

April 8, 2021

**PowerHome Solar**  
 919 N. Main St  
 Mooresville, NC 28115

**RE: Ormond Residence**  
**1866 Chriitain Light Road , Fuquay-Varinia , NC 27526**  
**Client Project #: 1866ORMO**  
**PFE Project #: 212206**

On behalf of PowerHome Solar, Penn Fusion Engineering LLC (PFE) performed a site visit and structural analysis of the roof design at the above referenced location. The purpose of our analysis was to determine if the existing design of the roof system is structurally sufficient to support the new photovoltaic modules in addition to the code required design loads. Information used for this analysis was determined by a site survey performed by a representative of PFE and is isolated only to the areas where the modules are intended to be placed. If any discrepancies are found by the contractor during installation, please contact PFE.

**System Specifications:**

 Panel Specs: (40) Silfab – SIL  
 Racking System: Quick Mount PV – QRail Light

The modules are to be located on the following roof planes:

Mounting Plane	Rafter Size	Rafter Spacing	Horizontal Span	Collar Ties	Collar Tie Spacing	Sheathing	Shingle Type	Number of Shingle Layers	Ceiling Profile
1	2x6	16"	26ft. 2in.		"	CDX 1/2"	Metal Corrugated	1	Flat
2	2x6	16"	18ft. 5in.		"	CDX 1/2"	Metal Corrugated	1	Flat

The roof design has been analyzed in accordance with the 2018 North Carolina Residential/Building Code with design loads as follows:

 Ground Snow (Pg): 15 psf  
 Wind Speed (V): 117 mph

**Mounting Plane 1**

The calculations for these structural members are attached. It has been determined by this office that the rafters, as specified above, exceed the allowable span for the total design loading. Attached are repair details that, when installed, will render the roof design structurally adequate to support the new PV modules in addition to the code required design loading.

Attach the module rail brackets to the roof with S-5! Protea Brackets at 48 on center maximum

**Mounting Plane 2**

The calculations for these structural members are attached. It has been determined by this office that the rafters, as specified above, exceed the allowable span for the total design loading. Attached are repair details that, when installed, will render the roof design structurally adequate to support the new PV modules in addition to the code required design loading.

Attach the module rail brackets to the roof with S-5! Protea Brackets at 48 on center maximum

If you have any questions regarding this analysis, please feel free to contact us.

 Best Regards,  
**Penn Fusion Engineering LLC**  
 Firm License No. P-1848

 Andrew D. Leone, P.E.  
 Principal


Client Name: PowerHome Solar  
 PFE Project Number: 212206  
 Client Project Number: 1866ORMO  
 Project: Ormond Residence  
 Address: 1866 Chriitain Light Road  
 Fuquay-Varinia , NC 27526  
 Description: Mounting Plane 1  
 Calculations By: ADL  
 Date: April 8, 2021

### Roof Construction

2x6 Rafters at 16" on center

A=	8.25 in <sup>2</sup>
Ix=	20.8 in <sup>4</sup>
Sx=	7.56 in <sup>3</sup>
Wood Species=	Doug-Fir Larch #2
Fb=	900 psi
Fv=	180 psi
E=	1600000 psi
Roof Slope=	14 °
Rafter Span=	26.17 ft
Ceiling Attached to Rafters?:	No

### Design Criteria

Ground Snow (P <sub>g</sub> ):	15 psf
Design Wind Speed:	117 mph
Live Load:	20 psf
Dead Load:	4.2 psf
PV Modules:	3.09 psf

### Wind Calculations

Directionality Factor (K <sub>d</sub> ):	0.85
Topographic Factor (K <sub>zt</sub> ):	1
Velocity Pressure Exposure Coefficient (K <sub>z</sub> ):	0.7
Importance Factor (I):	1
Velocity Pressure (q <sub>z</sub> ):	20.85 psf
Tributary Square Footage on Component:	10.83 ft <sup>2</sup>
Component Roof Pressures:	14.03 / -57.53 psf

### Snow Load Calculations

Exposure Factor (C <sub>e</sub> ):	1
Thermal Factor (C <sub>t</sub> ):	1
Importance Factor (I):	1
Flat Roof Snow Loads (P <sub>f</sub> ):	15 psf
Roof Slope Factor (C <sub>s</sub> ):	0.86153846153846
Sloped Snow Loads (P <sub>s</sub> ):	12.923076923077 psf
Unbalanced Snow Load:	15 psf

