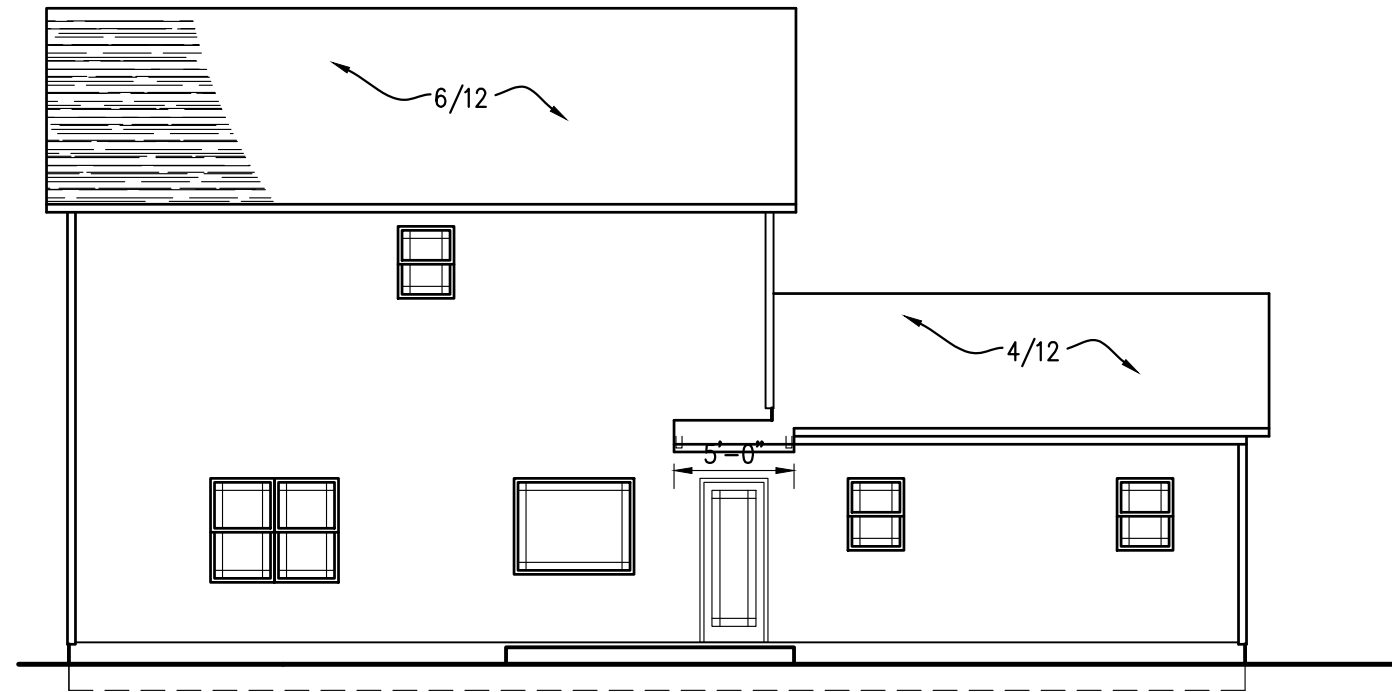


**APPROVED**  
 Limited building only review  
 Permit holder responsible for  
 full compliance with the code

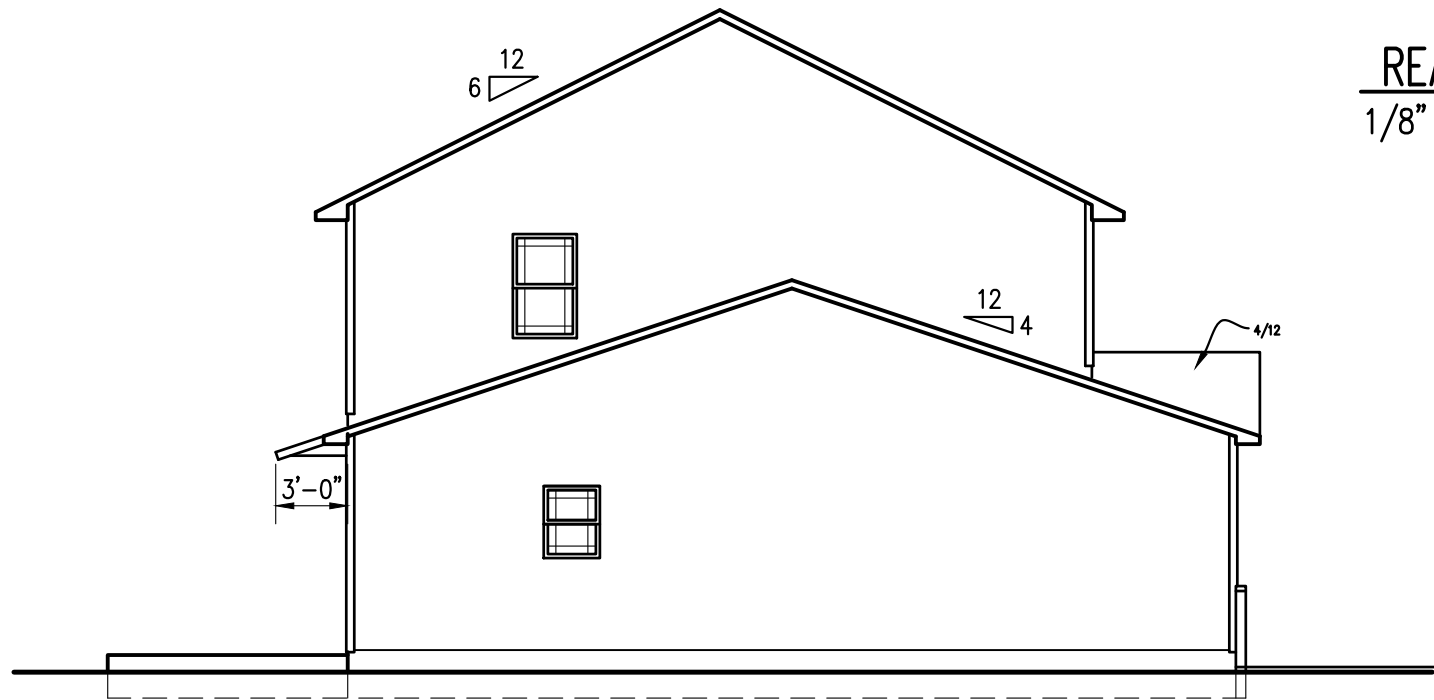
**NOTICE TO CONTRACTOR**  
 All construction must comply with current NC Building Codes  
 and is subject to field inspection and verification.



