PREPARED 7/12/16, 14:31:04 Harnett County

INSPECTION TICKET INSPECTOR: IVR

PAGE DATE

7/13/16

ADDRESS . : 46 BUNTING DR

SUBDIV: OAKMONT PH1 SC3 52LOTS

CONTRACTOR : GML DEVELOPMENT INC

PHONE: (919) 793-5237 PHONE :

OWNER . . : OAKMONT DEV PTNRS LLC

PARCEL . . : 03-0507-01- -0046- -24-

APPL NUMBER: 16-50038419 CP NEW RESIDENTIAL (SFD)

_____ DIRECTIONS : T/S: 04/07/2016 11:40 AM JBROCK ----

OAKMONT #104

STRUCTURE: 000 000 74X70 4BDR MONO W/ GARAGE

DA

TI

FLOOD ZONE . . . : FLOOD ZONE X

7/12/16

7/13/16

B114 02

BEDROOMS 4000000.00 PROPOSED USE : SFD WATER SUPPLY : COUNTY SEPTIC - EXISTING? . . . : NEW TANK

PERMIT: CPSF 00 CP * SFD REQUESTED INSP DESCRIPTION COMPLETED RESULT RESULTS/COMMENTS ADDRESS CONFIRMATION TIME: 17:00 VRU #: 002840668 A814 01 7/07/16 TW 7/06/16 AP 46 BUNTING DR LILLINGTON 27546 T/S: 07/06/2016 04:40 PM TWARD -----P309 01 7/07/16 MR R*PLUMB UNDER SLAB TIME: 17:00 VRU #: 002840684 T/S: 07/12/2016 02:09 PM MREARIC -----7/12/16 AΡ B114 01 7/12/16 R*BLDG MONO SLAB/TEMP SVC POLE TIME: 17:00 VRU #: 002841856 MR

> * OVERRIDE TAKEN BY JBROCK DATE: 07/08/16 TIME: 14:06:43 T/S: 07/08/2016 02:06 PM JBROCK ----someone please sign off on the plumbing under slab T/S: 07/12/2016 02:10 PM MREARIC -----

rec.d compaction , but slab not ready

R*BLDG MONO SLAB/TEMP SVC POLE TIME: 17:00 VRU #: 002843134

----- COMMENTS AND NOTES ------

Rend

comp. Report



McKee Homes 101 Hay Street, 2nd Floor Fayetteville, NC 28301

06/22/2016

Attention : C Morgan

RE: Daily Field Report for 06/21/2016

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

Building & Earth Project No : RD160330

Ladies and Gentlemen:

On this date, representative(s) of Building & Earth were present to perform construction material testing services at this project site. Our testing and observations for this date include the following:

FO-1: Field Observations made on this date.

· dcps on lot 104

Project Management review

Passed

Passed

ST-1: In place field density testing was performed for Finished Subgrade Soils -Building. The field density testing was performed in general accordance with ASTMD1556, using values from the laboratory proctors. A total of 2 in-place field density tests were performed on this date. The testing results indicate that in-place compaction and moisture content at the location and depth tested meet or exceed the specified requirements outlined in the project plans and specifications. For additional details of our testing, please refer to the attached Field Density Test Report.

Closing

The testing and observations identified above have been reviewed by our project manager. If you have questions regarding this information, please do not hesitate to contact us.

Respectfully Submitted, Building & Earth Sciences, LLP

Enclosures: FO-1, ST-1

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Rockard Heath

Page 1 of 6



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Lillington, NC

Project Number:

RD160330

FO-1

Client Name:

McKee Homes
McKee Homes

Placement#: Technician:

Damian Thomas

Contractor: Monitoring:

DCP

1: dcps on lot 104

Passed

We arrived onsite to evaluate the building pad area for this residential lot. We understand the residence has been designed to be supported on a monolithic slab foundation. Upon arrival, the contractor had not finished excavating the footings. Our evaluation as documented in this report includes:

- 1) A visual description of the residential lot
- 2) Comments on any improvements that hat affect the foundations of the residence
- 3) Hand rod probing of the footing excavations
- 4) Performing Dynamic Cone Penetration (DCP) tests at representative locations
- 5) Soil Density tests on fill, if applicable.

