

ADDRESS : 246 CYPRESS DR SUBDIV: ROLLING SPRINGS  
 CONTRACTOR : WELLCO CONTRACTORS, INC. PHONE : (910) 436-3131  
 OWNER : WELLCO CONTRACTORS INC PHONE : (910) 436-3131  
 PARCEL : 01-0506- - -0068- -15-  
 APPL NUMBER: 16-50040354 CP NEW RESIDENTIAL (SFD)  
 DIRECTIONS : HIDDEN LAKES #89 - CYPRESS DR  
 T/S: 12/09/2016 10:07 AM JBROCK ----

STRUCTURE: 000 000 48.3X42 4BDR SLAB W/GARAGE &  
 FLOOD ZONE : FLOOD ZONE X  
 # BEDROOMS : 4.00 PROPOSED USE : SFD  
 SEPTIC - EXISTING? : NEW TANK WATER SUPPLY : COUNTY

PERMIT: CPSF 00 CP \* SFD

TYP/SQ	REQUESTED COMPLETED	INSP RESULT	DESCRIPTION RESULTS/COMMENTS
B101 01	2/07/17	JH	R*BLDG FOOTING / TEMP SVC POLE VRU #: 002929790
	2/07/17	AP	
B103 01	2/23/17	JH	R*BLDG FOUND & TEMP SVC POLE VRU #: 002936737
	2/23/17	AP	
A814 01	2/23/17	SB	ADDRESS CONFIRMATION TIME: 17:00 VRU #: 002936482
	2/24/17	AP	246 CYPRESS DR SPRING LAKE 28390 T/S: 02/24/2017 03:30 PM SBENNETT
P309 01	3/10/17	JH	R*PLUMB UNDER SLAB VRU #: 002943505
	3/10/17	AP	T/S: 03/10/2017 01:20 PM JHALL
B111 01	3/14/17	DT	R*BLDG SLAB INSP/TEMP SVC POLE VRU #: 002944470
	3/14/17	AP	T/S: 03/14/2017 12:33 PM DETAYLOR
R425 01	6/01/17	JH	FOUR TRADE ROUGH IN VRU #: 002978861
	6/01/17	DA	1) Seal all cracks in air barriers in garage & corner of laundry room. 2) Missing structural nail guard over electrical panel in garage. 3) Missing air barrier under stairs in garage. 4) Need 2 jack studs each end of front door header. 5) Missing anchor bolts in garage on right side, to right of window in dining room, half bath & all interior load bearing walls. 6) Need engineer letter for drilled holes in floor truss in stairs & broken roof truss web at top of stairs. 7) Missing fire caulk & nail guards At top & bottom plates on drain line in right back bed closet. 8) Need chase for duct in right front bedroom closet. 9) Need 5 studs under each end of 2ply truss in front right bedroom & laundry room. 10) Missing nails in roof truss hangers in bonus room
R425 02	6/05/17	TI	FOUR TRADE ROUGH IN VRU #: 002979780

*6-5-17 AP JH*

COMMENTS AND NOTES

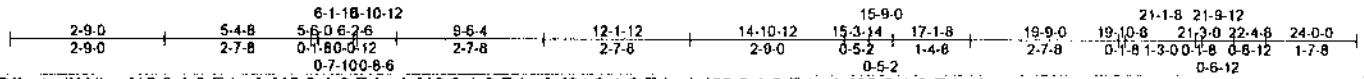
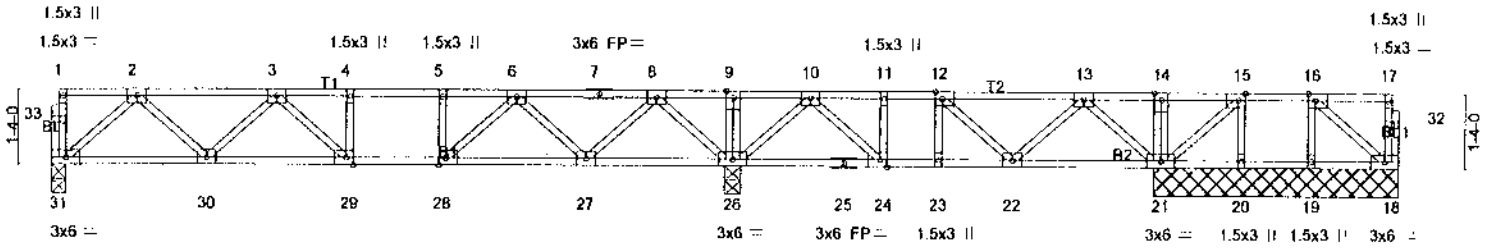