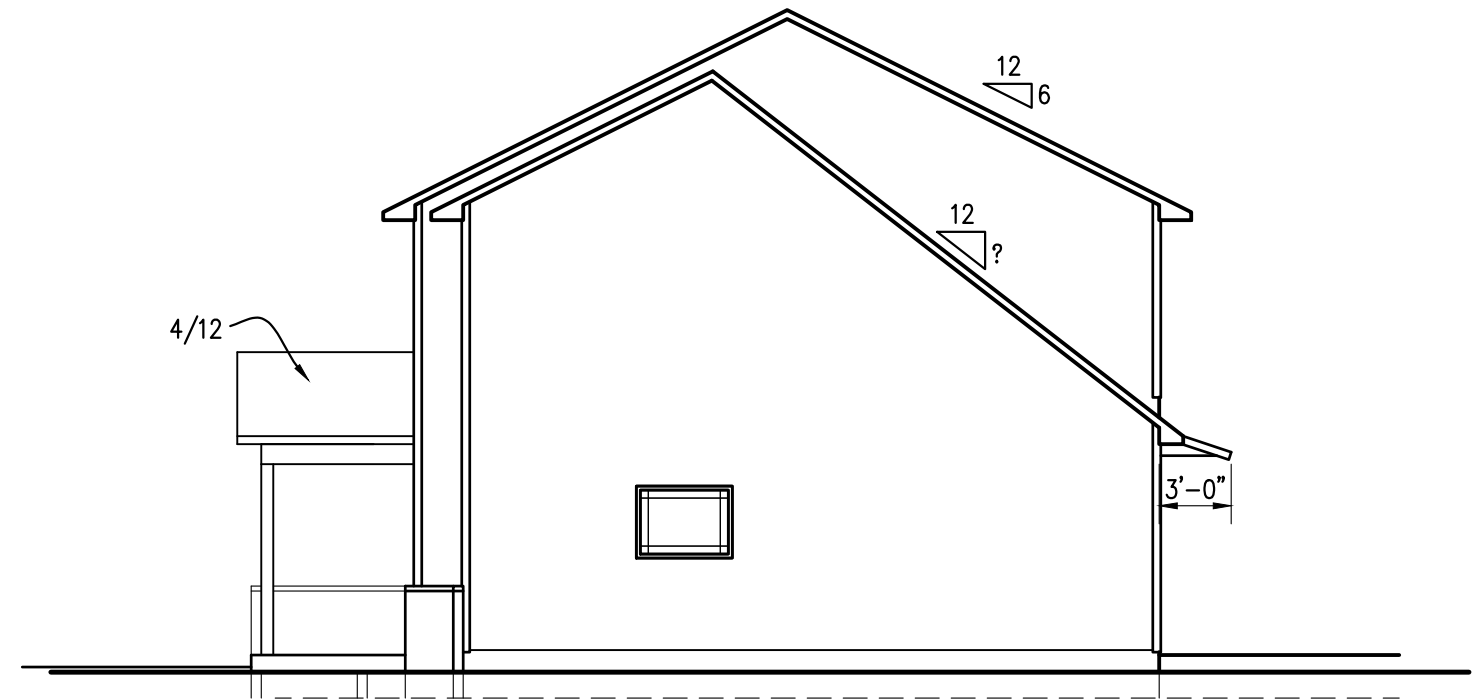
**03/30/2020**



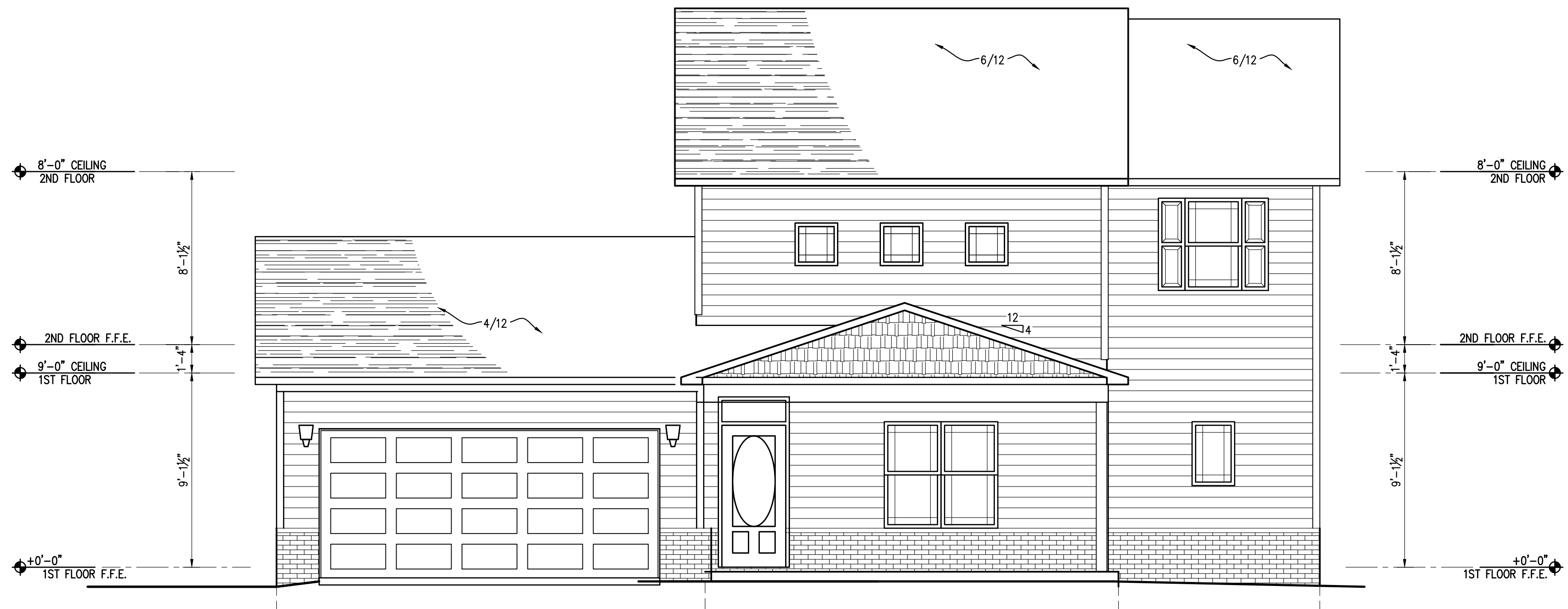
**REAR ELEVATION**  
 $1/8" = 1'-0"$



**LEFT ELEVATION**  
 $1/8" = 1'-0"$



**RIGHT ELEVATION**  
 $1/8" = 1'-0"$

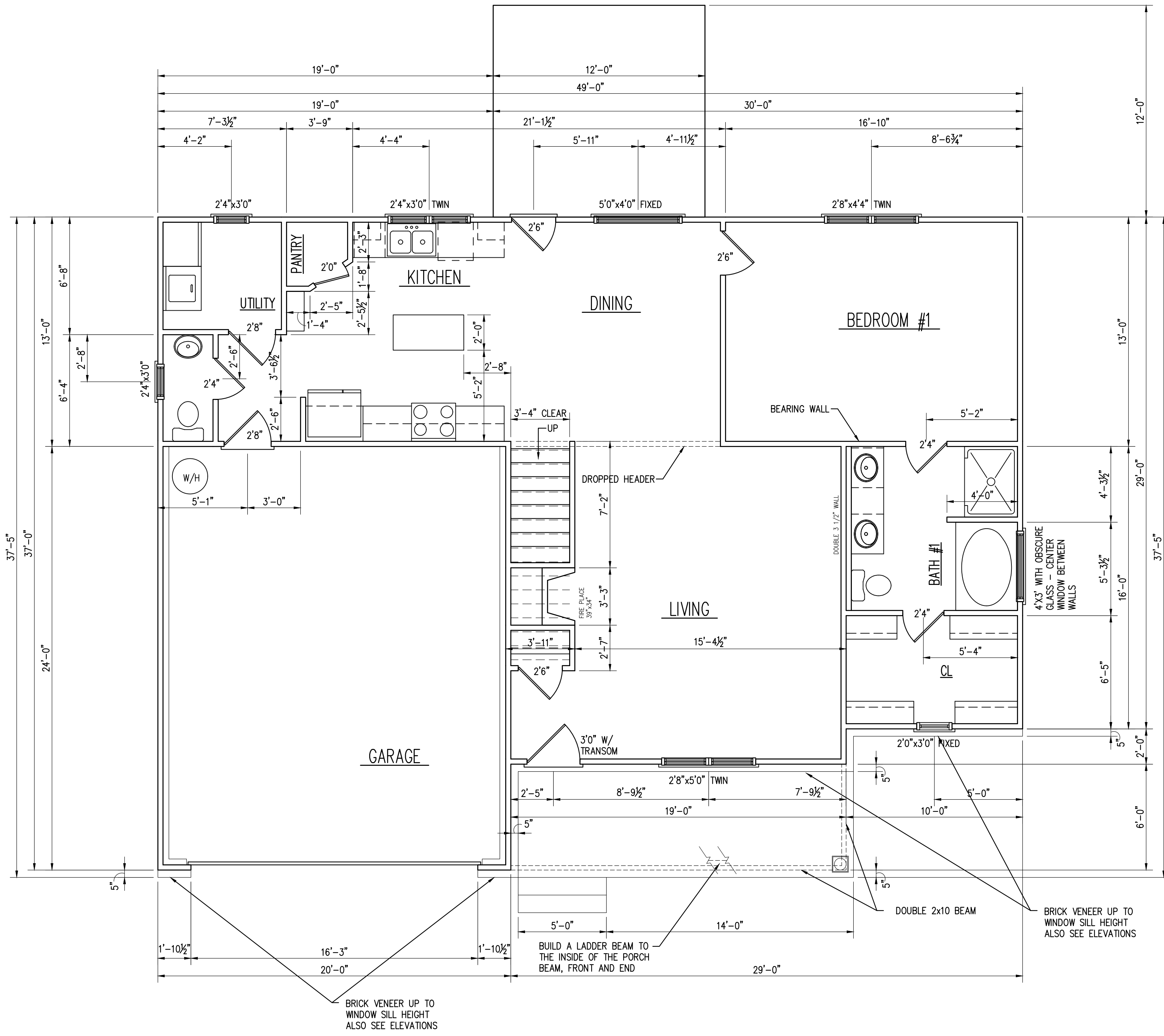


**FRONT ELEVATION**  
 $1/4" = 1'-0"$

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**OUR ASHER DESIGN**  
 LOT #: 19  
 Subdivision: Sweetwater

**IVERCON, INC.**  
 PO Box 64778 910-237-3461-ph  
 Fayetteville, NC 28306 910-717-5076-fax



ALL DIMENSIONS PROVIDED ARE TO FACE OF FRAMING MEMBERS UNLESS NOTED OTHERWISE

AREA DATA  
 FRAMED FIRST FLOOR 1144 SF  
 FRAMED SECOND FLOOR 760 SF  
 FRAMED GARAGE 474 SF (SLAB 436 SF)  
 BONUS ROOM N/A  
 TOTAL HEATED 1,904 SF  
 GROSS HEATED WITH GARAGE 2,378 SF

