

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	
MASTER	A01G	GABLE	99	1	savvy-carmichael	18147040

Builders FirstSource, Apex, NC, 27523
 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Apr 30 14:12:14 2014 Page 1
 ID:shlkXampymc7Y1x7z4ZB9tzrDYz-pn27fO7PEeBoV24teInPICWY74H073Hjgm7azLVn
 41-6-0
 42-6-0
 20-9-0
 1-0-0
 Scale = 1/2" = 1'-0"

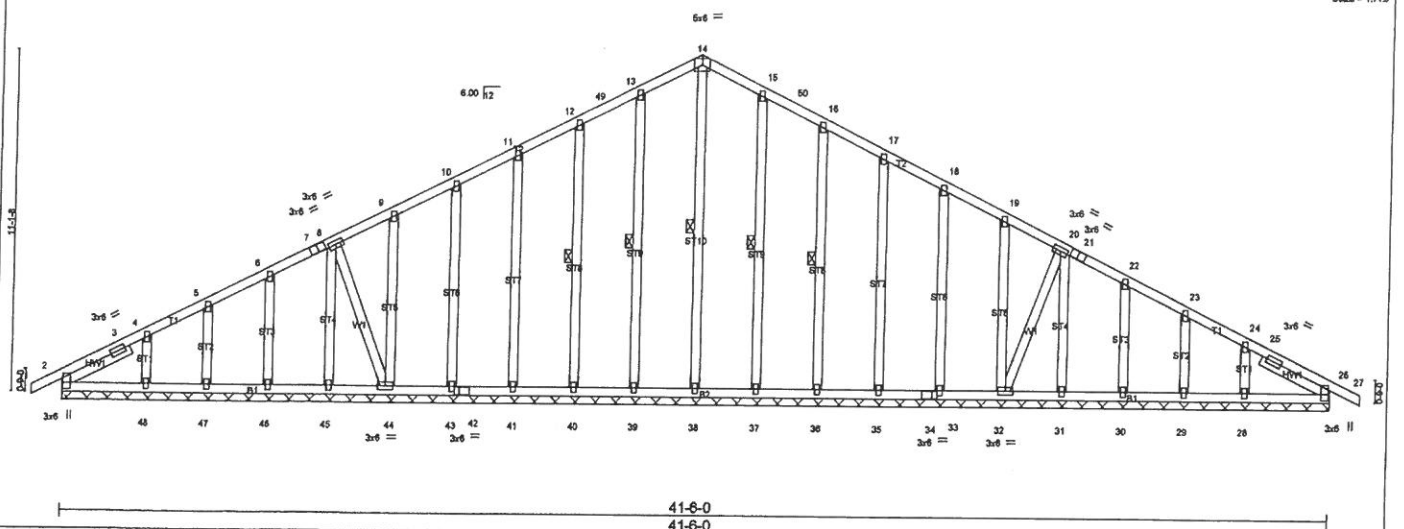


Plate Offsets (X,Y): [2-0-4-1,0-0-5], [26-0-4-1,0-0-5], [34-0-2-0,0-1-8], [42-0-2-0,0-1-8]								
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.10	Vert(LL) -0.00	26	n/r	120	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.06	Vert(TL) -0.00	26	n/r	120		
BCLL 0.0	Rep Stress Incr NO	WB 0.14	Horz(TL) 0.01	26	n/a	n/a		
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)						
							Weight: 321 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3
 SLIDER Left 2x4 SP No.2 2-6-0, Right 2x4 SP No.2 2-6-0

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 14-38, 13-38, 12-40, 15-37, 16-36

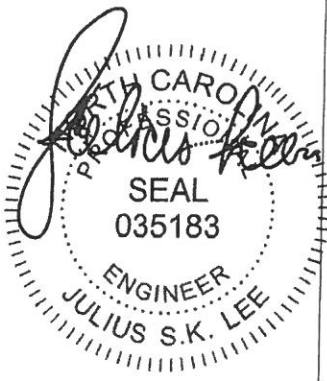
MITek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

REACTIONS All bearings 41-6-0.
 (lb) - Max Horz 2=114(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 41, 43, 44, 46, 47, 48, 37, 36, 35, 33, 32, 30, 28, 26
 Max Grav All reactions 250 lb or less at joint(s) 2, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 37, 36, 35, 33, 32, 31, 30, 29, 28, 26

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 12-49=-35/275, 13-49=0/279, 13-14=-37/320, 14-15=-37/321, 15-50=0/280, 16-50=-35/276

- NOTES** (12-14)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph (3-second gust); TCDL=4.2psf; BCDL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 20-9-0, Corner(3) 20-9-0 to 23-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) All plates are 2x4 MT20 unless otherwise indicated.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) Gable studs spaced at 2-0-0 oc.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 39, 40, 41, 43, 44, 46, 47, 48, 37, 36, 35, 33, 32, 30, 29, 28.
 - 10) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 11) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 - 12) If Southern Pine (SP) lumber is specified, the design values are those effective 08/01/2013 by ALSC or proposed by SPIB.
 - 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 14) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

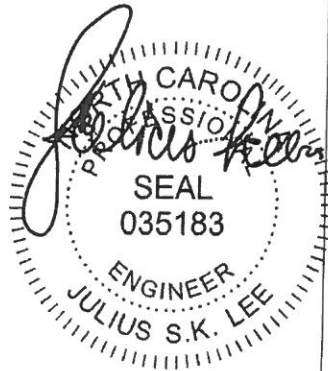
LOAD CASE(S) Standard



April 30, 2014

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>Julius Lee PE. 1109 Coastal Bay Boynton Beach, FL 33435</p>
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Job MASTER	Truss A01S	Truss Type COMMON	Qty 99	Ply 1	savvy-carmichael	18147041																																				
Builders FirstSource, Apex, NC, 27523		Job Reference (optional)																																								
<p style="font-size: small;">7.350 s Sep 27 2012 MITek Industries, Inc. Wed Apr 30 14:12:15 2014 Page 1 ID:shlkXampymc7Y1x7z4ZB9tzrDYz-HzcWk01_yJ7Ce3CSJeQPCUay7pOL3CVNPKg0zLVf</p>																																										
<p>Scale = 1/16"</p> <p>Plate Offsets (X,Y): [16.0-2.4,0-1-8]</p>																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>LOADING (psf)</th> <th>SPACING</th> <th>CSI</th> <th>DEFL</th> <th>PLATES</th> <th>GRIP</th> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1.15</td> <td>TC 0.96</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 10.0</td> <td>Lumber Increase 1.15</td> <td>BC 0.92</td> <td>Vert(LL) -0.25 16-17 >999 360</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.67</td> <td>Vert(TL) -0.55 16-17 >903 240</td> <td></td> <td></td> </tr> <tr> <td>BCDL 10.0</td> <td>Code IRC2009/TPI2007</td> <td>(Matrix)</td> <td>Horz(TL) 0.20 11 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td></td> <td></td> <td>Wind(LL) 0.12 16 >999 240</td> <td>Weight: 281 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	Plates Increase 1.15	TC 0.96	in (loc) l/defl L/d	MT20	244/190	TCDL 10.0	Lumber Increase 1.15	BC 0.92	Vert(LL) -0.25 16-17 >999 360			BCLL 0.0 *	Rep Stress Incr YES	WB 0.67	Vert(TL) -0.55 16-17 >903 240			BCDL 10.0	Code IRC2009/TPI2007	(Matrix)	Horz(TL) 0.20 11 n/a n/a						Wind(LL) 0.12 16 >999 240	Weight: 281 lb	FT = 20%
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REACTIONS (lb/size) 1=1805/0-3-8 (min. 0-2-2), 11=1871/0-3-8 (min. 0-2-3) Max Horz 1=-115(LC 5) Max Uplift 1=-63(LC 7), 11=-97(LC 8)																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-20=-3227/417, 2-20=-3124/420, 2-3=-3125/438, 3-4=-2763/411, 4-5=-2684/434, 5-21=-2185/400, 6-21=-2080/416, 6-22=-2240/438, 7-22=-2253/422, 7-8=-2939/476, 8-9=-3038/450, 9-10=-3136/408, 10-23=-3135/386, 11-23=-3232/382 BOT CHORD 1-19=-288/2732, 18-19=-288/2732, 17-18=-288/2732, 17-24=-201/2407, 16-24=-197/2413, 16-25=-177/2303, 25-26=-177/2303, 15-26=-177/2303, 14-15=-177/2303, 13-14=-309/2806, 11-13=-283/2752 WEBS 3-19=0/284, 3-17=-415/124, 5-17=0/355, 5-16=-783/188, 6-16=-236/1626, 7-16=-789/234, 7-14=-64/690, 9-14=-302/160																																										
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 100mph (3-second gust); TCDL=4.2psf; BCDL=8.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 20-9-0, Exterior(2) 20-9-0 to 24-11-15 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) All plates are 3x6 MT20 unless otherwise indicated. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11. 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 8) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss. 9) If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC or proposed by SPIB. 10) This manufactured product is designed as an Individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 11) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.																																										
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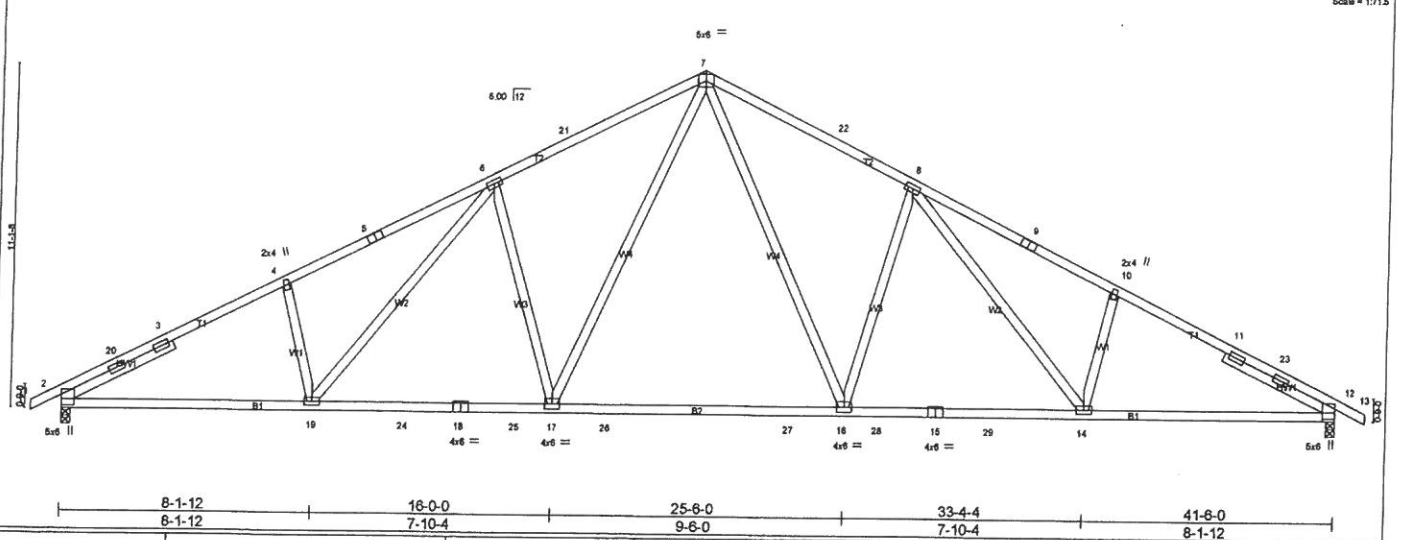


April 30, 2014

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Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)	18147042
MASTER	A02	COMMON	99	1	savvy-carmichael	

Builders FirstSource, Apex, NC, 27523
 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Apr 30 14:12:16 2014 Page 1
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 1-0-0 7-1-5 13-11-3 20-9-0 27-6-13 34-4-11 41-6-0 42-6-9
 1-0-0 7-1-5 6-9-14 6-9-13 6-9-13 6-9-14 7-1-5 1-0-0
 Scale = 1/31.5



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.91	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.98	Vert(LL) -0.43 16-17 >999 360		
BCLL 0.0 *	Lumber Increase 1.15	WB 0.75	Vert(TL) -0.86 16-17 >578 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.20 12 n/a n/a		
	Code IRC2009/TPI2007		Wind(LL) 0.12 17 >999 240		
				Weight: 242 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2 *Except*
 T1: 2x4 SP M 31
 BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.1
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.2 4-0-6, Right 2x4 SP No.2 4-0-6

BRACING
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

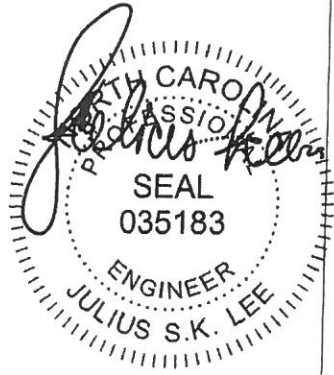
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1986/0-3-8 (min. 0-2-5), 12=1986/0-3-8 (min. 0-2-5)
 Max Horz 2=114(LC 5)
 Max Uplift 2=96(LC 7), 12=96(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-20=-3459/409, 3-20=-3354/411, 3-4=-3354/430, 4-5=-3283/481, 5-6=-3224/496, 6-21=-2806/466, 7-21=-2703/482, 7-22=-2703/482, 8-22=-2806/466, 8-9=-3224/496, 9-10=-3283/481, 10-11=-3354/430, 11-23=-3354/411, 12-23=-3459/409
 BOT CHORD 2-19=-291/2931, 19-24=-203/2589, 18-24=-203/2589, 18-25=-203/2589, 17-25=-203/2589, 17-26=-76/1966, 26-27=-76/1966, 16-27=-76/1966, 16-28=-195/2589, 15-28=-195/2589, 15-29=-195/2589, 14-29=-195/2589, 12-14=-283/2931
 WEBS 7-16=-148/1142, 8-16=-655/250, 8-14=-92/517, 7-17=-148/1142, 6-17=-655/250, 6-19=-92/517

- NOTES** (9-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph (3-second gust); TCCL=4.2psf; BCDL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 20-9-0, Exterior(2) 20-9-0 to 24-11-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 3x6 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.
 - This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - If Southern Pine (SP) lumber is specified, the design values are those effective 08/01/2013 by ALSC or proposed by SPIB.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

LOAD CASE(S) Standard



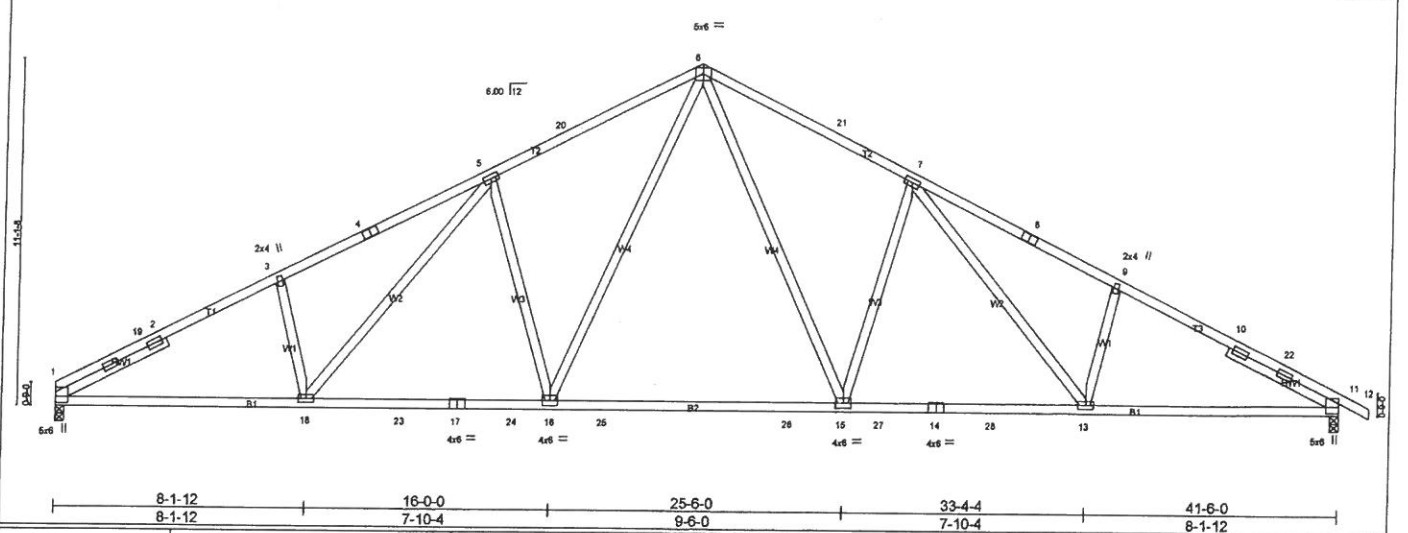
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Julius Lee PE.
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job MASTER	Truss A02S	Truss Type COMMON	Qty 99	Ply 1	savvy-carmichael	18147043
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Builders FirstSource, Apex, NC, 27523
 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Apr 30 14:12:18 2014 Page 1
 ID:shikXampymc7Y1x7z4ZB9tzDYz-hYleVI2wHhE_gNelal.12q73A7IDhXeCLe_GLzLVh
 27-6-13 34-4-11 41-6-0 42-6-9
 7-1-5 6-9-14 6-9-13 6-9-13 6-9-14 6-9-14 7-1-5 1-0-0
 Scale = 1/108



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.91	in (loc) l/defl L/d	MT20	244/190
BCDL 10.0	Lumber Increase 1.15	BC 0.96	Vert(LL) -0.43 15-16 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.76	Vert(TL) -0.86 15-16 >578 240		
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)	Horz(TL) 0.20 11 n/a n/a		
			Wind(LL) 0.12 16 >999 240	Weight: 240 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2 *Except*
 T1,T3: 2x4 SP M 31
 BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.1
WEBS
 2x4 SP No.3
SLIDER
 Left 2x4 SP No.2 4-0-6, Right 2x4 SP No.2 4-0-6

BRACING
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

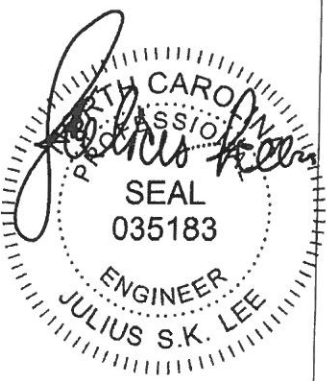
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1925/0-3-8 (min. 0-2-4), 11=1987/0-3-8 (min. 0-2-6)
 Max Horz 1=-115(LC 5)
 Max Uplift 1=-63(LC 7), 11=-97(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-19=-3467/425, 2-19=-3360/427, 2-3=-3361/445, 3-4=-3290/491, 4-5=-3231/514, 5-20=-2808/471,
 6-20=-2706/487, 6-21=-2705/482, 7-21=-2807/466, 7-8=-3225/496, 8-9=-3284/482,
 9-10=-3356/430, 10-22=-3355/411, 11-22=-3460/409
BOT CHORD 1-18=-293/2938, 18-23=-204/2592, 17-23=-204/2592, 17-24=-204/2592, 16-24=-204/2592,
 16-25=-77/1968, 25-26=-77/1968, 15-26=-77/1968, 15-27=-198/2591, 14-27=-198/2591,
 14-28=-198/2591, 13-28=-198/2591, 11-13=-286/2932
WEBS 6-15=-148/1142, 7-15=-655/250, 7-13=-92/517, 6-16=-148/1144, 5-16=-657/251, 5-18=-106/523

