

**Trenco**  
818 Soundside Rd  
Edenton, NC 27932

Re: GUYV1108-1  
Concrete Supply Office

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Peak Truss Builders, LLC.  
Pages or sheets covered by this seal: E12515675 thru E12515695

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844

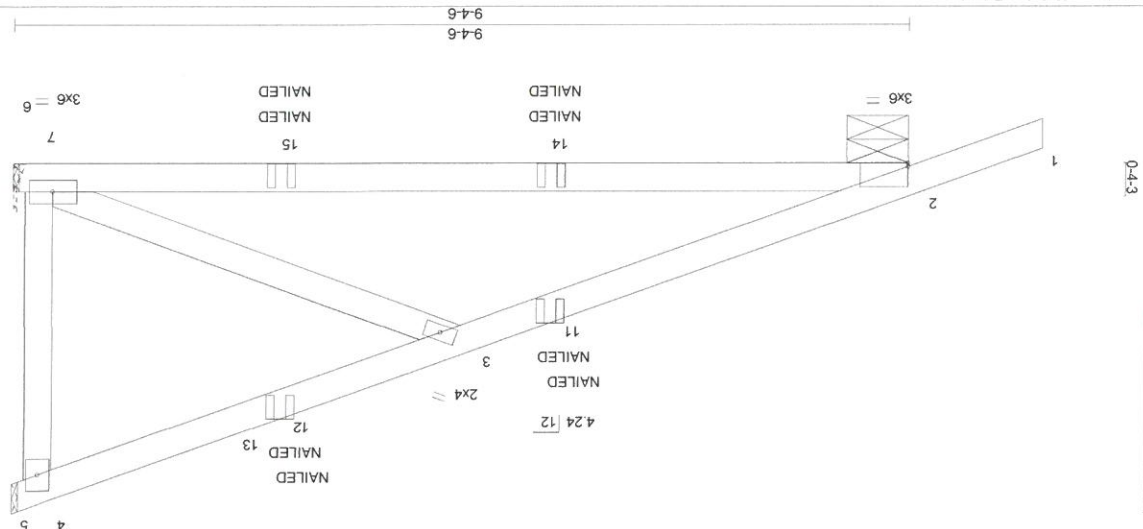


December 14, 2018  
Eric Gilbert, Eric

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for the reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANS/TP-1, Chapter 2.

Job	Truss	H1	Diagonal Hip Girder	Qty	4	Ply	Concrete Supply Office	Job Reference (optional)	8 220 S Nov 16 2018 MITTEK Industries, Inc. Thu Dec 13 14:19:50 2018 Page 1
GUVY1108-1								ID: 8mFASAZ272mQ6cN12K374yLl6i-9w3d1r1j5sQzVzVqzIda6LRSIO_yw_pV08BKy9MNO	

Peak Truss Builders, LLC, New Hill, NC - 27562, 4-10-12 4-10-12 4-5-10 9-4-6 1-5-0 4-10-12 4-10-12 4-5-10 9-4-6



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d
TCLL (roof) 20.0	2-0-0	TC 0.84	Vert(LL) -0.15	7-10	>746	240
Snow (P/Pg) 7.7/11.0	Plate Gnp DOL 1.15	BC 0.57	Vert(TL) -0.39	7-10	>279	180
TCLL (floor) 7.7/11.0	Lumber DOL 1.15	WB 0.25	Horz(TL) 0.01	7	n/a	n/a
BCLL 0.0	Rep Stress Incr NO	Mathx-MS				
BCDL 10.0	Code IBC2009/TP12007					

LUMBER-	BRACING-
TOP CHORD 2x4 SP No. 1	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No. 3	Rigid ceiling directly applied or 8-8-13 oc bracing.

**REACTIONS.** (lb/size) 2=35/0-7-12, 7=334/Mechanical  
 Max Horiz 2=290(LC 11)  
 Max Uplift 2=369(LC 12), 7=310(LC 9)  
 Max Grav 2=500(LC 2), 7=469(LC 2)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=64/1450  
 BOT CHORD 2-7=507/601  
 WEBS 3-7=576/507

**NOTES-**  
 (1) Wind: ASCE 7-05; 130mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate gnp DOL=1.60  
 (2) TCLL: ASCE 7-05; P=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp; Cf=1.10  
 (3) Unbalanced snow loads have been considered for this design.  
 (4) This truss has been designed for greater or min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (7) \* This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (8) Refer to girder(s) for truss to truss connections.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 2 and 310 lb uplift at joint 7.  
 (10) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.  
 (11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard  
 (1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (psf)  
 Vert: 1-4=-35, 4-5=-35, 6-8=-20  
 Concentrated Loads (lb)  
 Vert: 12=49(F=-24, B=-24) 14=5(F=-3, B=-3) 15=46(F=-23, B=-23)

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE ML-1473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MITTEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI-1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road  
 Edenton, NC 27932  
**TRENCO**  
 ENGINEERING BY  
 A PROFESSIONAL ENGINEER



December 14, 2018

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- NOTES:**
- Wind: ASCE 7-05: 130mph; TCDF=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp. C; Enclosed; MWFRS (all heights) and C-C Corner(3)-1-5-0 to 2-9-15, Exterior(2) 2-9-15 to 4-2-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
  - TCLL: ASCE 7-05; P=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=10.0 psf (ground snow); P=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp. C; Partially Exp.; Ct=1.10
  - Unbalanced snow loads have been considered for this design.
  - This truss has been designed for greater of 1.0 times flat roof live load of 12.0 psf or 1.0 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.
  - This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
  - This truss system has been designed for a 1.0 psf bottom chord live load nonconcurrent with any other live loads.
  - This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - Refer to girder(s) for truss to truss connections.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 6 and 235 lb uplift at joint 2.

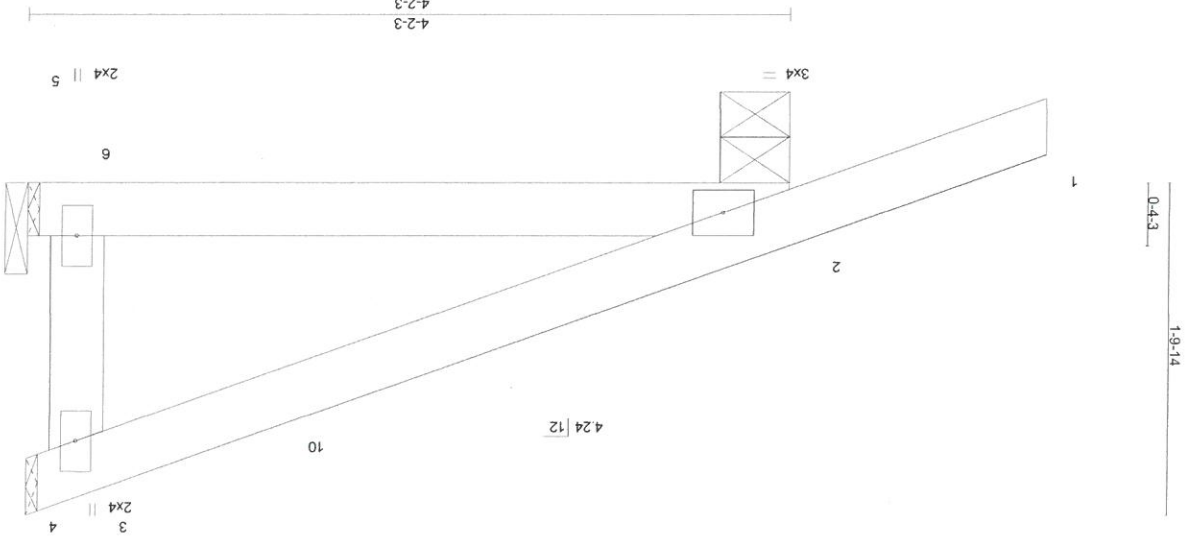
**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**REACTIONS.** (lb/size) 6=115/Mechanical, 2=167/0-4-9  
 Max HORIZ 2=138(LC 13)  
 Max Uplift 6=75(LC 14), 2=235(LC 14)  
 Max GRAV 6=152(LC 2), 2=256(LC 2)

**LUMBER-** TOP CHORD 2x4 SP No.1  
**WEBS** BOT CHORD 2x4 SP No.1  
**TOP CHORD** BOT CHORD 2x4 SP No.3  
 Structural wood sheathing directly applied or 4-2-3 oc purlins, except end verticals  
 Rigid ceiling directly applied or 1-0-0 oc bracing.

LOADING (psf)		SPACING-		CSI.		DEFLL		BRACING-	
TCLL (roof)	Snow (P/Pg)	Plate Gnp DOL	Lumber DOL	WB	BC	in (loc)	in (occ)	VERT (TL)	VERT (TL)
20.0	7.7/10.0	1.15	1.15	0.00	0.19	0.02	0.02	0.00	0.00

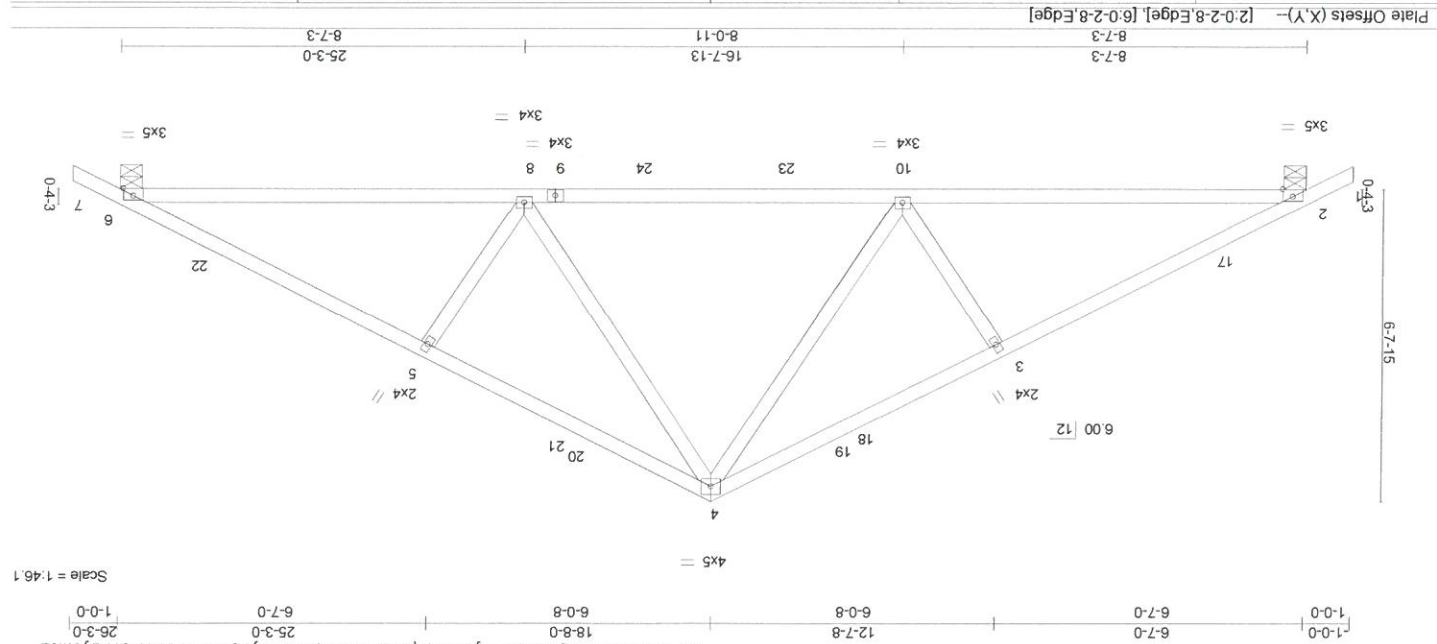
Weight: 17 lb FT = 20%  
 PATES MT20 244/190 GRIP



Peak Truss Builders, LLC, New Hill, NC - 27562, 8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:51 2018 Page 1  
 ID: 8mAFsAZ2277mQ6cNI2K374yL6i-e6RRr5ZU1Dj22Y3Y5\_PqjOgFu\_7J3DTeajny9MOM

Job	Truss	Truss Type	Qty	Ply	Concrete Supply Office
GVY1108-1	H2	Diagonal Hip Girder	2	1	E12515676

Job	Truss	T1	Common	Qty	4	Ply	Concrete Supply Office	E12515677
Peak Truss Builders, LLC, New Hill, NC - 27562, 8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:52 2018 Page 1 ID: 8mAfSAZ727m6cN112k374yL6i-6j7p2RZHFKLaTc64dfCDy2GvF9ssZDR70FDy9MDL 1-0-0 6-7-0 12-7-8 6-0-8 6-0-8 18-8-0 6-0-8 25-3-0 6-7-0 1-0-0 26-3-0 1-0-0 Scale = 1:46.1								

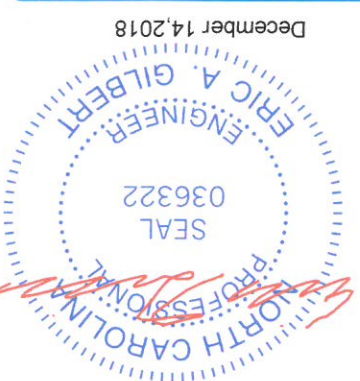


LOADING (psf)	TCL (roof)	Snow (P/FG)	TCDL	BCLL	BCLD
20.0	7.7/10.0	10.0	10.0	0.0	10.0
<b>SPACING-</b> 2-0-0 Plate Gnp DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2009/TP12007					
<b>CSI,</b> TC 0.44 BC 0.47 WB 0.37 Matrix-MS					
<b>DEFL.</b> in (loc) Vert(LL) -0.15 8-10 >999 240 Vert(TL) -0.31 8-10 >992 180 Horz(TL) 0.05 6 n/a n/a					
<b>LUMBER-</b> TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 Rigid ceiling wood sheathing directly applied or 7-0-12 oc bracing.					