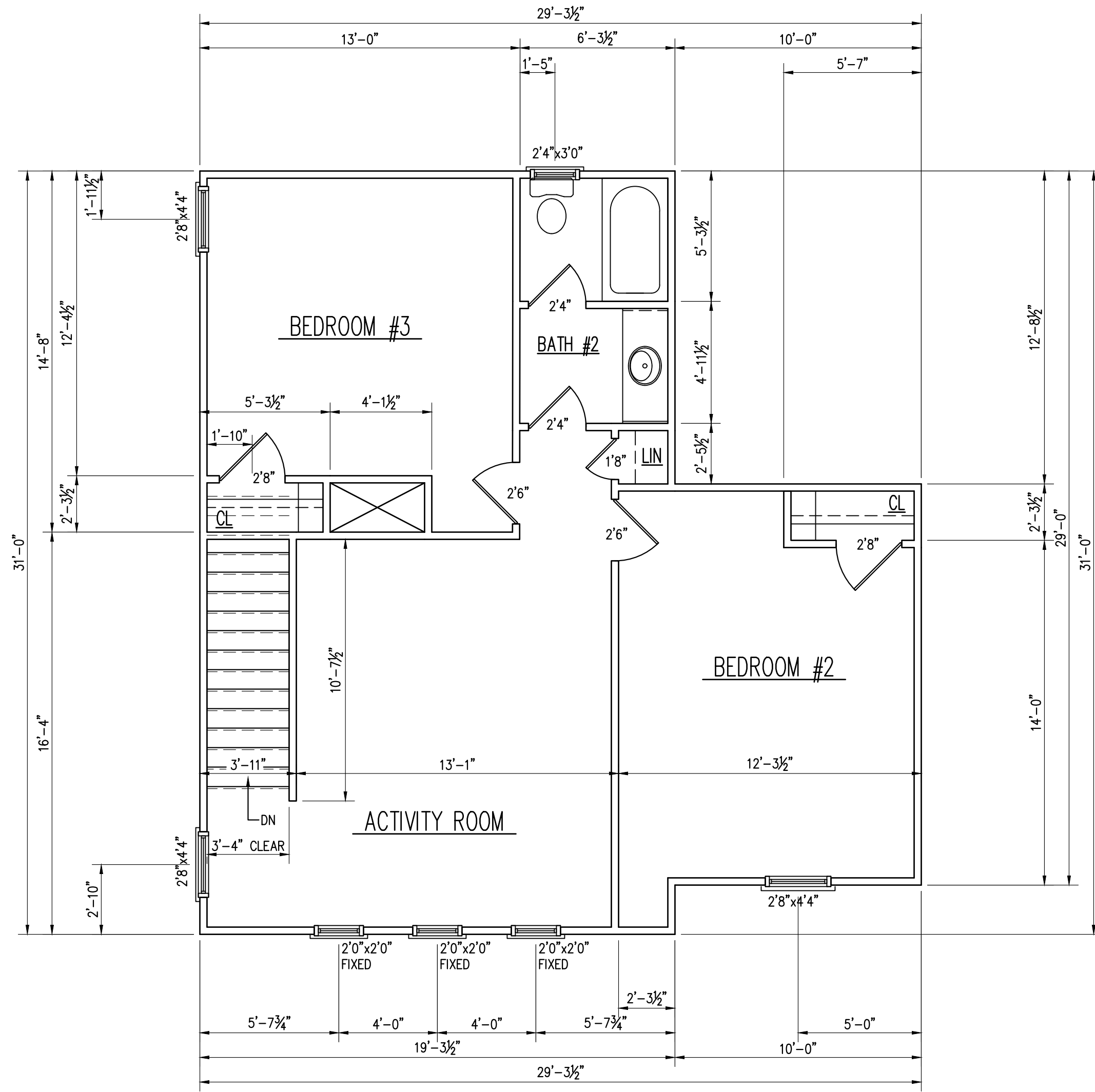
COVERED PORCH FRONT 114 SF (SLAB 98)  
 PATIO PATIO 144 SF

**FIRST FLOOR PLAN**  
 1/4" = 1'-0"

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**OUR ASHER DESIGN**  
 LOT #: 19  
 Subdivision: Sweetwater

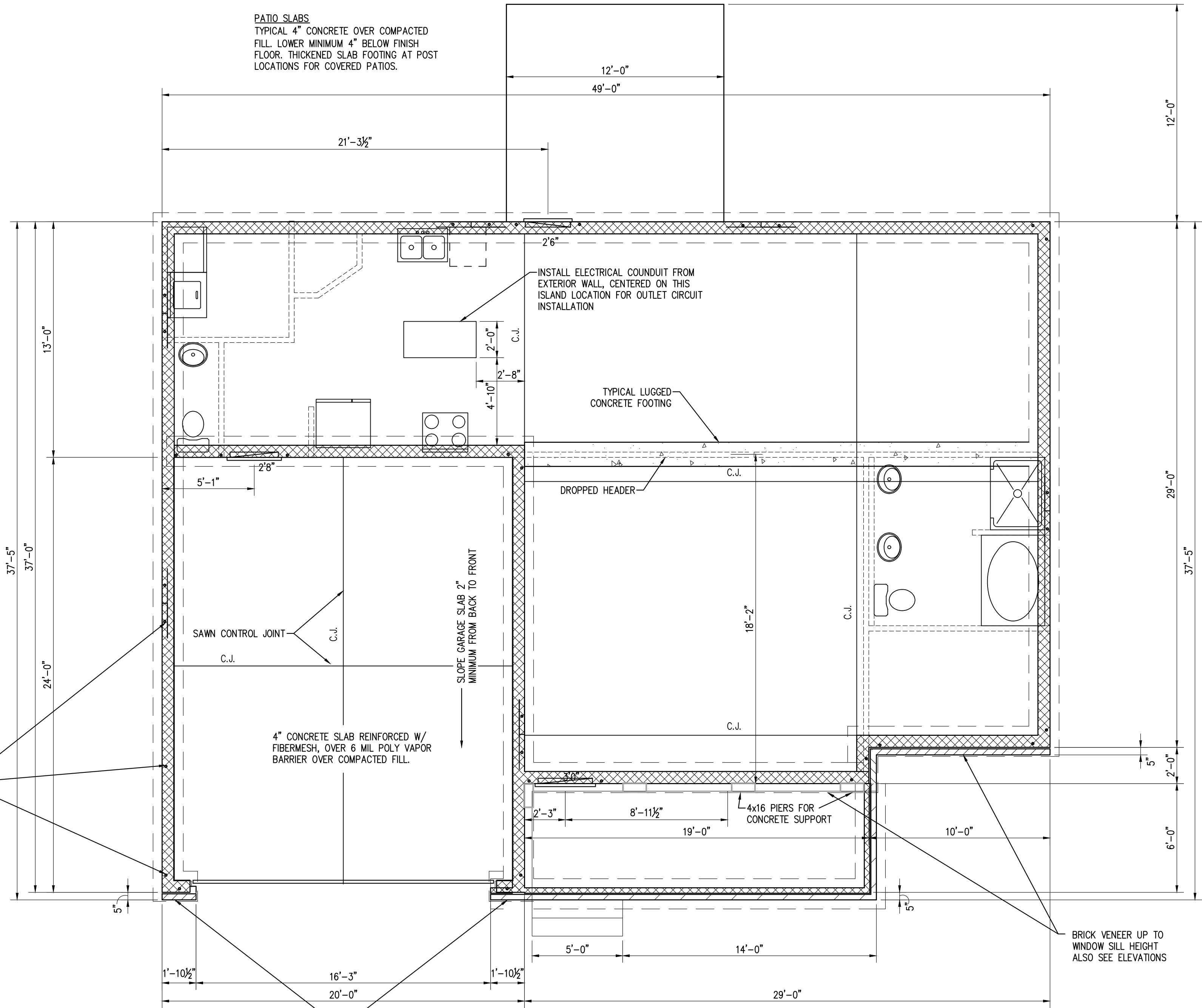
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 910-717-5076-fax



**SECOND FLOOR PLAN**  
 1/4" = 1'-0"

1/2"x12" ANCHOR BOLTS @ 6'-0" O.C., 1'-0" FROM ENDS AT CORNERS, 1'-0" FROM ENDS AT ALL DOOR JAMBS, AND 1'-0" FROM ENDS AT ALL SPLICES IN WALL PLATES

**PATIO SLABS**  
 TYPICAL 4" CONCRETE OVER COMPACTED FILL. LOWER MINIMUM 4" BELOW FINISH FLOOR. THICKENED SLAB FOOTING AT POST LOCATIONS FOR COVERED PATIOS.



4" CONCRETE SLAB REINFORCED W/ FIBERMESH, OVER 6 MIL POLY VAPOR BARRIER OVER COMPACTED FILL.