- NOTES** (9-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph (3-second gust); TCDF=4.2psf, BCDL=6.0psf, h=29ft, Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 20-9-0, Exterior(2) 20-9-0 to 24-11-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 3x6 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.
 - This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC or proposed by SPIB.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

LOAD CASE(S) Standard



April 30, 2014

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job MASTER	Truss A03	Truss Type COMMON	Qty 99	Ply 1	savvy-carmichael	18147044
Builders FirstSource, Apex, NC, 27523		7.350 s Sep 27 2012 MITek Industries, Inc. Wed Apr 30 14:12:19 2014 Page 1				
6-8-13 6-8-13		13-6-11 6-9-14		20-4-8 6-9-13		27-2-5 6-9-13
34-0-3 6-9-14		41-1-8 7-1-5		42-1-8 1-0-0		Scale = 1/16" = 1'-0"

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.92	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.96	Vert(LL) -0.42 15-16 >999 360		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.75	Vert(TL) -0.84 15-16 >587 240		
BCDL 10.0	Code IRC2009/TPI2007	(Matrx)	Horz(TL) 0.20 11 n/a n/a		
			Wind(LL) 0.12 15 >999 240	Weight: 243 lb	FT = 20%

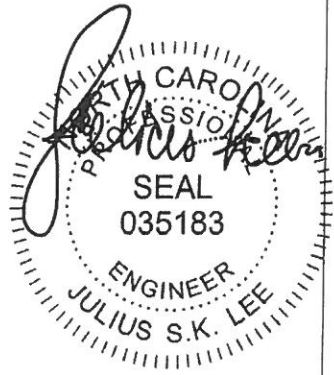
LUMBER TOP CHORD 2x4 SP No.2 *Except T1,T3: 2x4 SP M 31 BOT CHORD 2x4 SP No.2 *Except B2: 2x4 SP No.1 WEBS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 4-6-0, Right 2x4 SP No.2 4-0-6	BRACING TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. <div style="border: 1px solid black; padding: 2px; font-size: small;"> MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS (lb/size) 1=1913/Mechanical, 11=1969/0-3-8 (min. 0-2-5)
 Max Horz 1=115(LC 5)
 Max Uplift 1=61(LC 7), 11=97(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-19=3331/408, 2-19=3204/409, 2-3=3204/428, 3-4=3131/478, 4-5=3072/493, 5-20=2751/458,
 6-20=2648/481, 6-21=2668/478, 7-21=2770/462, 7-8=3191/492, 8-9=3250/478,
 9-10=3321/426, 10-22=3320/407, 11-22=3424/405
 BOT CHORD 1-18=2732/278, 18-23=1972/2528, 17-23=1972/2528, 17-24=1972/2528, 16-24=1972/2528,
 16-25=73/1934, 25-26=73/1934, 15-26=73/1934, 15-27=194/2558, 14-27=194/2558,
 14-28=194/2558, 13-28=194/2558, 11-13=283/2902
 WEBS 6-15=148/142, 7-15=656/250, 7-13=92/519, 6-16=143/1104, 5-16=818/247, 5-18=89/412

NOTES (9-11)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-05; 100mph (3-second gust); TCDL=4.2psf; BCDL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-8 to 3-4-8, Interior(1) 3-4-8 to 20-9-0, Exterior(2) 20-9-0 to 24-11-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) All plates are 3x6 MT20 unless otherwise indicated.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.
 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 9) If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC or proposed by SPIB.
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

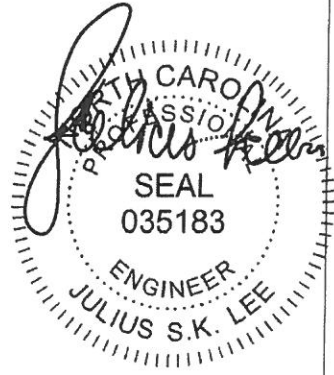
LOAD CASE(S) Standard



April 30, 2014

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	Julius Lee PE. 1109 Coastal Bay Boynton Beach, FL 33435
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Job MASTER	Truss A03G	Truss Type GABLE	Qty 99	Ply 1	savvy-cemichael	I6147045																																				
Builders FirstSource, Apex, NC, 27523		7.350 s Sep 27 2012 Mitek Industries, Inc. Wed Apr 30 14:12:20 2014 Page 1 ID: shikXampymc7Y1x7z4ZB9tzrDYz-ewPOwR4ApUxyDzX17?up6TvYKz2Uhkcxff75LDzLVf																																								
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.10</td> <td>in (loc) U/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 10.0</td> <td>Plates Increase 1.15</td> <td>BC 0.06</td> <td>Vert(LL) -0.00 24 n/r 120</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.15</td> <td>WB 0.14</td> <td>Vert(TL) -0.00 24 n/r 120</td> <td></td> <td></td> </tr> <tr> <td>BCDL 10.0</td> <td>Rep Stress Incr NO</td> <td>(Matrix)</td> <td>Horz(TL) 0.01 24 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code IRC2009/TPI2007</td> <td></td> <td></td> <td>Weight: 321 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	2-0-0	TC 0.10	in (loc) U/defl L/d	MT20	244/190	TCDL 10.0	Plates Increase 1.15	BC 0.06	Vert(LL) -0.00 24 n/r 120			BCLL 0.0 *	Lumber Increase 1.15	WB 0.14	Vert(TL) -0.00 24 n/r 120			BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 24 n/a n/a				Code IRC2009/TPI2007			Weight: 321 lb	FT = 20%
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	Code IRC2009/TPI2007			Weight: 321 lb	FT = 20%																																					
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:30%;"> LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 2-7-10, Right 2x4 SP No.2 2-6-0 </td> <td style="width:70%;"> BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 1 Row at midpt 12-36, 11-37, 10-38, 13-35, 14-34 <div style="border: 1px solid black; padding: 2px; font-size: small;"> Mitek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide. </div> </td> </tr> </table>							LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 2-7-10, Right 2x4 SP No.2 2-6-0	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 1 Row at midpt 12-36, 11-37, 10-38, 13-35, 14-34 <div style="border: 1px solid black; padding: 2px; font-size: small;"> Mitek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide. </div>																																		
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REACTIONS All bearings 41-1-8. (lb) - Max Horz 1=115(LC 5) Max Uplift All uplift 100 lb or less at joint(s) 1, 37, 38, 39, 41, 42, 44, 45, 46, 35, 34, 33, 31, 30, 28, 27, 26 Max Grav All reactions 250 lb or less at joint(s) 1, 36, 37, 38, 39, 41, 42, 43, 44, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, 26, 24																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 10-48=34/273, 11-48=0/277, 11-12=37/318, 12-13=37/318, 13-49=0/278, 14-49=34/274																																										
NOTES (12-14) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 100mph (3-second gust); TCCL=4.2psf; BCCL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Corner(3) 0-4-8 to 3-4-8, Exterior(2) 3-4-8 to 20-9-0, Corner(3) 20-9-0 to 23-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2-0-0 oc. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 37, 38, 39, 41, 42, 44, 45, 46, 35, 34, 33, 31, 30, 28, 27, 26. 10) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 12) If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC or proposed by SPIB. 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 14) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.																																										
LOAD CASE(S) Standard																																										



April 30, 2014

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Job MASTER	Truss B01G	Truss Type GABLE	Qty 99	Ply 1	savvy-carlmichael	18172592
Builders FirstSource, Apex, NC, 27523		7.350 s Sep 27 2012 Mitek Industries, Inc. Tue May 06 14:32:57 2014 Page 1 ID:shlkXampymc7Y1x7z4ZB9tzrDYz-aV1OoWkvFNDMatEVI55kuQdyRAbehlpHFZokovzJBik				

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.10	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.07	Vert(LL) -0.00 13 n/r 120		
BCLL 10.0	Rep Stress Incr NO	WB 0.18	Vert(TL) -0.01 13 n/r 120		
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)	Horz(TL) 0.00 14 n/a n/a		
				Weight: 130 lb	FT = 20%

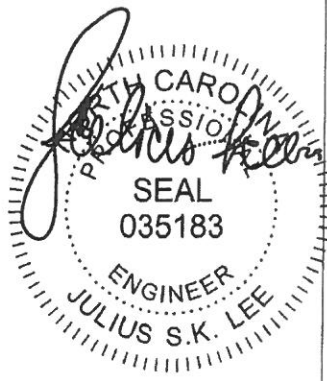
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. <div style="border: 1px solid black; padding: 2px; font-size: small;"> Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS All bearings 20-7-0.
 (b) - Max Horz 25=177(LC 5)
 Max Uplift All uplift 100 lb or less at joint(s) 25, 14, 20, 22, 23, 24, 18, 17, 16, 15
 Max Grav All reactions 250 lb or less at joint(s) 25, 14, 19, 20, 22, 23, 24, 18, 17, 16, 15

FORCES (b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (12-14)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-05; 100mph (3-second gust); TCDL=4.2psf; BCDL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-3-8, Interior(1) 2-3-8 to 10-3-8, Exterior(2) 10-3-8 to 14-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) All plates are 2x4 MT20 unless otherwise indicated.
 5) Gable requires continuous bottom chord bearing.
 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 7) Gable studs spaced at 2-0-0 oc.
 8) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 14, 20, 22, 23, 24, 18, 17, 16, 15.
 10) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 11) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 12) If Southern Pine (SP) lumber is specified, the design values are those effective 08/01/2013 by ALSC or proposed by SPIB.
 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 14) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

LOAD CASE(S) Standard



May 6, 2014

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MJI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	savvy-camilchael	18147046
MASTER	B01-3PL	COMMON	99	3	Job Reference (optional)	

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Apr 30 14:21 2014 Page 1
 Builders FirstSource, Apex, NC, 27523 ID:shlkXampymc7Y1x7z4ZB9tzrDYz-67zn7n4oac3pr76DZJP2lgSYDNI7Q3O5uJsetgzLVp

Plate Offsets (X,Y): [8-0-5-0,0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.79	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.15	BC 0.42	Vert(LL) -0.09 9-11 >999 360		
BCLL 0.0	Lumber Increase 1.15	WB 0.68	Vert(TL) -0.22 9-11 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 7 n/a n/a		
	Code IRC2009/TPI2007		Wind(LL) 0.06 9-11 >999 240	Weight: 403 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP M 31
 BOT CHORD 2x6 SP DSS
 WEBS 2x4 SP No.2
 SLIDER Left 2x4 SP No.2 3-2-15, Right 2x4 SP No.2 3-2-15

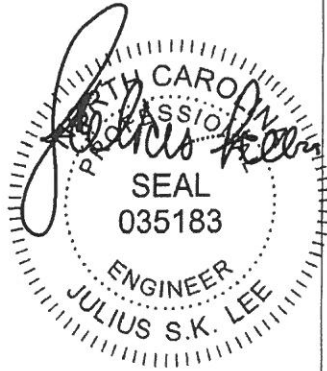
BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-9-4 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=8487/0-3-8 (min. 0-2-14), 7=9270/0-3-8 (min. 0-3-2)
 Max Horz 1=169(LC 4)
 Max Uplift 1=341(LC 5), 7=373(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-11559/456, 2-3=-11468/477, 3-4=-8089/387, 4-5=-8090/387, 5-6=-11548/481,
 6-7=-11638/459
 BOT CHORD 1-12=-389/9186, 11-12=-389/9186, 10-11=-389/9186, 9-10=-389/9186, 8-9=-331/9257,
 7-8=-331/9257
 WEBS 4-9=-342/8353, 5-9=-3263/224, 5-8=-120/4334, 3-9=-3172/220, 3-11=-115/4240

NOTES (10-12)
 1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-4-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 3) Unbalanced roof live loads have been considered for this design.
 4) Wind: ASCE 7-05; 100mph (3-second gust); TCDL=4.2psf, BCDL=6.0psf; h=29ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 1=341, 7=373.
 8) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 10) If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC or proposed by SPIB.
 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 12) This truss design conforms with NC State residential code 2012 and ANSI/TPI 1-2007 based on the parameters shown.

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.15, Plate Increase=1.15



April 30, 2014

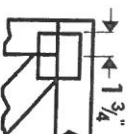
Job	Truss	Truss Type	Qty	Ply	savvy-carmichael	I8147046
MASTER	B01-3PL	COMMON	99	3	Job Reference (optional)	
Builders FirstSource, Apex, NC, 27523		7.350 s Sep 27 2012 MITEK Industries, Inc. Wed Apr 30 14:12:22 2014 Page 2				
		ID:shkXampymc7Y1x7z4ZB9izrDYz-aJX8L75QL5BfSHhP6QxHCu7jzneE9WeE6zcCQ6zLVt				
LOAD CASE(S) Standard						
Uniform Loads (plf)						
Vert: 1-4=-60, 4-7=-60, 1-12=-20, 7-12=-843(B=-823)						

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-B9 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

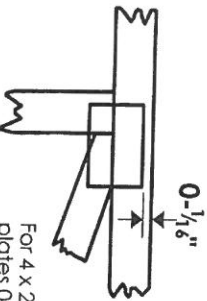
Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in fractions. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

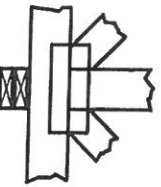
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

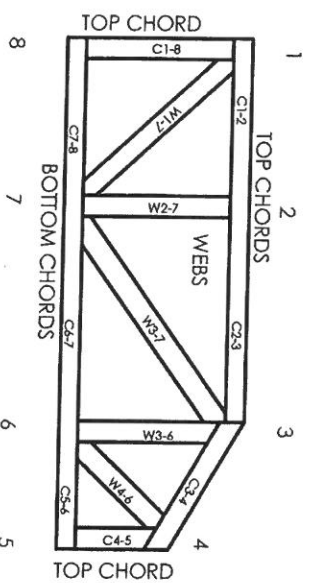
Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in fractions (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Julius Lee PE
1109 Coastal Bay,
Boynton Beach, FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/FP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP11 Quality Criteria.

General Notes:

- Per ANSI/TPI 1-2002 all "Truss to Wall" connections are the responsibility of the Building Designer, not the Truss Manufacturer.
- Dimensions are Feet-Inches- Sixteenths.
- Trusses are to be 24" o.c. unless noted otherwise (U.N.O.)
- Trusses are not designed to support brick U.N.O.
- Do not cut or modify trusses without first contacting Builders FirstSource.
- Immediately contact Builders FirstSource if trusses are damaged.

Connection Notes:

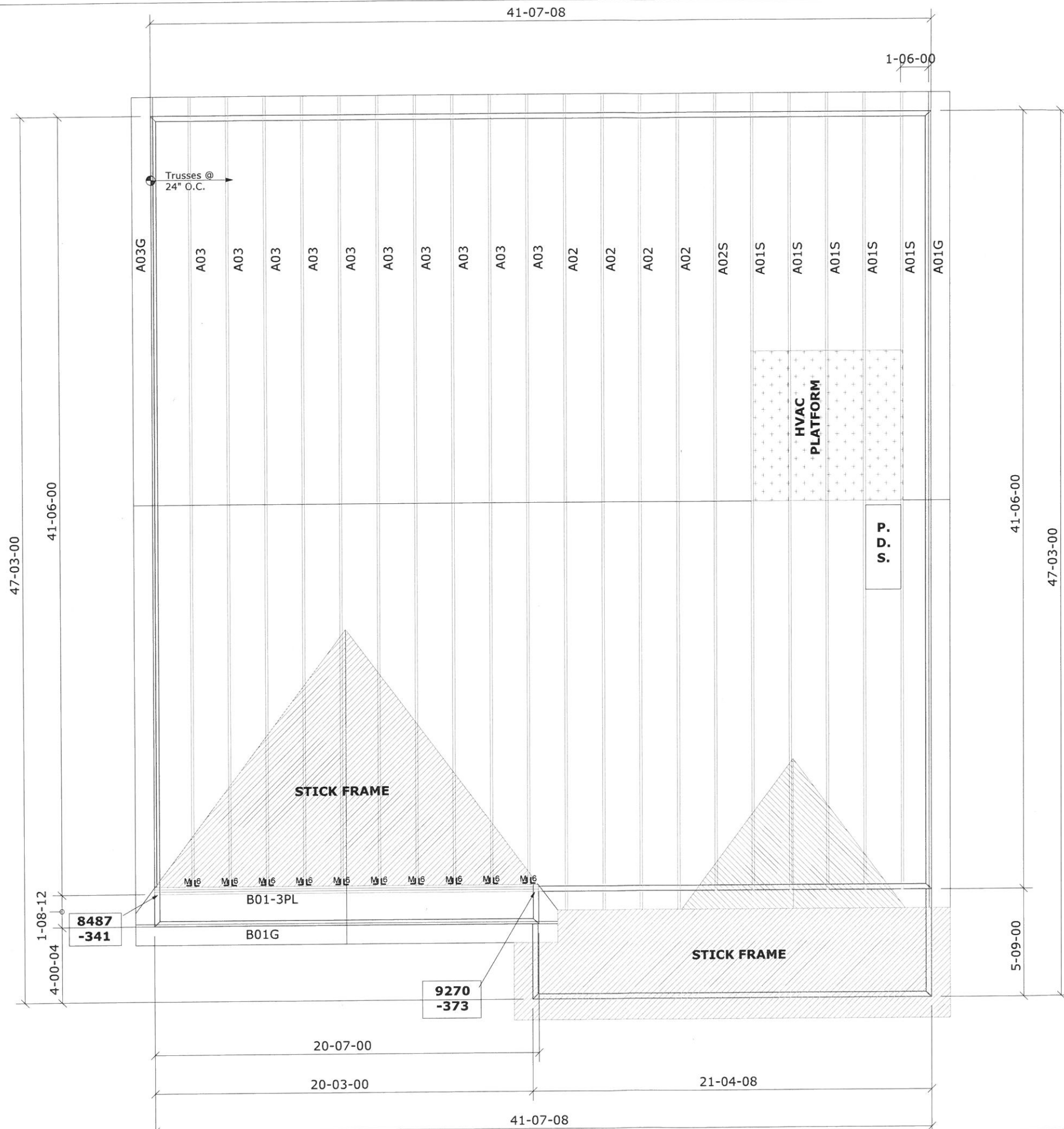
- All hangers are to be Simpson or equivalent U.N.O.
- Use Manufacturer's specifications for all hanger connections U.N.O.
- Use 10d x 1 1/2" Nails in hanger connections to single ply roof girder trusses.