**REACTIONS.** (lb/size) 2=735/0-5-8, 6=735/0-5-8  
 Max Horz 2=267(LC 13)  
 Max Uplift 2=651(LC 14), 6=651(LC 14)  
 Max Gray 2=1070(LC 2), 6=1070(LC 2)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1684/1068, 3-4=-1488/1057, 4-5=-1488/1057, 5-6=-1684/1068  
 BOT CHORD 2-10=-756/1438, 8-10=-364/961, 6-8=-790/1438  
 WEBS 4-8=-340/555, 5-8=-360/455, 4-10=-340/555, 3-10=-360/455

**NOTES-**  
 (1) Unbalanced roof live loads have been considered for this design.  
 (2) Wind: ACE7 7-05; 130mph; TCDL=6.0psf; BCLD=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extent(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-7-8, Extent(2) 12-7-8 to 15-7-8, Interior(1) 15-7-8 to 26-3-0 zone;  
 cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown;  
 Lumber DOL=1.60 plate gnp DOL=1.60  
 (3) TCLL: ASCE 7-05; P=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; P<sub>g</sub>=10.0 psf (ground snow); P<sub>f</sub>=7.7 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp C; Partially Exp.; C<sub>t</sub>=1.10  
 (4) Unbalanced snow loads have been considered for this design.  
 (5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (8) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 2 and 651 lb uplift at joint 6.

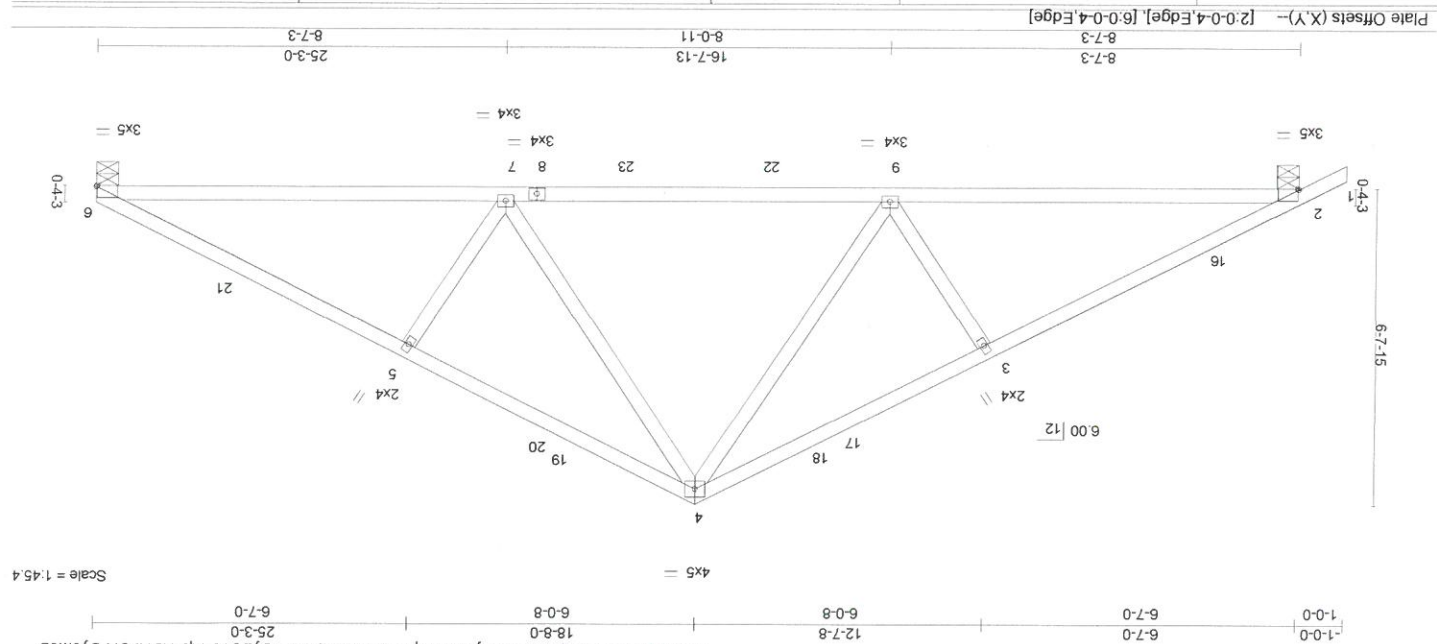


December 14, 2018

**TRENCO**  
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A FEDERAL AFFILIATE  
818 Soundside Road  
Edenton, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MR-7473 rev. 10/02/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see  
 ANSII/TPI1 Quality Criteria, DSB-89 and BCSI Building Component  
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	T1A	Common	Qty	Ply	Concrete Supply Office
GUVY1108-1				1		
Job Reference (optional)						
8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:52 2018 Page 1						
ID: 8mAFsAZ7277m06cN112k374YL6i-6Jp2RZHFKlaFC64dFcdY2Gv5f9qssXDR7O7FDy9MOL						
12-7-8 6-0-8 6-0-8 6-7-0 6-7-0 6-7-0 12-7-8 6-0-8 6-0-8 18-8-0 25-3-0 6-7-0						



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	L/d	L/d
TCLL (roof) 20.0	2-0-0	TC 0.44	Vert(LL) -0.15	7-9	>999	240
Snow (P/Pg) 7.7/10.0	Plate Grp DOL 1.15	BC 0.47	Vert(TL) -0.30	7-9	>995	180
TCDL 10.0	Rep Stress Incr YES	WB 0.37	Horz(TL) 0.05	6	n/a	n/a
BCLL 0.0	Code IBC2009/TP12007	Matrix-MS				
BCLL 10.0						

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD
BOT CHORD 2x4 SP No.1	BOT CHORD
WEBS 2x4 SP No.3	

**REACTIONS.** (lb/size) 6=699/0-5-8, 2=735/0-5-8  
 Max Horz 2=265(C 13)  
 Max Uplift 6=559(LC 14), 2=651(LC 14)  
 Max Grav 6=1010(LC 2), 2=1070(LC 2)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1685/1068, 3-4=-1489/1057, 4-5=-1491/1074, 5-6=-1687/1086  
 BOT CHORD 2-9=-824/1439, 7-9=-398/962, 6-7=-820/1442  
 WEBS 4-7=-341/558, 5-7=-361/456, 4-9=-339/555, 3-9=-360/456

**NOTES-**  
 (1) Unbalanced roof live loads have been considered for this design.  
 (2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extension(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-7-8, Exterior(2) 12-7-8 to 15-7-8, Interior(1) 15-7-8 to 25-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grp DOL=1.80  
 (3) TCLL: ASCE 7-05; P=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15 (ground snow); P=7.7 psf (flat roof snow); Lumber DOL=1.15; Category II; Exp C; Partially Exp.; Ct=1.10  
 (4) Unbalanced snow loads have been considered for this design.  
 (5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 6 and 651 lb uplift at joint 2.

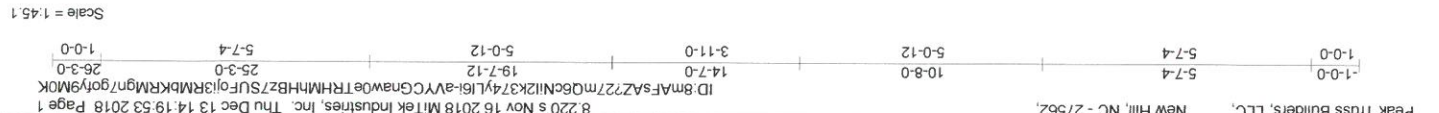


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**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and truss systems, see ANSITM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss Type	Truss	Hip	Qty	PLY	Concrete Supply Office	Job Reference (Optional)
GUVV1108-1	Truss	T18	Hip	1			8 220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:53 2018 Page 1
							ID: 8mAFsAZ277mQ6cNI2K374yL6i-avYCgNawoT8RHMHbZ7SUF0j3RMBKRMgn7gofy9MOK
							1-0-0 5-7-4 10-8-0 14-7-0 3-11-0 19-7-12 5-0-12 5-7-4 25-3-0 1-0-0
							5-6-8 5-6-8 5-6-8 5-6-8 5-6-8 5-6-8 5-6-8



LOADING (psf)	SPACING	CSL	DEFL.	in (loc)	L/d	L/d	PLATES	GRIP	Weight: 123 lb	FT = 20%
TCLL (roof) 20.0	2-0-0	0.39	0.06	7	n/a	n/a	MT20	244/190		
Snow (P/Fg) 12.7/10.0	1.15	0.70	-0.54	10-17	>999	180				
TCDL 10.0	1.15	0.39	-0.20	10-17	>999	240				
Rep Stress Incr YES										
Code IBC2009/FP12007										
Matrix-MS										
WB 0.27										
Horz(TL) 0.06										
Vert(TL) -0.54										
Vert(LL) -0.20										

RECTIONS. (lb/size)	WEBS	TOP CHORD	BOT CHORD	BRACING-	TOP CHORD	BOT CHORD
2=754/0-5-8, 7=754/0-5-8	2x4 SP No.3	2x4 SP No.1	2x4 SP No.1	Structural wood sheathing directly applied or 4-9-0 oc purlins, except 2-0 oc purlins (5-9-8 max.); 4-5.	Rigid ceiling directly applied or 6-0-15 oc bracing.	
Max Horiz 2=226(LC 12)						
Max Uplift 2=651(LC 14), 7=651(LC 14)						
Max Grav 2=1070(LC 2), 7=1070(LC 2)						
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.						
TOP CHORD 2-3=-1679/1309, 3-4=-1338/1040, 4-5=-1144/1039, 5-6=-1338/1040, 6-7=-1679/1308						
BOT CHORD 2-11=-1015/1448, 10-11=-596/1143, 7-10=-1034/1448						
WEBS 3-11=-384/497, 4-11=-170/327, 5-10=-145/327, 6-10=-384/496						

**NOTES-**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05: 130mph; TCDD=6.0psf; BCDD=6.0psf; h=25ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extent(2) -1-0-0 to 2-0-0; Interior(1) 2-0-0 to 10-8-0; Exterior(2) 10-8-0 to 18-9-15; Interior(1) 18-9-15 to 26-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60

3) TCLL: ASCE 7-05: Pr=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=10.0 psf (ground snow); Ft=12.7 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp C; Partially Exp.; Ct=1.10; Lu=50-0-0; Min. flat roof snow load governs; Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

7) Provide adequate drainage to prevent water ponding.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 2 and 651 lb uplift at joint 7.