Plate Offsets (X, Y) -- [12:0-1-8, Edge], [15:0-1-8, Edge], [16:0-1-8, Edge], [24:0-1-8, Edge], [28:0-1-8, Edge], [29:0-1-8, Edge]					
TCLL 40.0	Plate Grip DOL 1.00	TC 0.36	Vert(LL) -0.06 29-30 >999 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.42	Vert(TL) -0.09 29-30 >999 360		
BCLL 0.0	Rep Stress Incr NO	WB 0.31	Horz(TL) -0.02 31 n/a n/a		
BCDL 5.0	Code IRC2009/TPI2007	Matrix-S			
				Weight: 130 lb	FT = 20%F, 11%E

**LUMBER-**  
 TOP CHORD 2x4 SP No.1(flat)  
 BOT CHORD 2x4 SP No.1(flat)  
 WEBS 2x4 SP No.3(flat)

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** All bearings 4-4-8 except (jt=length) 26=0-3-8, 31=0-3-0.  
 (lb) - Max Uplift All uplift 100 lb or less at joint(s) except 18=-136(LC 2), 20=-294(LC 2)  
 Max Grav All reactions 250 lb or less at joint(s) except 21=924(LC 6), 26=1321(LC 2), 19=299(LC 10), 31=595(LC 9)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD** 2-3=-970/0, 3-4=-1324/0, 4-5=-1324/0, 5-6=-1324/0, 6-7=-623/0, 7-8=-623/0, 8-9=0/700, 9-34=0/700, 10-34=0/700, 10-35=-417/53, 11-35=-417/53, 11-12=-417/53, 13-14=0/576, 14-15=0/576  
**BOT CHORD** 30-31=0/629, 29-30=0/1269, 28-29=0/1324, 27-28=0/1061, 25-26=-328/160, 24-25=-328/160, 23-24=-53/417, 22-23=-53/417  
**WEBS** 15-21=-539/0, 16-19=-282/0, 15-20=0/301, 13-21=-718/0, 10-26=-690/0, 13-22=0/313, 10-24=0/462, 12-22=-250/0, 2-31=-834/0, 8-26=-1004/0, 2-30=0/475, 8-27=0/650, 3-30=-416/0, 6-27=-632/0, 6-28=0/467

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
  - 2) All plates are 3x4 MT20 unless otherwise indicated.
  - 3) Plates checked for a plus or minus 1 degree rotation about its center.
  - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 18 and 294 lb uplift at joint 20.
  - 5) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
  - 7) CAUTION, Do not erect truss backwards.
  - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 173 lb down at -36-5-12, 173 lb down at -38-5-12, 173 lb down at -40-5-12, 132 lb down at 12-5-12, and 153 lb down at 14-5-12, and 153 lb down at 16-5-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
  - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard  
 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00  
 Uniform Loads (plf)  
 Vert: 18-31=-10, 1-17=-100