BRICK VENEER UP TO WINDOW SILL HEIGHT ALSO SEE ELEVATIONS

**PORCH SLABS**  
 TYPICAL 4" CONCRETE SLAB OVER COMPACTED FILL. LOWER MINIMUM 4" BELOW FINISH FLOOR.

**2-STORY HOUSE PLANS**  
 INSTALL 3 COURSES OF BRICK TO TOP OF MASONRY FOUNDATION FOR 8" SOLID CAP. FOR BLOCK FOUNDATIONS FILL CELLS SOLID IN TOP 8" COURSE OF MASONRY

BRICK VENEER UP TO WINDOW SILL HEIGHT ALSO SEE ELEVATIONS

**FOUNDATION PLAN**

1/4" = 1'-0"

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**OUR ASHER DESIGN**  
 LOT #: 19  
 Subdivision: Sweetwater

**IVERCON, INC.**  
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 Fayetteville, NC 28306  
 910-237-3461-ph  
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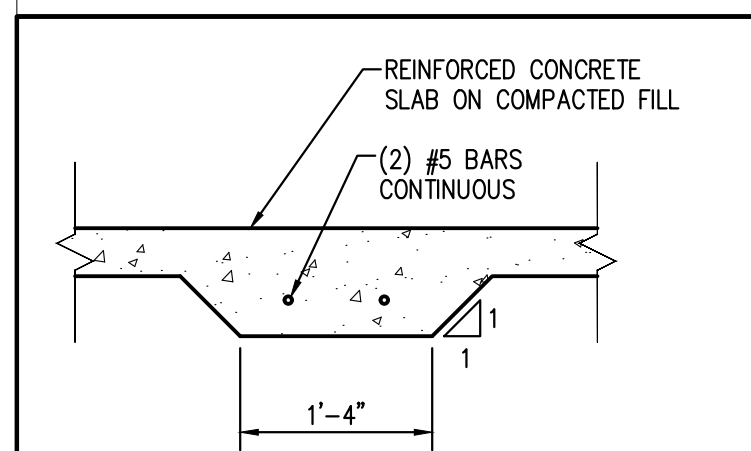
THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND BRING THE ATTENTION OF THE DESIGNER AS SOON AS IS PRACTICAL. THE CONTRACTOR IS REQUIRED TO MAKE ALL NECESSARY ADJUSTMENTS TO THE FOUNDATION CONSTRUCTION AS IS REQUIRE TO ADAPT TO THE SITE CONDITIONS.

**FIXTURE / DEVISE LEGEND**

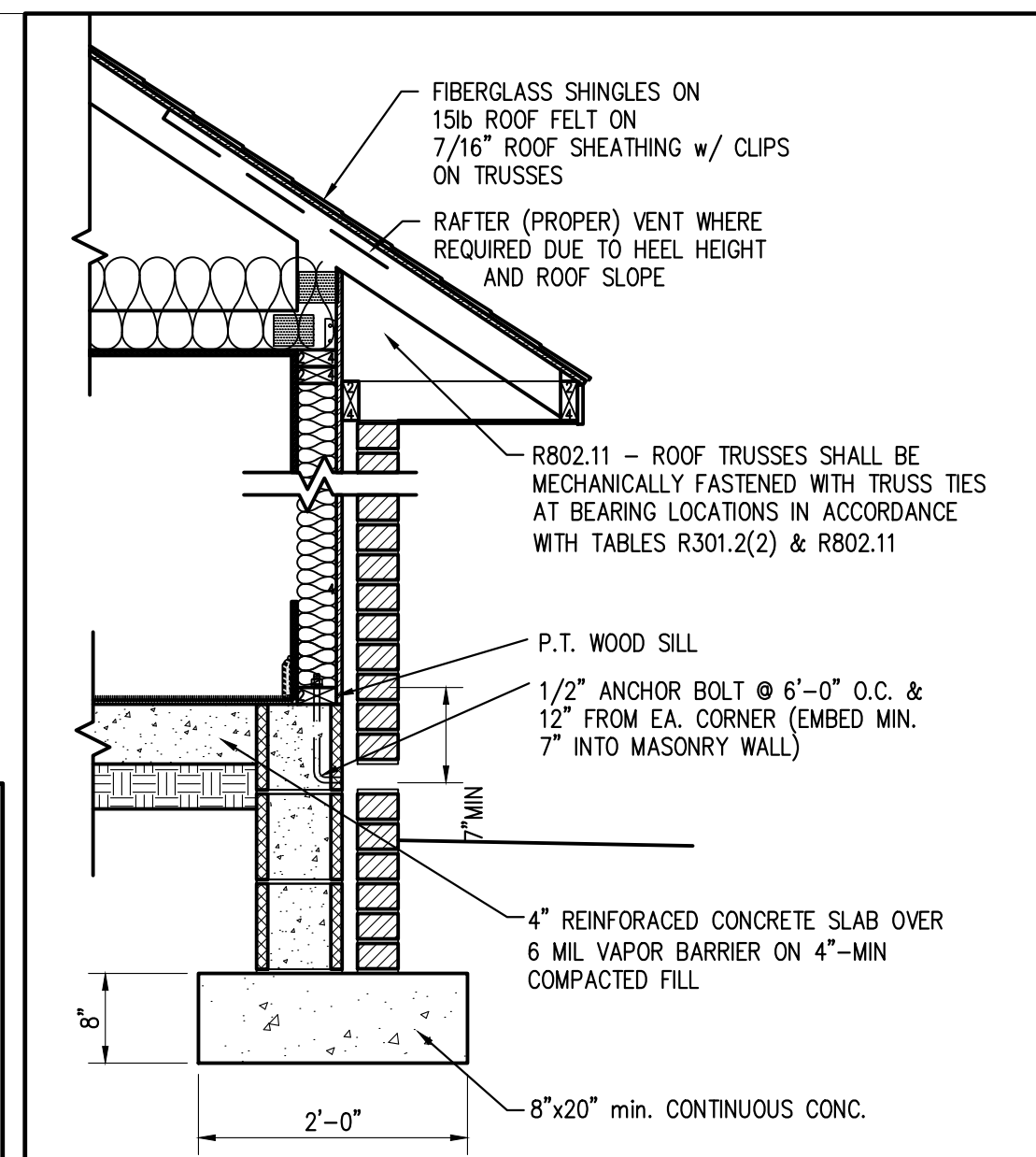
	CEILING FAN		SMOKE DETECTOR		WALL OUTLET
	CEILING FAN WITH LIGHT		EXHAUST FAN		220 VOLT OUTLET
	CEILING-MOUNTED FLUORESCENT LIGHT		EXHAUST FAN W/ LIGHT		GROUND-FAULT OUTLET
	CEILING-MOUNTED LIGHT		TELEPHONE JACK		WATER PROOF OUTLET
	6" FLUSH MOUNT ROUND LED		DOOR BELL		OUTLET MOUNTED @ 44" AFF (ALL OUTLETS AT CABINETS)
	4" FLUSH MOUNT ROUND LED		CABLE JACK		DUPLEX OUTLET W/DUPLEX USB
	WALL-MOUNTED LIGHT		THERMOSTAT		STACKED DOUBLE SWITCH
	RECESSED DIRECTIONAL LIGHT		FLOOR OUTLET		ONE POLE SWITCH
	FLOOD LIGHT W/ MOTION SENSOR		RETURN AIR		THREE-WAY SWITCH
	FLUORESCENT		GARAGE DOOR OPENER		FOUR-WAY SWITCH
			DOOR BELL		DIMMER SWITCH
					ELECTRICAL PANEL
					EXTERIOR WATER FAUCET