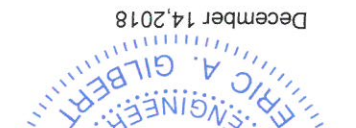
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

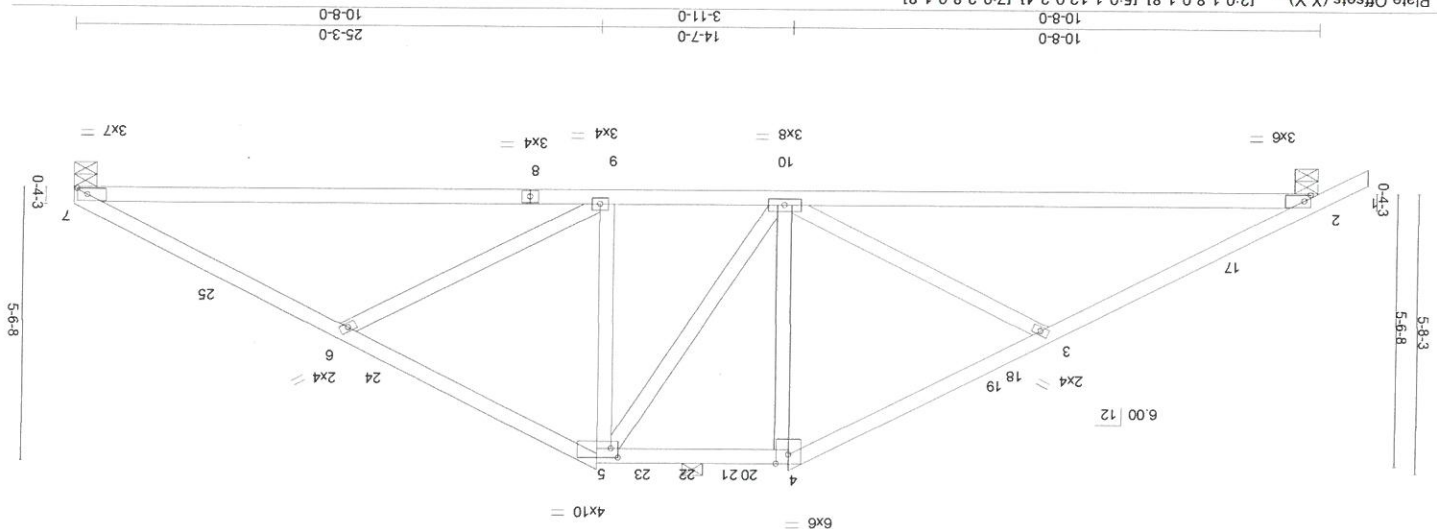
818 Soundside Road  
Edenton, NC 27932



December 14, 2018

**TRENCO**  
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A MITEK AFFILIATE

Job	Truss	T1C	Hip	Qty	1	Ply	Concrete Supply Office	E12515680
Job Reference (optional)								
8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:54 2018 Page 1								
ID: 8maFsaZ272mQ6cN12K374L6i2heaT7bYnybWVGtKgeh1TLuRSodkNvVRiEKsY9M0J								
1-0-0 5-7-4 5-0-12 3-11-0 14-7-0 5-0-12 5-7-4 1-0-0								



LOADING (psf)	SPACING	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	Weight: 122 lb FT = 20%
TCLL (roof) 20.0	2-0-0	0.39	0.06	7	n/a	n/a	MT20	244/190	
Snow (P/Bg) 12.7/10.0	1.15	0.70	-0.20	9-13	>999	240			
TCDL 10.0	1.15	0.70	-0.54	9-13	>558	180			
BCLL 0.0	YES	0.27	0.06	7	n/a	n/a			
BCDL 10.0	Code IBC2009/FP12007	Matrix-MS	Horz(TL)	0.06	7	n/a			

REACTIONS. (lb/size)	TOP CHORD	BOT CHORD
7=719/0-5-8, 2=755/0-5-8	2x4 SP No.1	2x4 SP No.3
Max Horiz 2=224(LC 13)		
Max Uplift 7=559(LC 14), 2=651(LC 14)		
Max Grav 7=1010(LC 2), 2=1070(LC 2)		

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1680/1315, 3-4=-1339/1047, 4-5=-1145/1045, 5-6=-1340/1043, 6-7=-1682/1310  
 BOT CHORD 2-10=-1083/1449, 9-10=-625/1145, 7-9=-1063/1452  
 WEBS 3-10=-384/496, 4-10=-170/327, 5-9=-146/329, 6-9=-386/498

**NOTES.**  
 (1) Unbalanced roof live loads have been considered for this design.  
 (2) Wind: ASCE 7-05: 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extent(2) 1-0-0 to 2-0-0, Interior(1) 10-8-0 to 18-9-15, Interior(1) 18-9-15 to 25-3-0 zone; cantilever left and right exposed; and vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 (3) TCLL: ASCE 7-05: Pr=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=10.0 psf (ground snow); Pf=12.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; C=1.10, Lu=50-0-0; Min. flat roof snow load governs; Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.  
 (4) Unbalanced snow loads have been considered for this design.  
 (5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (7) Provide adequate drainage to prevent water ponding.  
 (8) This truss has been designed for a 1.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (9) \* This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 7 and 651 lb uplift at joint 2.  
 (11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



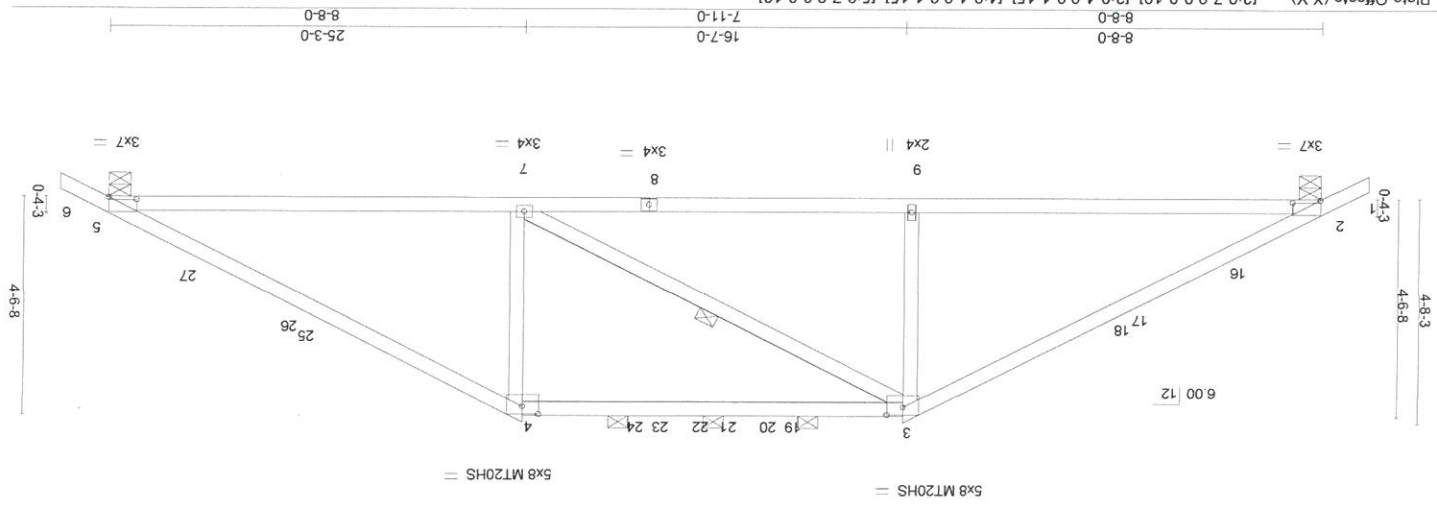
December 14, 2018

**TRENCO**  
 ENGINEERS BY  
 A HILTI COMPANY  
 818 Soundside Road  
 Eden, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-173 REV. 10/03/2015 BEFORE USE.**  
 Design shall be used only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of trusses and bracing systems. See ANS/ITP Quality Criteria, DSB-69 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and bracing systems. 218 N. Lee Street, Suite 312, Alexandria, VA 22314

Job	Truss	T1D	Hip	Qty	1	Concrete Supply Office	E12515681
Job Reference (optional)							
8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:55 2018 Page 1							
ID: 8mAfSAZ722mQ6cN12K374YL6i-WUgYhTCAVF9WgmT09wagu_qsbT3H8T5cnsY9M01							
1-0-0 8-8-0 16-7-0 7-11-0 25-3-0 8-8-0 1-0-0 26-3-0							

Peak Truss Builders, LLC, New Hill, NC - 27562, Scale = 1/45.1



LOADING (psf)	TCL (roof)	Snow (Pf/Pg)	Plate Gnp DOL	SPACING	2.0-0	CSI	DEFL.	DEFL.	in (loc)	L/d	L/d	PLATES	GRIP	Weight: 107 lb	FT = 20%
20.0	12.7/10.0	12.7/10.0	1.15	2.0-0	0.73	0.73	0.11	7-15	>999	240	240	MT20	244/190	187/143	
10.0	10.0	10.0	1.15	Lumber DOL	0.47	0.47	-0.22	7-9	>999	180	180	MT20HS	187/143		
0.0	0.0	0.0	1.15	Rep Stress Incr	0.13	0.13	0.05	5	n/a	n/a	n/a				
10.0	10.0	10.0	1.15	Code IBC2009/TFP12007	Matrix-MS										

LUMBER-	TOP CHORD	2x4 SP No. 1	*Except	BRACING-	TOP CHORD	Structural wood sheathing directly applied or 3-10-3 cc purlins,	except	BRACING-	BOT CHORD	2x4 SP No. 1	WEBS	1 Row at midpt	3-7
REACTIONS.	(b)/size	2=774/0-5-8, 5=774/0-5-8											
	Max Horiz	2=186(LC 13)											
	Max Uplift	2=651(LC 14), 5=651(LC 14)											
	Max Grav	2=1070(LC 2), 5=1070(LC 2)											
FORCES.	(b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.												
	TOP CHORD	2-3=-1557/1034, 3-4=-1289/1079, 4-5=-1557/1034											
	BOT CHORD	2-9=-681/1294, 7-9=-684/1288, 5-7=-699/1295											
	WEBS	3-9=0/331, 4-7=0/332											

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 130mph; TCL=0.0psf; BCDL=0.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extent(2) - 1.0-0 to 2.0-0, Interior(1) 2.0-0 to 8-8-0, Extent(2) 8-8-0 to 12-10-15, Interior(1) 12-10-15 to 16-7-0, Extent(2) 16-7-0 to 20-9-15, Interior(1) 20-9-15 to 26-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
- TCL: ASCE 7-05; P=20.0 psf roof live load; Lumber DOL=1.15 Plate DOL=1.15; Pg=10.0 psf (ground snow); Pf=12.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; C=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- All plates are MT20 plates unless otherwise indicated.
- Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 2 and 651 lb uplift at joint 5.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



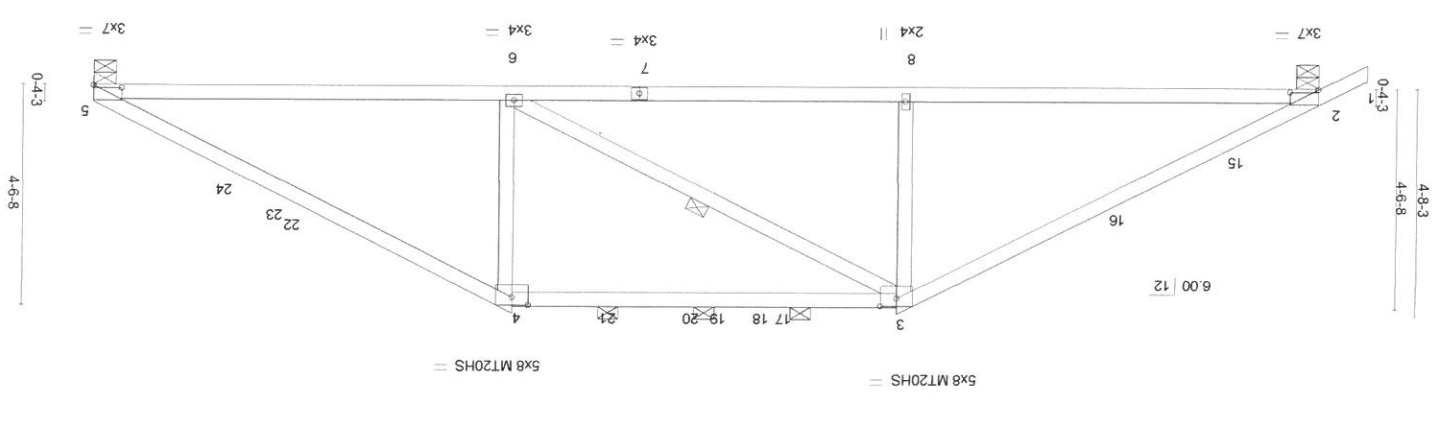
December 14, 2018

**TRENCO**  
ENGINEERS BY  
A PUBLIC COMPANY  
818 Soundside Road  
Edenton, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MILL-747 rev. 10/02/2015 BEFORE USE.**  
Design shall be used only with MITEK connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIP/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	T1E	Hip	1	Qty	Ply	Concrete Supply Office
GUVY1108-1				1			Job Reference (optional)
Peak Truss Builders, LLC, New Hill, NC - 27562, 8220 s Nov 16 2018 MTRK Industries, Inc. Thu Dec 13 14:19:56 2018 Page 1							
ID: 8mAFSAZ7272mQ6cN12K374YL6l6_4EKuocoiZr78qGrs96uQ9ZGwjoJomIMLLO_Y9MOH							
1-0-0 1-0-0 8-8-0 8-8-0 16-7-0 7-1-0 25-3-0 8-8-0							