## Member Calculations

### Bending

$M_d$ :	3114.69 ft*lb		
$f_b$ :	4942.32 psi		
Load Duration Factor ( $C_d$ ):	1.15		
Stability Factor ( $C_L$ ):	1		
Wet Service Factor ( $C_M$ ):	1		
Temperature Factor ( $C_T$ ):	1		
Size Factor ( $C_F$ ):	1.3		
Flat Use Factor ( $C_{fu}$ ):	1		
Incising Factor ( $C_i$ ):	1		
Repetitive Member Factor ( $C_r$ ):	1.15		
$F_b$ :	900 psi		
$F'_b$ :	1547.33 psi	4942.32 > 1547.33	No Good in Bending

### Shear

$V_d$ :	476.1 lb		
$f_v$ :	86.56 psi		
Load Duration Factor ( $C_d$ ):	1.15		
Wet Service Factor ( $C_M$ ):	1		
Temperature Factor ( $C_T$ ):	1		
Size Factor ( $C_F$ ):	1.3		
Flat Use Factor ( $C_{fu}$ ):	1		
Incising Factor ( $C_i$ ):	1		
$F_v$ :	180 psi		
$F'_v$ :	207 psi	86.56 <= 207	OK in Shear

### Deflection

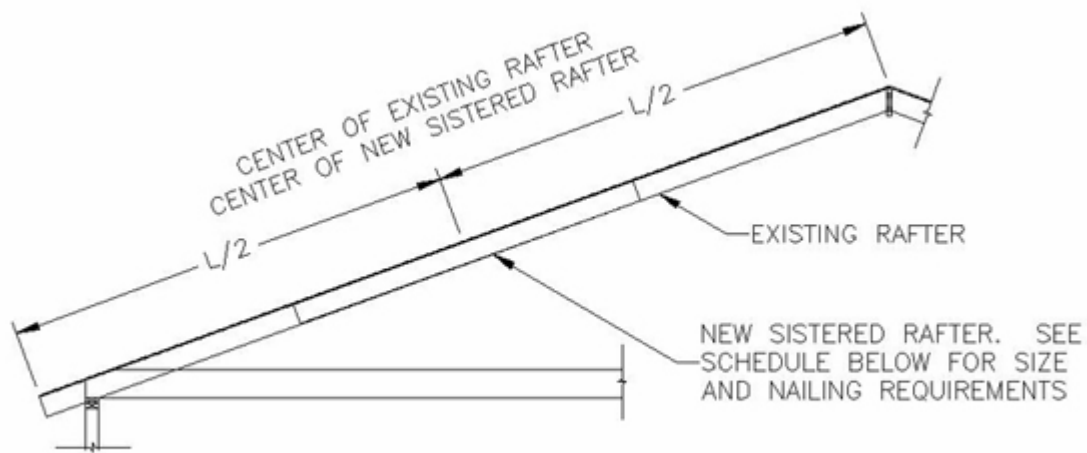
Live Load Deflection ( $\Delta_L$ ):	8.46 in	L/37	No Good in Live Load Deflection
Total Load Deflection ( $\Delta_T$ ):	11.54 in	L/27	No Good in Total Load Deflection

### Uplift Calculation

Tributary Square Footage on Component:	10.83 ft <sup>2</sup>
Uplift Pressure:	-57.53 psf
Uplift per Lag:	-623.25 lbs
Lag Screw Diameter:	5/16 in
Allowable Withdrawal per Inch:	0 lbs/in
Minimal Screw Penetration:	INF in

**Install 5/16" diameter lag screws @ 48 on center with minimum penetration of INF" into rafter.**

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Description: Mounting Plane 1  
Calculations By: ADL  
Date: April 8, 2021



New Sistered Rafter Size: 2x8x7' Doug-Fir Larch #2 or better  
Nailing Requirements: (2) 10d Nails @ 12" on center with (3) additional 10d Nails at each end  
10d = .12" shank diameter x 3" long minimum

Note: Apply repair to each rafter under PV system

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 Description: Mounting Plane 2  
 Calculations By: ADL  
 Date: April 8, 2021

### Roof Construction

2x6 Rafters at 16" on center

A=	8.25 in <sup>2</sup>
Ix=	20.8 in <sup>4</sup>
Sx=	7.56 in <sup>3</sup>
Wood Species=	Doug-Fir Larch #2
Fb=	900 psi
Fv=	180 psi
E=	1600000 psi
Roof Slope=	14 °
Rafter Span=	18.41 ft
Ceiling Attached to Rafters?:	No