Floor Notes:

- Shift truss as required to avoid plumbing traps.
- Installation Contractor and/or Field Supervisor are to verify all dimensions, trap locations, and options prior to installation

Dimension Notes:

- Drawing not to scale. Do not scale dimensions



Hanger List		All Tie Downs H2.5A Unless noted	
100	H2.5		
10	HTU26		

Special Items List	

Misc Material	

SAVVY	
CARMICHEAL	Elev: C

NC	Lot: ---

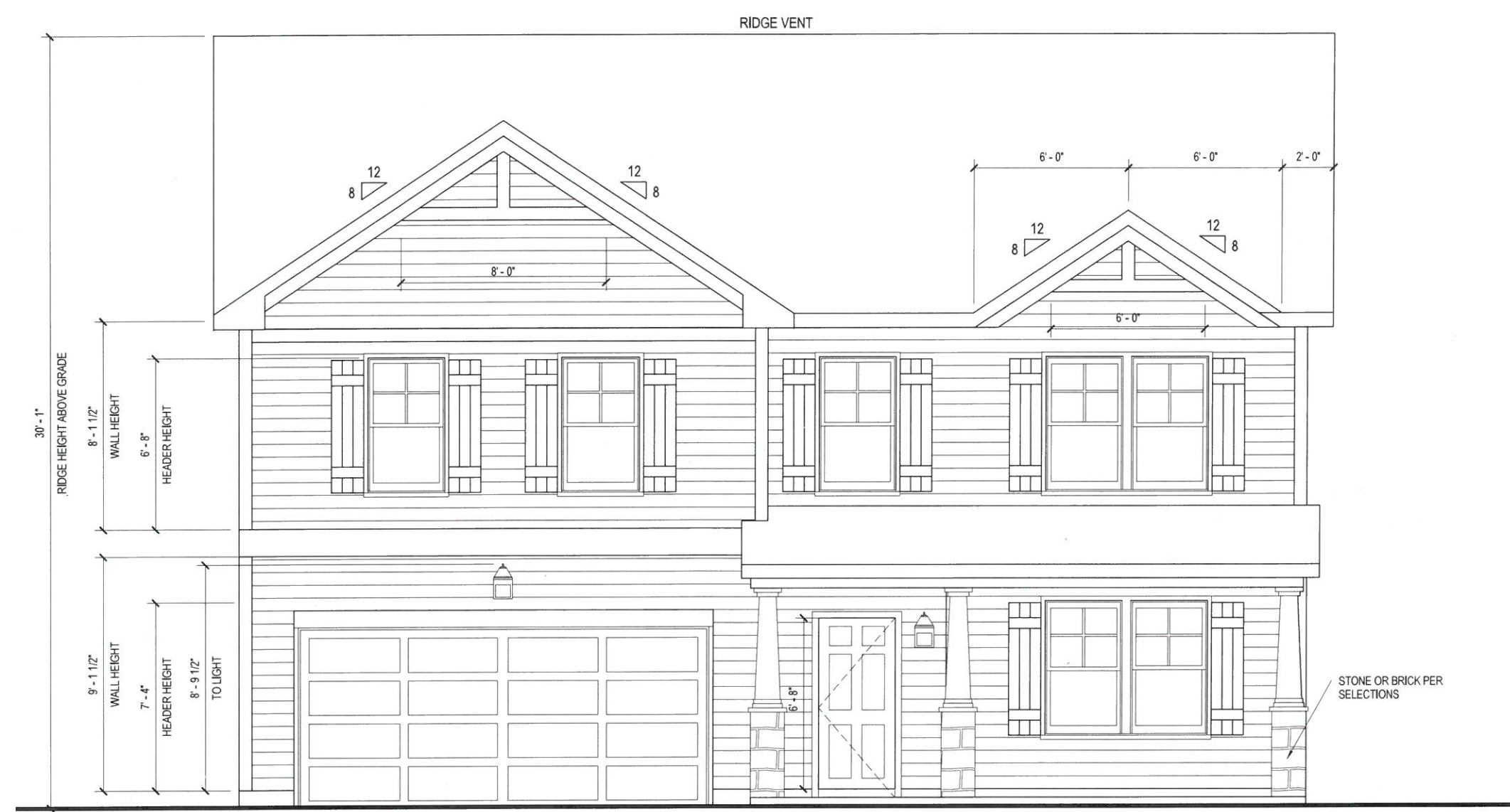
Appwright #	
BASE/LH	---
Code:	IRC 2009

Loading:	
T.C.L.L.	20
T.C.D.L	10
B.C.L.L.	0
B.C.D.L.	10

Revision History		Wind:	
Rev1:	xx/xx/xx	M.P.H.	100 mph
Rev2:	xx/xx/xx	Exposure Category	
Rev3:	xx/xx/xx	EXPOSURE B	
Pick Ticket:	---	Job No:	---
Sales No:	---	Acct No:	307844

Hatch Legend	
	Attic Room
	Volume Ceiling
	Stick Framing

Life Amplified.



A FRONT ELEVATION (13)
3/16" = 1'-0"

NOTICE TO CONTRACTOR
All construction must comply with
current NC Building Codes and is subject
to field inspection and verification.

ROOF VENTILATION CALCULATIONS	
VENTING	
ATTIC AREA EQUALS	1717 SQFT
VENTILATION REQUIRED EQUALS	11 SQFT

VENTILATION REQUIRED			
2"CONT. VENT EAVE STRIP:	80	LF=	7.5
RIDGE VENT:	51	LF=	4.5
GABLE VENT:	0	SQFT=	0
TOTAL			12

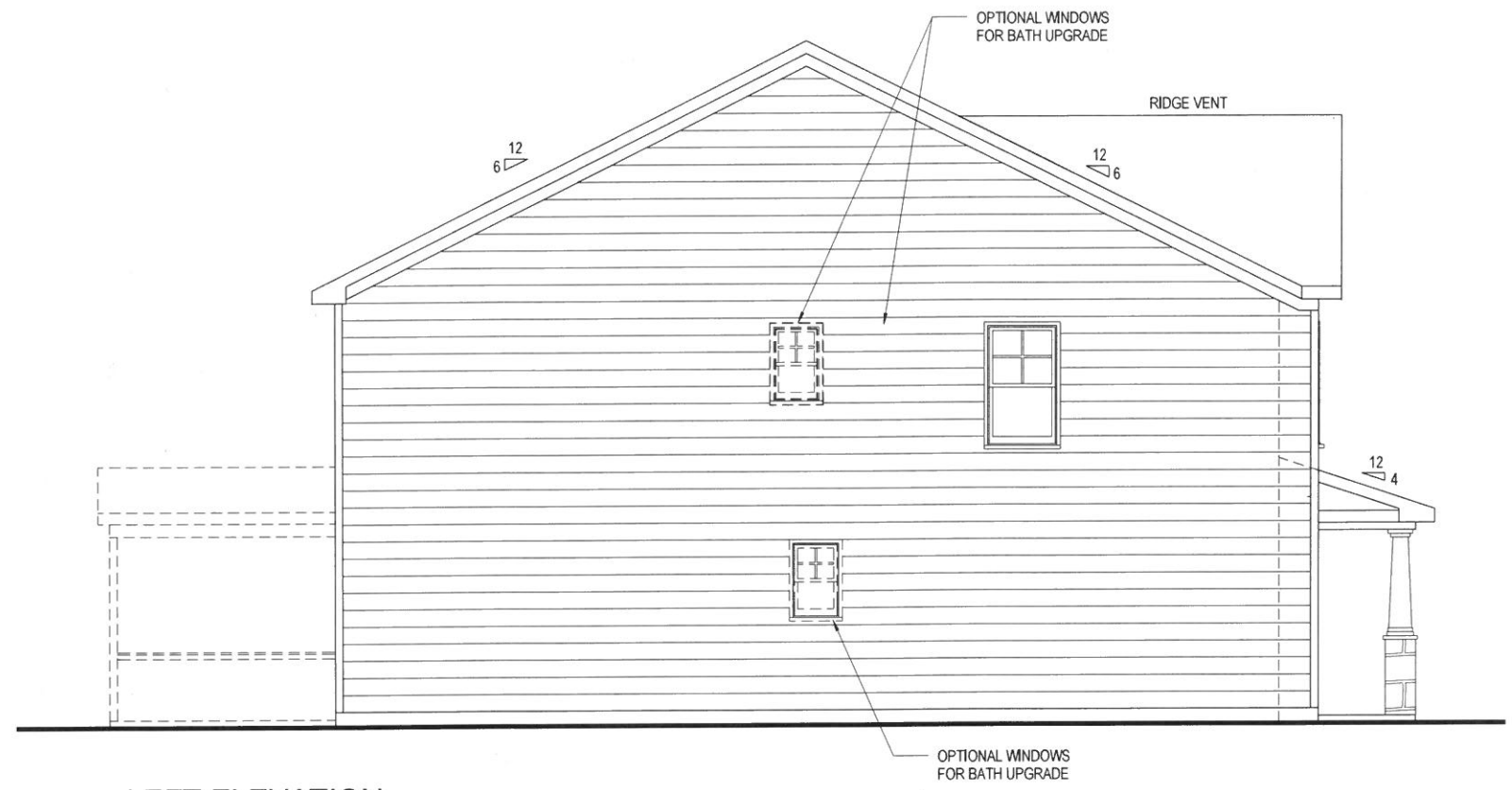
SQUARE FOOTAGES	
	SQ. FT.
1ST FLOOR	1316
2ND FLOOR	1717
GARAGE	413
FRONT PORCH	162
OPT. PATIO	110
SQUARE FOOTAGES	
	SQ. FT.
TOTAL LIVING	3033

NOTE:
BANDS ARE 10", TYP.
PEDIMENTS ARE 6" TYP.

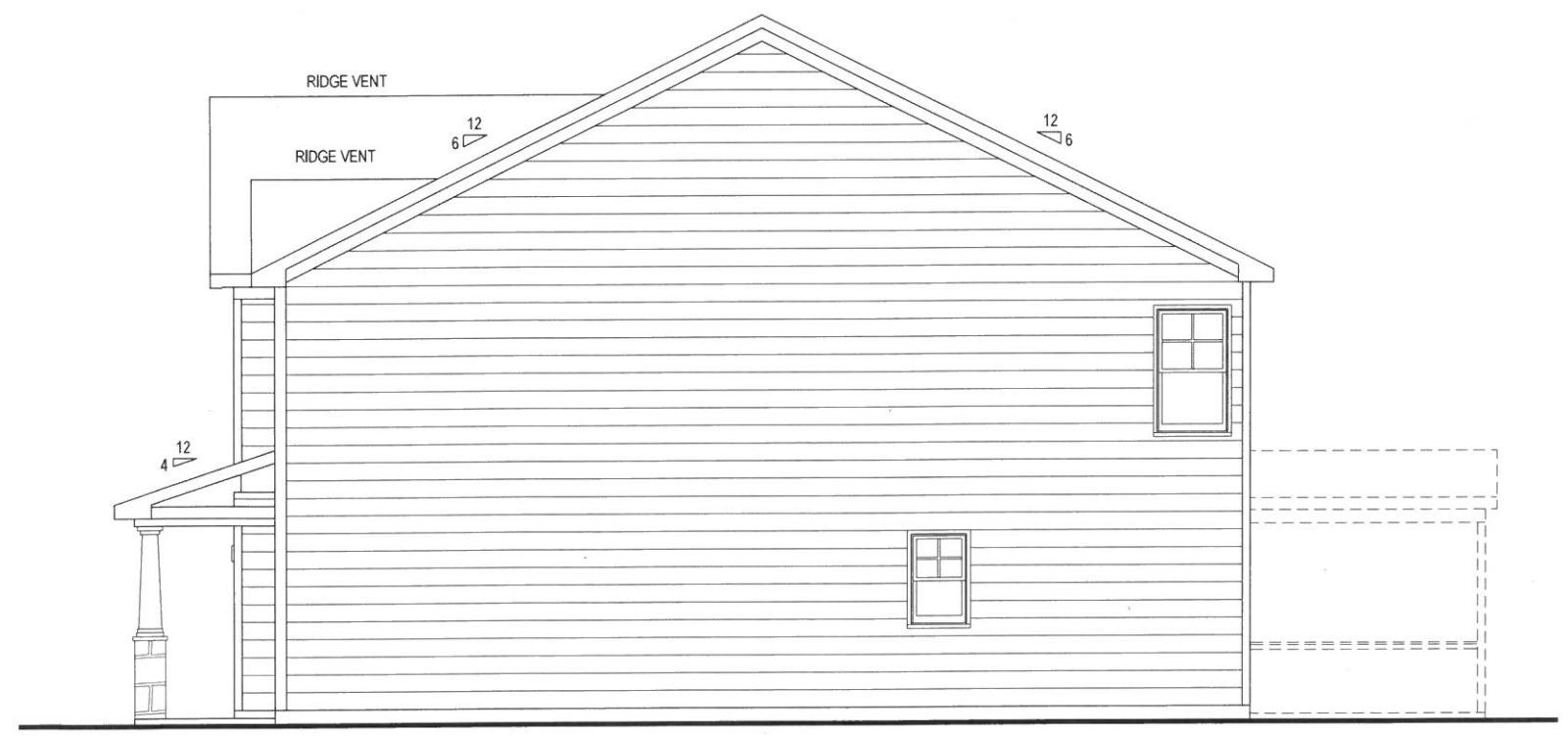
SIDING PER COMMUNITY REQUIREMENT.
GUTTERS PER COMMUNITY REQUIREMENT.

This plan is designed to the
2009 International Residential Code
with the most stringent of
amendments required by the following
states: AL., GA., NC., SC., VA.

B7 Memo
HARNETT COUNTY CENTRAL PERMITTING
APPLICATION # 1350030726
JOB NAME Savvy
DATE PLANS RECEIVED 7-10-15
SITE PLANS APPROVED _____
APPROVED BY MR 7-10-15



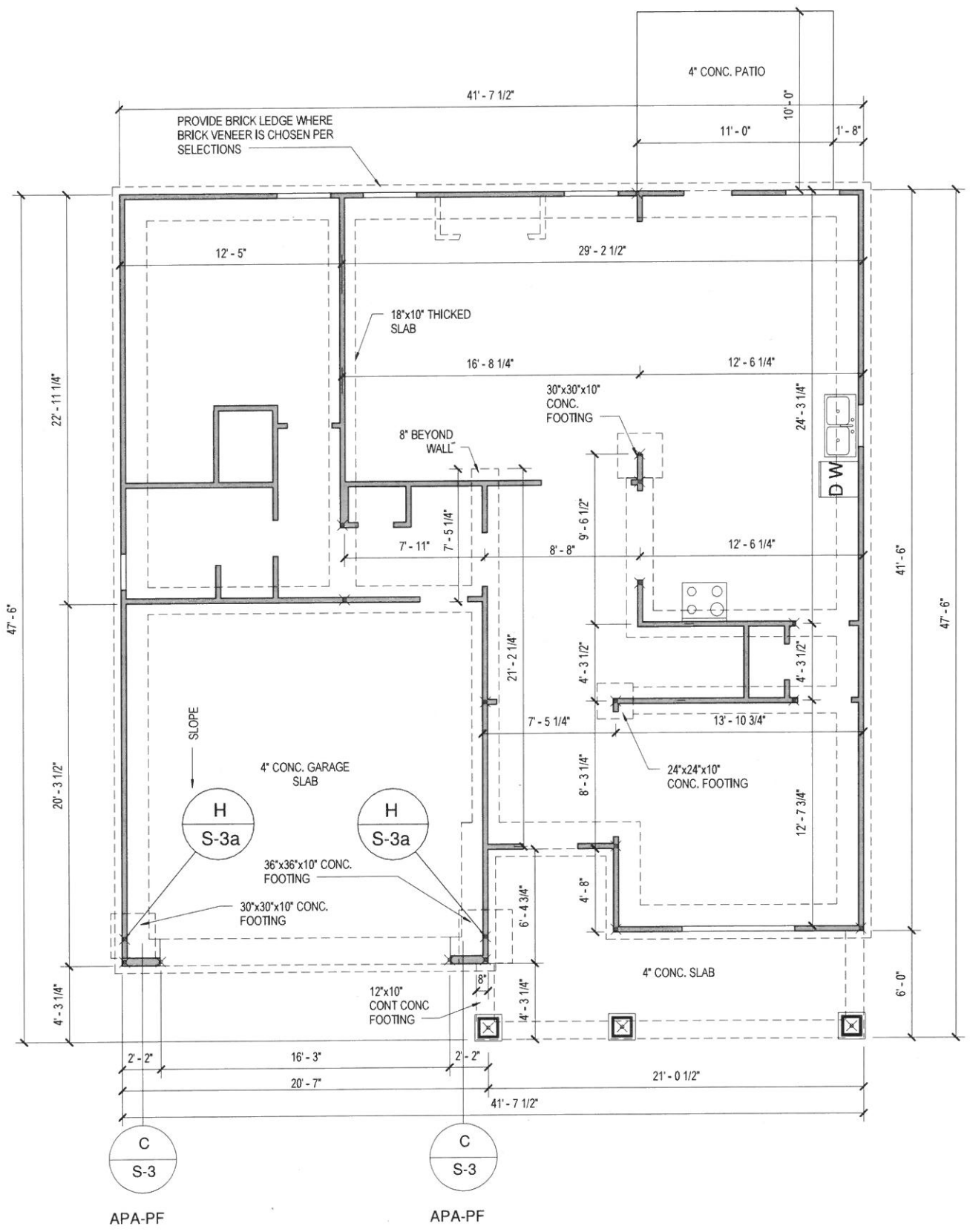
B LEFT ELEVATION
1/8" = 1'-0"



C RIGHT ELEVATION
1/8" = 1'-0"



D REAR ELEVATION
1/8" = 1'-0"



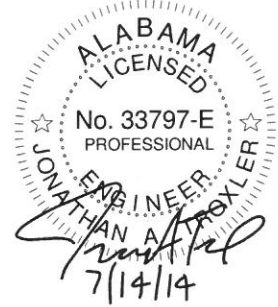
A SLAB PLAN
1/8" = 1'-0"

- REPRESENTS A POINT LOAD END FROM AN ABOVE OCCURRENCE
- ALL EXTERIOR WALLS ARE LOAD BEARING
- SEE SHEET S-1 FOR SLAB DETAILS
- HIGH WIND AREAS ONLY

 TIE DOWN FOR UPLIFT LOADS WHERE INDICATED. REQUIRED ONLY IN HIGH WIND AREAS. SEE S-3a

STRUCTURAL DESIGN BY:
 SOUTHERN ENGINEERS, P.A.
 3716 BENSON DR., RALEIGH, NC 27609
 LICENSE: C-1287, PHONE: 919-878-1617

- * Engineers seal applies only to structural components on this document. Seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
- * Any deviation or discrepancies on plans are to be brought to the immediate attention of Southern Engineer's liability
- * Seal is valid for a project permitted within one year from date of seal.
- * Use of these plans constitutes approval of terms & conditions as defined in the customer agreement.




JOISTS
TJI 210 (OR EQUAL)

HEADER AND COLUMN NOTES

1. ALL EXTERIOR AND LOAD BEARING HEADER SHALL BE MIN. (2) 2x6 WITH (1) SUPPORT STUD (1) KING STUD, UNLESS NOTED OTHERWISE.
2. THE NUMBER SHOWN AT BEAM AND HEADER SUPPORTS INDICATES THE NUMBER OF SUPPORT STUDS REQUIRED IN STUD POCKET OR COLUMN
3. COLUMN CONSISTING OF (7) OR MORE STUDS SHALL BE WRAPPED WITH 22-GAUGE METAL STRAPS AT 2" O.C.

HIGH WIND AREAS ONLY


TIE DOWN FOR UPLIFT LOADS WHERE INDICATED. REQUIRED ONLY IN HIGH WIND AREAS. SEE S-3a

WHOLE HOUSE BRACING SUMMARY
(CITY OF RALEIGH)
TOTAL REQUIRED BRACING:92
TOTAL PROVIDED BRACING:248
(IN FEET)

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NOTE: INSTALL ALL ENGINEERED LUMBER IN ACCORDANCE W/ MANUF. INSTRUCTIONS.