LOADING (psf)	TCL (roof)	Snow (P/Fg)	Plate Gnp DOL	Lumber DOL	Rep Stress Incr	YES	WB	0.13	Matrix-MS	CSL	0.73	TC	0.47	BC	0.47	Ver(TL)	0.11	6-11	>999	240	L/D	in (loc)	ld/df	5	n/a	n/a	Weight: 105 lb	FT = 20%
20.0	12.7/10.0	12.7/10.0	1.15	1.15	1.15	1.15	0.47	0.13			0.73	0.47	0.47	0.47	0.47	0.11	6-11	>999	240	L/D	in (loc)	ld/df	5	n/a	n/a	MT20	244/190	
																0.05	5	n/a	180	MT20HS	187/143							

LUMBER-	TOP CHORD	2x4 SP No. 1	3-4: 2x4 SP DSS	2x4 SP No. 3	WEBS	2x4 SP No. 3	
BRACING- <td>TOP CHORD</td> <td>Structural wood sheathing directly applied or 3-9-10 oc putlins, except</td> <td></td> <td></td> <td>BOT CHORD</td> <td>Rigid ceiling directly applied or 7-0-10 oc bracing.</td>	TOP CHORD	Structural wood sheathing directly applied or 3-9-10 oc putlins, except			BOT CHORD	Rigid ceiling directly applied or 7-0-10 oc bracing.	
REACTIONS,	(lb/size)	5=139/0-5-8, 2=775/0-5-8	Max Horiz 2=185(LC 13)	Max Uplift 5=559(LC 14), 2=651(LC 14)	Max Grav 5=1009(LC 2), 2=1071(LC 2)		
FORCES,	(lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.	TOP CHORD	2-3=-1558/1039, 3-4=-1291/1081, 4-5=-1560/1036	BOT CHORD	2-8=-749/1295, 6-8=-752/1289, 5-6=-729/1297	WEBS	3-8=0/331, 4-6=0/332

**NOTES-**

1) Unbalanced roof live loads have been considered for this design.  
 2) Wind: ASCE 7-05; 130mph; TCDL=0.0psf; BCDL=0.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Extension(1) 2-0-0 to 8-8-0, Extension(2) 8-8-0 to 12-10-15, Interior(1) 12-10-15 to 16-7-0, Extension(2) 16-7-0 to 20-9-15, Interior(1) 20-9-15 to 25-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 3) TLL: ASCE 7-05; P=20.0 psf roof live load; Lumber DOL=1.15 Plate DOL=1.15; Pg=10.0 psf (ground snow); Pf=12.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp; C=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.  
 4) Unbalanced snow loads have been considered for this design.  
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 7) Provide adequate drainage to prevent water ponding.  
 8) All plates are MTT20 plates unless otherwise indicated.  
 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 5 and 651 lb uplift at joint 2.  
 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



December 14, 2018

**TRENCO**  
ENGINEERING BY  
A HETEL AFFILIATE  
818 Soundside Road  
Edenton, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.**  
 Design shall be used only with MITEK® connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIPPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	T1FGRD	Hip Girder	Qty	1	Concrete Supply Office	E12515683
8 220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:19:59 2018 Page 1							
Job Reference (optional)							
ID: mAFsAZ227m06cN12K74yL6i-FwTqhbUEa7H9QXDskWZcXTC7yOeZj27y9M0E							
1-0-0 6-8-0 12-7-8 18-7-0 5-11-8 25-3-0 6-8-0							



LOADING (psf)	TCL (roof)	Snow (P/Pg)	TCDL	BCLL	BDDL
20.0	12.7/10.0	1.15	1.15	0.0 *	10.0
<b>SPACING-</b>					
2-0-0	Plate Gnp DOL	1.15	TC	BC	WB
		1.15	1.00	0.80	0.58
<b>CSI.</b>					
DEFL. in (loc)					
	L/defl	9	0.35	9	>868
	L/d	240		9	>683
	L/d	180		6	n/a
<b>PLATES</b>					
	GRIP	244/190			
	MT20	187/143			
	MT20HS	180			
Weight: 143 lb FT = 20%					

**LUMBER-** 2x4 SP DSS "Except"

**BRACING-** TOP CHORD 2-0 oc purlins (3-3-1 max.): 3-5. Rigid ceiling directly applied or 4-3-3 oc bracing. WEBS 1 Row at midpt 4-10, 4-7

**REACTIONS.** (lb/size) 6=1506/0-5-8, 2=1542/0-5-8  
 Max Horiz 2=145(LC 39)  
 Max Uplift 6=-1414(LC 12), 2=-1509(LC 12)  
 Max Grav 6=2078(LC 2), 2=2141(LC 2)

**FORCES.** (lb) - Max. Comp. Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=4342/2990, 3-4=-3782/273, 4-5=-3784/2743, 5-6=-4346/3002  
 BOT CHORD 2-10=-2547/3837, 9-10=-3386/4984, 7-9=-3386/4984, 6-7=-2577/3840  
 WEBS 3-10=-803/1404, 4-10=-1458/1026, 4-9=0/488, 4-7=-1459/1017, 5-7=-809/1408

**NOTES-**

1) Unbalanced roof live loads have been considered for this design.  
 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCCL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate gnp DOL=1.60  
 3) TCLL: ASCE 7-05; P=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15 (ground snow); P=12.7 psf (flat roof snow); Lumber DOL=1.15; Category II; Exp C; Partially Exp.; C=1.10, Lu=50-0-0 Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.  
 4) Unbalanced snow loads have been considered for this design.  
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 7) Provide adequate drainage to prevent water ponding.  
 8) All plates are MT20 plates unless otherwise indicated.  
 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangular 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1414 lb uplift at joint 6 and 1509 lb uplift at joint 2.  
 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.  
 13) Use USP HJUC26 (With 16d nails into Truss) or equivalent spaced at 11-10-4 oc max, starting at 6-8-6 from the left end to 18-6-10 to connect truss(es) to front face of bottom chord.  
 14) Fill all nail holes where hanger is in contact with lumber.  
 15) "NAILED" indicates 3-10d (0.148"x3.25") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.  
 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connector. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse of trusses and truss systems. See ANS/ITP Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road  
 Edenton, NC 27932

**TRENCO**  
 ENGINEERS BY  
 A PROFESSIONAL CORPORATION

December 14, 2018



Job	Truss	T1FGRD	Hip Girder	Qty	1	Ply	1	Concrete Supply Office	E12515683
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Peak Truss Builders, LLC, New Hill, NC - 27562, 8 220 s Nov 16 2018 MTRK Industries, Inc. Thu Dec 13 14:19:59 2018 Page 2

ID:8mAfsAZ2727mQ6cN12K374yL6i-FwTWqhbUEa7H9QXDEskWZcXTC7y0E2ja72jy9MOE

**LOAD CASE(S) Standard**

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-35, 3-5=-45, 5-6=-35, 2-6=-20

Concentrated Loads (lb)

Vert: 3=-72(F) 5=-72(F) 10=-373(F) 9=-54(F) 7=-373(F) 4=-67(F) 17=-67(F) 18=-67(F) 20=-67(F) 21=-67(F) 24=-54(F) 25=-54(F) 26=-54(F) 27=-54(F)

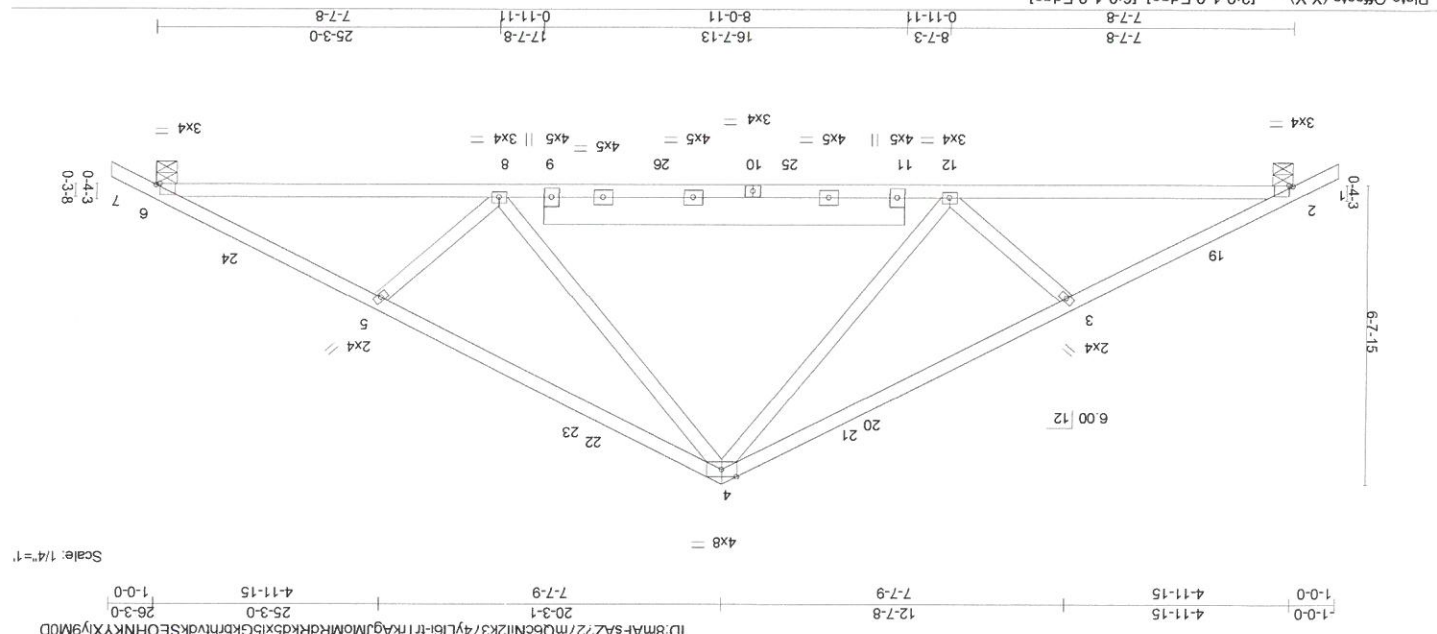
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE ML-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MTRK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road  
Edenton, NC 27932

Job	Truss	T1G	Common	Qty	9	Concrete Supply Office	E12515684
Job Reference (optional) 8 220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:00 2018 Page 1 ID: 8mAFsAZ277mQ6cNI2k374yL6i:trkAgJM0RdRkdx5GkbthvdkSEOHNKYXiy9M0D 12-7-8 4-11-15 4-11-15 7-7-8 7-7-8 20-3-1 7-7-9 4-11-15 25-3-0 1-0-0 26-3-0							

