ERECTION NOTES

1. All bracing shown and provided by the Metal Building Provider for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10).
2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.14).
4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
   4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems Manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met:
   5.1. Welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code - Steel or AWS D1.3 Structural Welding Code - Sheet Steel as applicable, for the processes, positions, and materials involved.
   5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.15).
10. The Metal Building Provider Field Modifications Policy:
    10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
    10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
    10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
11. WARNING - SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE. CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

1. The Metal Building Provider Customer, hereafter referred to as the "customer", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. It is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.
5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary notwithstanding. It is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3)
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5.1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The Metal Building Provider's red-oxide and gray oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions.
4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the Specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES

Building Code:	NCBC 18
Steel Specification:	AISC10
Cold-Formed Specification:	NAUS12

GENERAL LOADS

Roof Dead Load:	2.00	
Roof Collateral Load:	0.50	psf
Sprinkler Load:	0.00	psf
Roof Live Load:	20.00	psf
Tributary Live Load Reduction:	Yes	
Rainfall Intensity (5-minute duration 5-year recurrence):	6.76	in/hr

WIND LOAD

Wind Speed (3-sec gust) Vult:	119	mph
Vasd:	1.00	mph
V service:	1.00	mph
Wind Exposure Category:	B	
Wind Condition:	Enclosed	
Internal Pressure Coefficient (GCpi):	0.18/-0.18	
Edge Zone Width:	6.4	Ft

SNOW LOAD

Ground Snow Load (Pg):	15.00	psf
Roof Snow Load (Pf):	10.50	psf
Snow Exposure Factor (Ce):	1.00	
Snow Load Importance Factor (Is):	1.00	
Thermal Factor (Ct):	1.00	

DEFLECTION CRITERIA

Main Frames Lateral:	H/60	Roof Panels:	L/60
Main Frames Vertical:	L/180	Purlins:	L/150
Bearing Frame Rafter:	L/180	Wall Panels:	L/60
Endwall Columns:	L/120	Girts:	L/90

SEISMIC LOAD

Risk Category:	II - Normal		
Seismic Importance Factor (Ie):	1.00		
Spectral Response Acceleration (Ss):	0.1720		
Spectral Response Acceleration (S1):	0.0820		
Site Class:	D		
Spectral Response Coefficients (Sds):	0.1835		
Spectral Response Coefficients (Sd1):	0.1312		
Seismic Design Category:	B		
Basic Seismic Force Resisting Systems*:			
	Longitudinal		Lateral
Total Design Base Shear:	4.98 Kips		4.73 Kips
Sesimic Response Coefficient(s) (Cs):	0.061		0.061
Response Modification Factor(s) (R):	3		3
Deflection Amplification Factor(s):	3		3
Analysis Procedure: Equivalent Lateral Force			

* Ordinary Steel Concentrically Braced Frame(s) and/or Ordinary Steel Moment Frame(s)

ROOF PANEL

Profile: Super Span X Gauge: 26 Color: Black
UL580 Class 90: Yes
Clip Type if Standing Seam:

WALL PANEL

Profile: Super Span X Gauge: 26 Color: Cool White

PRIMARY FRAMING

Built-Up & Hot-Rolled: Red Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts: Red Oxide Primer
Girts, Light Gage Columns: Red Oxide Primer
Light Gage Jambs & Headers: Red Oxide Primer

Hot-Dip Galvanizing conforms to the ASTM A123 specification.
Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

TRIM COLORS

Roof Line:	Black		.
Wall Trim:	Black	Gauge:	.
Accessories:	Black	Gauge:	.
Downspouts:	Black	Gauge:	.

APPROVAL SPECIFICATIONS

1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contract requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
   5.1. Be made in contrasting ink.
   5.2. Be legible and unambiguous.
   5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.
9. Waiving the approval process by designating the order "For Production" supercedes notes 1, 2, 5, 6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING INDEX	
DATE	DESCRIPTION
C1	Cover Sheet
F1	Anchor Rod Plan
F2	Anchor Rod Details
F3	Anchor Rod Reactions
E1	Roof Framing Plan
E2	Roof Sheeting Plan
E3	Front Sidewall
E4	Back Sidewall
E5	Left Endwall
E6	Right Endwall
P1 >> P4	Frame Cross-Sections
D1 >> D4	Standard Details

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes parts such as doors, windows, foundation design, and erection of the building.