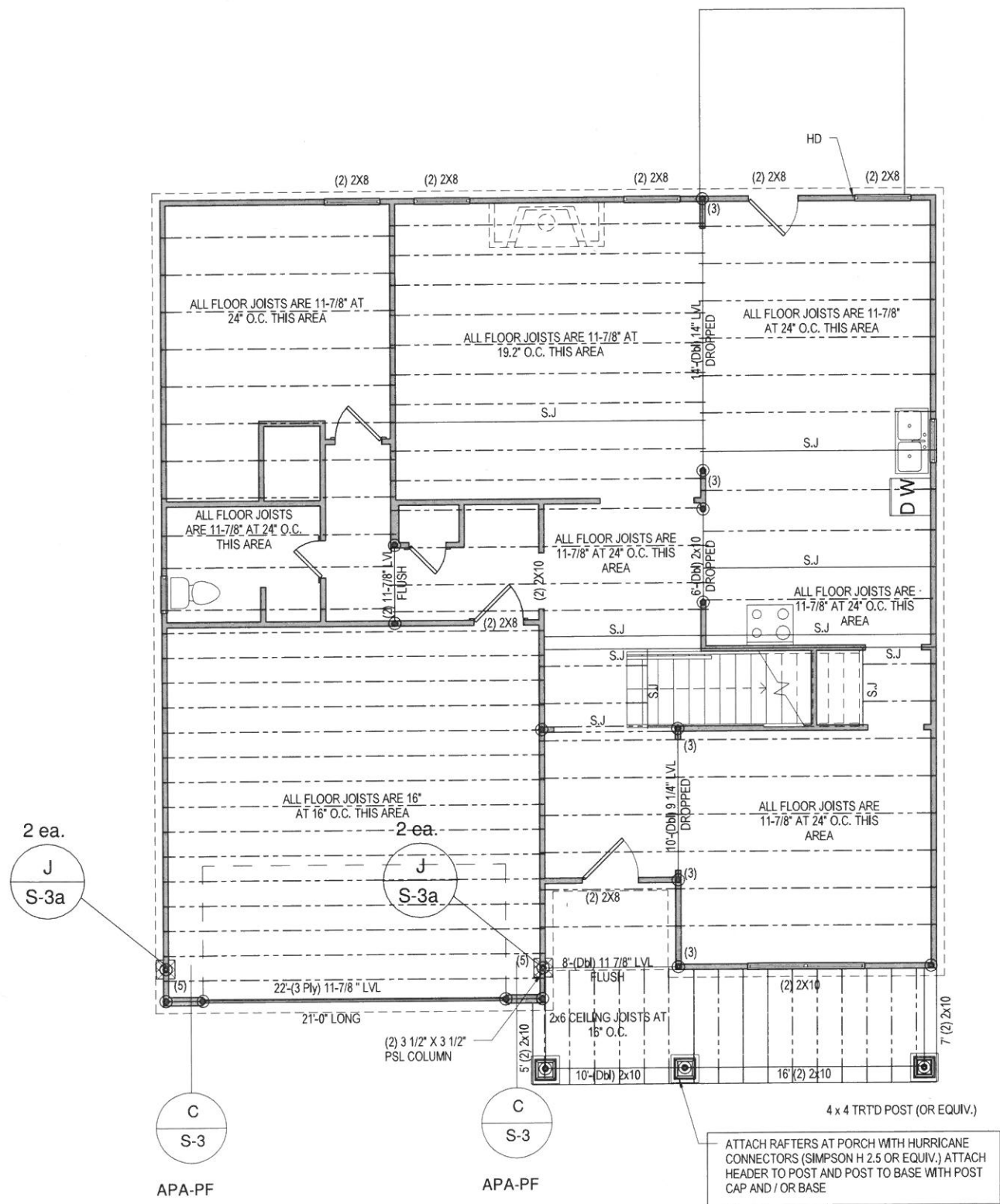
⊙ REPRESENTS A POINT LOAD FROM AN ABOVE OCCURRENCE

✕ REPRESENTS A POINT LOAD END FROM AN ABOVE OCCURRENCE

NOTE: REFER TO SHEET S-3 FOR FRAMING NOTE

BRACING NOTES:

1. WALL BRACING ANALYSIS BASED ON R602.10 - CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013; EFFECTIVE DATE SEPTEMBER 1, 2013).
FOR 130 MPH : WALLS SHALL BE BRACED ACCORDING TO SECTION R4506.2 AND R602.10- CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013; EFFECTIVE DATE SEPTEMBER 1, 2013)
2. NOTE THAT THE WALL BRACING AMOUNT PROVIDED ON THE PLANS (DETAILS AND SPECIFICATIONS) IS GREATER THAN THE AMOUNT OF WALL BRACING REQUIRED THE CODE. SEE NOTE BELOW FOR DETAILS AND SPECIFICATIONS FOR WALL BRACING.
3. BRACING METHOD AND TYPE: CONTINUOUS SHEATHING PER SECTION R602.10.3 USING WSP (WOOD STRUCTURAL PANEL SHEATHING).
4. EXTERIOR WALL SHEATHING : SHEATH EXTERIOR WALLS WITH 7/16" WSP (WOOD STRUCTURAL PANEL) SHEATHING AND ATTACH WITH 8D NAILS AT A 6"/12" NAILING PATTERN (6" OC AT PANEL EDGES AND 12" OC AT INTERMEDIATE SUPPORTS).
FOR 130 MPH: SHEATH EXTERIOR WALLS WITH 7/16" WSP (WOOD STRUCTURAL PANEL) SHEATHING (FOR EXPOSURE C, USE 15/32" WSP) AND ATTACH WITH 8D NAILS AT A 3"/6" NAILING PATTERN (3" OC AT PANEL EDGES AND 6" OC AT INTERMEDIATE SUPPORTS).
INSTALL BLOCKING AT ALL PANEL EDGES. (WSP SHEATHING SHALL EXTEND TO UPPERMOST DOUBLE BEARING PLATE). BLOCK AT ROOF PER R602.10.5.5.
5. MINIMUM WALL LENGTHS ARE BASED ON TABLE R602.10.1 AND ARE TO BE LOCATED AS SPECIFIED IN SECTION R602.10.3.2.
6. HOLD-DOWN DEVICE (NOTE AS "HD" ON PLANS) SHALL BE AN 800 POUND CAPACITY ASSEMBLY AS NOTED ON PLANS. SEE DETAILS FOR HD ASSEMBLY.
7. INTERIOR BRACED WALL: (NOTED AS "IBW" ON PLANS) ATTACH 1/2" GYPSUM BOARD ON EACH SIDE OF WALL WITH A MIN. OF 5D COOLER NAILS OR #6 SCREWS @ 7" O.C. ALONG THE EDGES AND AT INTERMEDIATE SUPPORTS. INTERIOR BRACED WALLS SHALL BE CONNECTED AS DESCRIBED IN R602.10.5.4 AND FIGURES CR602.10.5.4 (1) AND CR602.10.5.4(2).




A SECOND FLOOR FRAMING
1/8" = 1'-0"



BRACING NOTES:

1. WALL BRACING ANALYSIS BASED ON R602.10 - CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013; EFFECTIVE DATE SEPTEMBER 1, 2013).
FOR 130 MPH : WALLS SHALL BE BRACED ACCORDING TO SECTION R4506.2 AND R602.10- CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013; EFFECTIVE DATE SEPTEMBER 1, 2013)
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3. BRACING METHOD AND TYPE: CONTINUOUS SHEATHING PER SECTION R602.10.3 USING WSP (WOOD STRUCTURAL PANEL SHEATHING).
4. EXTERIOR WALL SHEATHING : SHEATH EXTERIOR WALLS WITH 7/16" WSP (WOOD STRUCTURAL PANEL) SHEATHING AND ATTACH WITH 8D NAILS AT A 6"/12" NAILING PATTERN (6" OC AT PANEL EDGES AND 12" OC AT INTERMEDIATE SUPPORTS).
FOR 130 MPH: SHEATH EXTERIOR WALLS WITH 7/16" WSP (WOOD STRUCTURAL PANEL) SHEATHING (FOR EXPOSURE C, USE 15/32" WSP) AND ATTACH WITH 8D NAILS AT A 3"/6" NAILING PATTERN (3" OC AT PANEL EDGES AND 6" OC AT INTERMEDIATE SUPPORTS). INSTALL BLOCKING AT ALL PANEL EDGES. (WSP SHEATHING SHALL EXTEND TO UPPERMOST DOUBLE BEARING PLATE). BLOCK AT ROOF PER R602.10.5.5.
5. MINIMUM WALL LENGTHS ARE BASED ON TABLE R602.10.1 AND ARE TO BE LOCATED AS SPECIFIED IN SECTION R602.10.3.2.
6. HOLD-DOWN DEVICE (NOTE AS "HD" ON PLANS) SHALL BE AN 800 POUND CAPACITY ASSEMBLY AS NOTED ON PLANS. SEE DETAILS FOR HD ASSEMBLY.
7. INTERIOR BRACED WALL: (NOTED AS "IBW" ON PLANS) ATTACH 1/2" GYPSUM BOARD ON EACH SIDE OF WALL WITH A MIN. OF 5D COOLER NAILS OR #6 SCREWS @ 7" O.C. ALONG THE EDGES AND AT INTERMEDIATE SUPPORTS. INTERIOR BRACED WALLS SHALL BE CONNECTED AS DESCRIBED IN R602.10.5.4 AND FIGURES CR602.10.5.4 (1) AND CR602.10.5.4 (2).

HIGH WIND AREAS ONLY



TIE DOWN FOR UPLIFT LOADS WHERE INDICATED. REQUIRED ONLY IN HIGH WIND AREAS. SEE S-3a

NOTE: INSTALL ALL ENGINEERED LUMBER IN ACCORDANCE W/ MANUF. INSTRUCTIONS.

⊙ REPRESENTS A POINT LOAD FROM AN ABOVE OCCURRENCE

NOTE: REFER TO SHEET S-3 FOR FRAMING NOTES

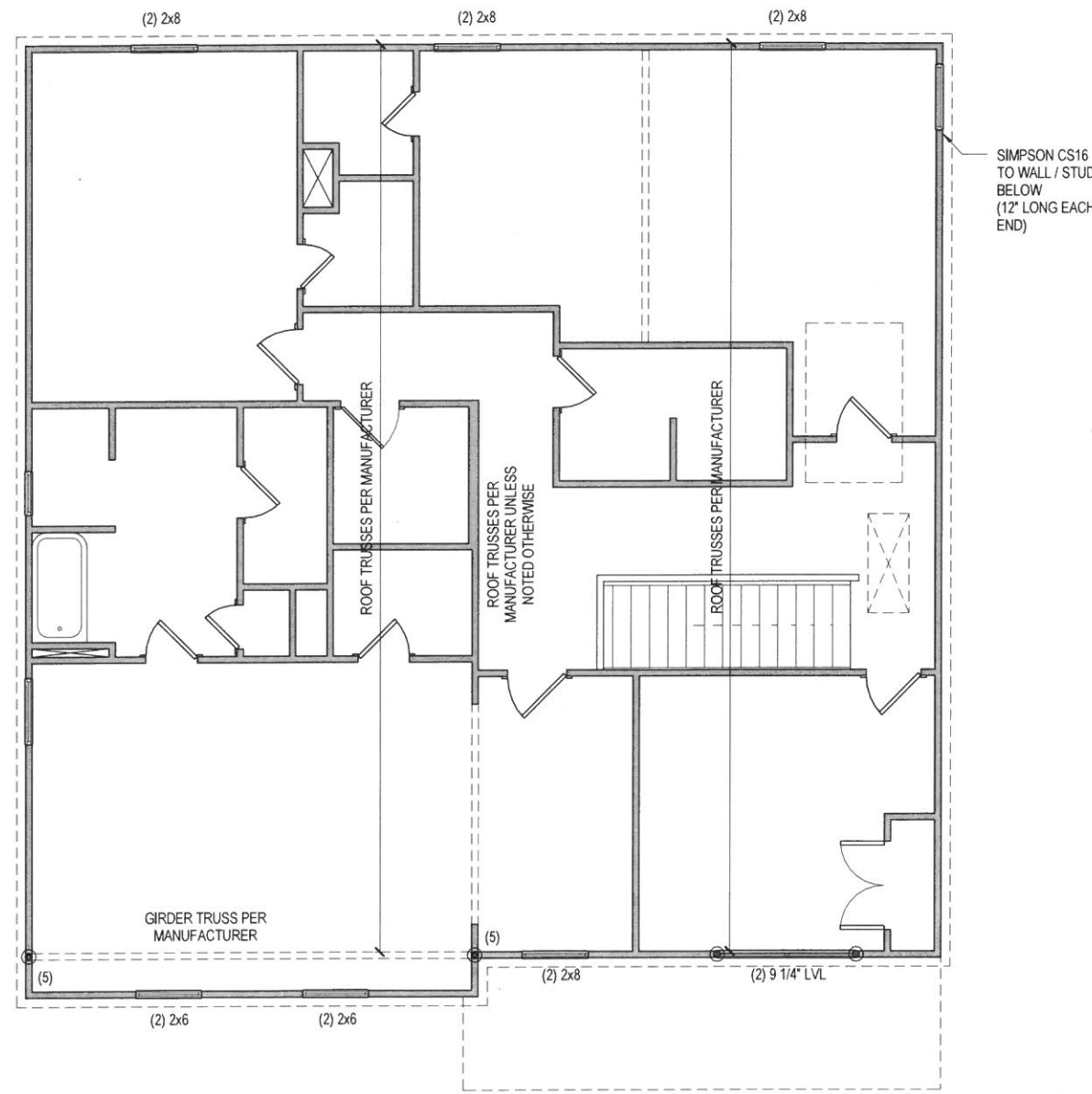
ALL STUD COLUMNS 2 - 2x4 UNLESS OTHERWISE NOTED

TRUSS SYSTEM REQUIREMENTS

1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS) SHALL BE DESIGNED IN ACCORDANCE WITH SEALED STRUCTURAL PLANS. ANY NEED TO CHANGE TRUSSES SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.
2. TRUSS SCHEMATIC (POFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.
3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNQ).
4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UP LIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATIC

STRUCTURAL DESIGN BY:
SOUTHERN ENGINEERS, P.A.
3716 BENSON DR., RALEIGH, NC 27609
LICENSE: C-1287, PHONE: 919-878-1617

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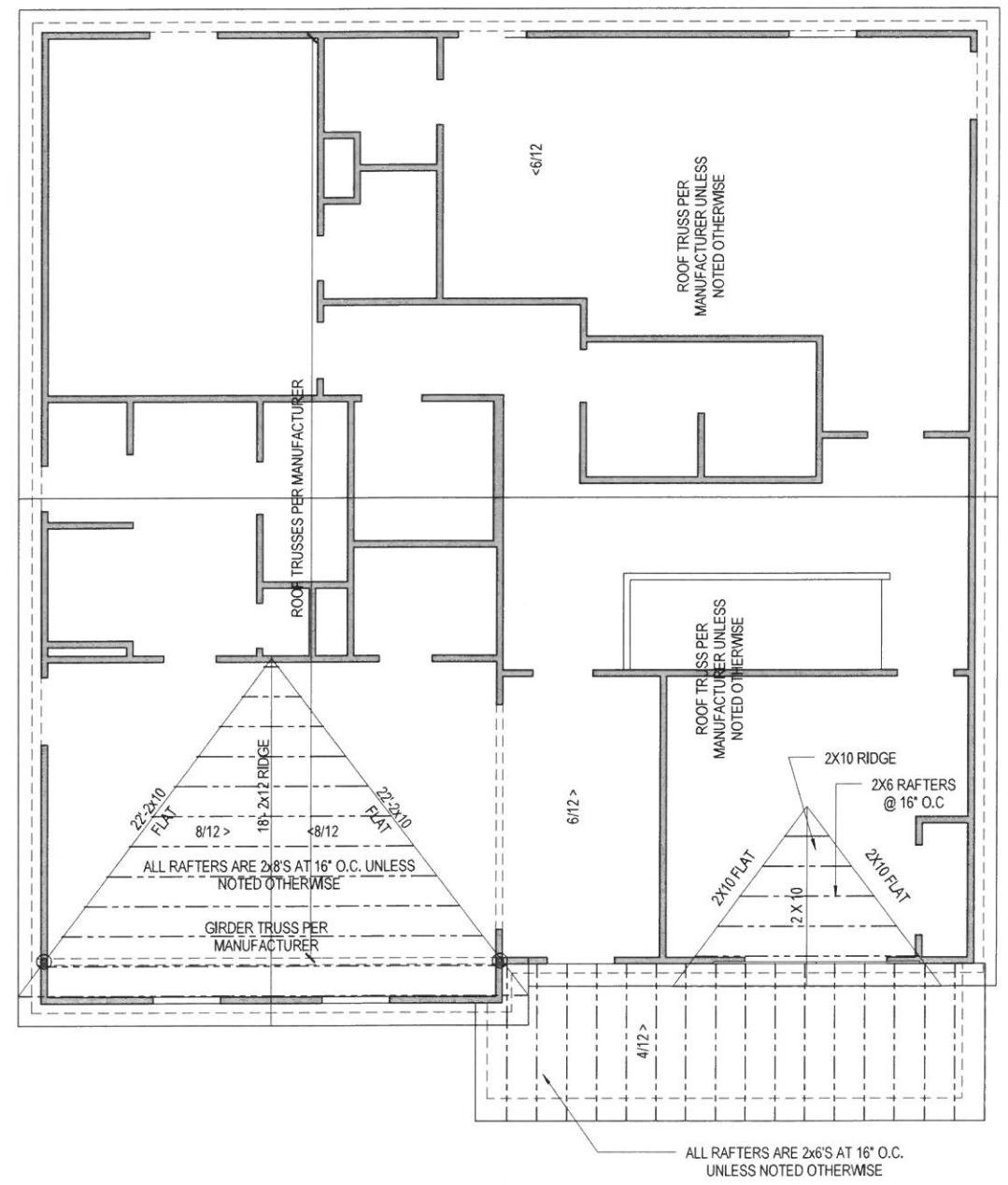
NOTE: TRUSS UPLIFT CONNECTIONS TO TOP PLATE PROVIDED BY TRUSS MANUFACTURER

A SECOND FLOOR CEILING FRAMING
1/8" = 1'-0"

NOTE: REFER TO SHEET S-3 FOR FRAMING NOTE

⊙ REPRESENTS A POINT LOAD FROM AN ABOVE OCCURRENCE

HIGH WIND AREAS ONLY

TIE DOWN FOR UPLIFT LOADS WHERE INDICATED. REQUIRED ONLY IN HIGH WIND AREAS. SEE S-3a



A ROOF FRAMING
1/8" = 1'-0"

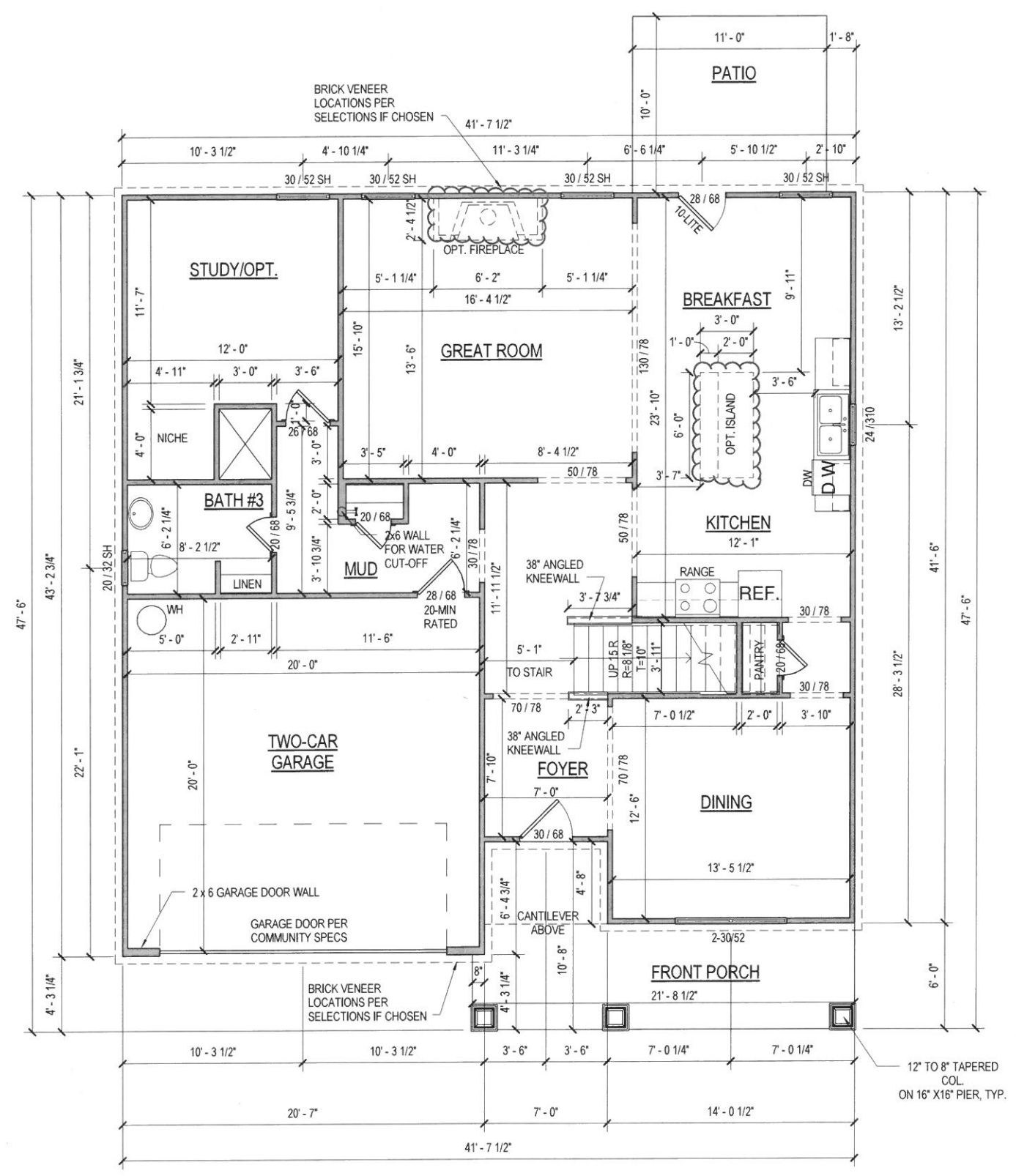
TRUSS SYSTEM REQUIREMENTS

1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS) SHALL BE DESIGNED IN ACCORDANCE WITH SEALED STRUCTURAL PLANS. ANY NEED TO CHANGE TRUSSES SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.
2. TRUSS SCHEMATIC (POFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.
3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNQ).
4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UP LIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATIC

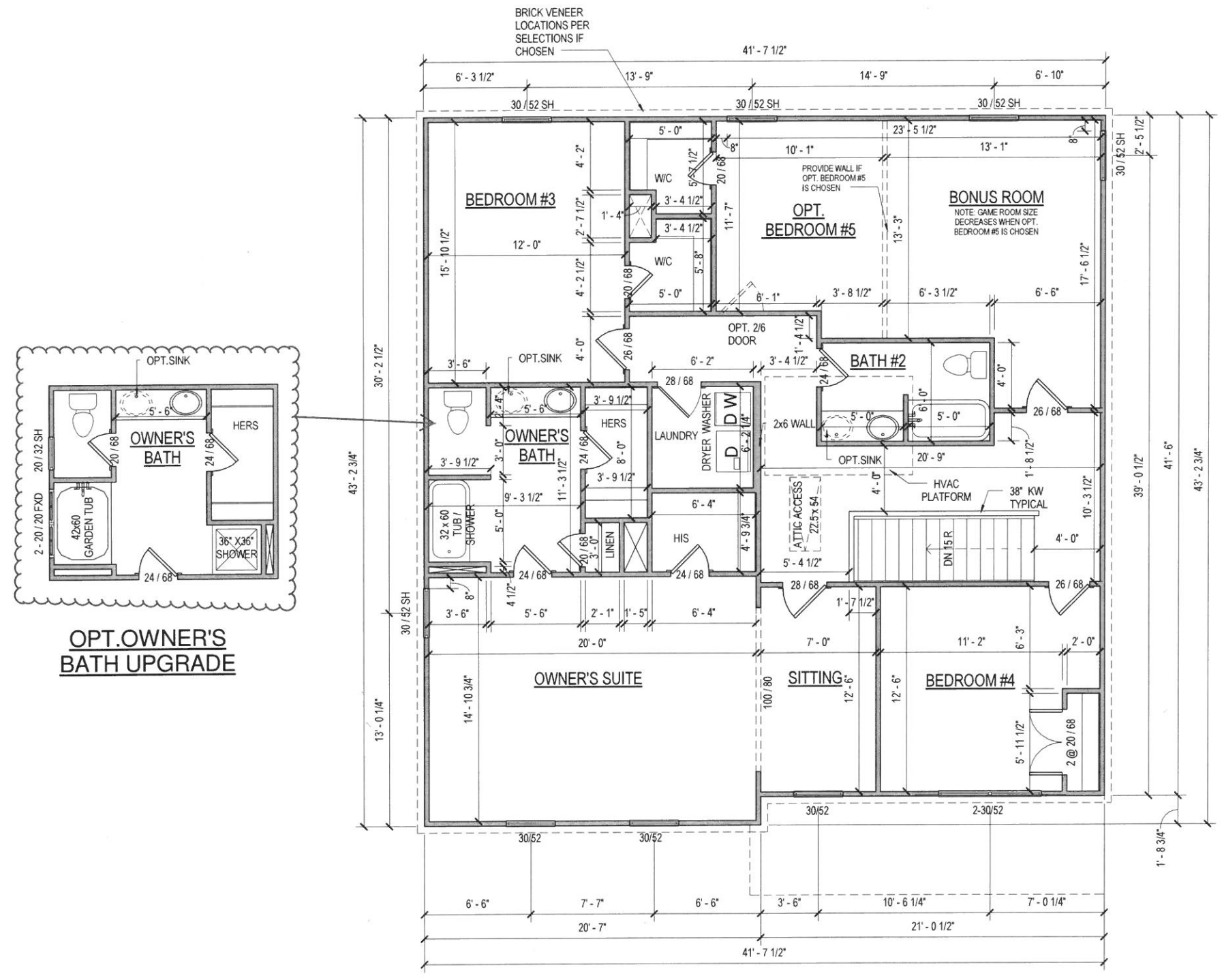
STRUCTURAL DESIGN BY:
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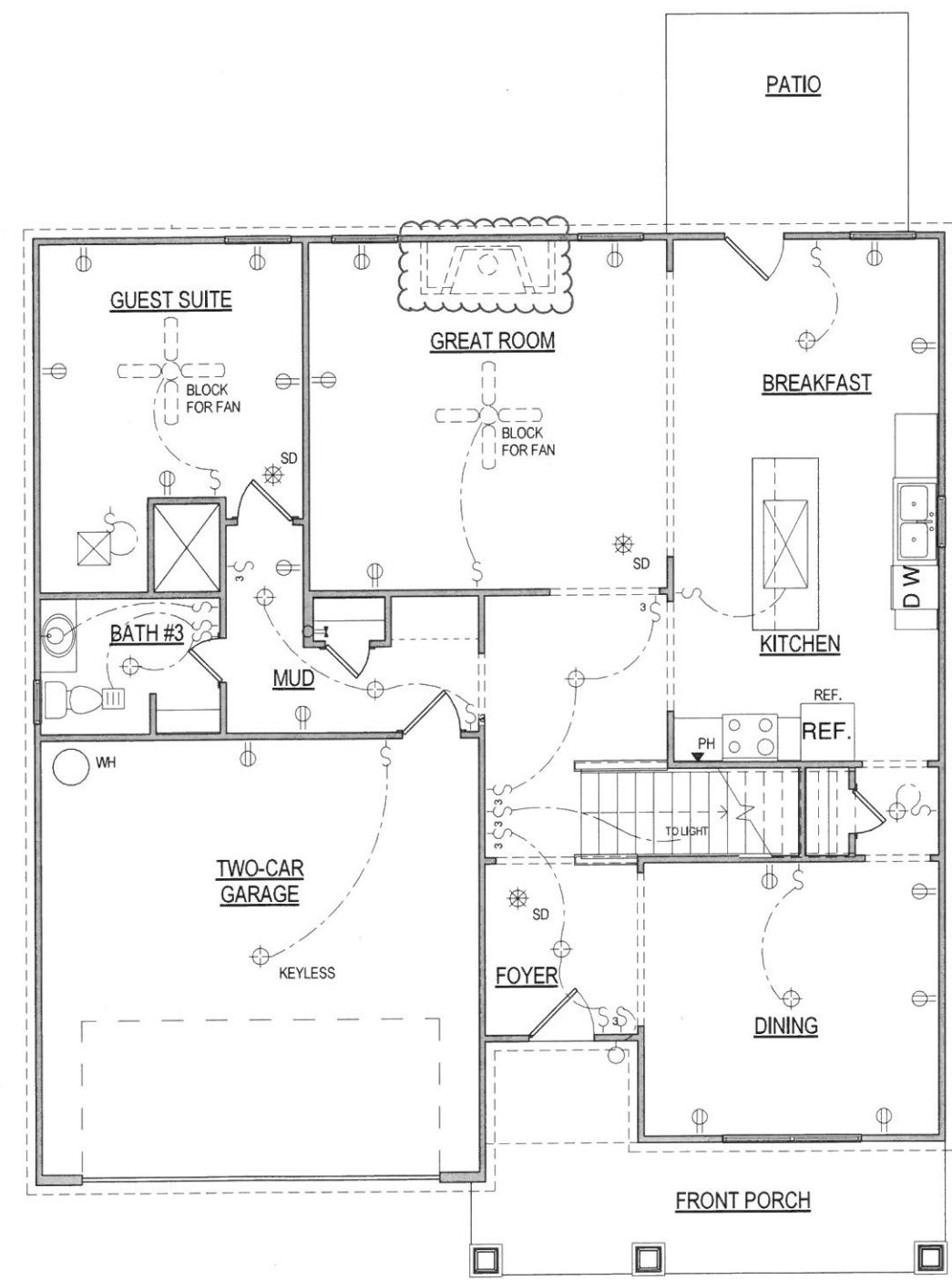


A FIRST FLOOR
1/8" = 1'-0"

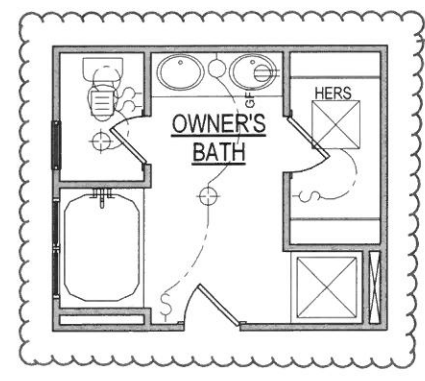


A Second Floor Plan
1/8" = 1'-0"

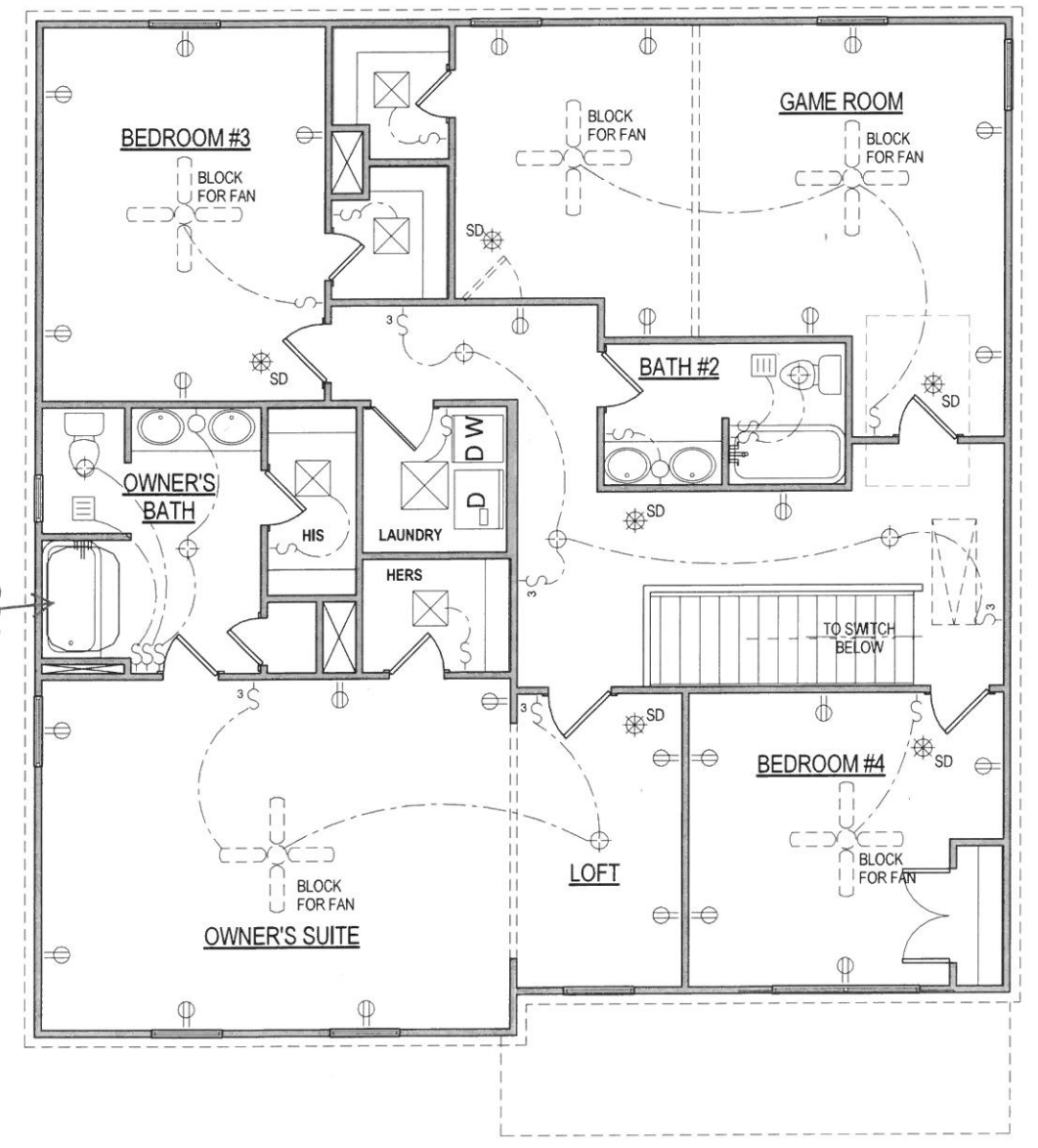
ELECTRICAL LEGEND	
	SWITCH
	THREE - WAY SWITCH
	DUPLEX
	220 VOLT OUTLET
	GFCI DUPLEX
	COUNTERTOP DUPLEX
	PHONE OUTLET
	SMOKE DETECTOR / CARBON MONOXIDE DETECTOR
	CEILING LIGHT
	WALL HUNG LIGHT
	VENT
	18" FLOURESCENT
	24" FLOURESCENT
	24" x 48" FLOURESCENT



A FIRST FLOOR - ELECTRICAL
1/8" = 1'-0"



**OPT.OWNER'S
BATH UPGRADE**



B SECOND FLOOR - ELECTRICAL
1/8" = 1'-0"

I-Joist Length, on Top of Flange

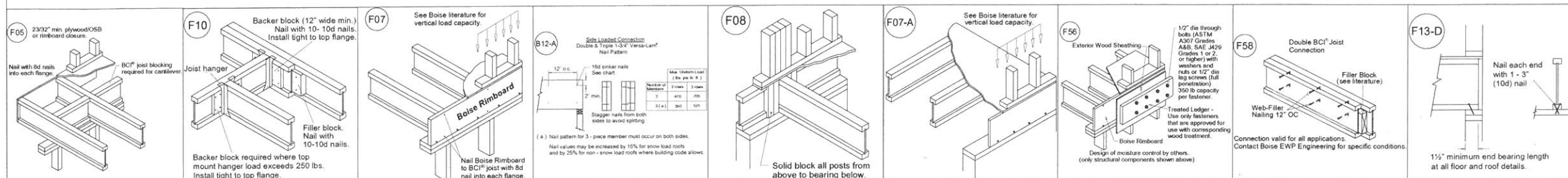
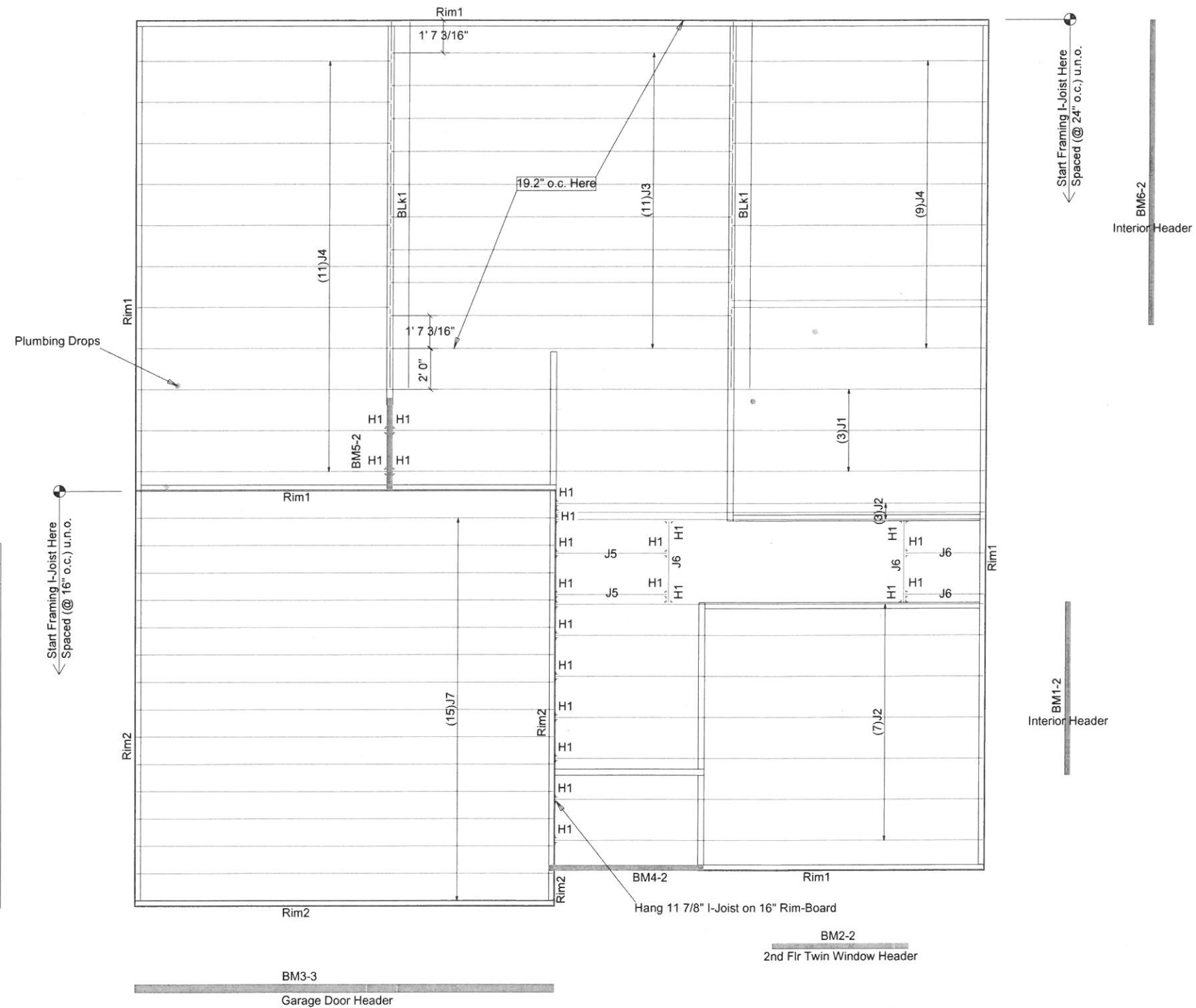
Note: Refer to current Boise Cascade Installation Guide for details not shown.

General Notes:

- 1.) "READ" Boise Installation Guide before installation of products.
- 2.) All I-Joist, LVL beams, and Rim Board must be field cut to length.
- 3.) Contractor must verify and approve the above material list.
- 4.) I-Joist may be moved 3" out of its own o.c. spacing, up to 19.2" o.c., to allow for plumbing drops.
- 5.) This layout, is a placement plan and, was designed in accordance with the original design of the structure (unless otherwise noted). See original plans for additional structural notes.
- 6.) Ceramic tile floors should be supported per APA standards. Additional joists may be required.
- 7.) HVAC & PLUMBER, "Review" Boise Installation Guide (Joist Hole Location & Sizing) Chart "BEFORE" cutting the I-joist product.
- 8.) "Blocking", are Random Length I-joist, Labeled as such.

Connector Summary				
PlotID	Qty	Manuf	Product	Flange
H1	24	Simpson	IUS 2.06/11.88	None

Products				
PlotID	Net Qty	Product	Length	Plies
J1	3	11-7/8" BCI® 5000s-1.8 SP	30' 0"	1
J2	10	11-7/8" BCI® 5000s-1.8 SP	22' 0"	1
J3	11	11-7/8" BCI® 5000s-1.8 SP	17' 0"	1
J4	20	11-7/8" BCI® 5000s-1.8 SP	13' 0"	1
J5	2	11-7/8" BCI® 5000s-1.8 SP	6' 0"	1
J6	4	11-7/8" BCI® 5000s-1.8 SP	5' 0"	1
J7	15	16" BCI® 6000s-1.8 SP	21' 0"	1
BM1-2	2	1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP	10' 0"	2
BM2-2	2	1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP	8' 0"	2
BM3-3	3	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	22' 0"	3
BM4-2	2	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	8' 0"	2
BM5-2	2	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	6' 0"	2
BM6-2	2	1-3/4" x 14" VERSA-LAM® 2.0 3100 SP	16' 0"	2
Rim1	12	1" x 11-7/8" BC RIM BOARD OSB	12' 0"	1
Rim2	6	1" x 16" BC RIM BOARD OSB	12' 0"	1
BLK1	1	11-7/8" BCI® 5000s-1.8 SP	30' 0"	1



2nd Floor Layout

Revisions: BY:

Boise Cascade

DN01 DO NOT cut, notch or drill flanges

DO NOT Cut Flanges

84 LUMBER
Build on what we know.
6301 Old Wake Forest Road, Raleigh, N.C. 27604 (919) 872-6984

Savvy Homes
L59 Kenlan Farms
Carmichael C
Second Floor Layout

BC FRAMER

Scale: Not to a Scale

Date: 04.22.2015

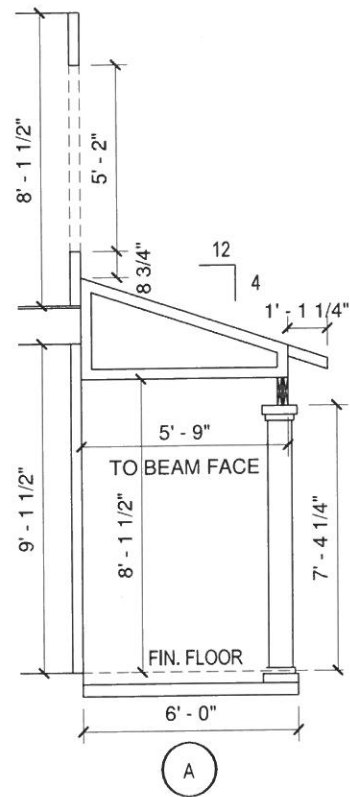
By: kelvinWOOD

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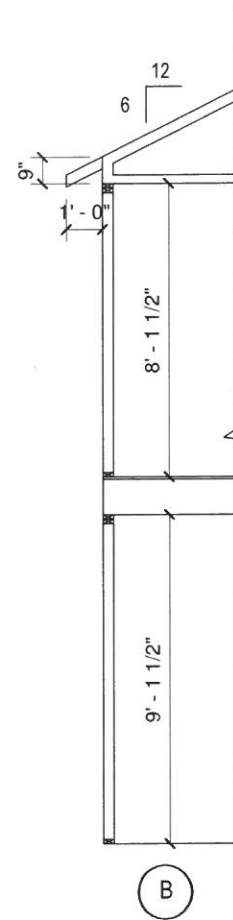
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Sheet: 1 of 1

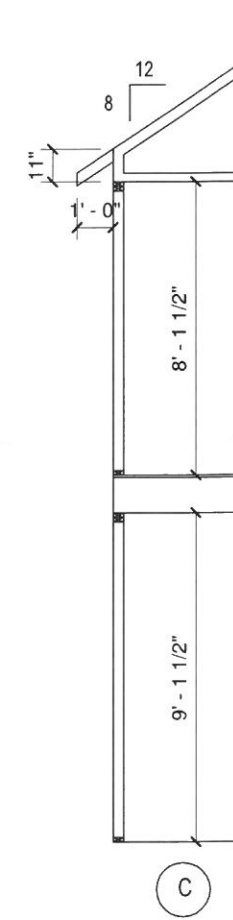
NOTE: SECTIONS AND DETAILS PROVIDED ON THIS SHEET ARE FOR SCHEMATIC USE ONLY. SEE TRUSS DOCUMENTATION AND/OR STRUCTURAL SHEETS FOR CONSTRUCTION INFORMATION



2 PORCH SECTION
3/16" = 1'-0"



B ROOF THUMBNAILS
3/16" = 1'-0"



STRUCTURAL NOTES

2012 NCRC (2009 IRC)
100 and 130 mph ZONE

1) ENGINEER'S SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS INCLUDING ROOF RAFTERS, RIPS, VALLEYS, RIDGES, FLOORS, WALLS, BEAMS AND HEADERS, COLUMNS, CANTILEVERS, OFFSET LOAD BEARING WALLS, PIER & GIRDER SYSTEM AND FOOTINGS. ENGINEER'S SEAL DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT INCLUDING ROOF SYSTEM. ALL REQUIREMENTS FOR PROFESSIONAL CERTIFICATION SHALL BE PROVIDED BY THE APPROPRIATE PROFESSIONAL. SOUTHERN ENGINEERS, P.A. CERTIFIES ONLY THE STRUCTURAL COMPONENTS AS SPECIFICALLY STATED.

2) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2012 EDITION (2009 IRC), PLUS ALL LOCAL CODES AND REGULATIONS.
(FOR 130 MPH: ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE 2012 NORTH CAROLINA RESIDENTIAL CODE (2009 IRC) (WITH SPECIAL CONSIDERATION TO CHAPTER 45-HIGH WIND ZONES AND CHAPTER 46-COASTAL AND FLOOD PLAIN CONSTRUCTION STANDARDS AND THE WOOD FRAME CONSTRUCTION MANUAL (WFCM) 2001 EDITION), PLUS ALL LOCAL CODES AND REGULATIONS).
THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK, NOR WILL THE ENGINEER BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
"CONSTRUCTION REVIEW" SERVICES ARE NOT PART OF OUR CONTRACT. ALL MEMBERS SHALL BE FRAMED, ANCHORED, TIED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BUILDING CODE.

3) DESIGN LOADS (R301.4) (LISTED AS: LIVE LOAD, DEAD LOAD, DEFLECTION LIMIT) ROOMS OTHER THAN SLEEPING ROOMS: (40 PSF, 10 PSF, L/360) SLEEPING ROOMS: (30 PSF, 10 PSF, L/360) ATTIC WITH PERMANENT STAIR: (40 PSF, 10 PSF, L/360) ATTIC WITH OUT PERMANENT STAIR: (20 PSF, 10 PSF, L/360) ATTIC WITH OUT STORAGE: (10 PSF, 10 PSF, L/240) STAIRS: (40 PSF, --, L/360) EXTERIOR BALCONIES: (60 PSF, 10 PSF, L/360) DECKS: (40 PSF, 10 PSF, L/360) GUARDRAILS AND HANDRAILS: (200 LBS) PASSENGER VEHICLE GARAGES: (50 PSF, 10 PSF, L/360) FIRE ESCAPES: (40 PSF, 10 PSF, L/360) SNOW: (20 PSF) WIND LOAD: (BASED ON 100 OR 130 MPH WIND VELOCITY)

4) WALL BRACING: WALLS SHALL BE BRACED ACCORDING TO R602.10- CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013: EFFECTIVE DATE SEPTEMBER 1, 2013).
(FOR 130 MPH: WALLS SHALL BE BRACED ACCORDING TO SECTION R4506.2 AND R602.10- CODE AND COMMENTARY FOR 2012 NC RESIDENTIAL CODE (FINAL 03-06-2013: EFFECTIVE DATE SEPTEMBER 1, 2013)).
NOTE THAT THE BRACING AS SPECIFIED ON THE PLANS IS BASED ON THE PRESCRIPTIVE BRACING REQUIREMENTS OF THE CODE AND SHALL BE VERIFIED AND/OR APPROVED BY THE CODE OFFICIAL.

5) CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF 5 INCHES UNLESS NOTED OTHERWISE (UNO). AIR ENTRAINED PER TABLE 402.2. ALL CONCRETE SHALL BE PROPORTIONED, MIXED, HANDLED, SAMPLED, TESTED, AND PLACED IN ACCORDANCE WITH ACI STANDARDS. ALL SAMPLES FOR PUMPING SHALL BE TAKEN FROM THE EXIT END OF THE PUMP.

6) ALLOWABLE SOIL BEARING PRESSURE ASSUMED TO BE 2000 PSF. THE CONTRACTOR MUST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTURAL ENGINEER IF UNSATISFACTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED. THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE, AND SHALL BE GRADED SO AS TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS.

7) ALL FRAMING LUMBER SHALL BE SPF #2 (Fb = 875 PSI) UNLESS NOTED OTHERWISE (UNO). ALL TREATED LUMBER SHALL BE SYP # 2 (Fb=975 PSI). PLATE MATERIAL MAY BE SPF # 3 OR SYP #3 (Fc(perp) = 425 PSI - MIN).

8) ALL WOODEN BEAMS AND HEADERS SHALL HAVE THE FOLLOWING END SUPPORTS: (1) 2x4 STUD COLUMN FOR 6'-0" MAX. BEAM SPAN (UNO), (2) 2x4 STUDS FOR BEAM SPAN GREATER THAN 6'-0" (UNO).

9) L.V.L. SHALL BE LAMINATED VENEER LUMBER: Fb=2600 PSI, Fv=285 PSI, E=1,900,000 PSI. P.S.L. SHALL BE PARALLEL STRAND LUMBER: Fb=2900 PSI, Fv=290 PSI, E=2,000,000 PSI. L.S.L. SHALL BE LAMINATED STRAND LUMBER: Fb=2250 PSI, Fv=400 PSI, E=1,550,000 PSI. INSTALL ALL CONNECTIONS PER MANUFACTURERS INSTRUCTIONS.

10) ALL ROOF TRUSS AND I-JOIST LAYOUTS SHALL BE PREPARED IN ACCORDANCE WITH THE SEALED STRUCTURAL DRAWINGS. TRUSSES AND I-JOISTS SHALL BE INSTALLED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS. ANY CHANGE IN TRUSS OR I-JOIST LAYOUT SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.

11) ALL STRUCTURAL STEEL SHALL BE ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOIST ARE TOE NAILED TO THE SOLE PLATE, AND SOLE PLATE IS NAILED OR BOLTED TO THE BEAM FLANGE @ 48" O.C. ALL STEEL TUBING SHALL BE ASTM A500.

12) REBAR SHALL BE DEFORMED STEEL, ASTM615, GRADE 60.

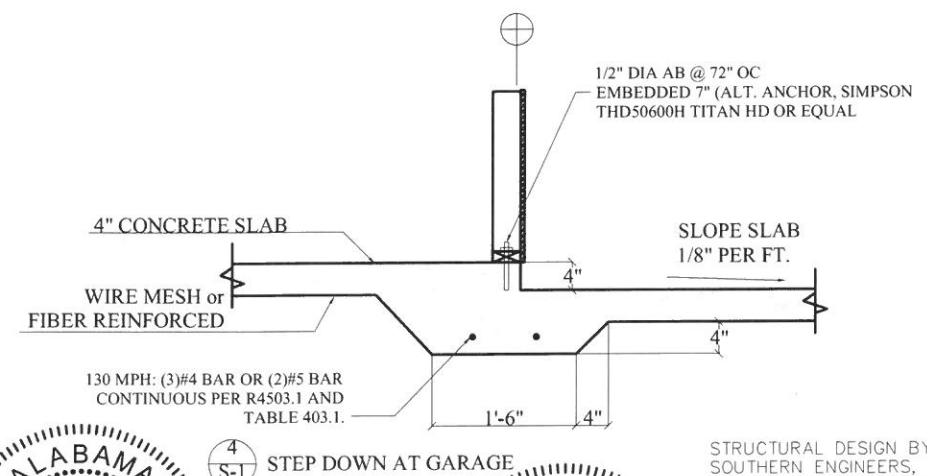
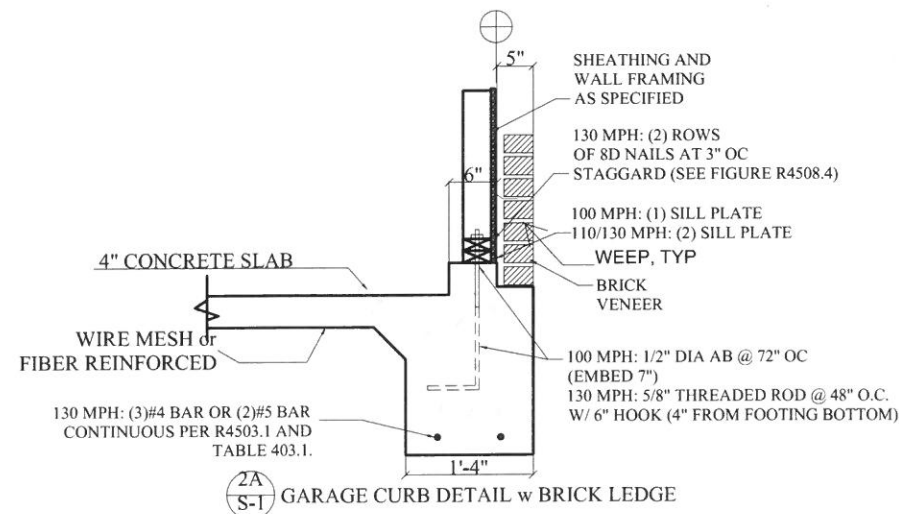
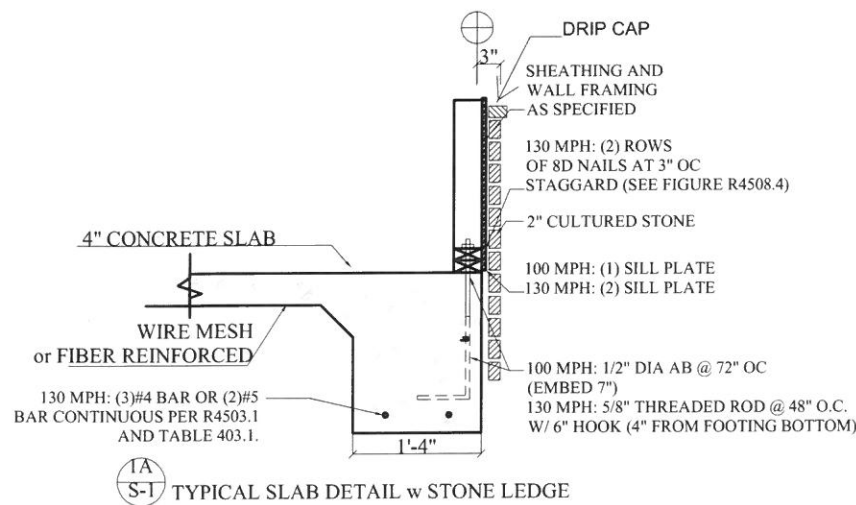
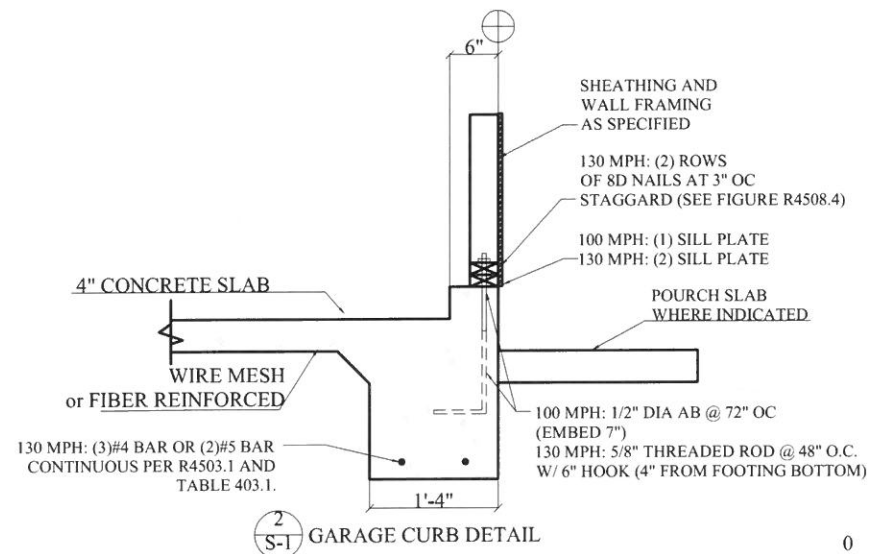
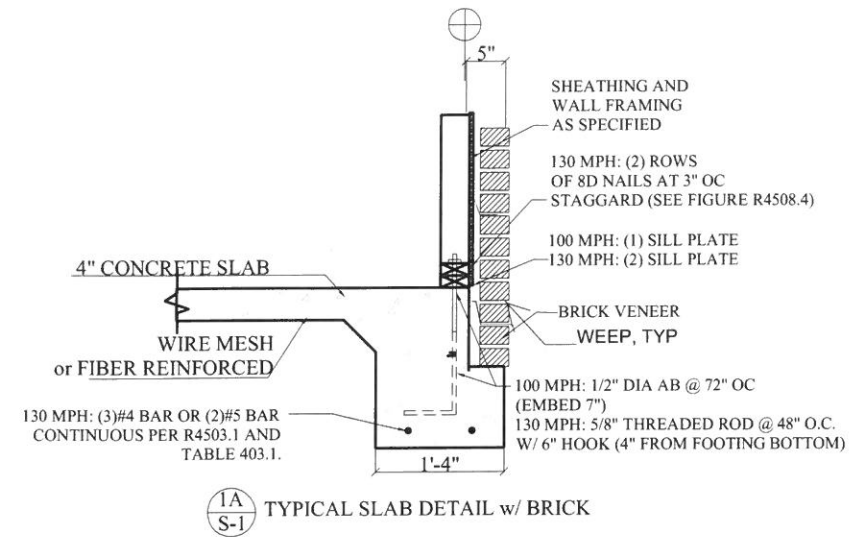
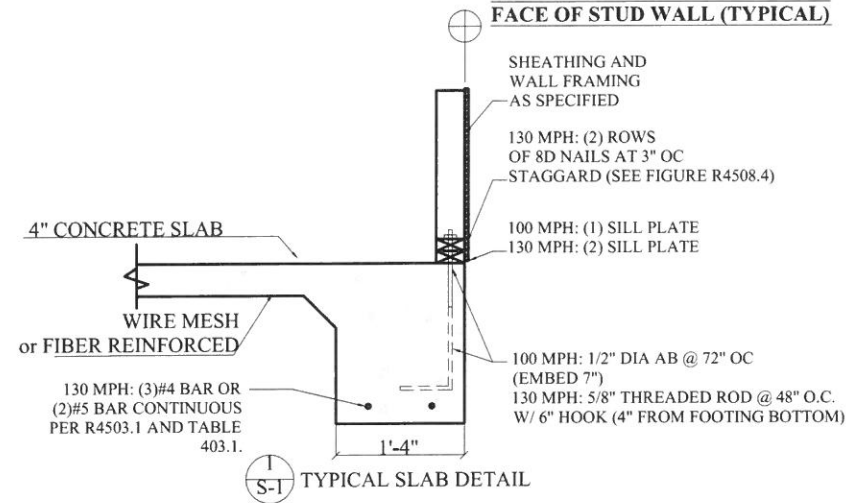
13) FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROWS OF 1/2" DIAMETER BOLTS (ASTM A307) WITH WASHERS PLACED UNDER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" O.C. (MAX), AND STAGGERED AT THE TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT 6" FROM EACH END.

14) BRICK LINTELS SHALL BE 3 1/2"x3 1/2"x1/4" STEEL ANGLE FOR UP TO 6'-0" SPAN AND 6"x4"x5/16" STEEL ANGLE WITH 6" LEG VERTICAL FOR SPANS UP TO 9'-0" (UNO).

15) 100 MPH: THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND WINDOWS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF. THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR WALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2012 EDITION SHALL BE AS FOLLOWS:

ROOF
45.4 PSF - 2.25:12 PITCH OR LESS
34.8 PSF - 2.25:12 TO 7:12 PITCH
21 PSF - 7:12 TO 12:12 PITCH
WALLS
24.1 PSF - WALLS

DIMENSION LINE ON EXTERIOR FACE OF STUD WALL (TYPICAL)



STRUCTURAL DESIGN BY:
SOUTHERN ENGINEERS, P.A.
3716 BENSON DR., RALEIGH, NC 27609
LICENSE: C-1287, PHONE: 919-878-1617

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PLAN NAME
Details

8/16/2013

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SAVVY HOMES
RESIDENTIAL LIVING

Company name:
Savvy Homes

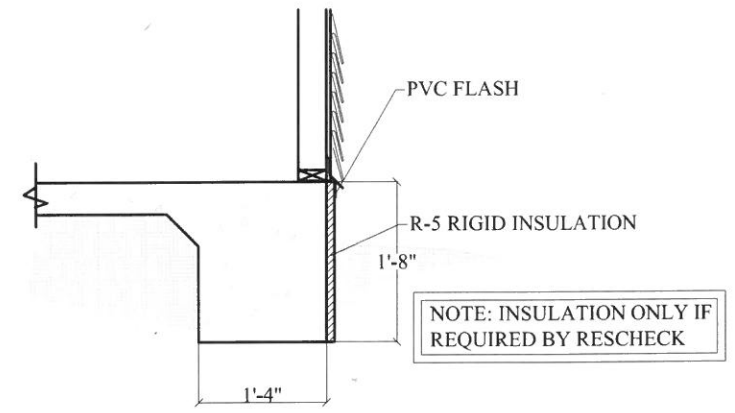
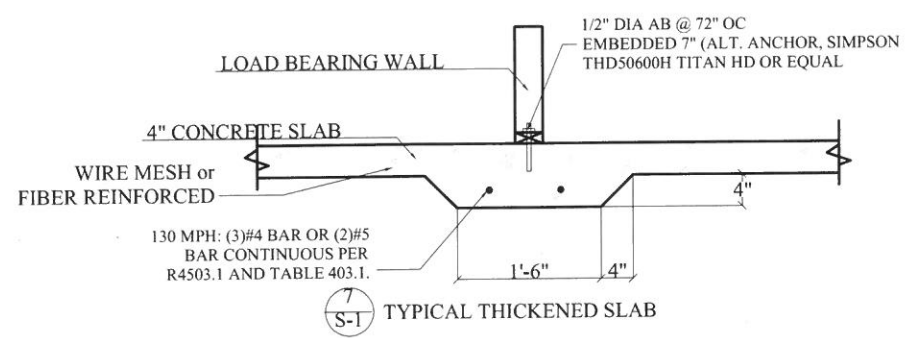
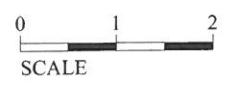
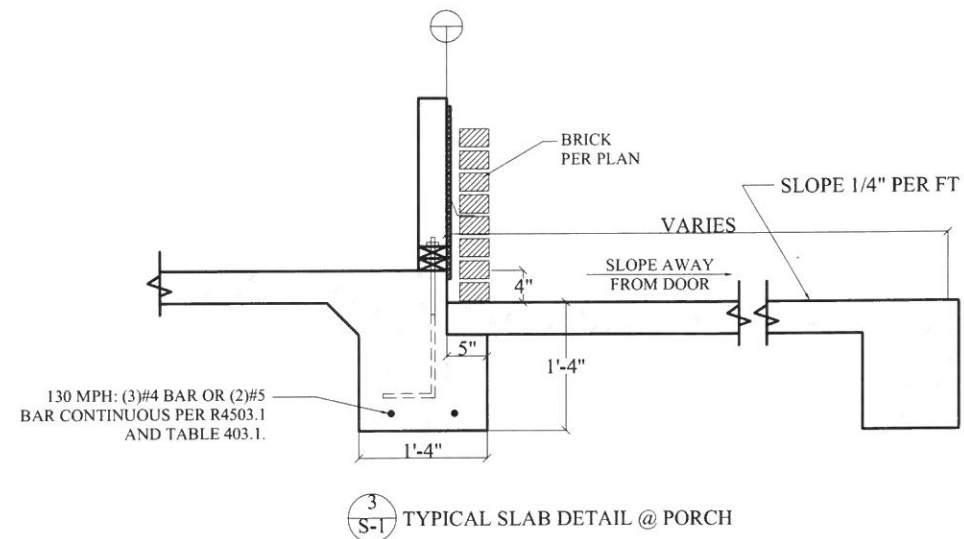
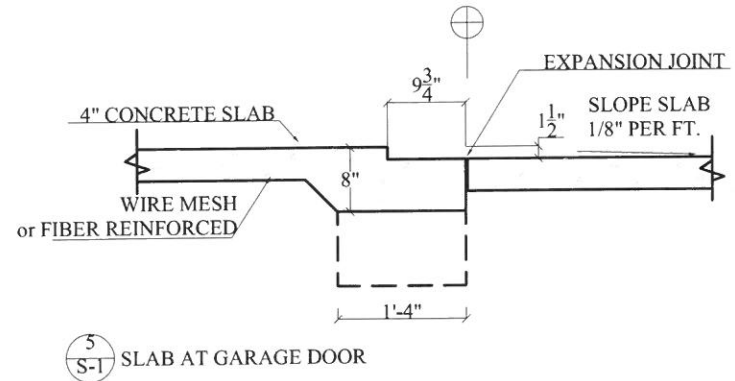
SAVVY HOMES
8025 GREENWOOD RD., SUITE 100
RALEIGH, NC 27613
PH 919-781-8104

Project name:
**Sections and Details
Mono Slab Details**

Date
8/16/2013

Sheet #
S-1.1

Savvy Holdings, LLC 2013 ©

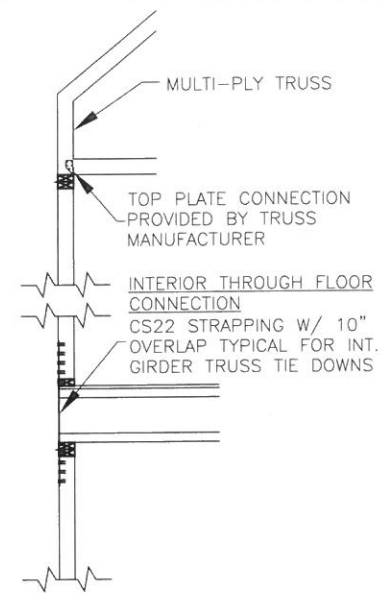


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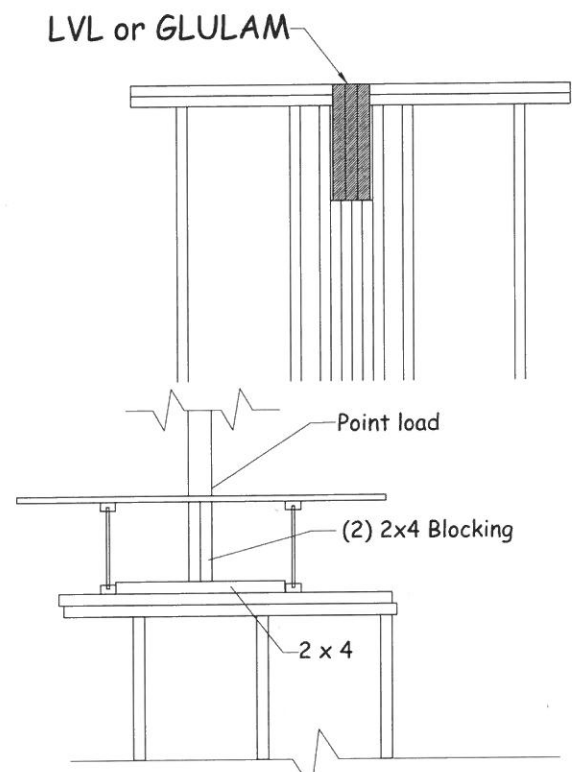


NOTE:
* INTERIOR TIE DOWNS ARE ONLY REQUIRED FOR GIRDER TRUSSES OR TRUSSES WITH MORE THAN 1 PLY.
* ALL TIE DOWNS ARE PROVIDED BY TRUSS MANUFACTURER FOR TOP PLATE CONNECTIONS.
* FOR EXTERIOR WALLS, CONTINUOUS SHEATHING METHOD SHALL PROVIDE REQUIRED UPLIFT TO FOUNDATION.
* ADDITIONAL FOUNDATION CONNECTIONS NOT REQUIRED IN WIND ZONES 110MPH OR LESS
* SEE S-3a FOR TIE DOWN SPECIFICATIONS IN WIND ZONES OF 120MPH OR MORE ONLY

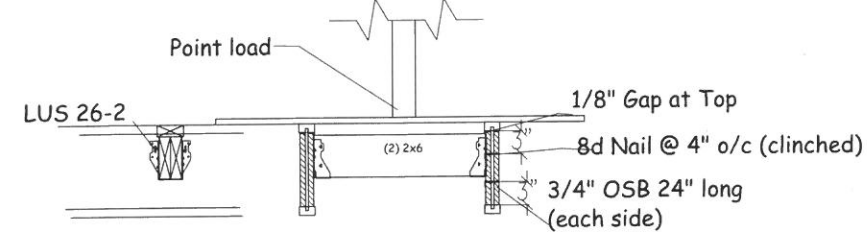


INTERIOR GIRDER TRUSS TIE DOWNS

TYPICAL MULTI-STUD BEAM POCKET - (IN GARAGE)

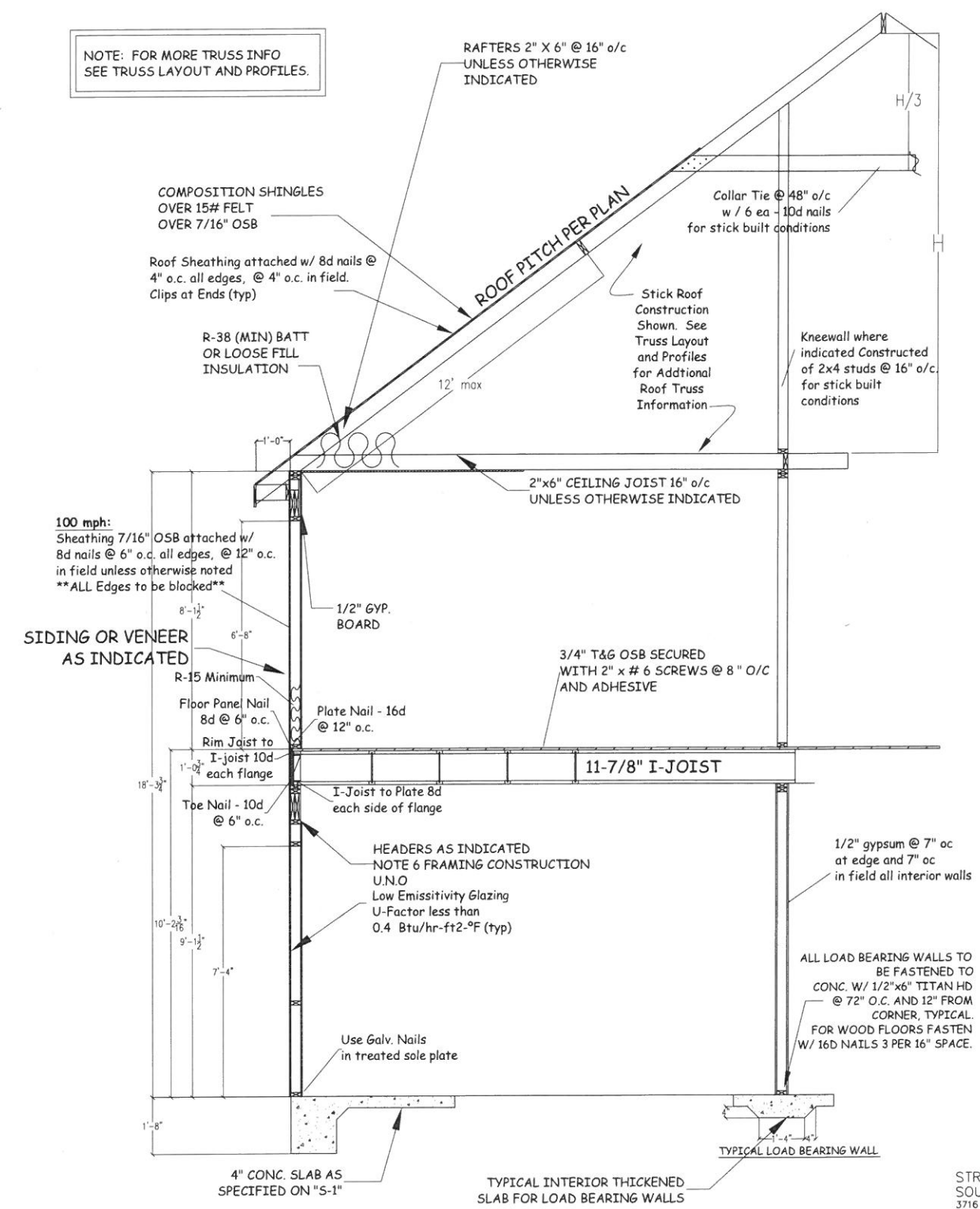


At End Span



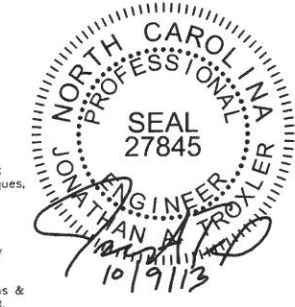
Intermediate Supports
BLOCKING DETAILS

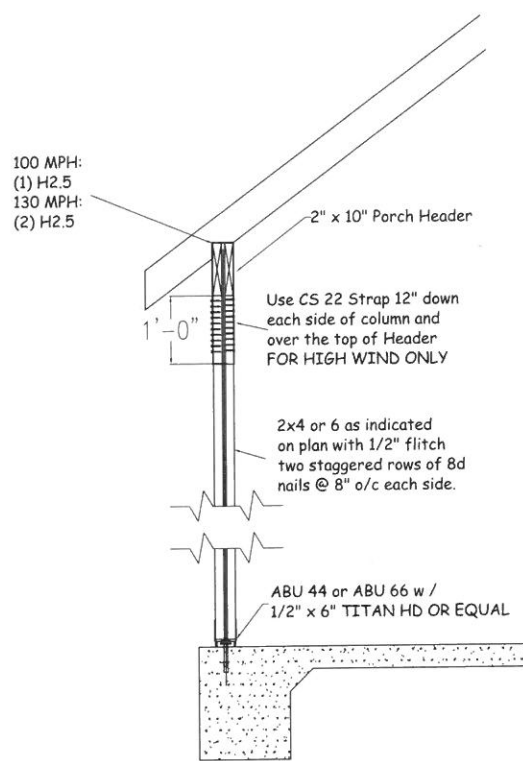
NOTE: FOR MORE TRUSS INFO SEE TRUSS LAYOUT AND PROFILES.



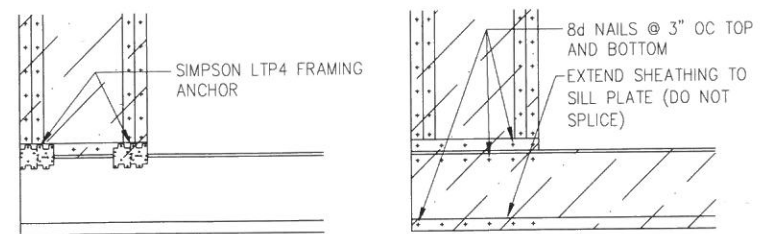
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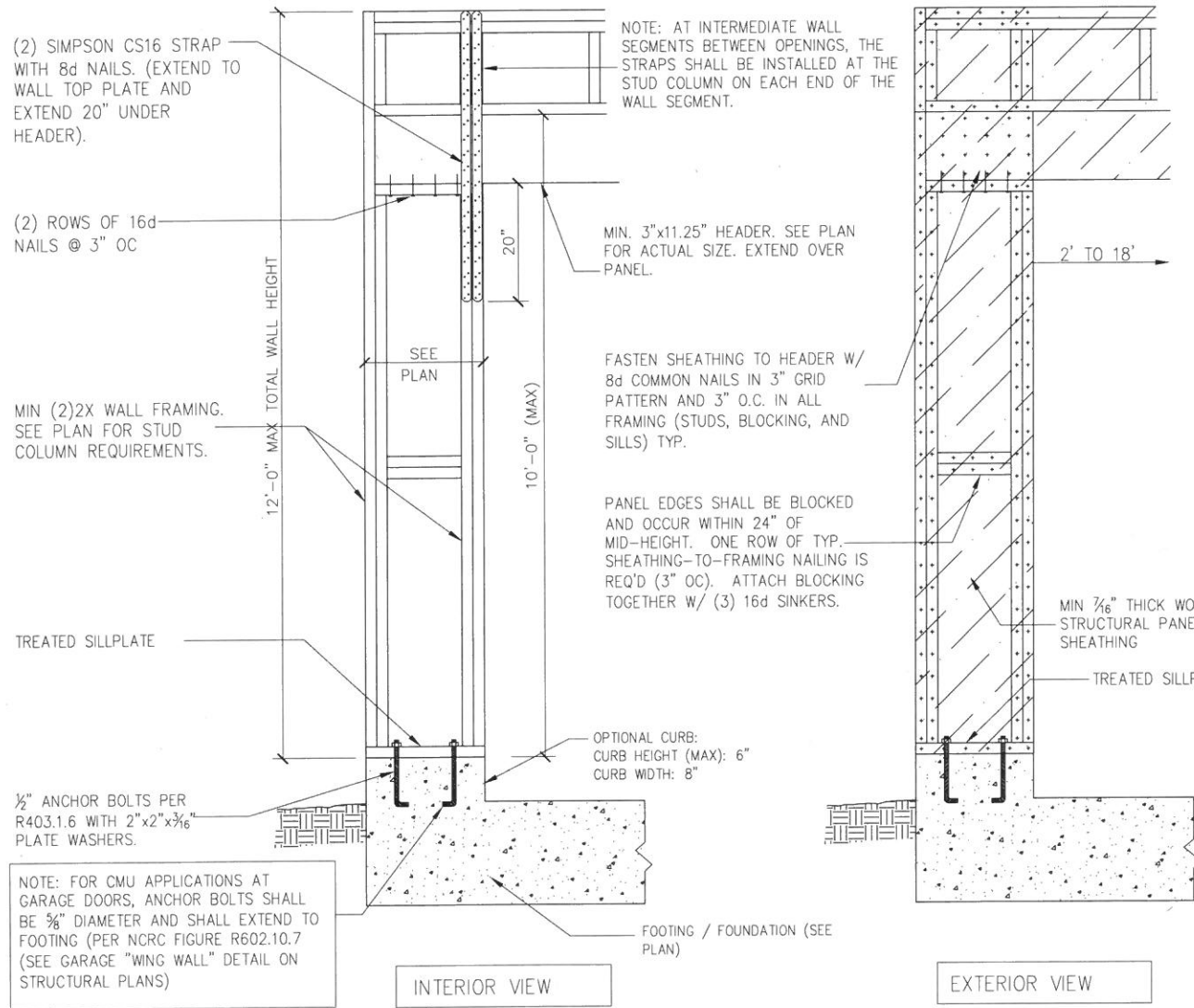




TYPICAL EXTERIOR COLUMN TIE DOWN

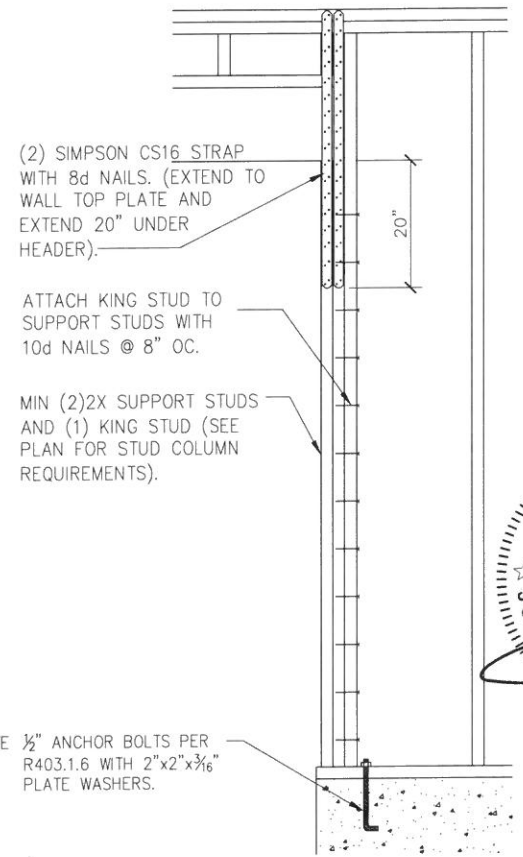


FRAMING ANCHOR OPTION
WSP OVERLAP OPTION
CS-PF - OVER WOOD FLOOR

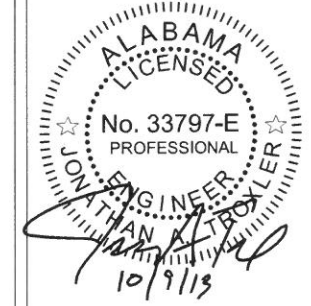


INTERIOR VIEW
EXTERIOR VIEW

A CS-PF: CONTINUOUS PORTAL FRAME CONSTRUCTION
DETAIL AND APPLICATION BASED ON NCRF FIGURE R602.10.4.1.1



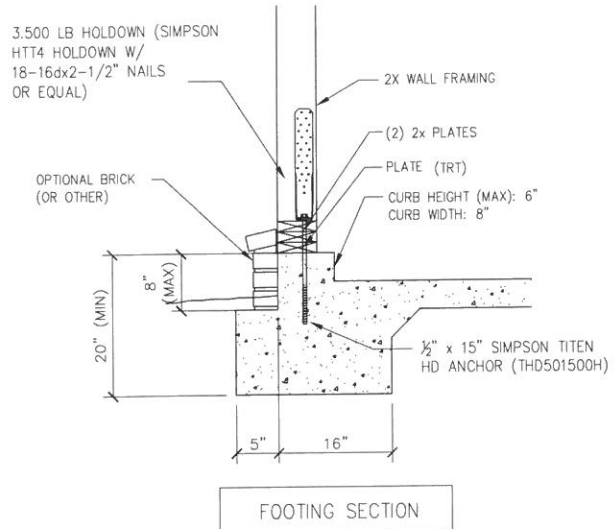
B END CONDITION DETAIL
(FOR USE WITH SINGLE CS-PF CONDITION)
DETAIL AND APPLICATION BASED ON NCRF FIGURE R602.10.1 - PORTAL FRAME CONSTRUCTION



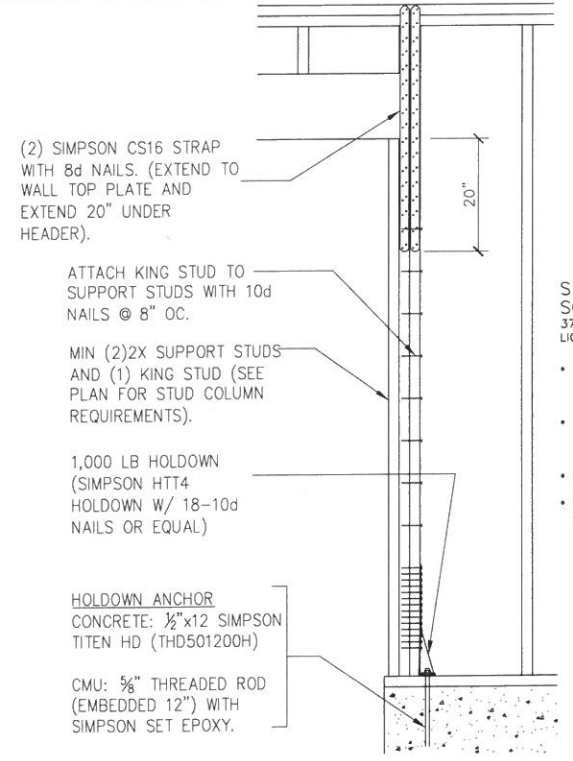
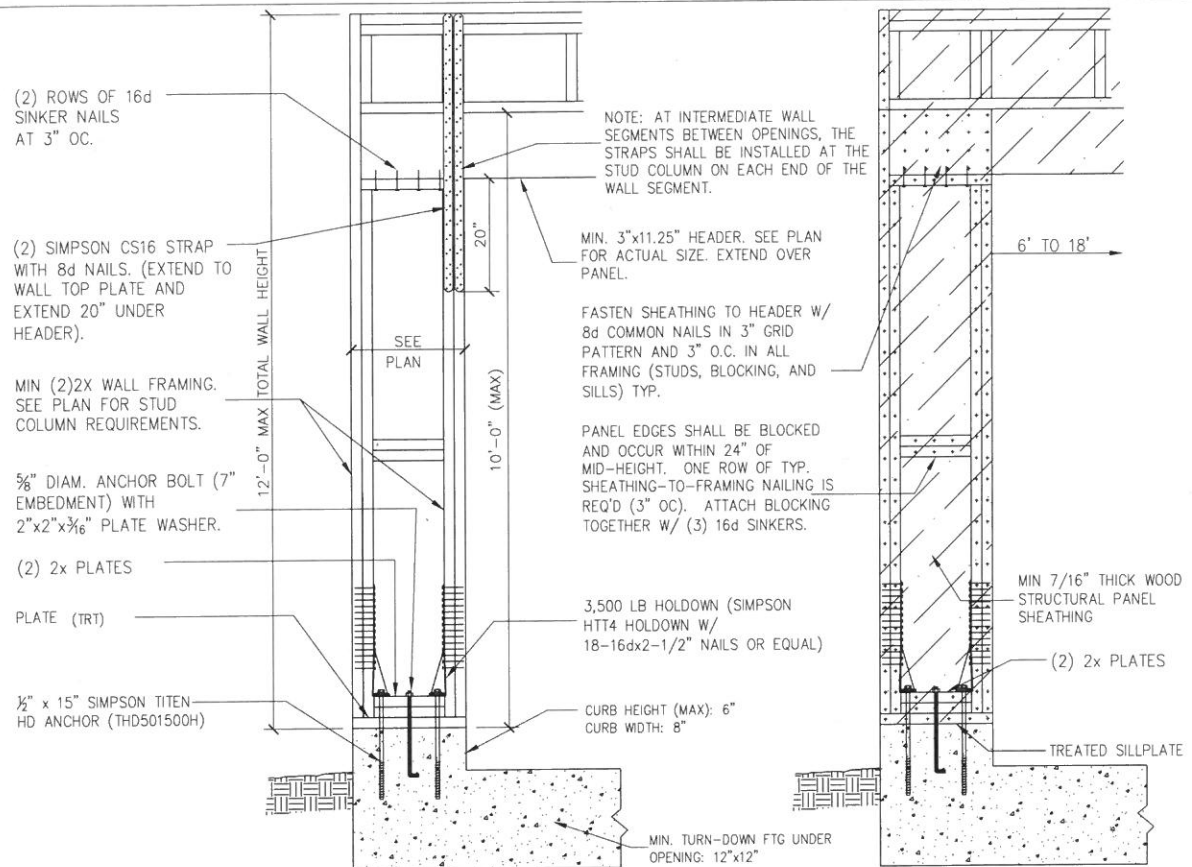
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Savvy Holdings, LLC 2013



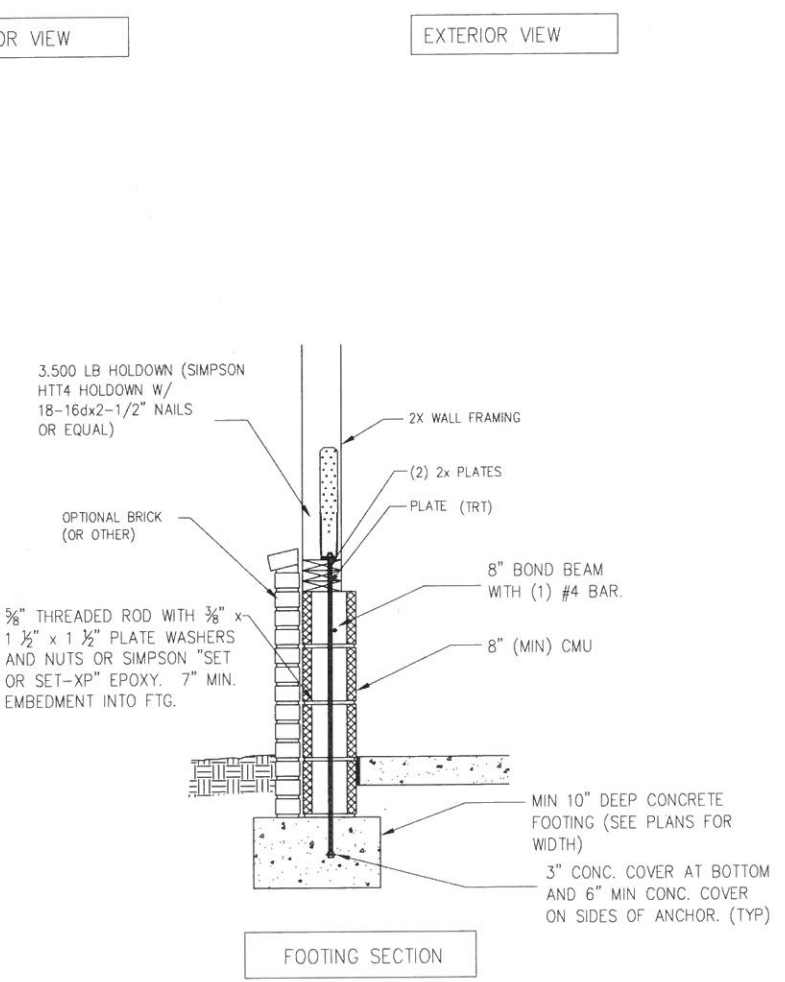
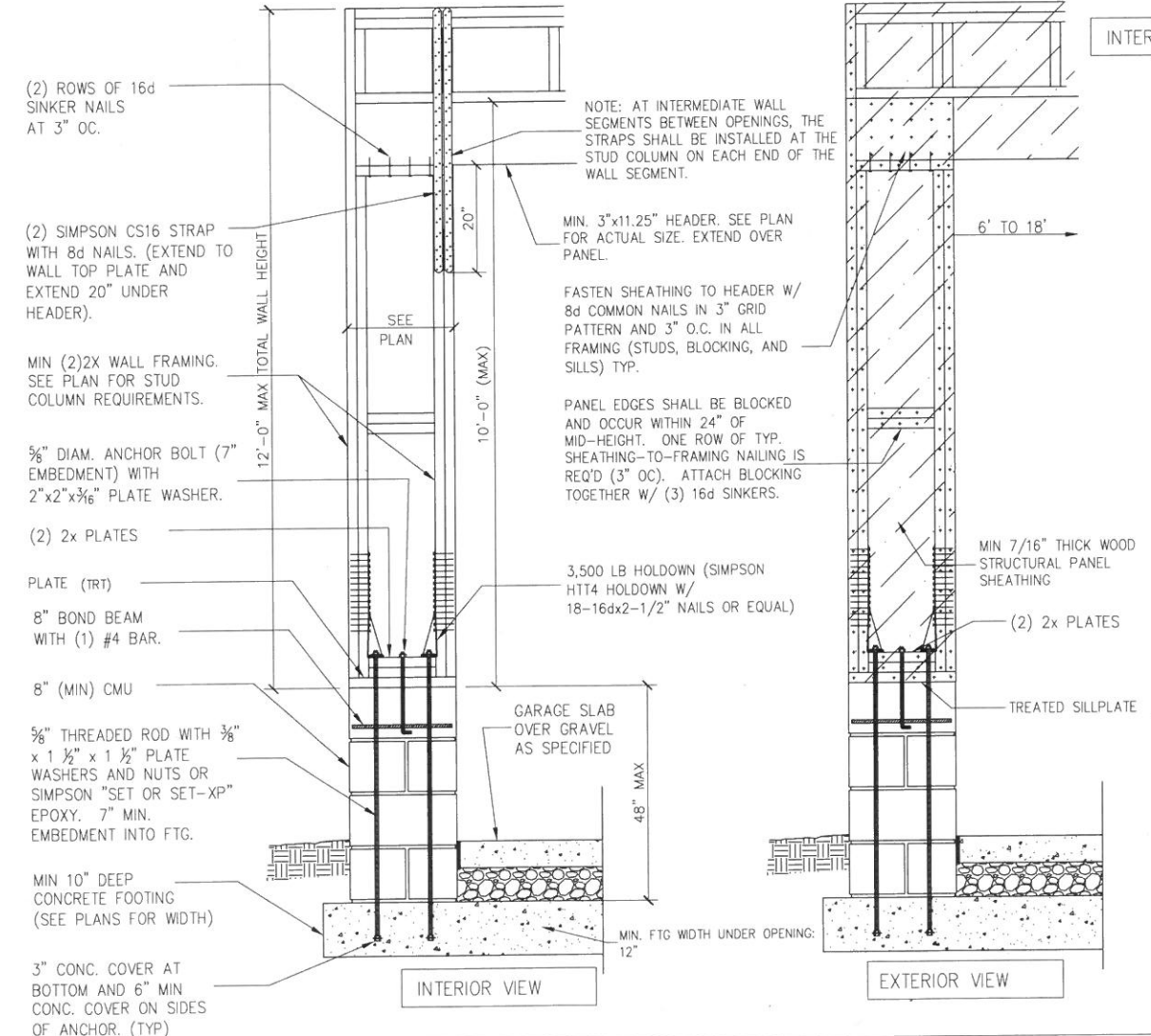
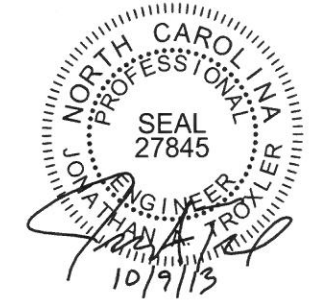
(C) APA PORTAL FRAME W/ HOLD-DOWNS
 DETAIL AND APPLICATION BASED ON APA TT-100E WITH USE OF TABLE 1 FOR APA PORTAL FRAME WITH HOLD-DOWN CAPACITIES.



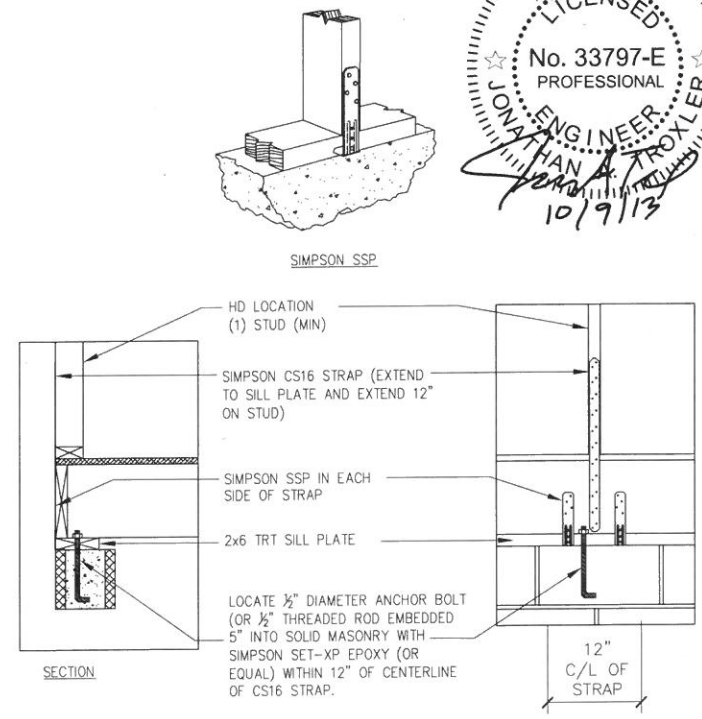
(D) END CONDITION DETAIL
 (FOR USE WITH SINGLE APA PORTAL FRAME CONDITION)
 DETAIL AND APPLICATION BASED ON APA TT-100E WITH USE OF TABLE 1 FOR APA PORTAL FRAME WITH HOLD-DOWN CAPACITIES.

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 3716 BENSON DR., RALEIGH, NC 27609
 LICENSE: C-1287, PHONE: 919-878-1617

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(E) APA PORTAL FRAME W/ HOLD-DOWNS
 DETAIL AND APPLICATION BASED ON APA TT-100E WITH USE OF TABLE 1 FOR APA PORTAL FRAME WITH HOLD-DOWN CAPACITIES.



(F) 'HD' HOLD-DOWN DETAIL
 (OVER WOOD FLOOR)

NOTE: ALTERNATE HD HOLD-DOWN DEVICES OR SYSTEMS MAY BE USED TO MEET THE CODE REQUIRED 800 LB CAPACITY IN LIEU OF THE ABOVE DETAIL.