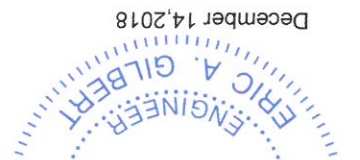


LOADING (psf)	TCLL (roof)	Snow (P/F/g)	TCDL	BCLL	BDDL
20.0	7.7/110.0	1.15	1.15	0.0	10.0
SPACING-	Plate Gnp DOL	Lumber DOL	Rep Stress Incr	Code IBC2009/TP12007	
2-0-0	1.15	1.15	YES		
CSI.	TC	BC	WB	Matrix-MS	
0.72	0.41	0.39	0.39		
DEFL.	Vert(TL)	Horz(TL)			
in (loc)	0.12	0.05			
L/d	8-12	8-12			
180	>999	>999			
PLATES	MT20	GRIP			
244/190					
Weight: 142 lb	FT = 20%				

**LUMBER-** 2x4 SP No.1 'Except'  
**TOP CHORD** 2x4 SP No. 1 'Except'  
**BOT CHORD** 2x4 SP No. 2  
**WEBS** 2x4 SP No. 3  
**REACTIONS.** (lb/size) 2=735/0-5-8, 6=735/0-5-8  
 Max Horiz 2=267(LC 13)  
 Max Uplift 2=651(LC 14), 6=651(LC 14)  
 Max Gray 2=1070(LC 2), 6=1070(LC 2)  
**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1843/1225, 3-4=-1580/1087, 4-5=-1579/1087, 5-6=-1843/1225  
 BOT CHORD 2-12=-957/1616, 8-12=-410/975, 6-8=-992/1616  
 WEBS 4-12=-308/570, 3-12=-424/528, 4-8=-308/570, 5-8=-424/528

**NOTES-**  
 (1) Unbalanced roof live loads have been considered for this design.  
 (2) Wind: ASCE 7-05: 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-7-8, Exterior(2) 12-7-8 to 15-7-8, Interior(1) 15-7-8 to 26-3-0 zone cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 (3) TLL: ASCE 7-05: P=20.0 psf (roof live load; Lumber DOL=1.15; Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10  
 (4) Unbalanced snow loads have been considered for this design.  
 (5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (7) This truss has been designed for a 1.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (8) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 2 and 651 lb uplift at joint 6.

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE ML-7473 rev. 10/02/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, erection and bracing of trusses and truss systems, see ANSITM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



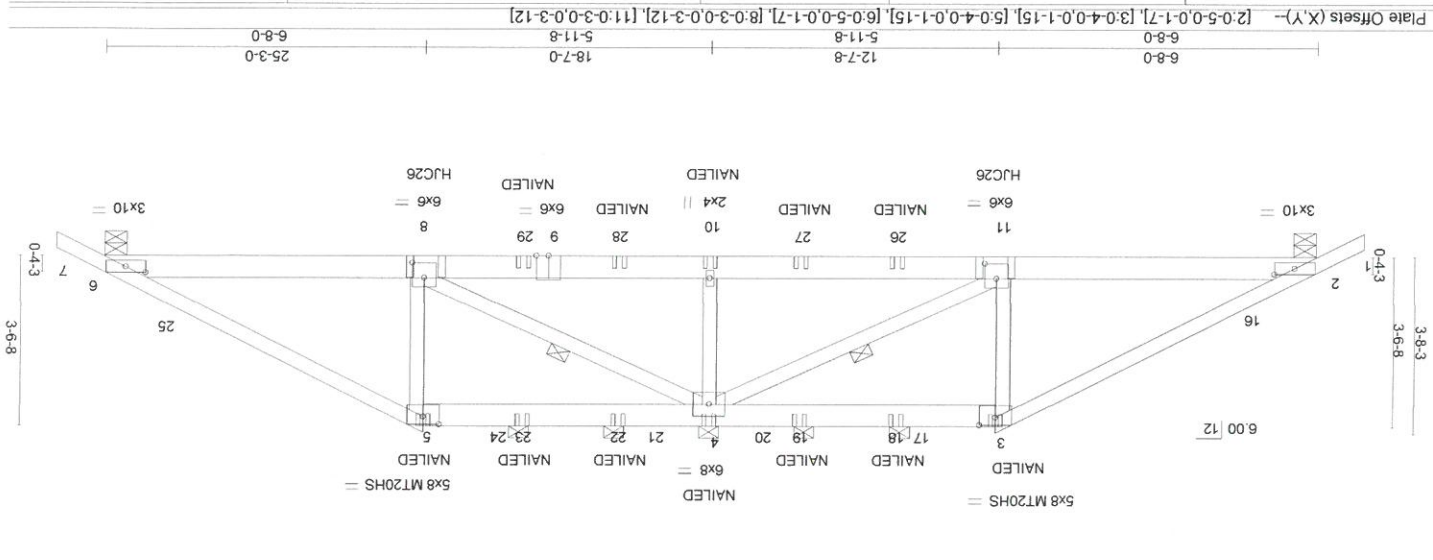
December 14, 2018



818 Soundside Road  
 Edenton, NC 27932

Job	Truss Type	Truss Type	Qty	Ply	Concrete Supply Office
GUYV1108-1	T1GRD	Hip Girder	1		
E12515685					Concrete Supply Office

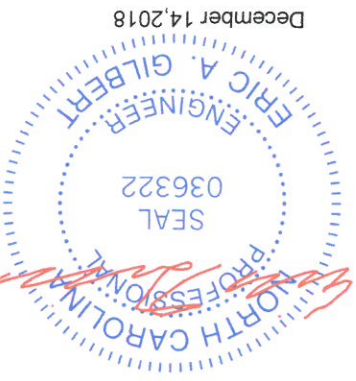
8 220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:01 2018 Page 1  
 ID: 8mAFsAZ7277mQ6N12K374YL6L11DXWgx75UfBpfeGkpx72pH9CTsXXW1364CyMOC  
 1-0-0 6-8-0 12-7-8 18-7-0 5-11-8 25-3-0 6-8-0 1-0-0  
 Scale = 1:45.1



LOADING (psf)	SPACING	CSI	DEFL.	in (loc)	L/d	L/d	PLATES	GRIP	Weight: 144 lb FT = 20%
TCL (roof) 20.0	2-0-0	0.63	0.34	10	>896	240	MT20	244/190	
Snow (P/Pg) 12.7/10.0	1.15	0.77	0.43	10	>702	180	MT20HS	187/143	
TCDL 10.0	1.15	0.58	0.13	6	n/a	n/a			
BCLL 0.0 *	NO								
BDDL 10.0									

LUMBER-	BRACING-	REACTIONS.	FORCES.
TOP CHORD 2x4 SP DSS * Except	Structural wood sheathing directly applied or 2-1-5 oc purlins, except	Max Horiz 2=-147(LC 10)	(lb) - Max. Comp Max. Ten. - All forces 250 (lb) or less except when shown.
BOT CHORD 2x6 SP No. 1	2-0-0 oc purlins (3-3-3 max.); 3-5.	Max Uplift 2=-1507(LC 12), 6=-1507(LC 12)	TOP CHORD 2-1=-2489/3834, 10-11=-3325/4979, 8-10=-3325/4979, 6-8=-2489/3834
WEBS 2x4 SP No. 3	Rigid ceiling directly applied or 4-3-12 oc bracing.	Max Gray 2=2139(LC 2), 6=2139(LC 2)	WEBS 3-11=-800/1402, 4-11=-1454/1002, 4-10=0/487, 4-8=-1454/1003, 5-8=-800/1402

**NOTES-**  
 1) Unbalanced roof live loads have been considered for this design.  
 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60  
 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=10.0 psf (ground snow); F=12.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; C=1.10, Lu=50-0-0 Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.  
 4) Unbalanced snow loads have been considered for this design.  
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 7) Provide adequate drainage to prevent water ponding.  
 8) All plates are MT20 plates unless otherwise indicated.  
 9) This truss has been designed for a 1.0 psf bottom chord live load nonconcurrent with any other live loads.  
 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1507 lb uplift at joint 2 and 1507 lb uplift at joint 6.  
 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.  
 13) Use USP HJC26 (With 16d nails into Girder & 10d nails into Truss) or equivalent spaced at 11-10-4 oc max, starting at 6-8-6 from the left end to 18-6-10 to connect truss(es) to front face of bottom chord.  
 14) Fill all nail holes where hanger is in contact with lumber.  
 15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.  
 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



December 14, 2018



818 Soundside Road  
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**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	T1GRD	Hip Girder	Qty	1	Ply	Concrete Supply Office	E12515685
8 220 s Nov 16 2018 MTEK Industries, Inc. Thu Dec 13 14:20:01 2018 Page 2 ID: 8mAFAZ727m06cM12k374yLL6-L11DXWVgx75UjFbpreGkpx72pH9CTsXXW1364CyMOC								

**LOAD CASE(S) Standard**

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-35, 3-5=-45, 5-7=-35, 2-6=-20

Concentrated Loads (lb)

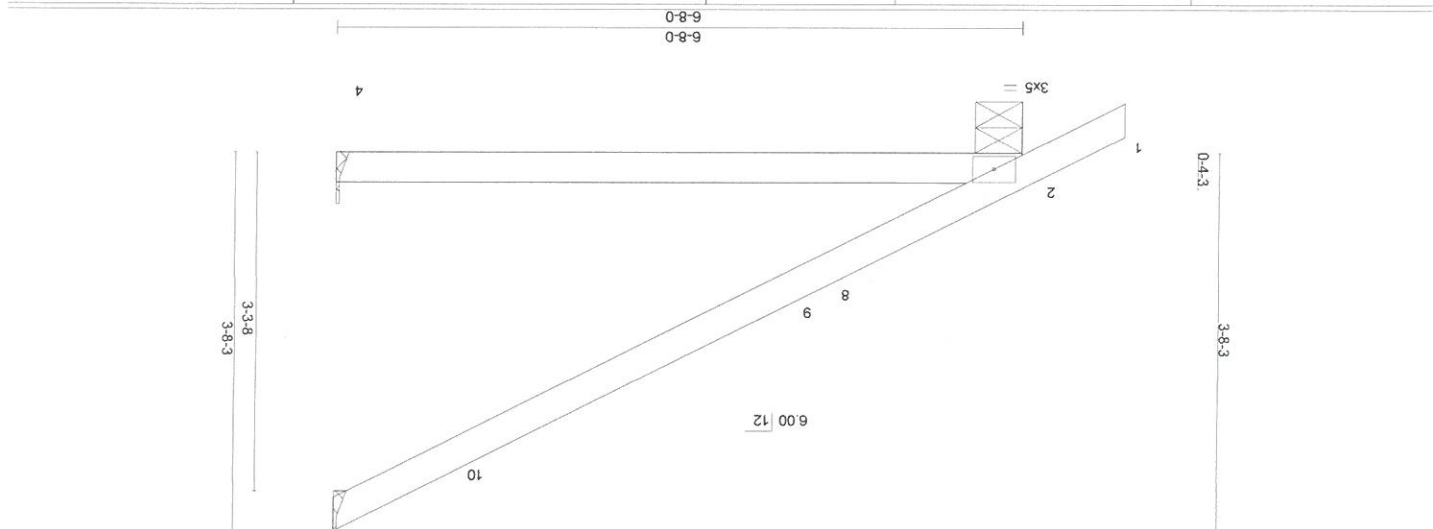
Vert: 3=-72(F) 5=-72(F) 11=-373(F) 10=-54(F) 8=-373(F) 4=-67(F) 18=-67(F) 19=-67(F) 22=-67(F) 23=-67(F) 26=-54(F) 27=-54(F) 28=-54(F) 29=-54(F)

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M11-1473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and tracing of trusses and truss systems, see ANSIPM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 ENGINEERING  
 A HUBBARD COMPANY  
 818 Soundside Road  
 Edenton, NC 27932

Job	Truss	T2	Jack-Open	Qty	Ply	Concrete Supply Office	E12515686
ID: 8mAFSAZ727m06cN1ZK374yLL6i-pEbb9szuPc9s17cMnZM9gAYXKFC5mkgpcey9M0B 8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:02 2018 Page 1 Job Reference (optional)							

