assume no responsibility for misuse or improper installation. This analysis is not stamped for water leakage. Framing was determined based on information in provided plans and/or photos, along with engineering judgement. Prior to commencement of work, the contractor shall verify the framing sizes, spacings, and spans noted in the stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any discrepancies prior to starting construction. Contractor shall also verify that there is no damaged framing that was not addressed in stamped plans, calculations, and cert letter (where applicable) and notify the Engineer of Record of any concerns prior to starting construction.