Visual Description of the Lot:

The lot generally slopes downward from back to front. Building locations are referenced from the street looking at the front of the residence. Maximum relief across the lot is approximately 15 feet. Surface water runoff appears to drain toward the road.

Comments on Improvements:

The site (is not stripped, has been stripped of surface cover and topsoil. It appears that 12-24 inches of topsoil has been removed from the building pad area.

Structural fill has been placed at the site to level the building pad. Based on our observation, we understand the pad has been cut or filled according to the following:

Section-----Thickness of Cut or Fill Left Front-----36 inches of fill Left Rear-------12 inches of cut Center-------12 inches of cut Right Front------36 inches of fill Right Rear------24 inches of cut

Measurements:

- 1) What is the proposed depth of footing? 2ft
- 2) What is the distance from the outside edge of footing to top edge of the nearest slope? 5ft
- 3) What is the distance from the outside edge of footing to the outside edge (toe) of the nearest slope? 10ft
- 4) What is the vertical height of the slope? 3ft

Future Footing Tests

Hand Rod Probing: Our representative performed hand rod probing of the surface of the building pad. Hand rod probing of the bearing material generally showed an average penetration of approximately 1 inch. Areas of soft material were noted near the front left corner with the hand rod probing to a depth of about 6 inches. Our representative did not observe standing water or evidence of standing water on the footing's bearing surface.

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Page 2 of 6



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Lillington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

DCP

Technician:

Damian Thomas

Monitoring:

DCP Testing: Our representative performed Dynamic Cone Penetration (DCP) testing in general accordance with ASTM STP-399 at four representative locations to a depth of 36 inches. Our representative did not observe water within the DCP boreholes as noted below.

The following information provides the results of our hand auger borings and DCP testing:

Test 1: [Front right corner]

```
-- Depth----"N"-----Soil Color---USCS------Notes:
--- FSG -- 7 --- orange --- SM ------- Note 1: Water (not) encountered at 0.0 feet
--- -1' ---- 9--- orange brown ----- SM ------- Note 2: Water (not) encountered (at 0.0 feet)
--- -2' ----12 --- Orange brown ------ CL ----- Note 3: Water (not) encountered (at 0.0 feet)
--- -3' ---- 14 --- Orange red ------- CL -----
```

Test 2: [Back right corner]

```
-- Depth----"N"-----Soil Color---USCS------Notes:
-- FSG -- 9 --- Orange --- SM ------- Note 1: Water (not) encountered at 0.0 feet
--- -1' ---- 10 --- Orange brown ----- SM ------- Note 2: Water (not) encountered (at 0.0 feet)
--- -2' ---- 10 --- Orange brown ------ CL ---- Note 3: Water (not) encountered (at 0.0 feet)
--- -3' ---- 13 --- Orange red ------ CL ----
```

Test 3: [Back left corner]

```
-- Depth----"N"-----Soil Color---USCS------Notes:
--- FSG -- 6--- Orange--- SM ------- Note 1: Water (not) encountered at 0.0 feet
--- -1' ---- 10 --- Orange brown ----- SM ----- Note 2: Water (not) encountered (at 0.0 feet)
--- -2' ---- 7 --- Orange brown------ SM ----- Note 3: Water (not) encountered (at 0.0 feet)
--- -3' ---- 8--- Orange brown------ SM -----
```

Test 4[Front left corner]

```
-- Depth----"N"-----Soil Color---USCS------Notes:
-- FSG -- 6 --- Orange --- SM ------- Note 1: Water (not) encountered at 0.0 feet
-----1' ---- 11 --- Orange brown ------ SM ------ Note 2: Water (not) encountered (at 0.0 feet)
-----2' ---- 12 --- Orange brown ------- SM ----- Note 3: Water (not) encountered (at 0.0 feet)
-----3' ---- 12--- Orange brown-------SM -----
```

Soil Density Testing:

610 Spring Branch Road Dunn, NC 28334 Phone 910-292-2085 Fax 910-292-2192 www.BuildingandEarth.com Rachard Heath

Page 3 of 6



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Lillington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

Technician:

Damian Thomas

Monitoring:

DCP

Soil density testing was performed using the sand cone method of compaction in general accordance with ASTM D1556. The results of our tests are attached as ST-1.