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH
Kyle McDonough
10-21-2025

DRAWING STATUS

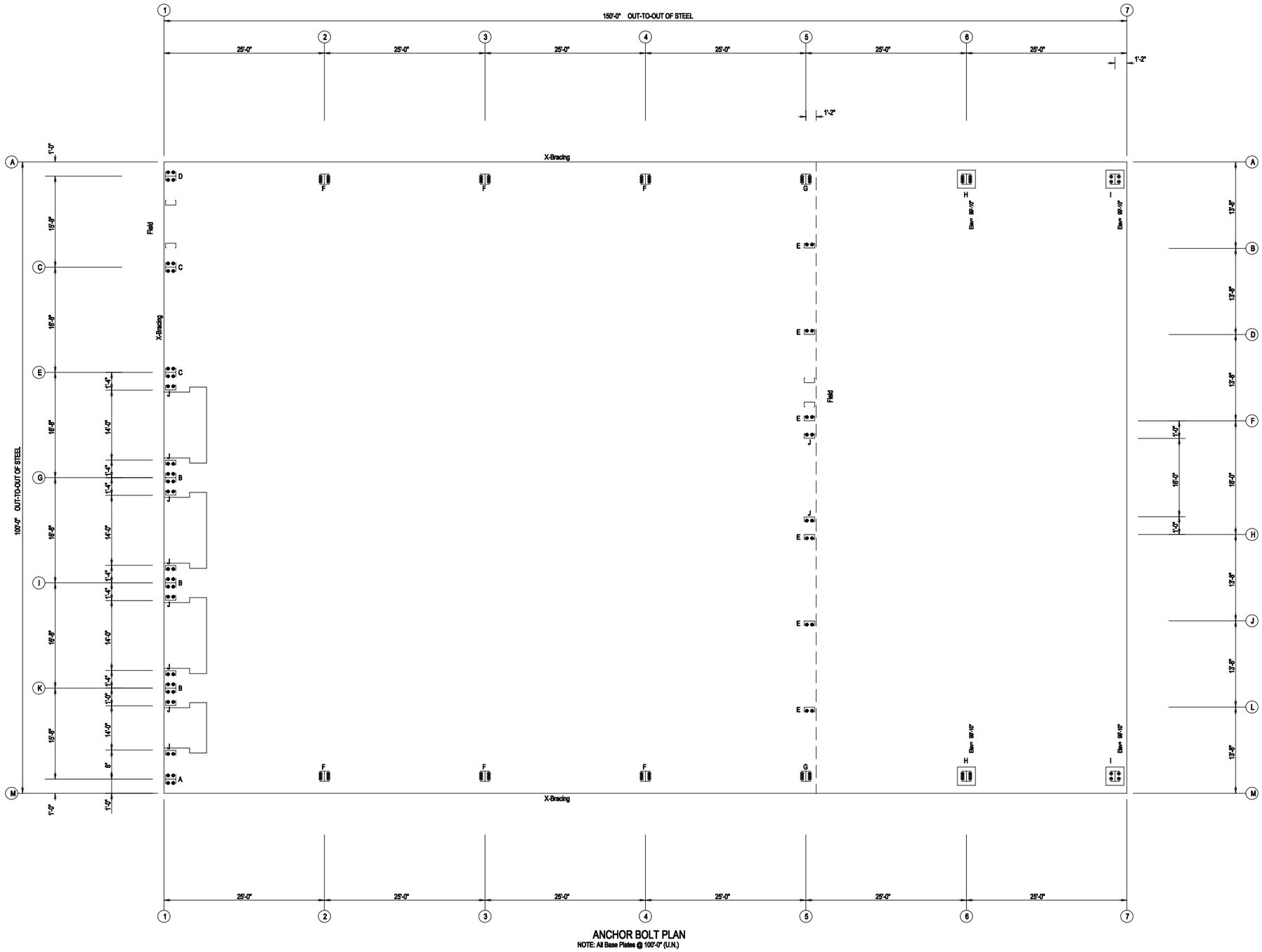
- ☐ FOR APPROVAL: These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.
- ☐ FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.
- ☐ FOR ERECTOR INSTALLATION: Final drawings for construction.

ISSUE	DATE	DESCRIPTION	BY	CHK
1	9/26/2025	FOR PERMITS	CCD	CCD
2	10/21/2025	FOR CONSTRUCTION	TE	TE
3	10/21/2025	FOR R01 APPROVALS	CCD	CCD

SHEET DESCRIPTION: COVER SHEET
BLDG. SIZE: 100'-0" x 150'-0" x 16'-0"
CUSTOMER: MAVERICK STEEL BUILDINGS
CUSTOMER LOCATION: JEFFERSON, GA 30549
PROJECT REFERENCE: Ricky Earnhardt
JOBSITE LOCATION: Lillington, NC 27504
JOBSITE COUNTY: County

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
	.	10/20/25		25-448	C1	R01

Dia= 5/8"
Dia= 1"



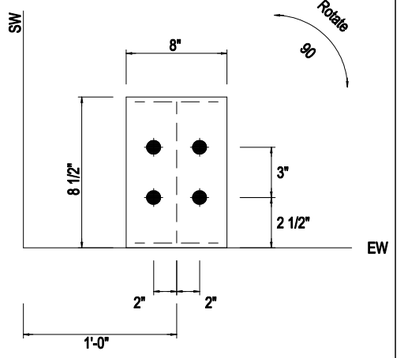
ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH
Kyle McDonough
10-21-2025

MAVERICK STEEL BUILDINGS INC.
WWW.MAVERICKSTEELBUILDINGS.COM
52 Apex Dr Jefferson, GA 30549
Ph. (888) 346-2426 Mavericksteelbuildings.com

DESCRIPTION: ANCHOR BOLT PLAN						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

Dia= 5/8"



DETAIL A

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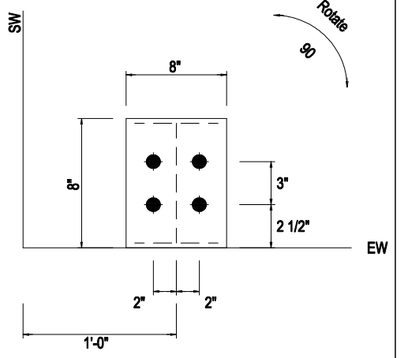
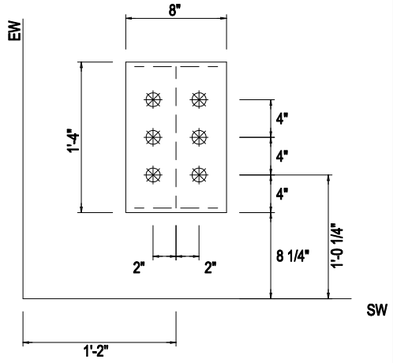


DETAIL D

Dia= 1"



DETAIL G

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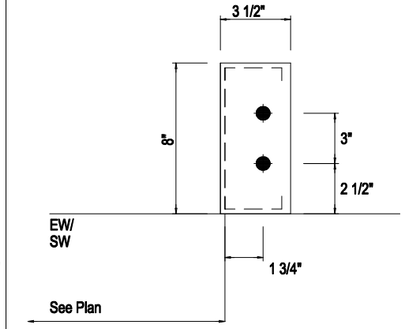