Peak Truss Builders, LLC, New Hill, NC - 27562, Scale = 1/21.0

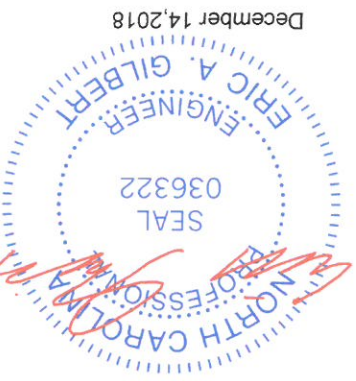


LOADING (psf)		SPACING-		DEFL.		BRACING-	
TCLL (roof)	20.0	2-0-0	CSI	in (loc)	in (loc)	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
TCLL (PFFg)	7.7/10.0	Plate Gnp DOL	TC	0.18	0.18	BOT CHORD	Rigid ceiling directly applied or 10-0 oc bracing.
Snow (PFFg)	1.15	Lumber DOL	BC	0.59	-0.22		
TCLL	10.0	Rep Stress Incr	WB	0.00	-0.01		
BCLL	0.0	YES			3		
BCLL	0.0	Code IBC2009/TP12007	Matrix-MP		n/a		
BCDL	10.0				n/a		

**REACTIONS.** (lb/size) 3=106/Mechanical, 2=221/0-5-8, 4=74/Mechanical  
 Max Uplift 3=-194(LC 14), 2=-188(LC 14), 4=-1(LC 14)  
 Max Grav 3=174(LC 2), 2=329(LC 2), 4=123(LC 7)

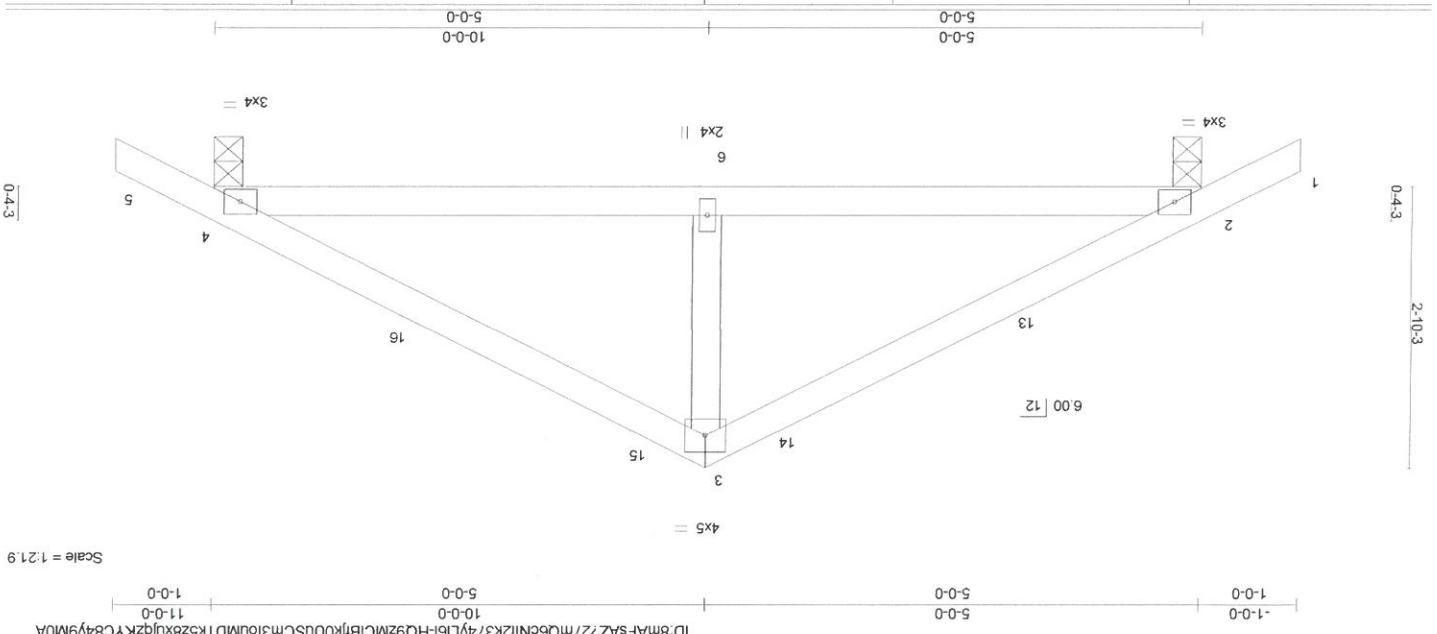
**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**  
 (1) Wind: ASCE 7-05; 130mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C-Extensor(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 6-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 (2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15); Category II; Exp C; Partially Exp.; Cf=1.10  
 (3) Unbalanced snow loads have been considered for this design.  
 (4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (6) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (8) Refer to girder(s) for truss to truss connections.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 3, 188 lb uplift at joint 2 and 1 lb uplift at joint 4.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MR-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Backing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-49 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.  
 818 Soundside Road  
 Edenton, NC 27932  
**TRENCO**  
 ENGINEERS, PC  
 A TRUSS SPECIALIST

Job	Truss	T3	Common	Qty	1	Ply	Concrete Supply Office	E12515687
Job Reference (optional) 8 220 S Nov 16 2018 MITEK Industries, Inc. Thru Dec 13 14:20:03 2018 Page 1 ID: 8mFAZ727mQ6cN12K374YL6i-HQ9MCGiBfjkUuScm3iouMDTK5z8xjqzKCYC84y9M0A 11-0-0 10-0-0 5-0-0 5-0-0 5-0-0 5-0-0 1-0-0 1-0-0								



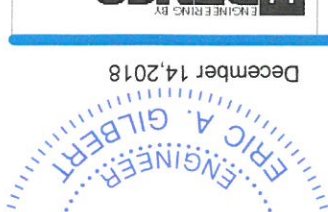
LOADING (psf)		SPACING-		CSI.		DEFL.		in (loc)		I/defl		L/D	
TCLL (roof)	20.0	Plate Grrp DOL	1.15	TC	0.29	Vert(TL)	0.03	6-12	>999	240			
Snow (FPFg)	7.7/10.0	Lumber DOL	1.15	BC	0.23	Vert(TL)	-0.05	6-12	>999	180			
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	4	n/a	n/a			
BCLL	0.0	Code IBC2009/TP12007		Matrx-MS									
BCLD	10.0												

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

**REACTIONS.** (lb/size) 2=312/0-3-8, 4=312/0-3-8  
 Max Horiz 2=117(LC 13)  
 Max Uplift 2=313(LC 14), 4=313(LC 14)  
 Max Grav 2=460(LC 2), 4=460(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-554/467, 3-4=-554/467  
 BOT CHORD 2-6=-223/446, 4-6=-223/446

**NOTES.**  
 1) Unbalanced roof live loads have been considered for this design.  
 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C-Extensor(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 5-0-0, Extensor(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 11-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grrp DOL=1.60  
 3) TCLL: ASCE 7-05; P=20.0 psf (roof live load); Lumber DOL=1.15; P<sub>g</sub>=10.0 psf (ground snow); P<sub>f</sub>=7.7 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp C; Partially Exp.; C<sub>t</sub>=1.10  
 4) Unbalanced snow loads have been considered for this design.  
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 8) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2 and 313 lb uplift at joint 4.



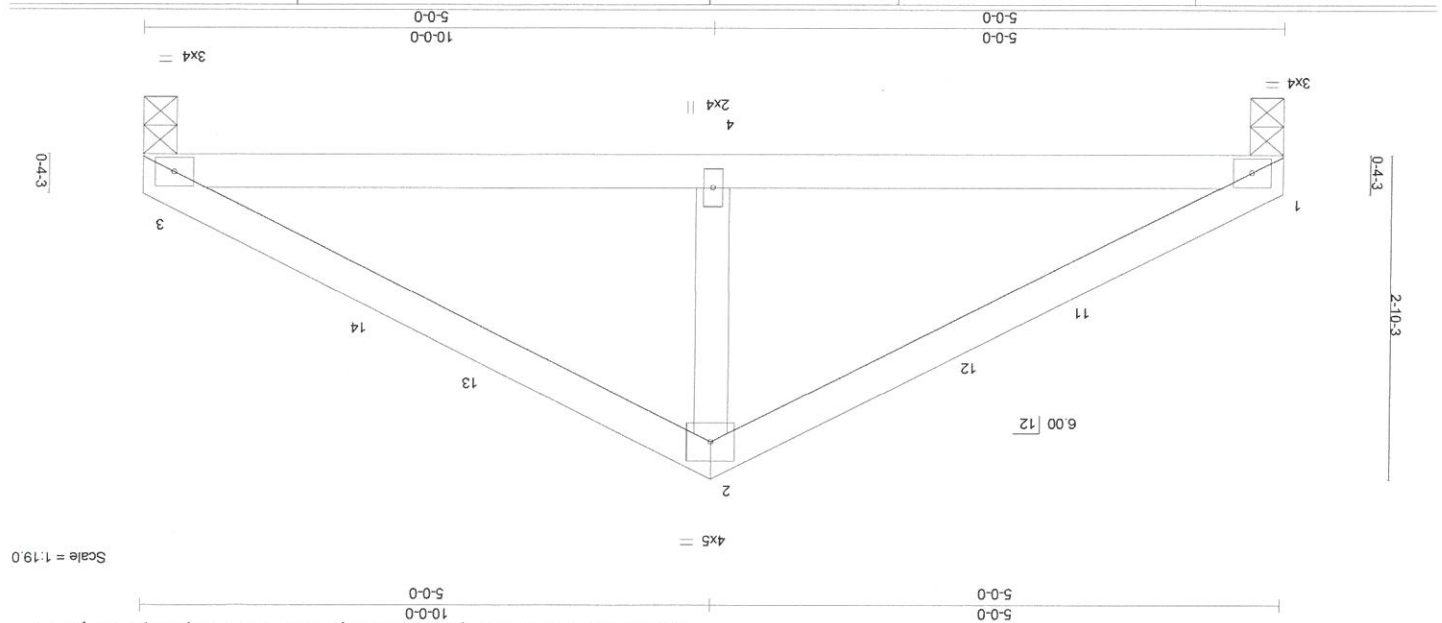
December 14, 2018



Job	Truss	T3A	Common	Qty	1	Concrete Supply Office	E12515688
GUYV1108-1							

Peak Truss Builders, LLC, New Hill, NC - 27562.

8 220 s Nov 16 2018 MTEK Industries, Inc. Thu Dec 13 14:20:03 2018 Page 1  
 ID: 8mFAZ727mQ6cN12k374yL6:HQZMCiBfK0UUScm3iouMDTi5ynxuzkRC84y9M0A



LOADING (psf)		SPACING-		CSI.		DEFL.		BRACING-	
TCLL (roof)	20.0	Plate Gnp DOL	1.15	TC	0.29	in (loc)	1/del	L/D	PLATES
Snow (P/Fg)	7.7/10.0	Lumber DOL	1.15	BC	0.26	Vert(TL)	>999	>999	MT20
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	4-10	n/a	GRIP
BCLL	0.0	Code IBC2009/TP1207		Maintx-MS					244/190
BCDL	10.0								

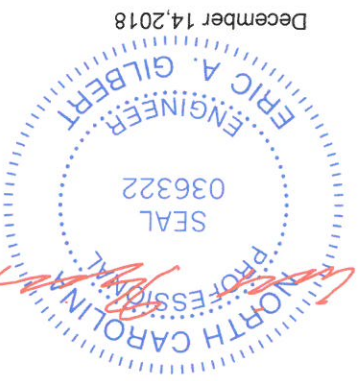
LUMBER-		TOP CHORD		BOT CHORD	
2x4 SP No.1	2x4 SP No.1	Structural wood sheathing directly applied or 6'-0-0 oc purlins.	Structural wood sheathing directly applied or 10'-0-0 oc bracing.		
WEBS	2x4 SP No.3				

**REACTIONS.** (lb/size) 1=277/0-3-8, 3=277/0-3-8  
 Max Horz 1=98(LC 12)  
 Max Uplift 1=222(LC 14), 3=222(LC 14)  
 Max Gray 1=400(LC 2), 3=400(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-2=-571/524, 2-3=-571/524  
 BOT CHORD 1-4=-316/463, 3-4=-316/463

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C-Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 10-0-0 zone;
- 3) TCLL: ASCE 7-05; Pf=20.0 psf (roof live load); Lumber DOL=1.15; snow: Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp C; Partially Exp.; Cf=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 6) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 1 and 222 lb uplift at joint 3.