Results:

Based on our observations and test results, the newly placed fill/existing soils appear to be suitable to provide support for the floor slab and footings, provided the floor slab has a loading of less than 150 pounds per square foot, and the footings have a design bearing capacity of 2,000, or less.

Recommendations:

To minimize the potential for future softening of the bearing materials due to water infiltration, the surface soils should be protected from construction traffic and inclement weather. The construction of the footings and structure should commence without delay. In the event that the subgrade soils become wet, or otherwise compromised from their current condition, should be observed and retested as necessary by Building and Earth Sciences.

We note that our testing was isolated to the upper 3 feet of the soil profile from the finished subgrade elevation as observed on this date. As such, we cannot be aware of any soil or groundwater conditions below this depth that could adversely affect the support of the new construction. If additional information is required, please contact our office.

2: Project Management review

Passed

Our client has authorized Building & Dearth Sciences to perform an evaluation of the prepared building pad for this project. We understand that the structure will have a monolithic slab-on-grade floor system that will have foundations and a floor slab that will be supported by the newly placed structural fill soils. It appears that between 1 and 3 feet of structural fill soils have been placed to achieve the desired grades. The intent of our testing was to determine if the newly placed structural fill soils are adequate to provide a bearing capacity of 2,000 psf for the foundations, and have been compacted to 95% to support the floor slab for the new structure.

Our evaluation included hand rod probing, advancing hand auger borings with DCPs and performing a density test on the surface. Based upon our hand rod probing the newly placed soils are firm and resistant to significant penetration. Hand auger borings were then advanced at 4 selected location across the building envelope to determine the consistency of the below grade soils. At 12-inch increments in the hand auger boring, to a depth of 3 feet, Dynamic Cone Penetrometer (DCP) Testing was performed in accordance with ASTM STP-399. With proper evaluation, DCP Testing can be correlated to both bearing capacity and percent compaction. Based upon the results of this testing, the below grade soils that will support the foundations and floor slab are acceptable.

While on site, our representative also performed in place density testing to confirm compaction of the surface soils. Our testing was performed using the sand cone method in general accordance with ASTM D-1556. Our results were compared to an in-field proctor that was performed in general accordance with ASTM D-698. Based upon our tests results, the soils have been properly compacted at the surface.

It is important to note that our testing was isolated to the upper 3 feet. As such, we are not able to comment upon the settlement characteristics of deeper soils. Additionally, inclement weather (rain or snow), as well as construction traffic across the pad, can compromise the stability and support characteristics of the surface soils. If the surface soils become compromised, it will be necessary to return to the site for re-testing. This decision should be executed by your onsite Quality Control and Superintendents.

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Kachard Heath

Page 4 of 6



Project Name: Client Name:

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

McKee Homes

Contractor: Monitoring:

McKee Homes DCP

Project Number:

RD160330

Placement#:

FO-1

Technician:

Damian Thomas

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Rackard Heath

Page 5 of 6



ST-1

Test Date: 06/21/2016 Field Technician: Damian Thomas

Tests requested by: N/R Results provided to: N/R

Report of Field Density Testing

Project Name:

Notes:

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

Project Number: RD160330

Project Location: Lillington, NC

Client: McKee Homes

Contractor: McKee Homes

Test location by technician

2 Elevation by Technician

Fill/backfill placed prior to technician arriving

Ambient Temperature: 70-90

Weather: Overcast Wind Conditions: Breezy

Results Provided To: N/R

Superintendant: N/R

Design & Specification Data

Area ID	Area Description	Depth (ft)	Test Method	% Compaction				
ECC DI I					Min	Max		
FSG-Bldg	Finished Subgrade Soils -Building	0.0 - 2.0	ASTM D-698	95 %	- 10.0	+ 100		

Laboratory Proctors

Proctor ID	Description of Material	USCS/AASHTO	Maximum Dry Density (pcf)	Optimum Moisture Content (%)				
1-point			101.0	17.5%				
Density Test Data								