DETAIL J

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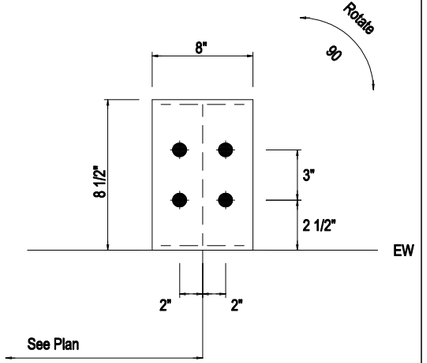

DETAIL B

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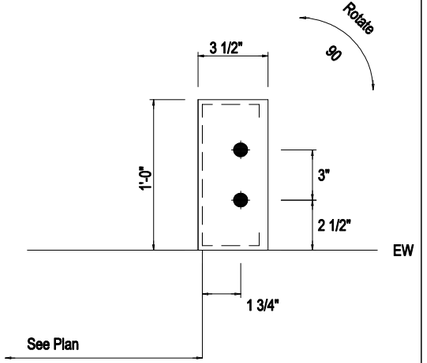

DETAIL E

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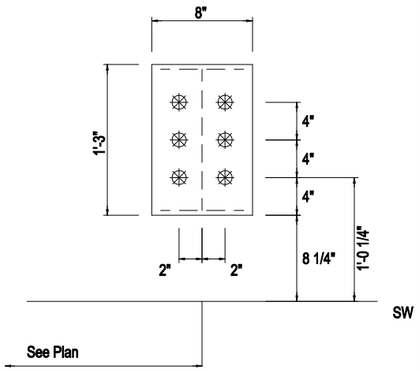


DETAIL H Base EL. 99'-10"

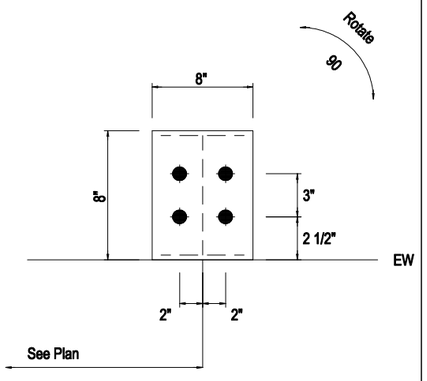
GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.

2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.

3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typicaily the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

Dia= 5/8"



DETAIL C

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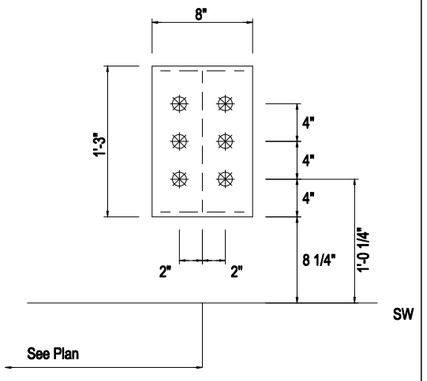

DETAIL F

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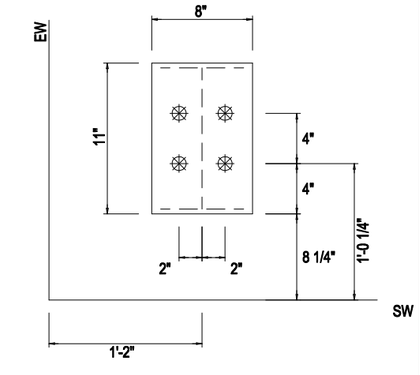


DETAIL I Base EL. 99'-10"

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

MAVERICK STEEL BUILDINGS INC.
WWW.MAVERICKSTEELBUILDINGS.COM
52 Apex Dr Jefferson, GA 30549
Ph. (888) 346-2426 Maverickssteelbuildings.com

DESCRIPTION: ANCHOR BOLT DETAILS

CUSTOMER: MAVERICK STEEL BUILDINGS | PROJECT: Ricky Earnhardt

LOCATION: Lillington, NC 27504

DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width	(ft)	= 100.0
Length	(ft)	= 150.0
Eave Height	(ft)	= 16.0/ 16.0
Roof Slope	(rise/12)	= 1.00/ 1.00
Roof Dead Load	(psf)	= 2.0
Wall Dead Load		
Left Endwall	(psf)	= 3.0
Right Endwall	(psf)	= 3.0
Front Sidewall	(psf)	= 3.0
Back Sidewall	(psf)	= 3.0
Roof Live Load	(psf)	= 20.0
Frame Live Load	(psf)	= 12.0
Collateral Load	(psf)	= 0.5
Snow Load	(psf)	= 10.5
Minimum Snow	(psf)	= 15.0
Wind Speed	(mph)	= 119.0
Wind Code		= IBC 15
Exposure		= B
Closure		= Enclosed
Internal Wind Coeff		= -0.18, +0.18
Risk Category		= II - Normal
Importance - Wind		= 1.00
Importance - Seismic		= 1.00
Seismic Design Category		= B
Seismic Coeff	(Sms)	= 0.28

5. Loading conditions are:

1 Dead+Collateral+Snow+Snow_Drift
2 0.6Dead+0.6Wind_Left1
3 0.6Dead+0.6Wind_Right1
4 0.6Dead+0.6Wind_Long1L
5 0.6Dead+0.6Wind_Long1R
6 0.6Dead+0.6Wind_Long2L
7 Dead+Collateral+MIN_SNOW
8 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
9 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
10 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
11 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
13 Dead+Collateral+E1PAT_LL_3
14 Dead+Collateral+E1PAT_LL_4
15 Dead+Collateral+E1PAT_LL_5
16 Dead+Collateral+E1PAT_LL_7
17 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
18 Dead+0.6Wind_Right2+0.6Wind_Suction