**GENERAL NOTES**

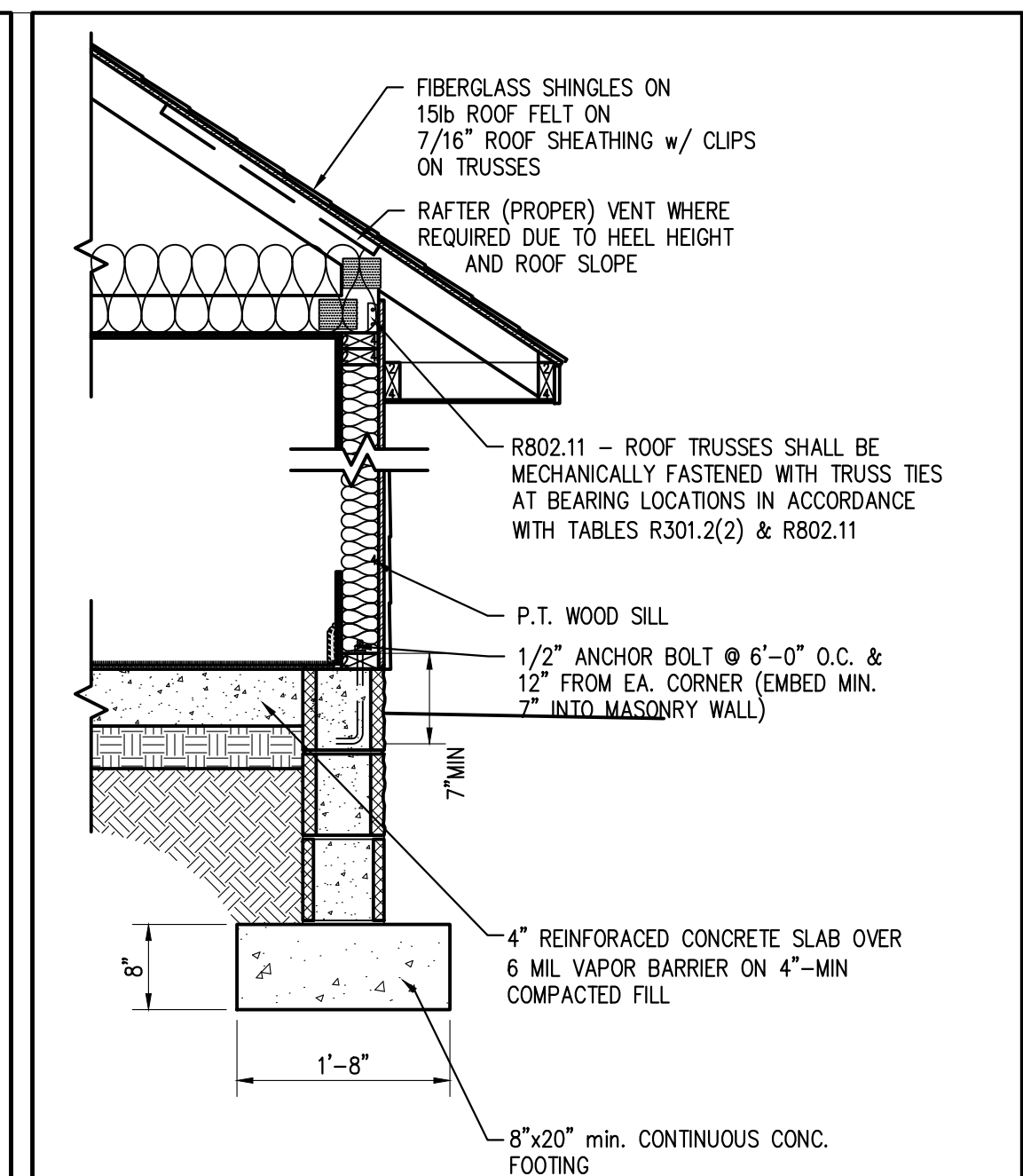
- PROVIDE CRAWL SPACE ACCESS OPENING MIN. 24"x36"
- MAINTAIN MIN 18" SPACE BETWEEN TOP OF PIERS AND CRAWL SPACE GRADE. ADJUST FOR MECHANICAL DUCT CLEARANCES AS REQUIRED
- PROVIDE VAPOR BARRIER IN CRAWL SPACE
- MIN 8"x16" FOUNDATION VENTS. QUANTITY AS REQUIRED TO MEET MIN FREE AIR AREA AS CALCULATED
- ANCHOR BOLTS @ 6'-0" OC AND 1'-0" FROM EACH CORNER OF BUILDING EMBEDDED MIN 7" INTO MASONRY CELL GROUTED SOLID
- ALL SILL PLATES ON SLAB CONSTRUCTION SHALL BE PRESSURE TREATED MIN 2X4
- ALL MUD SILLS SHALL BE PRESSURE TREATED LUMBER
- PROVIDE THRU-WALL FLASHING @ ALL EXTERIOR WALLS @ TOP OF FOUNDATION WALL, AND MINIMUM 8" ABOVE GRADE
- PROVIDE THRU WALL FLASHING AT ALL DOOR AND WINDOW HEADS IN BRICK VENEER, EXTENDED MIN 8" BEYOND EACH END OF OPENING. FULL HEAD WEEPS MIN 32". MIN TWO WEEPS PER OPENING
- PROVIDE STEEL LINTELS @ ALL DOOR AND WINDOW HEADS IN BRICK VENEER
- TYPICAL CEILING HEIGHT 8'-0" AFF (UNO)
- TYPICAL WINDOW HEAD HEIGHT 6'-8" AFF (UNO)
- FULL HEAD WEEP JOINTS AT ALL THRU-WALL FLASHING, MIN 32" OC
- 1'-9" OVERHANGE FROM FACE OF STUD FRAMING WITH BRICK VENEER (FOR 1'-4" EXPOSED OVERHANG))
- 1'-4" OVERHANGE FROM STUD FRAMING @ LOCATIONS WITH HORIZONTAL SIDING
- OVERHANGE ON COVERED PORCHES MATCH OVERHANGE @ EXTERIOR WALL
- TYPICAL 5" FROM EXTERIOR FACE OF STUD TO EXTERIOR FACE OF BRICK VENEER
- ALL DIMENSIONS PROVIDED ARE TO FACE OF FRAMING MEMBERS UNLESS NOTED OTHERWISE



**TYPICAL LUGGED FOOTING**  
3/4" = 1'-0"



**TYPICAL WALL SECTION W/ BRICK VENEER**  
3/4" = 1'-0" LESS THAN 110mph WIND ZONE



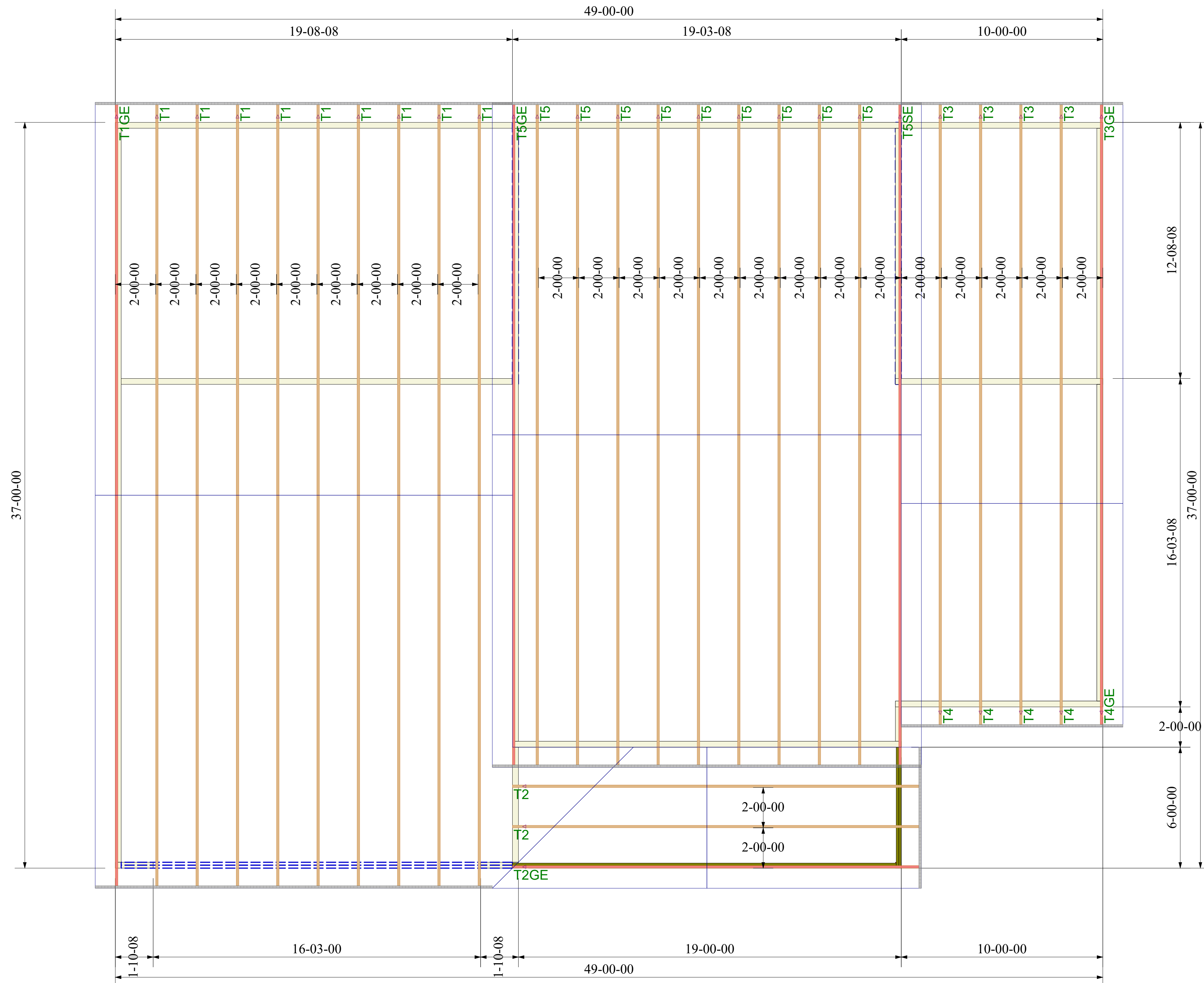
**TYPICAL WALL SECTION**  
3/4" = 1'-0" LESS THAN 110mph WIND ZONE  
FOUNDATION WALL HEIGHT VARIES AS GRADING REQUIRES

THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND BRING THE ATTENTION OF THE DESIGNER AS SOON AS IS PRACTICAL. THE CONTRACTOR IS REQUIRED TO MAKE ALL NECESSARY ADJUSTMENTS TO THE FOUNDATION CONSTRUCTION AS IS REQUIRE TO ADAPT TO THE SITE CONDITIONS.