APRIL 21, 2008

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

ED-REP01A1

Trenco, Edenton, North Carolina

ENGINEERED BY  
**TRENCO**  
A Mitek Affiliate

AITNESS

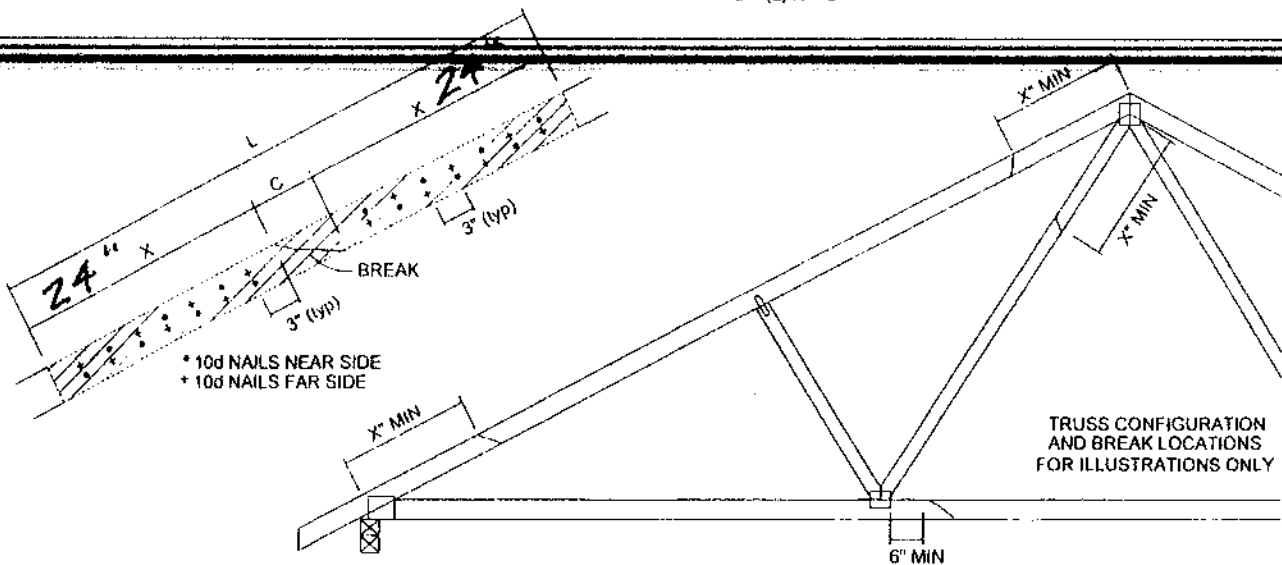
TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SYP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
14	21	24"	1626	2439	1497	2246	1272	1908	1288	1932
18	27	30"	2091	3136	1925	2888	1635	2453	1556	2484
22	33	36"	2555	3832	2353	3529	1999	2998	2024	3036
26	39	42"	3020	4530	2781	4171	2362	3543	2392	3588
30	45	48"	3485	5227	3209	4812	2726	4088	2760	4140

\* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE (OR BETTER) TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d NAILS 1 1/2" ON CENTER STAGGERED AS SHOWN. (NAIL ROWS FOR 2x4 TRUSS, THREE ROWS FOR 2x6)

IF TOTAL CSI AT BREAK/SPLICE LOCATION IS NO GREATER THAN 0.50, A SCAB OF THE SAME SIZE OF HF/SPF No. 2 MAY BE USED

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)  
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS  
 $L = (2) X + C$

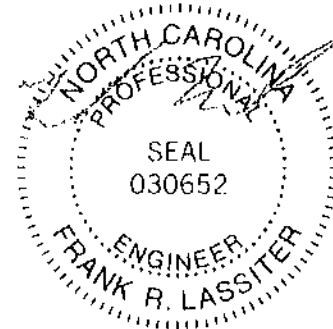


THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR SHALL BE USED FOR SINGLE PLY TRUSSES IN THE 2x4 OR 2x6 ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



April 8, 2009

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES PRIOR TO USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/APA Quality Criteria, DSR-85 and ICCI Building Component Safety Information available from Truss Plate Institute, 2811 N. Lee Street, Suite 312, Alexandria, VA 22314.