Test #	IDs		Test			Do	04	Γ	
	Area	Proctor	Туре	Location	Elev. (ft)	Dry Density(pcf)	% Moisture	% Compaction	Result
1	FSG-Bldg	1-point	ASTMD1556	Finished Subgrade Soils -Building : Right Back Corner :	FSG	101.2	11.7	100%	PASS
2	FSG-Bldg	1-point	ASTMD1556	Finished Subgrade Soils -Building : Right Front corner :	FSG	103.2	12.4	100+	PASS

Equipment Used: Last Calibration:

Standard Counts:

Density:

Moisture:

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Rackal Heath

Page 6 of 6

PREPARED 7/11/16, 14:24:18 Harnett County

INSPECTION TICKET INSPECTOR: IVR

PAGE

19 DATE 7/12/16

ADDRESS . : 46 BUNTING DR

SUBDIV: OAKMONT PH1 SC3 52LOTS

CONTRACTOR : GML DEVELOPMENT INC

PHONE: (919) 793-5237

OWNER . . : OAKMONT DEV PTNRS LLC

PHONE :

PARCEL . . : 03-0507-01- -0046- -24-

APPL NUMBER: 16-50038419 CP NEW RESIDENTIAL (SFD)

DIRECTIONS: T/S: 04/07/2016 11:40 AM JBROCK ----

OAKMONT #104 -------

SCANNED

STRUCTURE: 000 000 74X70 4BDR MONO W/ GARAGE

FLOOD ZONE . . . : FLOOD ZONE X

BEDROOMS : 4000000.00 PROPOSED USE : SFD SEPTIC - EXISTING? . . . : NEW TANK WATER SUPPLY : COUNTY

PERMIT: CPSF 00 CP * SFD

B114 01 7/12/16

REQUESTED INSP DESCRIPTION TYP/SQ COMPLETED RESULT RESULTS/COMMENTS

TI

TI P309 01 7/07/16 R*PLUMB UNDER SLAB TIME: 17:00 VRU #: 002840684

M AP-MP

R*BLDG MONO SLAB/TEMP SVC POLE TIME: 17:00 VRU #: 002841856 * OVERRIDE TAKEN BY JBROCK DATE: 07/08/16 TIME: 14:06:43

T/S: 07/08/2016 02:06 PM JBROCK -----

specone please sign off on the plumbing under slab

N.M.
Recod tion
LOMPac



McKee Homes 101 Hay Street, 2nd Floor Fayetteville, NC 28301

06/22/2016

Attention: C Morgan

RE: Daily Field Report for 06/21/2016

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

Building & Earth Project No: RD160330

Ladies and Gentlemen:

On this date, representative(s) of Building & Earth were present to perform construction material testing services at this project site. Our testing and observations for this date include the following:

FO-1: Field Observations made on this date.

· dcps on lot 104

Project Management review

Passed

Passed

ST-1: In place field density testing was performed for Finished Subgrade Soils -Building. The field density testing was performed in general accordance with ASTMD1556, using values from the laboratory proctors. A total of 2 in-place field density tests were performed on this date. The testing results indicate that in-place compaction and moisture content at the location and depth tested meet or exceed the specified requirements outlined in the project plans and specifications. For additional details of our testing, please refer to the attached Field Density Test Report.

Closing

The testing and observations identified above have been reviewed by our project manager. If you have questions regarding this information, please do not hesitate to contact us.

Respectfully Submitted, Building & Earth Sciences, LLP

Enclosures: FO-1, ST-1

SEA POINTER A MILLER BRANCH

Rockal Hoath



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Lillington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

Technician:

Damian Thomas

Monitoring:

DCP

1: dcps on lot 104

Passed

We arrived onsite to evaluate the building pad area for this residential lot. We understand the residence has been designed to be supported on a monolithic slab foundation. Upon arrival, the contractor had not finished excavating the footings. Our evaluation as documented in this report includes:

- 1) A visual description of the residential lot
- 2) Comments on any improvements that hat affect the foundations of the residence
- 3) Hand rod probing of the footing excavations
- 4) Performing Dynamic Cone Penetration (DCP) tests at representative locations
- 5) Soil Density tests on fill, if applicable.