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Elev. (in)
1	A	4	0.625	8.000	8.000	0.375	0.0
1	C	4	0.625	8.000	8.000	0.375	0.0
1	E	4	0.625	8.000	8.000	0.375	0.0
1	G	4	0.625	8.000	8.500	0.375	0.0
1	I	4	0.625	8.000	8.500	0.375	0.0
1	K	4	0.625	8.000	8.500	0.375	0.0
1	M	4	0.625	8.000	8.500	0.375	0.0
5	L	2	0.625	3.500	12.00	0.250	0.0
5	J	2	0.625	3.500	12.00	0.250	0.0
5	H	2	0.625	3.500	12.00	0.250	0.0
5	F	2	0.625	3.500	12.00	0.250	0.0
5	D	2	0.625	3.500	12.00	0.250	0.0
5	B	2	0.625	3.500	12.00	0.250	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	—Dead— Horz	Vert	—Collateral— Horz	Vert	—Live— Horz	Vert	—Snow— Horz	Vert	—Wind_Left1— Horz	Vert	—Wind_Right1— Horz	Vert
2*	A	4.0	4.3	0.7	0.6	15.8	15.0	13.8	13.1	-21.6	-20.9	-14.2	-15.6
2*	M	-4.0	4.3	-0.7	0.6	-15.8	15.0	-13.8	13.1	14.2	-15.6	21.6	-20.9

Frame Line	Column Line	—Wind_Left2— Horz	Vert	—Wind_Right2— Horz	Vert	—Wind_Long1— Horz	Vert	—Wind_Long2— Horz	Vert	—Seismic_Left— Horz	Vert	Seismic_Right Horz	Vert
2*	A	-13.6	-11.2	-6.2	-5.9	-16.8	-26.4	-17.6	-22.3	-0.4	-0.1	0.4	0.1
2*	M	6.2	-5.9	13.6	-11.2	17.6	-21.3	16.8	-25.5	-0.4	0.1	0.4	-0.1

Frame Line	Column Line	-Seismic_Long Horz	Vert	-MIN_SNOW— Horz	Vert	F1UNB_SL_L- Horz	Vert	F1UNB_SL_R- Horz	Vert
2*	A	0.0	-1.4	19.7	18.7	12.4	13.5	12.4	8.0
2*	M	0.0	-1.4	-19.7	18.7	-12.4	8.0	-12.4	13.5

Frame Line	Column Line	—Dead— Horz	Vert	—Collateral— Horz	Vert	—Live— Horz	Vert	—Snow— Horz	Vert	—Wind_Left1— Horz	Vert	—Wind_Right1— Horz	Vert
5	A	4.2	4.5	0.7	0.6	16.0	15.0	14.0	13.1	-24.2	-19.5	-17.0	-20.1
5	M	-4.2	4.5	-0.7	0.6	-16.0	15.0	-14.0	13.1	17.0	-20.1	24.2	-19.5

Frame Line	Column Line	—Wind_Left2— Horz	Vert	—Wind_Right2— Horz	Vert	—Wind_Long1— Horz	Vert	—Wind_Long2— Horz	Vert	—Seismic_Left— Horz	Vert	Seismic_Right Horz	Vert
5	A	-18.8	-16.4	-11.4	-10.4	-15.9	-17.5	-16.3	-15.4	-0.4	-0.1	0.4	0.1
5	M	11.4	-10.4	18.9	-16.4	16.3	-15.4	15.9	-17.5	-0.4	0.1	0.4	-0.1

Frame Line	Column Line	-MIN_SNOW— Horz	Vert	F2UNB_SL_L- Horz	Vert	F2UNB_SL_R- Horz	Vert
5	A	20.0	18.7	12.6	13.5	12.6	8.0
5	M	-20.0	18.7	-12.6	8.0	-12.6	13.5

Frame Line	Column Line	—Dead— Horz	Vert	—Collateral— Horz	Vert	—Live— Horz	Vert	—Snow— Horz	Vert	—Wind_Left1— Horz	Vert	—Wind_Right1— Horz	Vert
6	A	3.9	4.3	0.7	0.6	15.6	14.7	13.6	12.8	-23.3	-17.1	-22.3	-24.3
6	M	-3.9	4.3	-0.7	0.6	-15.6	14.6	-13.6	12.8	22.3	-24.3	23.3	-17.1

Frame Line	Column Line	—Wind_Left2— Horz	Vert	—Wind_Right2— Horz	Vert	—Wind_Long1— Horz	Vert	—Wind_Long2— Horz	Vert	—Seismic_Left— Horz	Vert	Seismic_Right Horz	Vert
6	A	-18.0	-20.6	-18.7	-15.3	-20.6	-19.5	10.9	10.3	-0.4	-0.1	0.4	0.1
6	M	18.7	-15.3	18.0	-20.6	20.6	-19.5	-10.9	10.3	-0.4	0.1	0.4	-0.1

Frame Line	Column Line	-MIN_SNOW— Horz	Vert	F3UNB_SL_L- Horz	Vert	F3UNB_SL_R- Horz	Vert
6	A	19.5	18.3	12.3	13.2	12.3	7.8
6	M	-19.5	18.3	-12.3	7.8	-12.3	13.2

Frame Line	Column Line	—Dead— Horz	Vert	—Collateral— Horz	Vert	—Live— Horz	Vert	—Snow— Horz	Vert	—Wind_Left1— Horz	Vert	—Wind_Right1— Horz	Vert
7	A	2.3	2.6	0.3	0.3	8.0	7.8	7.0	6.9	-11.9	-9.1	-11.4	-13.0
7	M	-2.3	2.6	-0.3	0.3	-8.0	7.8	-7.0	6.9	11.4	-13.0	11.9	-9.1