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**OUR ASHER DESIGN**  
LOT #: 19  
Subdivision: Sweetwater

**IVERCON, INC.**  
PO Box 64778 910-237-3461-ph  
Fayetteville, NC 28306 910-717-5076-fax



Wall Framing					
PlotID	Length	Product	Plies	Net Qty	Fab Type
	32-00-00	2x4 SPF No.2	1	3	FF
	20-00-00	2x4 SPF No.2	1	6	FF
	16-00-00	2x4 SPF No.2	1	3	FF
	14-00-00	2x4 SPF No.2	1	3	FF
	12-00-00	2x4 SPF No.2	1	6	FF
	8-00-00	2x4 SPF No.2	1	128	FF
	2-00-00	2x4 SPF No.2	1	3	FF
	8-00-00	2x4 SPF No.2	1	8	FF
		7/16" 4x8 OSB	1	31	FF

Truss Connector Total List			
Manuf	Product	Qty	
USP	One RT7A	73	
USP	THD26	4	

# ROOF TRUSS FRAMING

DRAWING SCALE : NTS



Ivercon Construction

19 Sweetwater

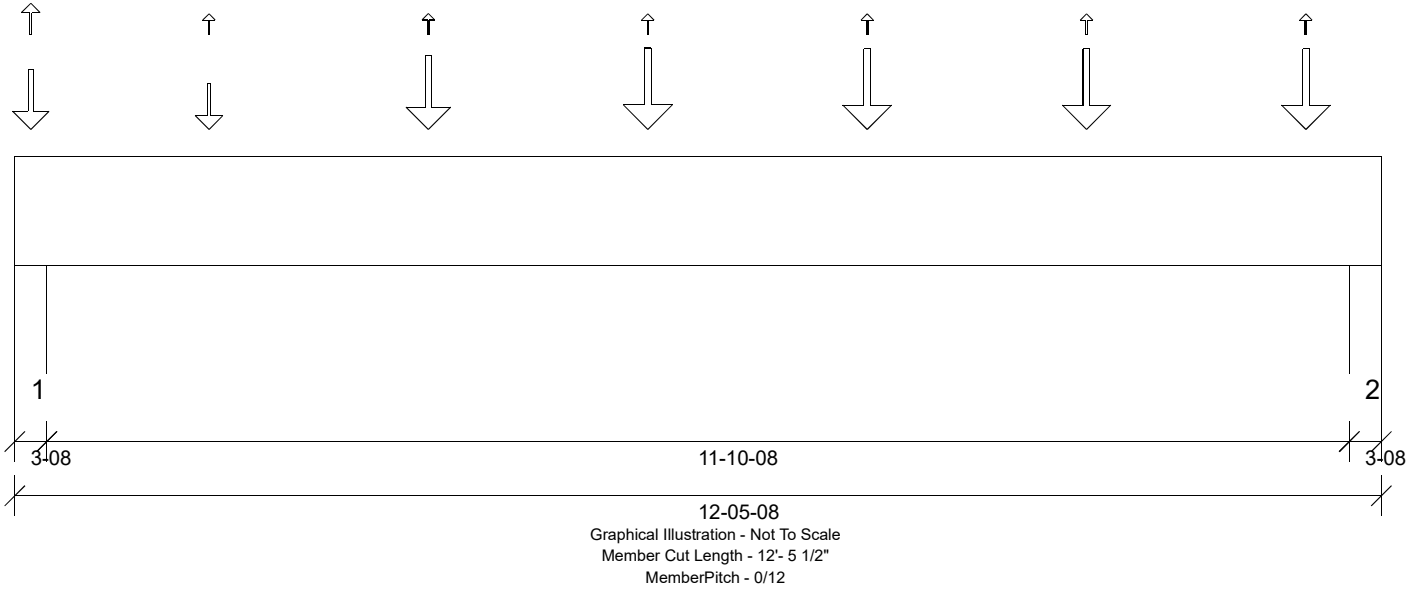
ROOF TRUSS PLACEMENT PLAN

REVISIONS	
DATE	BY

PROJECT NUMBER  
20020068

SHEET NUMBER

1 / 1



**Design Information:**

Building Code:	IRC2015	Floor Dead Load:	10.0 lb/ft <sup>2</sup>	Roof Dead Load:	10.0 lb/ft <sup>2</sup>	Ground Snow Load:	20.0 lb/ft <sup>2</sup>
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft <sup>2</sup>	Roof Live Load:	20.0 lb/ft <sup>2</sup>		
		Unbraced Length	Top: 1'- 8 1/2"	Bottom:	11'- 10 1/2"		

**Design Results:**

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	5'- 9 1/4"	18598.25 lb ft	21161.10 lb ft	Passed - 88%	1.00	D + L
Critical Moment (Neg)	0'- 2 1/2"	-64.86 lb ft	19839.68 lb ft	Passed - 0%	1.00	D + L
Critical Shear	11'- 2 1/8"	4739.05 lb	8035.42 lb	Passed - 59%	1.00	D + L
Live Load Deflection	6'- 3 9/16"	0'- 3/8"	0'- 3/4" (L/360)	Passed - L/398	-	L
Total Load Deflection	6'- 3 1/2"	0'- 1/2"	0'- 1" (L/240)	Passed - L/283	-	D + L
Max. Reaction			Supported Mt/ Supporting Mt/			
	0'- 2 1/2"	5756.60 lb	9187.42 lb 16206.61 lb	Passed - 63%	1.00	D + L
	12'- 3"	6918.10 lb	9187.48 lb 16206.71 lb	Passed - 75%	1.00	D + L

**Design Notes:**

\* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

**Loading:**

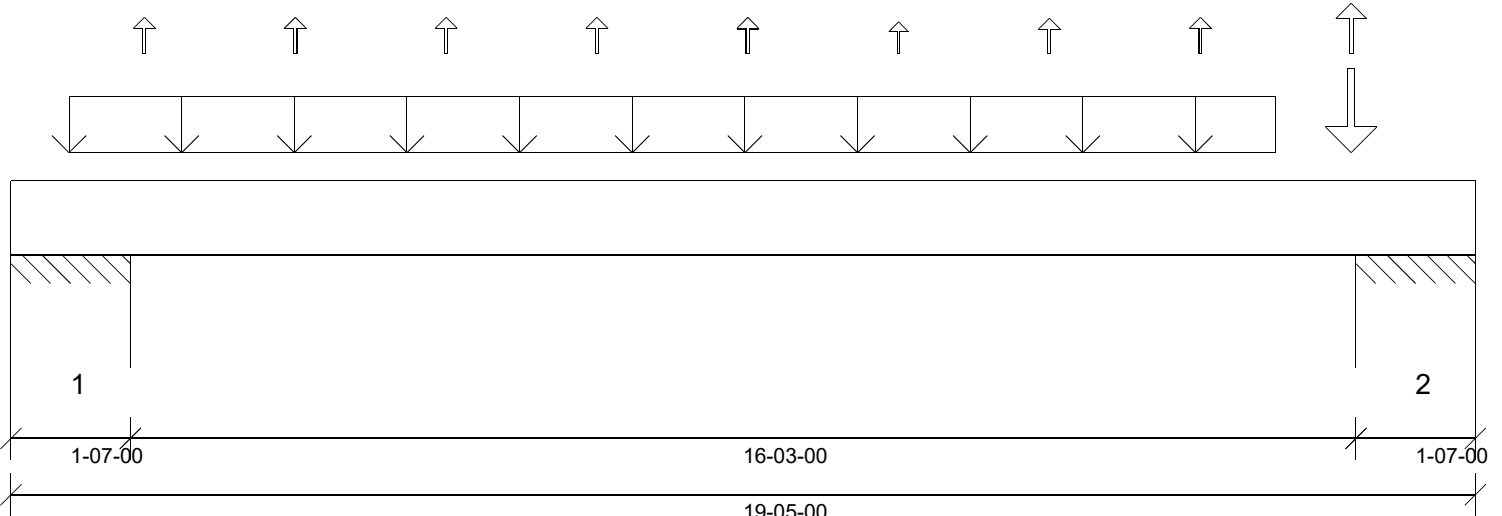
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	12'- 5 1/2"	Self Weight	11 lb/ft	-	-	-
Point	0'- 1 3/4"	0'- 1 3/4"	BM3(i64)	804.00 lb	230.00 lb	242.00 lb	104.00 lb
Point	1'- 9 1/4"	1'- 9 1/4"	F4(c01)	248.00 lb	651.00 lb	10.00 lb	5.00 lb
Point	3'- 9 1/4"	3'- 9 1/4"	F2GR(c01)	581.00 lb	1333.00 lb	16.00/-3.00 lb	6.00 lb
Point	5'- 9 1/4"	5'- 9 1/4"	F2(c04)	603.00 lb	1593.00 lb	8.00 lb	4.00 lb
Point	7'- 9 1/4"	7'- 9 1/4"	F2(c07)	597.00 lb	1568.00 lb	8.00 lb	4.00 lb
Point	9'- 9 1/4"	9'- 9 1/4"	F2(c02)	597.00 lb	1568.00 lb	8.00 lb	4.00 lb
Point	11'- 9 1/4"	11'- 9 1/4"	F2(c06)	597.00 lb	1568.00 lb	8.00 lb	4.00 lb

**Support Information:**

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3 1/2"	3(i10)	2204.00 lb	3565.00 lb	275.00/-2.00 lb	119.00 lb
2	12'- 2"	12'- 5 1/2"	1(i9)	1959.00 lb	4946.00 lb	25.00/-1.00 lb	12.00 lb

**Errors, Warnings & Notes:**

- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale  
 Member Cut Length - 19'- 5"  
 MemberPitch - 0/12

**Design Information:**

Building Code:	IRC2015	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	20.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 1'- 10 1/2"	Bottom:	16'- 3"		