Visual Description of the Lot:

The lot generally slopes downward from back to front. Building locations are referenced from the street looking at the front of the residence. Maximum relief across the lot is approximately 15 feet. Surface water runoff appears to drain toward the road.

Comments on Improvements:

The site (is not stripped, has been stripped of surface cover and topsoil. It appears that 12-24 inches of topsoil has been removed from the building pad area.

Structural fill has been placed at the site to level the building pad. Based on our observation, we understand the pad has been cut or filled according to the following:

Section-----Thickness of Cut or Fill Left Front------36 inches of fill Left Rear---------12 inches of cut Center-------36 inches of fill Right Front-------36 inches of cut Right Rear---------24 inches of cut

Measurements:

- 1) What is the proposed depth of footing? 2ft
- 2) What is the distance from the outside edge of footing to top edge of the nearest slope? 5ft
- 3) What is the distance from the outside edge of footing to the outside edge (toe) of the nearest slope? 10ft
- 4) What is the vertical height of the slope? 3ft

Future Footing Tests

Hand Rod Probing: Our representative performed hand rod probing of the surface of the building pad. Hand rod probing of the bearing material generally showed an average penetration of approximately 1 inch. Areas of soft material were noted near the front left corner with the hand rod probing to a depth of about 6 inches. Our representative did not observe standing water or evidence of standing water on the footing's bearing surface.

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Richard Heath



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Lillington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

Technician:

Damian Thomas

Monitoring:

DCP

DCP Testing: Our representative performed Dynamic Cone Penetration (DCP) testing in general accordance with ASTM STP-399 at four representative locations to a depth of 36 inches. Our representative did not observe water within the DCP boreholes as noted below.

The following information provides the results of our hand auger borings and DCP testing:

```
Test 1: [Front right corner]
```

```
-- Depth----"N"----Soil Color---USCS------Notes:
-- FSG -- 7 --- orange -- SM ------- Note 1: Water (not) encountered at 0.0 feet
-----1' --- 9-- orange brown ------ SM ------- Note 2: Water (not) encountered (at 0.0 feet)
----2' ----12 --- Orange brown ------ CL ----- Note 3: Water (not) encountered (at 0.0 feet)
----3' ---- 14 --- Orange red ------- CL -----
```

Test 2: [Back right corner]

Test 3: [Back left corner]

```
Depth----"N"-----Soil Color---USCS------Notes:
FSG - 6--- Orange--- SM ------- Note 1: Water (not) encountered at 0.0 feet
-----1' --- 10 --- Orange brown ----- SM ------ Note 2: Water (not) encountered (at 0.0 feet)
----2' ---- 7 --- Orange brown------ SM ----- Note 3: Water (not) encountered (at 0.0 feet)
----3' ---- 8--- Orange brown------ SM -----
```

Test 4[Front left corner]

```
-- Depth----"N"-----Soil Color--USCS------Notes:
--- FSG -- 6 -- Orange --- SM ------- Note 1: Water (not) encountered at 0.0 feet
--- -1' --- 11 --- Orange brown ------ SM ------ Note 2: Water (not) encountered (at 0.0 feet)
--- -2' ---- 12 --- Orange brown ------- SM ----- Note 3: Water (not) encountered (at 0.0 feet)
--- -3' ---- 12--- Orange brown--------SM -----
```

Soil Density Testing:

610 Spring Branch Road Dunn, NC 28334 Phone 910-292-2085 Fax 910-292-2192 www.BuildingandEarth.com

Rackard Hoath



Project Name:

Lot 104 Oakmont Subdivision (CMT)

Liflington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

Technician:

Damian Thomas

Monitoring:

DCP

Soil density testing was performed using the sand cone method of compaction in general accordance with ASTM D1556. The results of our tests are attached as ST-1.

Results:

Based on our observations and test results, the newly placed fill/existing soils appear to be suitable to provide support for the floor slab and footings, provided the floor slab has a loading of less than 150 pounds per square foot, and the footings have a design bearing capacity of 2,000, or tess.

