

April 1, 2020

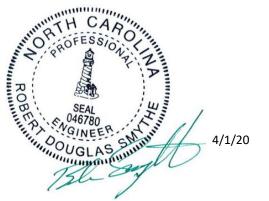
Power Home Solar and Roofing 919 North Main Street Mooresville, NC 28115 Design Criteria: Ultimate Wind Speed- 120 mph Ground Snow Load- 10 psf Risk Category- II Exposure category- C

RE: Structural Roof Evaluation for the *Trim Residence: 51 Richmond Park Drive*, *Cameron, North Carolina*

As per your request, we have evaluated the roof structure under the proposed solar panel array. The information used to evaluate this structure was gathered during a field visit by Power Home Solar and Roofing on behalf of Right Angle Engineering. The roof structure consists of pre-manufactured trusses spaced at 24" on center. The roof material consists of asphalt shingles. The design criteria used to analyze this structure are listed above and included with this letter. The adopted building codes in this jurisdiction are: *the 2018 North Carolina Building Code, the 2018 North Carolina Existing Building Code,* and *ASCE 7-16*.

North Carolina Existing Building Code (NCEBC) 2018 section 807.4 indicates that alterations to an existing building that results in less than a 10% increase in the total stress may be performed without a structural evaluation of the existing building. As demonstrated in the attached calculations, the additional weight of the solar panels will be less than 10% increase in the gravity loading and the stress on the existing roof framing.

Based on our assessment we have determined that the existing roof framing will safely and adequately support the additional loads imposed by the solar panels. In order for the loads to be evenly distributed, the roof attachments should be staggered and spread evenly throughout the panel array. Attachment points should be spaced at a maximum of 48" on center. The racking system should be installed per the manufacture's specifications. There should be a minimum of 29 L-foot attachment points to the roof. Each attachment should have a 5/16" or 18/8 SS lag screw with 2.5" minimum penetration centered on each truss top chord. Waterproofing around the roof penetrations is the responsibility of others. Right Angle Engineering assumes no responsibility for improper installation of the solar panels. Regards,



Robert D Smythe, P.E. Right Angle Engineering



Design Criteria:			
Design Wind Speed (3 second gust)	120	mph	-
Exposure Category	С	-	
Risk Category	2		
Mean Roof Height	30	ft	
Roof Type	Gable Roof		
Building Type	enclosed		
Roof Dead Load- ASCE Table	C3_1		
Asphalt Shingles	2	psf	
3/8" Plywood Sheathing	1.2	psf	
Roof Framing	4	psf	
Insulation	3.85	•	
Gypsum sheathing	2	psf	
Solar Panel Array	3	psf	
Solar Paller Allay	5	psf	
Dead Load Without Panels	13.05	psf	
Dead Load With Solar panels	16.05	psf	
Dead Load With Solar pariets	10.05	psi	
Roof Live Load			
Existing Roof Live Load	20	psf	ASCE 7-16 Table 4.3-1
Roof Live Load with Solar Panels	0	psf	2018 NCBC 1607.12.5
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Roof Snow Load-ASCE 7-16			
Ground Snow Load (pg)	10	psf	Section 7.2
Exposure Factor (Ce)	0.9		Table 7.3-1
Thermal Factor (Ct)	1.1		Table 7.3-2
Importance Factor (Is)	1		Table 1.5-2
Flat Roof Snow Load (Pf)	7		Equation 7.3-1
Slippery surface Slope Factor (Cs)	0.72		Figure 7-2
Nonslippery Surface Slope Factor			
(Cs)	1		Figure 7-2
Roof Snow Load	7	psf	Equation 7.4-1
Reduced Roof Snow Load (Slippery			-
Surface)	5	psf	Equation 7.4-1
Load Combinations - ASCE 7-	16 Section 2.4.1		
		With Solar	
	Without Solar Panels	panels	
D + Lr	33 psf	16 psf	
D + S	20 psf	21 psf	



Solar Array 1-		
Roof Slope	27	degrees
Number of panels	14	
Panel Area	245	ft^2

Wind Calculations- ASCE 7-16			
GCp Zone 1	-1		Figure 30.3-(2A-5B)
GCp Zone 2	-1.2		Figure 30.3-(2A-5B)
GCp Zone 3	-1.2		Figure 30.3-(2A-5B)
Gcpi	0.18		Table 26.13-1
Velocity Pressure (qh)	30.7	psf	
qh= .00256KhKhtKdV^2			Equation 26.10-1
Kh	0.98		Table 26.10-1
Kht	1		Equation 26.8-1
Kd	0.85		Table 26.6-1
Designed wind pressure (P)		psf	Equation 30.8-1
Р= qh(GCh) - (GChi))			
Zone 1 Pressure (P)	-36.2	psf	
Zone 2 Pressure (P)	-42.4	psf	
Zone 3 Pressure (P)	-42.4	psf	

Roof Connection		_	
Shear Capacity	190	lbs	NDS 2015 Table 12K
Shear tributary area	52.5	ft^2	
Pullout Capacity	266	lbs/in	
Lag screw embedment	2.5	in	
Total pullout capacity	665	lbs	NDS 2015 Table 12.2A
Pullout max tributary area	15.7	ft^2	
Factor of Safety	2.2		
Minimum number of connections	29		

Beam Stress NCEBC 2018 Section 806.	2			
Beam Span	16	ft		-
Spacing	2	ft		
Roof Framing type	pre-manufactured trusses			
Panel Orientation	landscape			
Number of Panels per rafter	3			
Panel distance from eave	4			
	Without Solar	With Solar	Percent	
	Panels	Panels	Increase	
Bending Moment	2115.2 ft-lbs	1281.2 ft-lbs	60.6%	Less than 105%
Vertical Reaction (V1)	528.8 lbs	356.6 lbs	67.4%	Less than 105%
Vertical Reaction (V2)	528.8 lbs	348.1 lbs	65.8%	Less than 105%