**Design Results:**

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	9'- 9 1/4"	15470.72 lb ft	24295.56 lb ft	Passed - 64%	1.15	D + Lr	
Critical Moment (Neg)	17'- 11 1/2"	-1752.07 lb ft	22741.96 lb ft	Passed - 8%	1.60	D + 0.75(L + S + 0.6W)	
Critical Shear	2'- 6 7/8"	3346.11 lb	9240.73 lb	Passed - 36%	1.15	D + Lr	
Live Load Deflection	9'- 8 9/16"	0'- 7/16"	0'- 3/4" (L/360)	Passed - L/467	-	Lr	
Total Load Deflection	9'- 8 9/16"	0'- 13/16"	0'- 1" (L/240)	Passed - L/247	-	D + Lr	
Max. Reaction			Supported Mt/	Supporting Mt/			
	0'- 1 1/2"	2995.50 lb	18375.00 lb	32413.50 lb	Passed - 16%	1.60	0.6D + 0.6W
	0'- 1 1/2"	-1294.80 lb	18375.00 lb	-	Passed - 7%	1.60	D + 0.75(L + S + 0.6W)
	1'- 5 1/2"	4659.91 lb	18375.00 lb	32413.50 lb	Passed - 25%	1.60	D + 0.75(L + Lr + 0.6W)
	1'- 5 1/2"	-3589.86 lb	18375.00 lb	-	Passed - 20%	1.60	0.6D + 0.6W
	17'- 11 1/2"	5308.82 lb	18375.00 lb	32413.50 lb	Passed - 29%	1.60	D + 0.75(L + Lr + 0.6W)
	17'- 11 1/2"	-3173.91 lb	18375.00 lb	-	Passed - 17%	1.60	0.6D + 0.6W
	19'- 3 1/2"	2663.32 lb	18375.00 lb	32413.50 lb	Passed - 14%	1.60	0.6D + 0.6W
	19'- 3 1/2"	-1305.29 lb	18375.00 lb	-	Passed - 7%	1.60	D + 0.75(L + S + 0.6W)

**Design Notes:**

\* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

**Loading:**

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	19'- 5"	Self Weight	11 lb/ft	-	-	-
Uniform	0'- 9 1/4"	16'- 9 1/4"	Smoothed Load	209 lb/ft	-	237 lb/ft	132 lb/ft
Point	1'- 9 1/4"	1'- 9 1/4"	T1(c08)	-	-	-	-
Point	3'- 9 1/4"	3'- 9 1/4"	T1(c07)	-	-	-	-
Point	5'- 9 1/4"	5'- 9 1/4"	T1(c06)	-	-	-	-
Point	7'- 9 1/4"	7'- 9 1/4"	T1(c05)	-	-	-	-
Point	9'- 9 1/4"	9'- 9 1/4"	T1(c09)	-	-	-	-
Point	11'- 9 1/4"	11'- 9 1/4"	T1(c04)	-	-	-	-
Point	13'- 9 1/4"	13'- 9 1/4"	T1(c03)	-	-	-	-
Point	15'- 9 1/4"	15'- 9 1/4"	T1(c02)	-	-	-	-
Point	17'- 9 1/4"	17'- 9 1/4"	T1(c01)	628.00 lb	-	899.00 lb	513.00 lb

**Support Information:**

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	1'- 7"	E12(i33)	1968.00 lb	-	2111.00 lb	1176.00 lb
==>	0'- 1 1/2"	0'- 1 1/2"	E12(i33)	-	-	-	-
==>	1'- 5 1/2"	1'- 5 1/2"	E12(i33)	1968.00 lb	-	2111.00 lb	1176.00 lb
2	17'- 10"	19'- 5"	E11(i8)	2217.00 lb	-	2580.00 lb	1449.00 lb
==>	17'- 11 1/2"	17'- 11 1/2"	E11(i8)	2217.00 lb	-	2580.00 lb	1449.00 lb
==>	19'- 3 1/2"	19'- 3 1/2"	E11(i8)	-	-	-	-

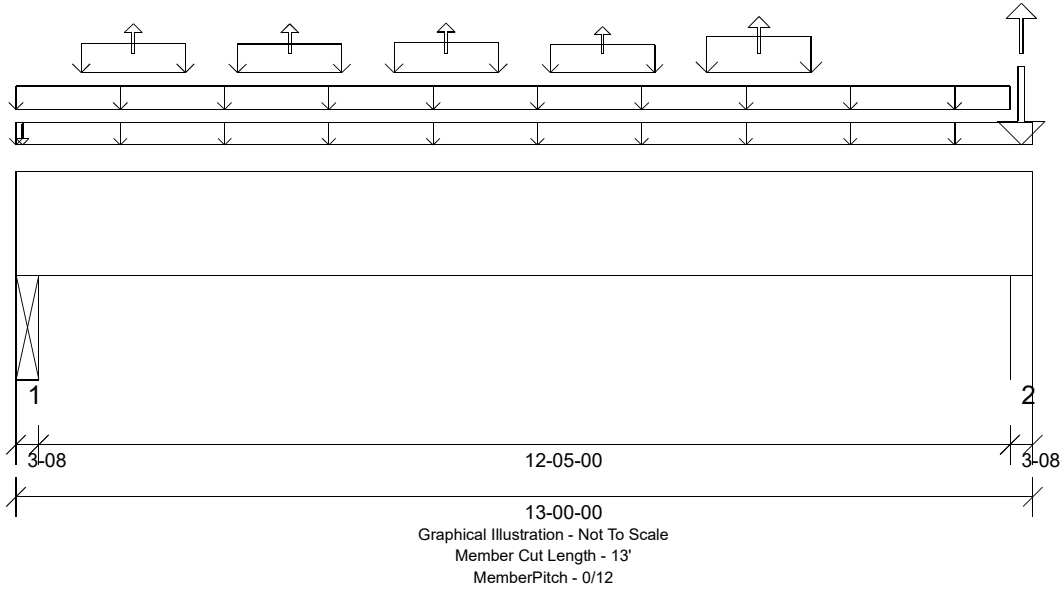
**Errors, Warnings & Notes:**

- \* CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.





**Design Information:**

Building Code:	IRC2015	Floor Dead Load:	10.0 lb/ft <sup>2</sup>	Roof Dead Load:	10.0 lb/ft <sup>2</sup>	Ground Snow Load:	20.0 lb/ft <sup>2</sup>
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft <sup>2</sup>	Roof Live Load:	20.0 lb/ft <sup>2</sup>		
		Unbraced Length Top:	0'	Bottom:	12'-5"		

**Design Results:**

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	6'- 5 7/16"	3161.15 lb ft	37216.21 lb ft	Passed - 8%	1.00	D + L
Critical Moment (Neg)	12'- 9 1/2"	-34.58 lb ft	42798.63 lb ft	Passed - 0%	1.15	D + Lr
Critical Shear	11'- 4 1/2"	789.36 lb	10826.67 lb	Passed - 7%	1.00	D + L
Live Load Deflection	6'- 6 1/16"	0'	0'- 3/4" (L/360)	Passed - L/999	-	0.75(L + Lr + 0.6W)
Total Load Deflection	6'- 6"	0'- 1/16"	0'- 1" (L/240)	Passed - L/999	-	D + 0.75(L + Lr + 0.6W)
Max. Reaction			<u>Supported Mt</u> <u>Supporting Mt</u>			
	0'- 2 1/2"	1033.12 lb	9187.57 lb    9187.57 lb	Passed - 11%	1.00	D + L
	12'- 9 1/2"	1548.55 lb	9187.57 lb    5206.29 lb	Passed - 30%	1.15	D + 0.75(L + Lr)

**Design Notes:**

\* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

**Loading:**

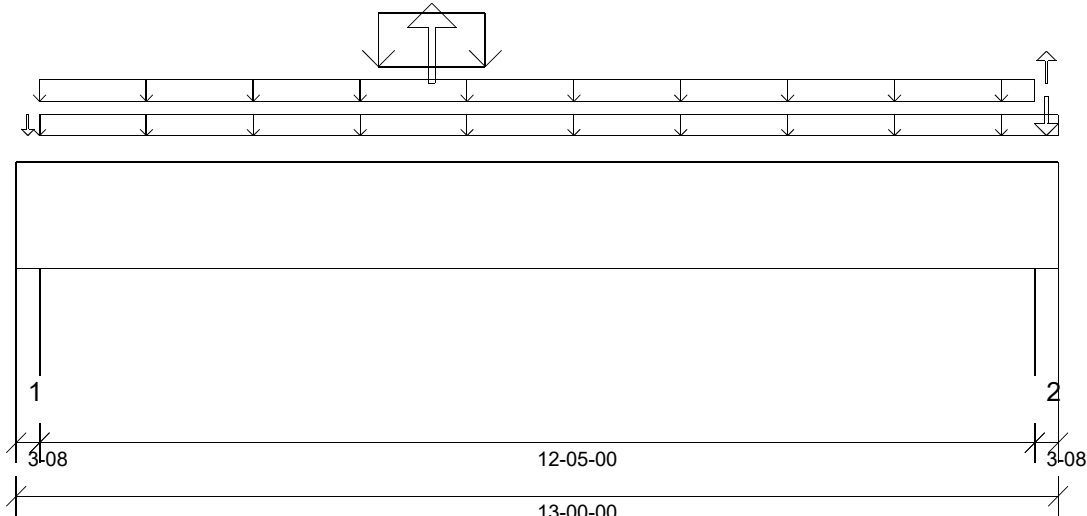
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	13'	Self Weight	15 lb/ft	-	-	-
Uniform	0'	13'	FC1 Floor Material	9 lb/ft	35 lb/ft	-	-
Uniform	-0'	12'- 8 1/2"	E16(i24)	65 lb/ft	-	-	-
Uniform	0'- 10"	2'- 2"	E16(i24)	55 lb/ft	-	56 lb/ft	25 lb/ft
Uniform	2'- 10"	4'- 2"	E16(i24)	54 lb/ft	-	55 lb/ft	25 lb/ft
Uniform	4'- 10"	6'- 2"	E16(i24)	58 lb/ft	-	60 lb/ft	26 lb/ft
Uniform	6'- 10"	8'- 2"	E16(i24)	38 lb/ft	-	59 lb/ft	18 lb/ft
Uniform	8'- 10"	10'- 2"	E16(i24)	96 lb/ft	-	93 lb/ft	43 lb/ft
Point	0'- 1"	0'- 1"	E16(i24)	-	-	9.00 lb	4.00 lb
Point	1'- 6"	1'- 6"	E16(i24)	-	-	-	-
Point	3'- 6"	3'- 6"	E16(i24)	-	-	-	-
Point	5'- 6"	5'- 6"	E16(i24)	-	-	-	-
Point	7'- 6"	7'- 6"	E16(i24)	-	-	-	-
Point	9'- 6"	9'- 6"	E16(i24)	-	-	-	-
Point	12'- 10 1/4"	12'- 10 1/4"	E17(i29)	305.00 lb	-	240.00 lb	121.00 lb