Frame Line	Column Line	—Wind_Left2— Horz	Vert	—Wind_Right2— Horz	Vert	—Wind_Long1— Horz	Vert	—Wind_Long2— Horz	Vert	—Seismic_Left— Horz	Vert	Seismic_Right Horz	Vert
7	A	-9.2	-11.0	-9.6	-8.2	-14.6	-14.4	6.1	6.1	-0.2	-0.1	0.2	0.1
7	M	9.6	-8.2	9.2	-11.0	14.6	-14.4	-6.1	6.1	-0.2	0.1	0.2	-0.1

Frame Line	Column Line	-MIN_SNOW— Horz	Vert	F4UNB_SL_L- Horz	Vert	F4UNB_SL_R- Horz	Vert
7	A	10.0	9.8	6.3	7.1	6.3	4.2
7	M	-10.0	9.8	-6.3	4.2	-6.3	7.1

2* Frame lines: 2 3 4

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Elev. (in)
2*	A	6	1.000	8.000	15.00	0.375	0.0
2*	M	6	1.000	8.000	15.00	0.375	0.0

2* Frame lines: 2 3 4

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Elev. (in)
5	A	6	1.000	8.000	16.00	0.375	0.0
5	M	6	1.000	8.000	16.00	0.375	0.0

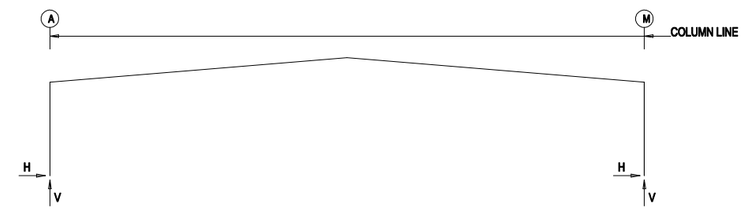
RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Elev. (in)
6	A	6	1.000	8.000	15.00	0.375	-2.0
6	M	6	1.000	8.000	15.00	0.375	-2.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Elev. (in)
7	A	4	1.000	8.000	11.00	0.375	-2.0
7	M	4	1.000	8.000	11.00	0.375	-2.0

FRAME LINES: 2 3 4 5 6 7



ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	—Dead— Horz	Vert	Collat Vert	—Live— Horz	Vert	—Snow— Horz	Vert	Snow_Drift Horz	Vert	Wind_Left1 Horz	Vert	Wind_Right1 Horz	Vert
1	A	-0.1	0.8	0.1	-0.8	3.7	-0.6	2.4	-1.0	3.4	0.0	-2.1	0.0	-1.4
1	C	-0.2	1.5	0.2	-1.5	6.6	-1.1	5.4	-1.9	6.7	-3.4	-9.4	0.0	0.4
1	E	-0.1	1.2	0.2	-0.7	6.1	-0.5	3.7	-0.9	3.5	0.0	-1.6	3.4	-6.5
1	G	0.0	0.9	0.1	0.0	4.3	0.0	2.3	0.0	0.0	0.0	-2.8	0.0	-3.4
1	I	0.0	0.8	0.1	0.0	4.1	0.0	2.2	0.0	0.0	0.0	-3.0	0.0	-4.9
1	K	0.0	0.9	0.1	0.0	4.6	0.0	2.4	0.0	0.0	0.0	-3.4	0.0	-5.9
1	M	0.0	0.5	0.0	0.0	1.7	0.0	0.9	0.0	0.0	0.0	-1.4	0.0	-2.1

Frm Line	Col Line	Wind_Left2 Horz	Vert	Wind_Right2 Horz	Vert	Wind_Press Horz	Vert	Wind_Suct Horz	Vert	Wind_Long1 Horz	Vert	Wind_Long2 Horz	Vert	Seis_Left Horz	Vert
1	A	0.0	-1.3	0.0	-0.6	0.6	-1.5	0.3	-0.8	0.0	-2.1	0.0	-1.4	0.0	0.0
1	C	-3.4	-7.6	0.0	2.2	-1.3	-3.0	3.4	-1.6	-2.0	-8.0	0.0	-1.1	-0.6	-0.6
1	E	0.0	0.0	3.4	-4.9	-2.2	-1.5	3.3	-0.8	0.0	-3.4	2.0	-5.0	0.0	0.6
1	G	0.0	-1.3	0.0	-1.9	-3.0	0.0	3.3	0.0	0.0	-3.9	0.0	-4.2	0.0	0.0
1	I	0.0	-1.4	0.0	-3.3	-2.7	0.0	3.0	0.0	0.0	-2.8	0.0	-5.3	0.0	0.0
1	K	0.0	-1.6	0.0	-4.0	-2.5	0.0	2.7	0.0	0.0	-3.4	0.0	-5.8	0.0	0.0
1	M	0.0	-0.6	0.0	-1.3	0.0	0.0	0.0	0.0	0.0	-1.4	0.0	-2.1	0.0	0.0

Frm Line	Col Line	Seis_Right Horz	Vert	Seis Long Vert	-MIN_SNOW— Horz	Vert	E1UNB_SL_L- Horz	Vert	E1UNB_SL_R- Horz	Vert	E1PAT_LL_1- Horz	Vert	E1PAT_LL_2- Horz	Vert
1	A	0.0	0.0	0.0	0.0	1.3	0.0	0.9	0.0	0.3	0.0	1.7	0.0	-0.2
1	C	0.0	0.7	0.0	0.0	3.5	0.0	2.3	0.0	0.8	0.0	4.9	0.0	1.9
1	E	0.6	-0.7	0.0	0.0	3.1	0.0	3.6	0.0	0.4	0.0	2.0	0.0	4.8
1	G	0.0	-0.1	0.0	0.0	3.2	0.0	2.6	0.0	2.6	0.0	-0.3	0.0	2.0
1	I	0.0	0.0	0.0	0.0	3.1	0.0	0.4	0.0	3.6	0.0	0.1	0.0	-0.3
1	K	0.0	0.0	0.0	0.0	3.5	0.0	0.8	0.0	2.3	0.0	0.0	0.0	0.1
1	M	0.0	0.0	0.0	0.0	1.3	0.0	0.3	0.0	0.9	0.0	0.0	0.0	0.0

Frm Line	Col Line	E1PAT_LL_3- Horz	Vert	E1PAT_LL_4- Horz	Vert	E1PAT_LL_5- Horz	Vert	E1PAT_LL_6- Horz	Vert	E1PAT_LL_7- Horz	Vert
1	A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	0.0	-0.2
1	C	0.0	-0.2	0.0	0.1	0.0	0.0	0.0	2.2	0.0	2.4
1	E	0.0	2.0	0.0	-0.3	0.0	0.1	0.0	2.1	0.0	2.0
1	G	0.0	4.8	0.0	2.0	0.0	-0.3	0.0	2.1	0.0	2.1
1	I	0.0	2.0	0.0	4.8	0.0	2.0	0.0	2.0	0.0	2.1
1	K	0.0	-0.2	0.0	2.0	0.0	4.9	0.0	2.4	0.0	2.2
1	M	0.0	0.0	0.0	-0.2	0.0	1.7	0.0	-0.2	0.0	2.0

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz	Seis Long Vert
5	L	0.1	-2.1	2.3	0.0
5	J	0.1	-2.2	2.5	0.0
5	H	0.1	-2.8	3.1	0.0
5	F	0.1	-2.8	3.1	0.0
5	D	0.1	-2.2	2.5	0.0
5	B	0.1	-2.1	2.3	0.0

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1	C,E	3.4	3.5	0.6	0.7			
F_SW	M	3,4	7.6	4.3	2.5	1.4			
R_EW	5								(h)
B_SW	A	4,3	9.3	5.2	2.5	1.4			

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH
Kyle McDonough
10-21-2025

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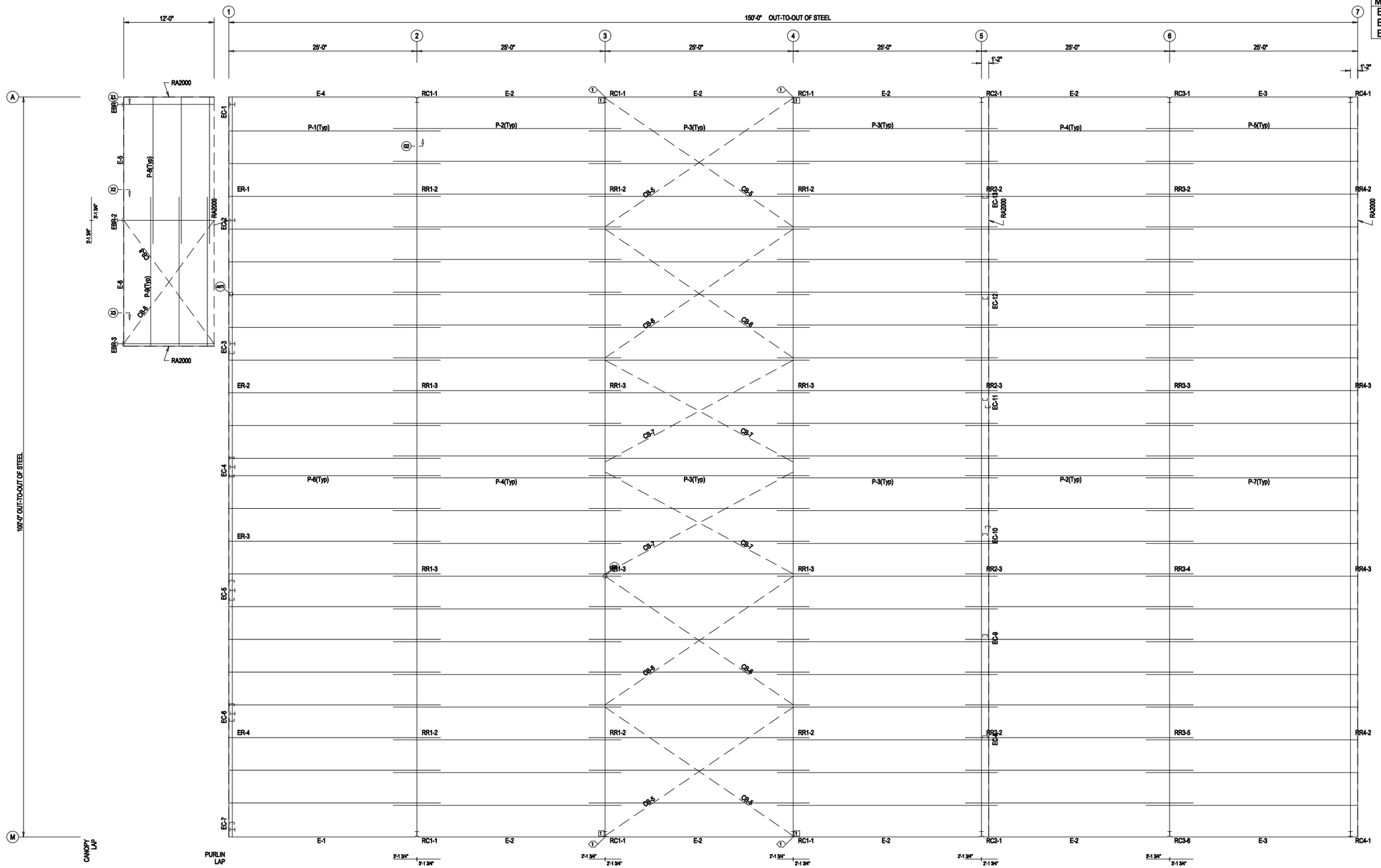
DESCRIPTION: ANCHOR BOLT REACTIONS

CUSTOMER: MAVERICK STEEL BUILDINGS		PROJECT:	Ricky Earnhardt
LOCATION:	Lillington, NC 27504		

DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF



ROOF FRAMING PLAN

EXTENSION/CANOPY BOLTS
ROOF PLAN

MARK	QUAN	TYPE	DIA	LENGTH
EBR-1	8	A325	5/8"	1 3/4"
EBR-2	8	A325	5/8"	2"
EBR-3	8	A325	5/8"	1 3/4"

SPECIAL BOLTS
ROOF PLAN

◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A307	1/2"	1 1/4"	0

MEMBER TABLE
ROOF PLAN

MARK	PART
EBR-1	BEAM
EBR-2	BEAM
EBR-3	BEAM
P-1	8X25Z14
P-2	8X25Z16
P-3	8X25Z16
P-4	8X25Z16
P-5	8X25Z16
P-6	8X25Z14
P-7	8X25Z16
P-8	8X25Z14
P-9	8X25Z14
E-1	8ES141
E-2	8ES141
E-3	8ES141
E-4	8ES141
E-5	8ES141
E-6	8ES141
CB-5	0.31_CBL
CB-6	0.25_CBL
CB-7	0.25_CBL
CB-8	0.25_CBL

CONNECTION PLATES
ROOF PLAN

□ID	MARK/PART
1	AK106

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Kyle McDonough
10-21-2025

GENERAL NOTES:

Refer to the cover sheet C1 for
General Framing and Sheeting & Trim notes.

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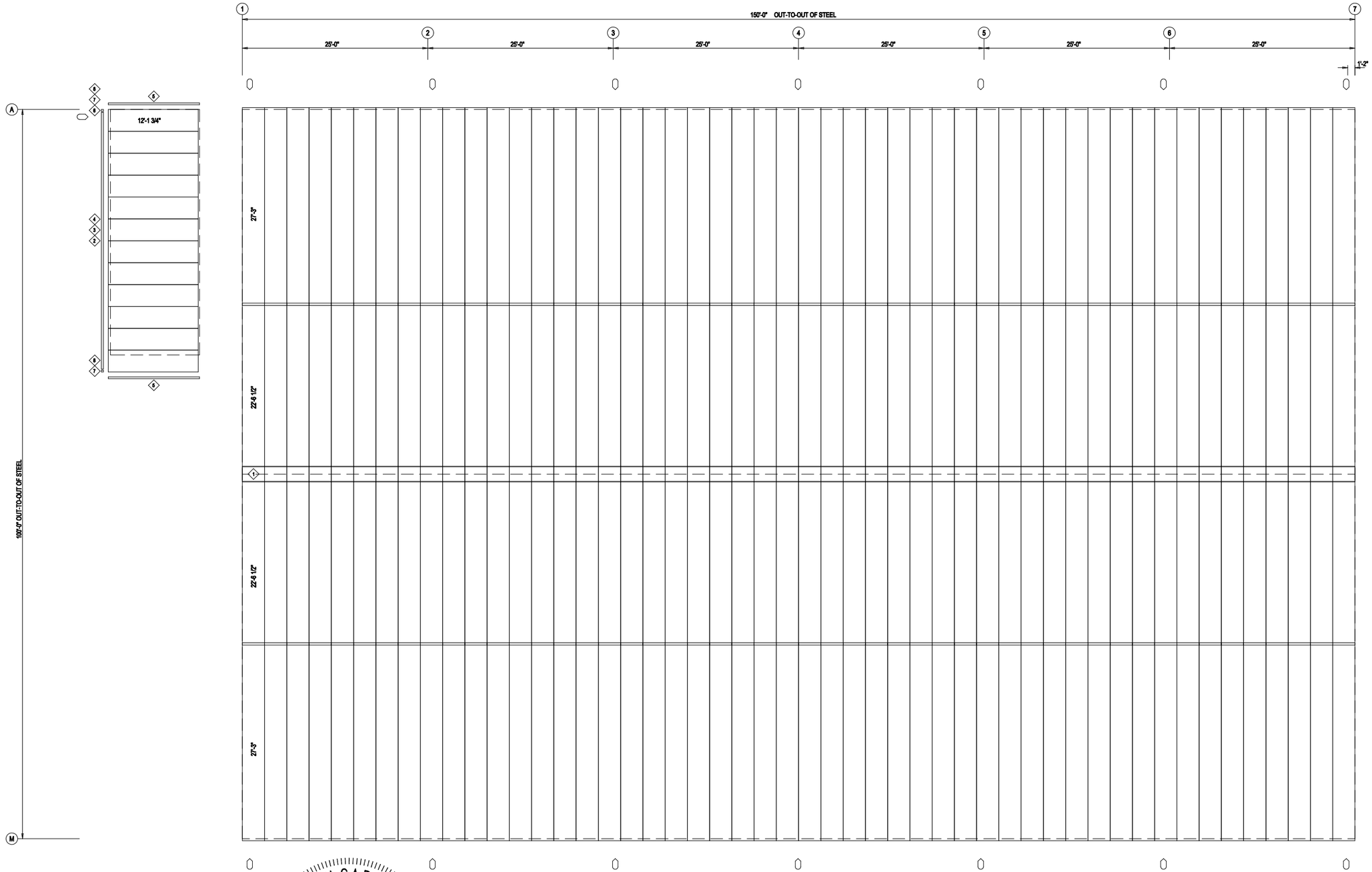
DESCRIPTION: ROOF FRAMING						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

DOWNSPOUT LOCATIONS

ROOF SHEETING TRIM TABLE			
◇ID	PART	LENGTH	QTY
1	SSRC30	3'-0"	50
2	FL18C	15'-2"	1
3	FL18D	20'-2"	1
4	GC-10_4	10'-2"	4
5	FL16	15'-2"	2
6	GS-121	9"	1
7	FL16C4	8"	2
8	FL18A4	8"	2