December 14, 2018

**TRENCO**  
 ENGINEERS, PC  
 A FLETCHER ALMIDA COMPANY  
 818 Soundside Road  
 Edenton, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-T473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MTEK connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITM Quality Criteria, DSB-49 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Oly	Ply	Concrete Supply Office	E12515689
Job Reference (optional)			1	1		

Peak Truss Builders, LLC, New Hill, NC - 27562, 8 220 s Nov 16 2018 MITek Industries, Inc. Thu Dec 13 14:20:05 2018 Page 2 ID: 8mAFsAZ?27mQ6cN1I2K374yLL6i-DpHknjRBK\_kjCcauULGzn1q1ue6F057Re1JDzyM08

**LOAD CASE(S) Standard**

Uniform Loads (plf)  
 Vert: 1-3=-35, 3-4=-45, 4-6=-35, 2-5=-20

Concentrated Loads (lb)  
 Vert: 4=-7(B) 8=-98(B) 7=-98(B) 3=-7(B) 14=-7(B) 16=-11(B)

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPP Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 A MITEK COMPANY  
 ENGINEERS BY  
 818 Soundside Road  
 Edenton, NC 27932



Job	Truss	T4A	Jack-Open	Qty	Ply	3	Concrete Supply Office
GUVV1108-1				1			Job Reference (optional)

Peak Truss Builders, LLC, New Hill, NC - 27562, 8 220 s Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:06 2018 Page 1  
 ID: 8mAFAZ727mQeCNlI2k374yLl6i-h76, Dk4yeb6bLMBmRBsVW7rzclzF8GqGfnsIPy9M07



LOADING (psf)		SPACING-		CSL		DEFL.		BRACING-		LUMBER-	
TCLL (roof)	20.0	Plate Gnp DOL	1.15	TC	0.37	in (loc)	0.05	TOP CHORD	2x4 SP No.1	BCDL	10.0
Snow (P/F/g)	7.7/10.0	Lumber DOL	1.15	BC	0.33	Vert(TL)	-0.05	BOT CHORD	2x4 SP No.1	BCLL	0.0
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	Structural wood sheathing directly applied or 4-8-7 oc purlins.		BCD	10.0
BCLL	0.0	Code IBC2009/TP12007		Matrx-MP				Rigid ceiling directly applied or 10-0 oc bracing.			
								Weight: 17 lb			
								FT = 20%			

**REACTIONS.** (lb/size) 3=73/Mechanical, 2=168/0-5-8, 4=51/Mechanical

Max Uplift 3=-131(LC 14), 2=-165(LC 14)  
 Max Grav 3=119(LC 2), 2=252(LC 2), 4=85(LC 7)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**

- 1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-7-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 3 and 165 lb uplift at joint 2.



December 14, 2018

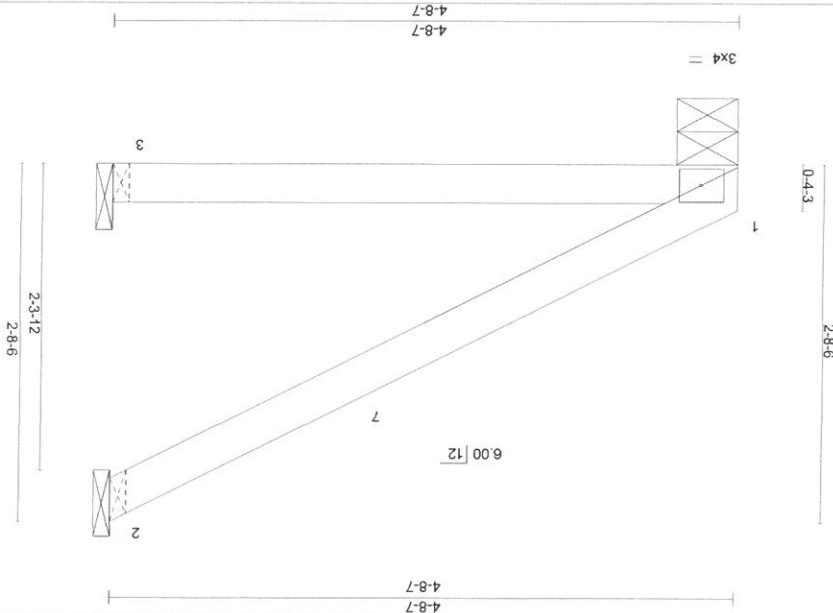


818 Soundside Road  
 Eden, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-717 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP14 Quality Criteria, DSB-49 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	T48	Jack-Open	Qty	Ply	Concrete Supply Office	E12515692
GUVY1108-1				1		(optional)	

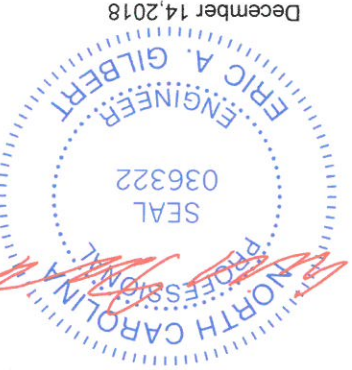
Peak Truss Builders, LLC, New Hill, NC - 27562, 8220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:06 2018 Page 1  
 ID:8mAf-AZ727mG6cNI12K374YL6h7r6\_Dk4y6bLMBmRBSVW7r1zTx8GqGfmsIPy9M07



LOADING (psf)	TCLL (roof)	Snow (P/F/g)	TCDL	10.0	SPACING-		CSL	DEFL.	in (loc)	L/D	BRACING-		2x4 SP No.1	2x4 SP No.1	REACTIONS.			
					Plate Gnp DOL	Lumber DOL					Rep Stress Incr	Code IBC2009/TP12007				WB	Matrix-MP	TOP CHORD
20.0	7.7/10.0	1.15	1.15	1.15	2.0-0	1.15	0.38	0.06	3-6	>981	240	Structural wood sheathing directly applied or 4-8-7 cc purlins.	2x4 SP No.1	1129/0-5-8, 2=75/Mechanical, 3=53/Mechanical	1=159(LC 14), 2=137(LC 14), 3=7(LC 14)	Max Horiz 1=159(LC 14)	Max Uplift 1=-62(LC 14), 2=-137(LC 14), 3=-7(LC 14)	Max Grav 1=186(LC 2), 2=122(LC 2), 3=86(LC 7)

**NOTES-**  
 (1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Exterior(2) 0-0-0 to 4-7-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 (2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Exp C; Category II; Exp C; Partially Exp.; Cf=1.10  
 (3) Unbalanced snow loads have been considered for this design.  
 (4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (7) Refer to girder(s) for truss to truss connections.  
 (8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 1, 137 lb uplift at joint 2 and 7 lb uplift at joint 3.

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



December 14, 2018

**TRENCO**  
 ENGINEERS BY  
 A TRUCK COMPANY  
 818 Soundside Road  
 Eden, NC 27932

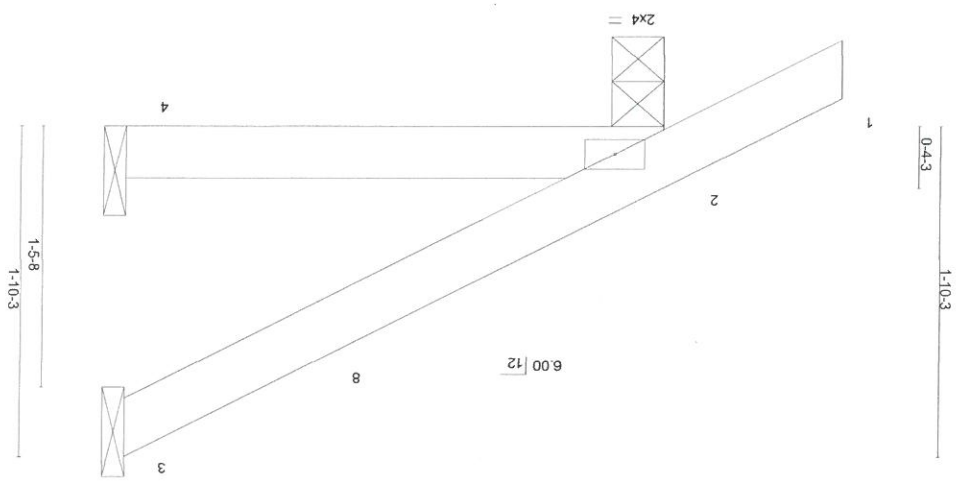
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1717, 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	T5	Jack-Open	Qty	3	Ply	Concrete Supply Office
							Job Reference (optional)
							1

Peak Truss Builders, LLC, New Hill, NC - 27562, 8/22/20 s Nov 16 2018 MTEK Industries, Inc. Thu Dec 13 14:20:07 2018 Page 1

ID: 8mAFA7277mQeCHN12K374YL6I-ABPUZiKXESZwMz?vNK3CNBRIN4i4QuYwOH9M06

Scale: 1"=1'



LOADING (psf)		SPACING-		CSI.		DEFL.		BRACING-	
TCLL (roof)	20.0	Plate Gnp DOL	1.15	TC	0.11	Vert(LL)	0.01	TOP CHORD	Structural wood sheathing directly applied or 3-0-0 oc purlins.
Snow (P/F/g)	7.7/10.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	-0.01	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00		
BCLL	0.0								
BCDL	10.0								

REACTIONS. (lb/size) 3=44/Mechanical, 2=123/0-3-8, 4=31/Mechanical

Max Horiz 2=152(LC 14)  
Max Uplift 3=-75(LC 14), 2=-149(LC 14)  
Max Grav 3=71(LC 2), 2=188(LC 2), 4=52(LC 7)

FORCES. (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (all heights) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15); Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 3 and 149 lb uplift at joint 2.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-171 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEK connectors. This design is based upon parameters shown, and is for an individual building component, not a building system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



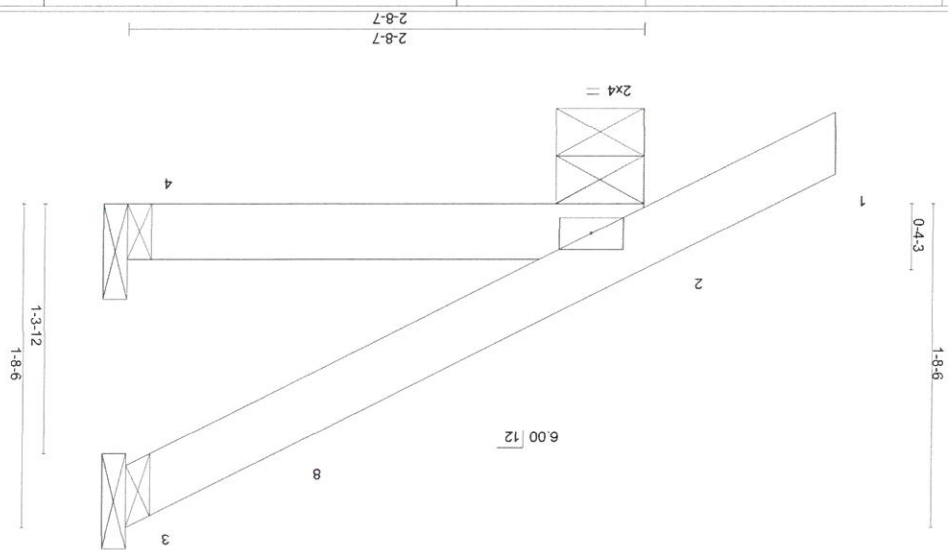
818 Soundside Road  
Edenton, NC 27932

December 14, 2018



Job	Truss	T6	Jack-Open	Qty	Ply	Concrete Supply Office
GUYV1108-1				4		
				1		

Peak Truss Builders, LLC, New Hill, NC - 27562, 8220 S Nov 16 2018 Mitek Industries, Inc. Thu Dec 13 14:20:08 2018 Page 1  
 ID: 8mAFA5AZ727mG6cNI2K374YL6i-OQysPvmKUFMIagL9ZcuzbQwM06jgAKZ7Czpjy9M05  
 Scale = 1:11.4



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d
TCLL (roof) 20.0	2-0-0	TC 0.10	Vert(LL)	-0.00	>999	240
Snow (PffPg) 7.7/10.0	Lumber DOL	BC 0.07	Horz(TL)	-0.00	4-7	>999
TCDL 10.0	Rep Stress Incr	WB 0.00		-0.00	3	n/a
BCLL 0.0	Code IBC2009/TP12007					n/a
BCDL 10.0						

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD
BOT CHORD 2x4 SP No.1	BOT CHORD

**REACTIONS.** (lb/size) 3=39/Mechanical, 2=115/0-5-8, 4=28/Mechanical  
 Max Horiz 2=142(LC 14)  
 Max Uplift 3=65(LC 14), 2=-147(LC 14)  
 Max Grav 3=63(LC 2), 2=177(LC 2), 4=47(LC 7)

**NOTES-**  
 (1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp. C; Enclosed; MWFRS (all heights) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-7-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions show; Lumber DOL=1.60 plate gnp DOL=1.60  
 (2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp. C; Partially Exp.; Cf=1.10  
 (3) Unbalanced snow loads have been considered for this design.  
 (4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (8) Refer to girder(s) for truss connections.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 3 and 147 lb uplift at joint 2.