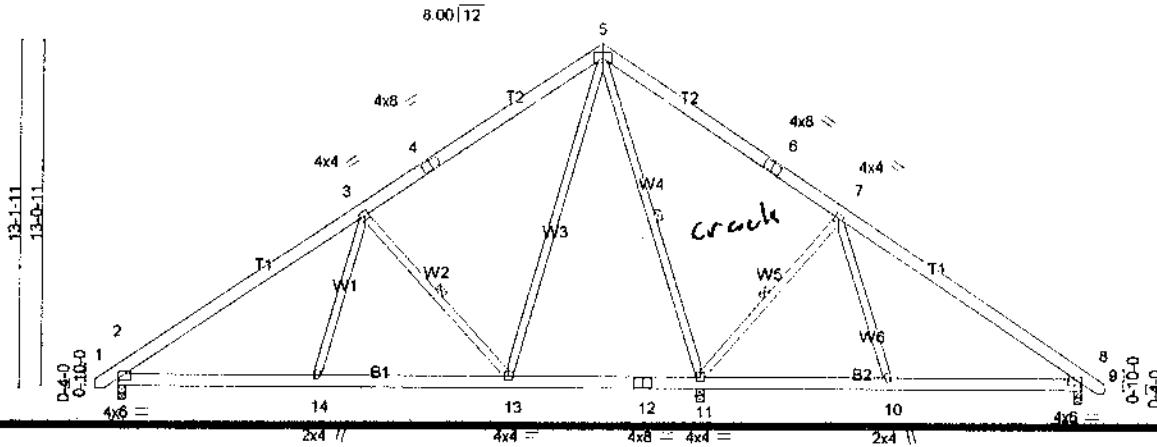
ENGINEERED BY  
**TRENCO**  
A Mitek Affiliate

816 Soundside Road  
Edenton, NC 27932

0-10-8	9-3-9	18-4-0	27-4-7	36-8-0	37-6-8
0-10-8	9-3-9	9-0-7	8-0-7	9-3-9	0-10-8

5x8 =

Scale = 1/87.1



7-6-14	14-10-9	22-2-4	29-3-9	36-8-0
7-6-14	7-3-11	7-3-11	7-1-5	7-4-7

LOADING (psf)	SPACING	CFI	DEFL	is (ps)	WALL	L/8	PLATES	CFIP
TCDL 10.0	Plate Grip DOL 1.15	TC 0.57	Vert(LL) -0.09	2-14 >999	240		WT20	24/190
BCLL 0.0	Rep Stress Incr YES	BC 0.25	Vert(TL) 0.02	2-14 >999	240			
BCDL 10.0	Code IRC2009/TPI2007	WB 0.88	Horz(TL) 0.02	8 n/a	n/a			
		Matrix-S	Wind(LL) 0.03	2-14 >999	240			

**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpl 3-13, 5-11, 7-11

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (size) 2=0-3-8 (min. 0-1-8), 11=0-3-8 (min. 0-2-2), 8=0-3-8 (min. 0-1-8)  
 Max Horz 2=349(LC 6)  
 Max Uplift 2=-81(LC 7), 11=-59(LC 8), 8=-66(LC 8)  
 Max Grav 2=948(LC 1), 11=1785(LC 1), 8=577(LC 12)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1203/174, 3-4=-685/205, 4-5=-515/259, 7-8=-570/78  
 BOT CHORD 2-14=-189/868, 3-14=-211/821, 12-13=-76/282, 11-12=-76/282, 10-11=0/307, 8-10=0/355  
 WEBS 3-14=0/345, 3-13=-612/298, 5-13=-137/783, 5-11=-1030/116, 7-11=-619/303, 7-10=0/327

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cal. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 2, 59 lb uplift at joint 11 and 66 lb uplift at joint 8.
  - 6) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard