PREPARED 6/20/17, 14:39:46 INSPECTION TICKET PAGE 23 Harnett County INSPECTOR: IVR DATE 6/21/17 ADDRESS . : 85 DOT CT SUBDIV: ATKINS VILLAGE PH 3 CONTRACTOR : ROYAL OAKS BUILDING GROUP LLC PHONE: (919) 233-3886 OWNER . . : ROYAL OAKS BUILDING GROUP PHONE : PARCEL . . : 04-0664- - -0020- -47-APPL NUMBER: 17-50040983 CP NEW RESIDENTIAL (SFD) DIRECTIONS: T/S: 03/21/2017 08:39 AM JBROCK ----ATKINS VILLAGE #45 STRUCTURE: 000 000 45X67 3BDR CRAWL W/ GARAGE & DECK FLOOD ZONE . . . : FLOOD ZONE X # BEDROOMS : 3.00 PROPOSED USE : SFD SEPTIC - EXISTING? . . . : NEW TANK WATER SUPPLY : COUNTY _______ PERMIT: CPSF 00 CP * SFD REQUESTED INSP DESCRIPTION TYP/SO COMPLETED RESULT RESULTS/COMMENTS A814 01 6/21/17 TI ADDRESS CONFIRMATION TIME: 17:00 VRU #: 002987469

B101 01 6/21/17 TI

6/21/17

B103 01

T/S: 06/20/2017 02:16 PM JBROCK -----

R*BLDG FOOTING / TEMP SVC POLE TIME: 17:00 VRU #: 002987477 T/S: 06/20/2017 02:16 PM JBROCK -----

R*BLDG FOUND & TEMP SVC POLE TIME: 17:00 VRU #: 002987485 T/S: 06/20/2017 02:16 PM JBROCK -----



June 7, 2017

Geoteoanibal Engineering
Environmental Joha Jung
Construction Materials Testins

Mr. Rich Sherman
Royal Oaks Homes
rsherman@royaloakshomes.com

Report of Observations Atkins Village North, Lot 45 Fuquay Varing, North Carolina Our Project Number 121-15-78801

Gentlemen:

As requested, a representative of TerraTech Engineers, Inc. was present at the above referenced site on June 5, 2017 to perform testing on the crawl space footing excavations for the proposed residential home located in the area of Lot #45. The purpose of our testing was to verify that the design soil bearing pressure is available for the concrete foundations. We understand that an allowable soil bearing pressure of 2,000 pounds per square foot (psf) is required. Our services did not include surveying. Locations are based on the excavations performed by others.

Our field examinations consisted of visual observations, dynamic cone penetrometer testing in accordance with ASTM STP-399, and hand rod probing at selected locations. Dynamic cone penetrometer testing was performed at select locations and to a maximum depth of 4 feet below the over-excavated foundation bearing elevation. Our scope did not include mechanically drilled soil test borings to evaluate deeper subsurface soil conditions that could affect foundation support. Such services can be provided, if desired.

During our site visit, soft, wet soils were observed to a depth of approximately 1 foot below the planned foundation bearing elevation. We recommended that the soft, wet materials be removed from the footing excavation. We were informed by the contractor that the footing excavations would be backfilled with consolidated #57 washed stone to re-establish the foundation bearing elevation. After these remedial measures were performed and prior to placement of #57 washed stone, the results of our footing examinations indicated that an allowable bearing capacity of 2,000 pounds per square foot (psf) was available at the locations and depths tested at the time of our investigation. Additionally, the foundation dimension met the plan minimum requirements.

Exposure to the environment, especially rainfall, may weaken the soils beneath the foundation bearing surface, if they are exposed for extended periods of time prior to concrete placement. If the over-excavated soils beneath the foundation bearing surface become softened due to exposure, the soft soils should be compacted or removed and replaced prior to placement of washed stone and concrete.

If you have any questions concerning this information, please do not hesitate to call.

Sincerely,

TerraTech Engineer

7/11 (US

Thomas O. Helms

Project Engineer

TOH/sk

cc: rsargent@royaloa

Christopher S. Pilz, P.E. Principal Geotechnical Engineer