December 14, 2018

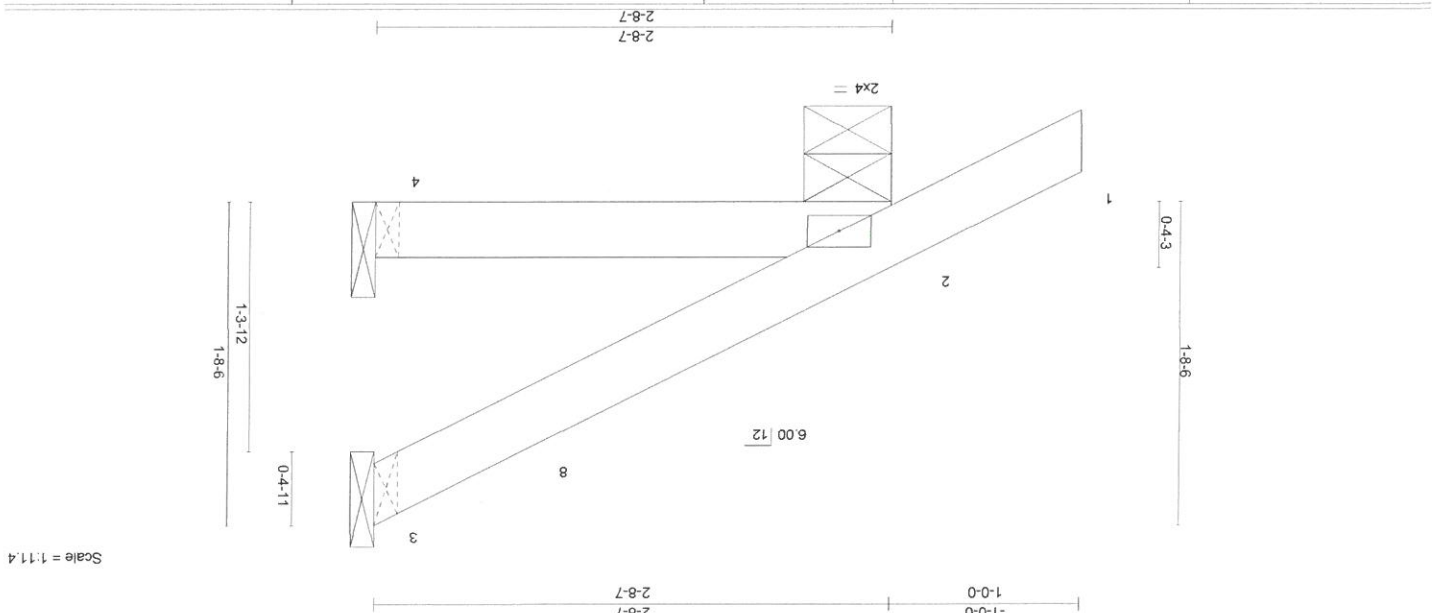
**TRENCO**  
 A MITEK COMPANY  
 818 Soundside Road  
 Eden, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIPPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	T6A	Jack-Open	Qty	4	Concrete Supply Office	Job Reference (optional)
GUYV1108-1				Ply	1		

8 220 s Nov 16 2018 MITEK Industries, Inc. Thru Dec 13 14 20 08 2018 Page 1  
 ID: 8mAFsAZ277mQ6cNIk374yLl6i-eOysPvmKUfMIagL9ZcuzbQwMMQ6jckAZ7cgpjy9M05



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	L/d	PLATES	GRIP	Weight: 10 lb	FT = 20%
TCLL (roof) 20.0	2-0-0	0.10	Vert(LL)	-0.00	7	240	244/190		
Snow (P/F/g) 7.7/10.0	Lumber DOL 1.15	0.07	Vert(TL)	-0.00	4-7	180			
TCDL 10.0	Rep Stress Incr YES	0.00	Horz(TL)	-0.00	3	n/a			
BCLL 0.0	Code IBC2009/TP12007	0.00				n/a			
BCDL 10.0									

**REACTIONS.** (lb/size) 3=39/Mechanical, 2=115/0-5-8, 4=28/Mechanical  
 Max Horiz 2=142(LC 14)  
 Max Uplift 3=65(LC 14), 2=-147(LC 14)  
 Max Grav 3=63(LC 2), 2=177(LC 2), 4=47(LC 7)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**  
 (1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp. C; Enclosed; MWFRS (all heights) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-7-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60  
 (2) TCLL: ASCE 7-05; P=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=10.0 psf (ground snow); Pf=7.7 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp. C; Partially Exp.; Cf=1.10  
 (3) Unbalanced snow loads have been considered for this design.  
 (4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 7.7 psf on overhangs non-concurrent with other live loads.  
 (5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.  
 (6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 (7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.  
 (8) Refer to girder(s) for truss to truss connections.  
 (9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 3 and 147 lb uplift at joint 2.



December 14, 2018

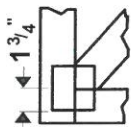
**TRENCO**  
 ENGINEERING BY  
 A KATIE ANHOLD  
 818 Soundside Road  
 Edenton, NC 27932

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-13 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse of trusses and truss systems; see ANSIT/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.  
 ANSIT/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

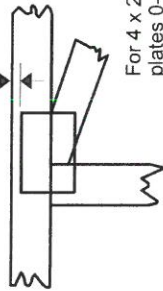
## Symbols

### PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



0- $\frac{1}{16}$ "



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



\* Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 X 4

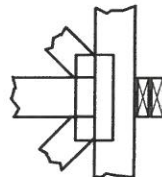
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### BEARING



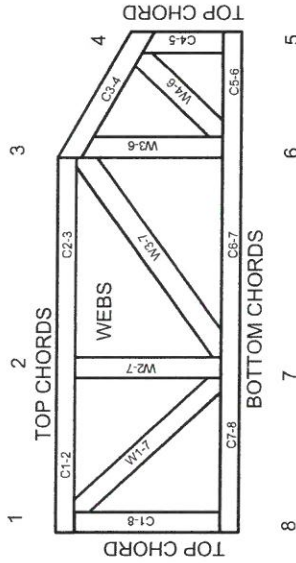
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
- DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.
- BCSI:

## Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

### PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988  
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

## General Safety Notes

### Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor 1 bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

**THIS LAYOUT IS TO BE USED AS A TRUSS PLACEMENT GUIDE ONLY.  
PLEASE REFER TO BUILDING PLANS FOR BUILDING CONSTRUCTION AND DETAILS,  
SUCH AS PLUMBING OR DUCT DROPS.**

**PROPOSED DESIGN-  
NOT FOR  
CONSTRUCTION**

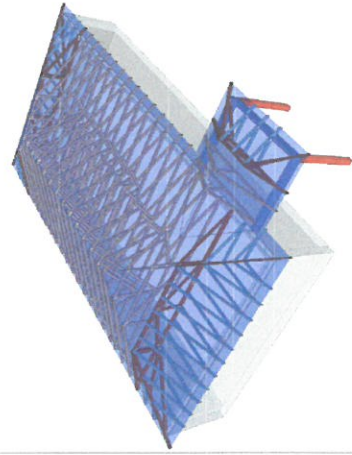
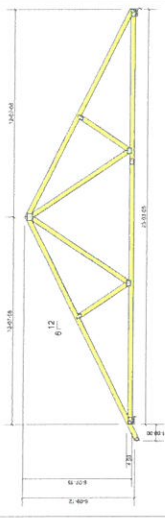
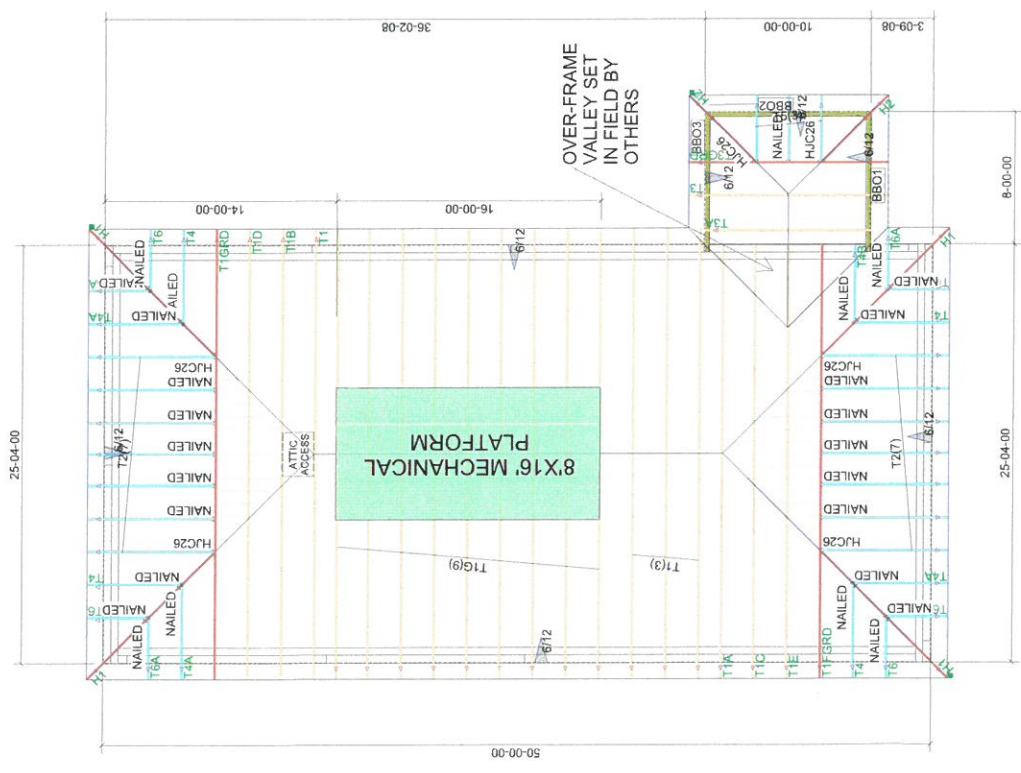
**Roof Truss Loading per 2012 NC Residential Code**  
 Top Chord Live Load 20# PSF  
 Top Chord Dead Load 10# PSF  
 Bottom Chord Live Load 10# PSF  
 Bottom Chord Dead Load 10# PSF  
 Trusses are designed for additional storage load wherever a 4'x24" box will fit between the webs.

△ This symbol denotes left end of truss as shown on truss drawings  
 ● Approximate location of toilet drop. Builders please confirm.  
 Truss connections by others:  
 (N) - Nailed  
 (L) - Ledger

Note: All dimensions shown are assumed to be:  
 1. Outside-out of stud  
 2. Outside-out of sheathing  
 3. Outside-out of framing  
 4. Outside-out of plumbing and mechanical clearance. Unless shown is not exceeded.  
 5. Outside-out of truss.  
 6. Without prior approval from Peak Builders with any questions. We will do our best to answer any way we can. We can be reached at 919-545-5555 or sales@peaktruss.com

**CONCRETE SUPPLY OFFICE  
ROOF TRUSSES  
2' OC, 1' OH**

Truss Connector Total List		
Manuf	Product	Qty
	HJC26	6

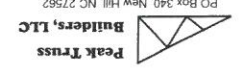


Job # GUYV1108-1

Concrete Supply Office  
Fuquay-Varina NC

Date Quoted: 11/08/18  
Designer: SB

Guy C Lee - Clayton  
151 Hwy 42 E  
Clayton, NC  
27520



PO Box 340, New Hill, NC 27562