**Support Information:**

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3 1/2"	HDR(i63)	804.00 lb	230.00 lb	242.00 lb	104.00 lb
2	12'- 8 1/2"	13'	E3(i40)	1046.00 lb	230.00 lb	438.00 lb	204.00 lb

**Errors, Warnings & Notes:**

- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale  
 Member Cut Length - 13'  
 MemberPitch - 0/12

**Design Information:**

Building Code:	IRC2015	Floor Dead Load:	10.0 lb/ft <sup>2</sup>	Roof Dead Load:	10.0 lb/ft <sup>2</sup>	Ground Snow Load:	20.0 lb/ft <sup>2</sup>
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft <sup>2</sup>	Roof Live Load:	20.0 lb/ft <sup>2</sup>		
		Unbraced Length Top: 0'		Bottom: 12'- 5"			

**Design Results:**

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	5'- 4 15/16"	5876.79 lb ft	42798.63 lb ft	Passed - 14%	1.15	D + Lr
Critical Moment (Neg)	5'- 2 1/4"	-619.77 lb ft	59545.93 lb ft	Passed - 1%	1.60	0.6D + 0.6W
Critical Shear	1'- 7 1/2"	1318.13 lb	12450.67 lb	Passed - 11%	1.15	D + Lr
Live Load Deflection	6'- 1 3/8"	0'	0'- 3/4" (L/360)	Passed - L/999	-	0.6W
Total Load Deflection	6'- 3 3/16"	0'- 1/16"	0'- 1" (L/240)	Passed - L/999	-	D + 0.75(L + Lr + 0.6W)
Max. Reaction			<u>Supported Mt</u> <u>Supporting Mt</u>			
	0'- 2 1/2"	1499.05 lb	9187.32 lb    5206.15 lb	Passed - 29%	1.15	D + 0.75(L + Lr)
	12'- 9 1/2"	1491.25 lb	9187.42 lb    5206.20 lb	Passed - 29%	1.15	D + 0.75(L + Lr)

**Design Notes:**

\* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

**Loading:**

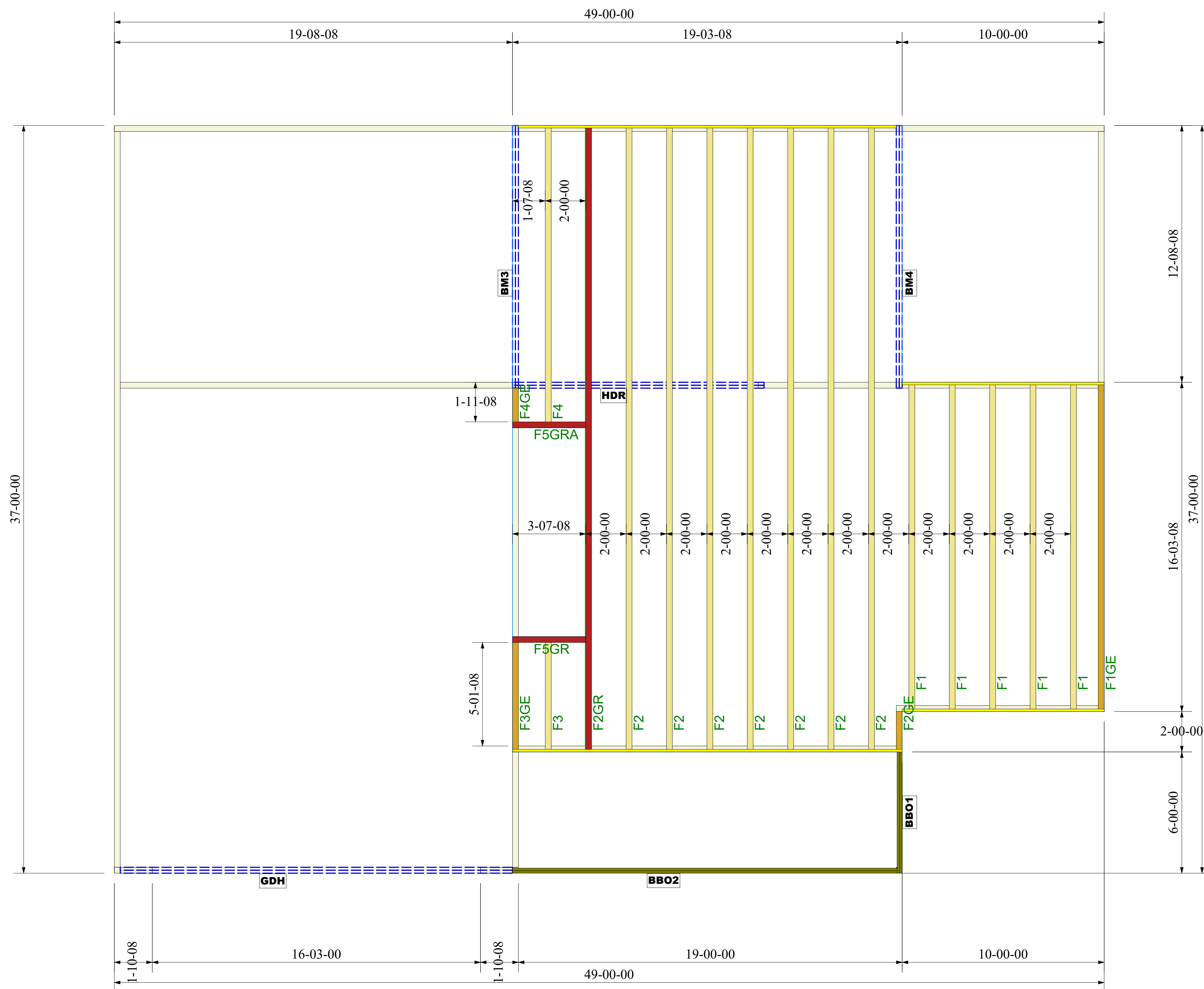
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	13'	Self Weight	15 lb/ft	-	-	-
Uniform	0'- 3 1/2"	13'	FC1 Floor Material	8 lb/ft	30 lb/ft	-	-
Uniform	0'- 3 1/2"	12'- 8 1/2"	E18(i23)	65 lb/ft	-	-	-
Uniform	4'- 6 1/4"	5'- 10 1/4"	E18(i23)	554 lb/ft	-	554 lb/ft	276 lb/ft
Point	0'- 1 3/4"	0'- 1 3/4"	E19(i28)	30.00 lb	-	-	-
Point	5'- 2 1/4"	5'- 2 1/4"	E18(i23)	-	-	-	-
Point	12'- 10 1/4"	12'- 10 1/4"	E17(i29)	148.00 lb	-	177.00/-27.00 lb	61.00 lb

**Support Information:**

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3 1/2"	1(i9)	1024.00 lb	189.00 lb	447.00 lb	223.00 lb
2	12'- 8 1/2"	13'	E3(i40)	990.00 lb	197.00 lb	470.00/-27.00 lb	207.00 lb

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Products					
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH	20-00-00	2.0 RigidLam LVL 1-3/4 x 11-7/8	2	2	FF
HDR	14-00-00	2.0 RigidLam LVL 1-3/4 x 11-7/8	2	2	FF
BM3	14-00-00	2.0 RigidLam LVL 1-3/4 x 16	2	2	FF
BM4	14-00-00	2.0 RigidLam LVL 1-3/4 x 16	2	2	FF
BBO2	20-00-00	1-08 x 9-04 Generic Material	2	2	FF
BBO1	8-00-00	1-08 x 9-04 Generic Material	2	2	FF
PBO1	10-00-00	3-08 x 3-08 Generic Material	1	1	FF

Truss Connector Total List		
Manuf	Product	Qty
USP	MSH422	4

# FLOOR TRUSS FRAMING

DRAWING SCALE : NTS



Ivercon Construction  
 19 Sweetwater  
 FLOOR TRUSS PLACEMENT PLAN

REVISIONS	
DATE	BY

PROJECT NUMBER  
20020068  
 SHEET NUMBER  
**1 / 1**