### Design Criteria

Ground Snow (P <sub>g</sub> ):	15 psf
Design Wind Speed:	117 mph
Live Load:	20 psf
Dead Load:	4.2 psf
PV Modules:	3.09 psf

### Wind Calculations

Directionality Factor (K <sub>d</sub> ):	0.85
Topographic Factor (K <sub>zt</sub> ):	1
Velocity Pressure Exposure Coefficient (K <sub>z</sub> ):	0.7
Importance Factor (I):	1
Velocity Pressure (q <sub>z</sub> ):	20.85 psf
Tributary Square Footage on Component:	10.83 ft <sup>2</sup>
Component Roof Pressures:	14.03 / -57.53 psf

### Snow Load Calculations

Exposure Factor (C <sub>e</sub> ):	1
Thermal Factor (C <sub>t</sub> ):	1
Importance Factor (I):	1
Flat Roof Snow Loads (P <sub>f</sub> ):	15 psf
Roof Slope Factor (C <sub>s</sub> ):	0.86153846153846
Sloped Snow Loads (P <sub>s</sub> ):	12.923076923077 psf
Unbalanced Snow Load:	15 psf

## Member Calculations

### Bending

$M_d$ :	1540.92 ft*lb		
$f_b$ :	2445.09 psi		
Load Duration Factor ( $C_d$ ):	1.15		
Stability Factor ( $C_L$ ):	1		
Wet Service Factor ( $C_M$ ):	1		
Temperature Factor ( $C_T$ ):	1		
Size Factor ( $C_F$ ):	1.3		
Flat Use Factor ( $C_{fu}$ ):	1		
Incising Factor ( $C_i$ ):	1		
Repetitive Member Factor ( $C_r$ ):	1.15		
$F_b$ :	900 psi		
$F'_b$ :	1547.33 psi	2445.09 > 1547.33	No Good in Bending

### Shear

$V_d$ :	334.87 lb		
$f_v$ :	60.89 psi		
Load Duration Factor ( $C_d$ ):	1.15		
Wet Service Factor ( $C_M$ ):	1		
Temperature Factor ( $C_T$ ):	1		
Size Factor ( $C_F$ ):	1.3		
Flat Use Factor ( $C_{fu}$ ):	1		
Incising Factor ( $C_i$ ):	1		
$F_v$ :	180 psi		
$F'_v$ :	207 psi	60.89 <= 207	OK in Shear

### Deflection

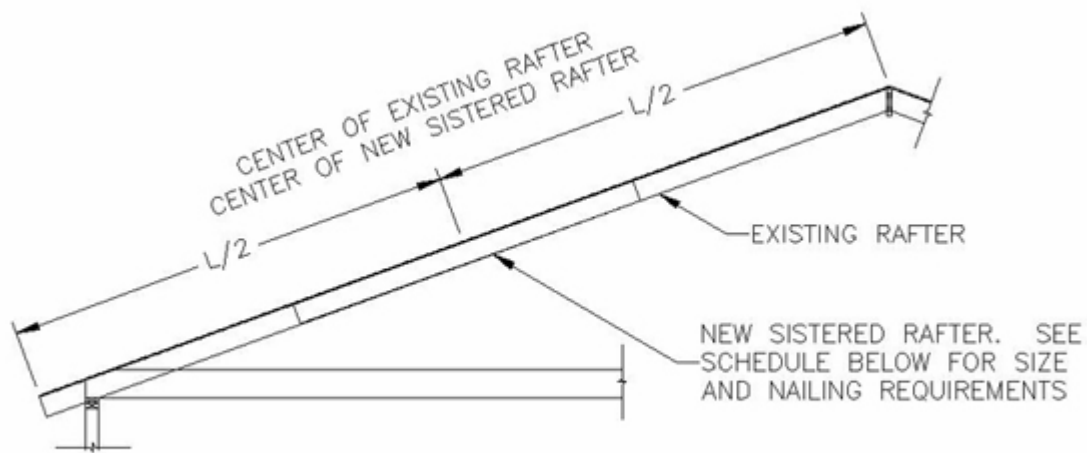
Live Load Deflection ( $\Delta_L$ ):	2.07 in	L/107	No Good in Live Load Deflection
Total Load Deflection ( $\Delta_T$ ):	2.82 in	L/78	No Good in Total Load Deflection

### Uplift Calculation

Tributary Square Footage on Component:	10.83 ft <sup>2</sup>
Uplift Pressure:	-57.53 psf
Uplift per Lag:	-623.25 lbs
Lag Screw Diameter:	5/16 in
Allowable Withdrawal per Inch:	0 lbs/in
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