Recommendations:

To minimize the potential for future softening of the bearing materials due to water infiltration, the surface soils should be protected from construction traffic and inclement weather. The construction of the footings and structure should commence without delay. In the event that the subgrade soils become wet, or otherwise compromised from their current condition, should be observed and retested as necessary by Building and Earth Sciences.

We note that our testing was isolated to the upper 3 feet of the soil profile from the finished subgrade elevation as observed on this date. As such, we cannot be aware of any soil or groundwater conditions below this depth that could adversely affect the support of the new construction. If additional information is required, please contact our office.

2: Project Management review

Passed

Our client has authorized Building & Derhorm an evaluation of the prepared building pad for this project. We understand that the structure will have a monolithic slab-on-grade floor system that will have foundations and a floor slab that will be supported by the newly placed structural fill soils. It appears that between 1 and 3 feet of structural fill soils have been placed to achieve the desired grades. The intent of our testing was to determine if the newly placed structural fill soils are adequate to provide a bearing capacity of 2,000 psf for the foundations, and have been compacted to 95% to support the floor slab for the new structure.

Our evaluation included hand rod probing, advancing hand auger borings with DCPs and performing a density test on the surface. Based upon our hand rod probing the newly placed soils are firm and resistant to significant penetration. Hand auger borings were then advanced at 4 selected location across the building envelope to determine the consistency of the below grade soils. At 12-inch increments in the hand auger boring, to a depth of 3 feet, Dynamic Cone Penetrometer (DCP) Testing was performed in accordance with ASTM STP-399. With proper evaluation, DCP Testing can be correlated to both bearing capacity and percent compaction. Based upon the results of this testing, the below grade soils that will support the foundations and floor stab are acceptable.

While on site, our representative also performed in place density testing to confirm compaction of the surface soils. Our testing was performed using the sand cone method in general accordance with ASTM D-1556. Our results were compared to an in-field proctor that was performed in general accordance with ASTM D-698. Based upon our tests results, the soils have been properly compacted at the surface.

It is important to note that our testing was isolated to the upper 3 feet. As such, we are not able to comment upon the settlement characteristics of deeper soils. Additionally, inclement weather (rain or snow), as well as construction traffic across the pad, can compromise the stability and support characteristics of the surface soils. If the surface soils become compromised, it will be necessary to return to the site for re-testing. This decision should be executed by your onsite Quality Control and Superintendents.

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Rackaul Heath



Project Name:

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

Project Number:

RD160330

Client Name:

McKee Homes

Placement#:

FO-1

Contractor:

McKee Homes

Technician:

Damian Thomas

Monitoring:

DCP



ST-1

Test Date: 06/21/2016

Field Technician: Damian Thomas

Tests requested by: N/R Results provided to: N/R

Report of Field Density Testing

Project Name:

Lot 104 Oakmont Subdivision (CMT) Lillington, NC

Ambient Temperature:

70-90

Project Number: RD160330

Weather: Overcast

Project Location: Lillington, NC

Wind Conditions: Breezy

Results Provided To: N/R

Contractor: McKee Homes

Client: McKee Homes

Superintendant: N/R

Notes:

1 Test location by technician

Elevation by Technician 2

Fill/backfill placed prior to technician arriving

Design & Specification Data

Area ID	Area Description	Depth (ft)	Test Method	% Compaction	Moisture Range	
					Min	Max
FSG-Bldg	Finished Subgrade Soils -Building	0.0 - 2.0	ASTM D-698	95 %	- 10.0	+ 10.0

Laboratory Proctors

Proctor ID	Description of Material	USCS/AASHTO	Maximum Dry Density (pcf)	Optimum Moisture Content (%)
1-point			101.0	17.5%

Density Test Data

L									
Test #	ID Area	Proctor	Test Type	Location	Elev. (ft)	Dry Density(pcf)	% Moisture	% Compaction	Result
1	FSG-Bldg	1-point	ASTMD1556	Finished Subgrade Soils -Building : Right Back Corner :	FSG	101.2	11.7	100%	PASS
2	FSG-Bldg	1-point	ASTMD1556	Finished Subgrade Soils -Building . Right Front corner :	FSG	103.2	12.4	100+	PASS

Equipment Used: Last Calibration: Standard Counts.

Density: Moisture:

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Rachael Heath