ROOF SHEETING PLAN

PANELS: 26 Ga. Super Span X - Black

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough 10-21-2025

GENERAL NOTES:

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DESCRIPTION: ROOF SHEETING

CUSTOMER: MAVERICK STEEL BUILDINGS | PROJECT: Ricky Earnhardt

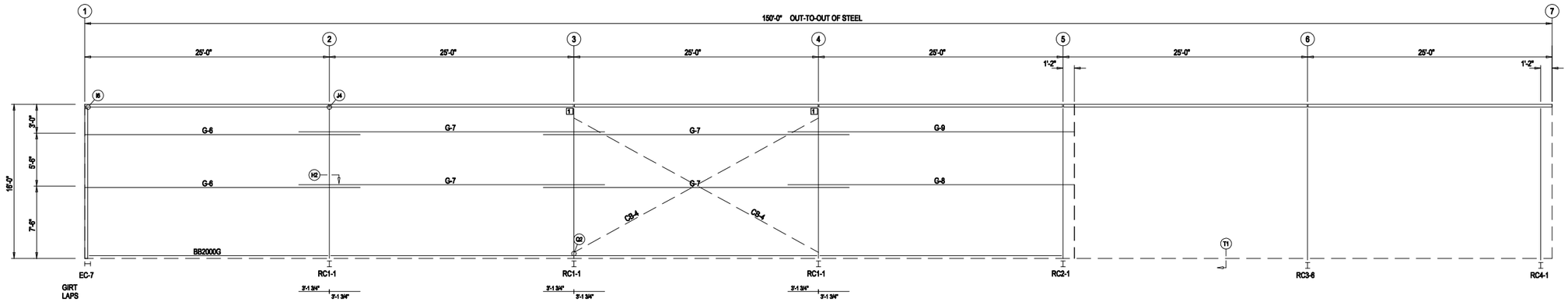
LOCATION: Lillington, NC 27504

DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

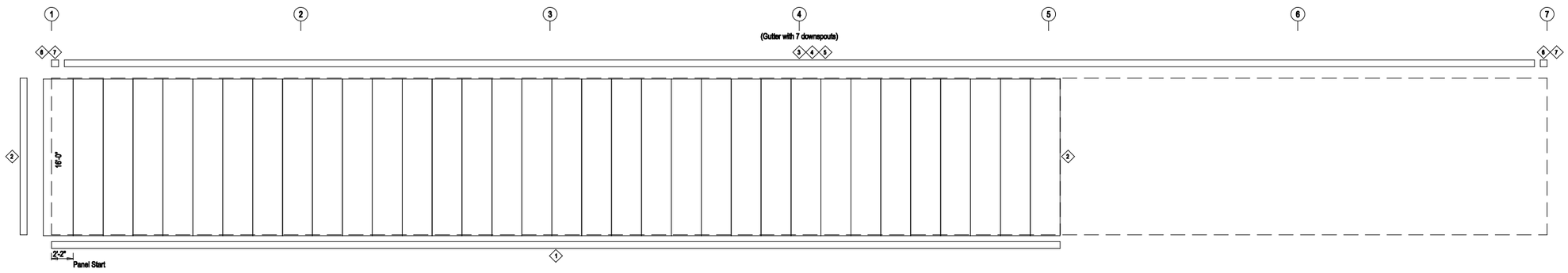
TRIM TABLE - THIS WALL ONLY FRAME LINE - M			
◇ID	PART	LENGTH	QTY
1	BT-101	10'-3"	10
2	CT-102	16'-0"	2
3	FL18C	15'-2"	1
4	FL18D	20'-2"	7
5	GC-10_4	10'-2"	15
6	FL16C4	8"	2
7	FL18A4	8"	2

MEMBER TABLE FRAME LINE M	
MARK	PART
G-6	8X25Z16
G-7	8X25Z16
G-8	8X25Z14
G-9	8X25Z16
CB-4	0.38_CBL

CONNECTION PLATES FRAME LINE M	
□ID	MARK/PART
1	AK106



SIDEWALL FRAMING: FRAME LINE M



SIDEWALL SHEETING & TRIM: FRAME LINE M

PANELS: 26 Ga. Super Span X - Cool White

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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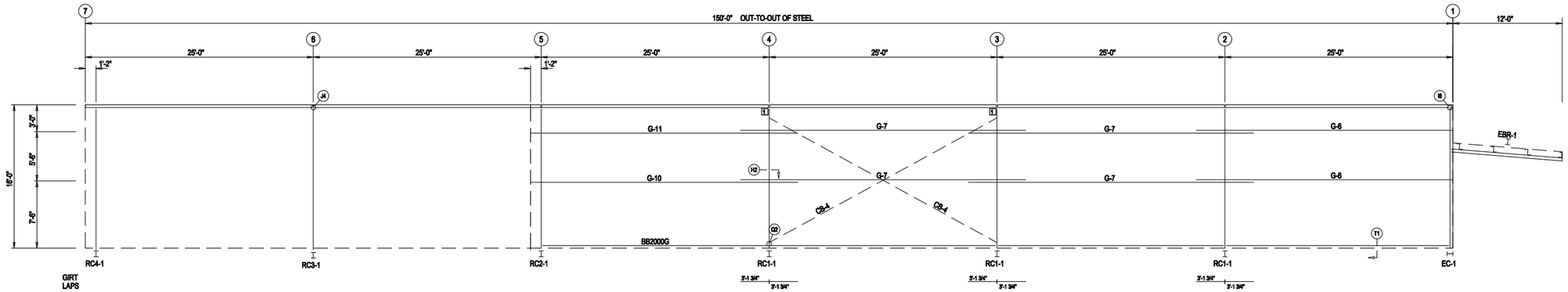
Ph. (888) 346-2426 Maverickssteelbuildings.com

DESCRIPTION: SIDEWALL FRAMING						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

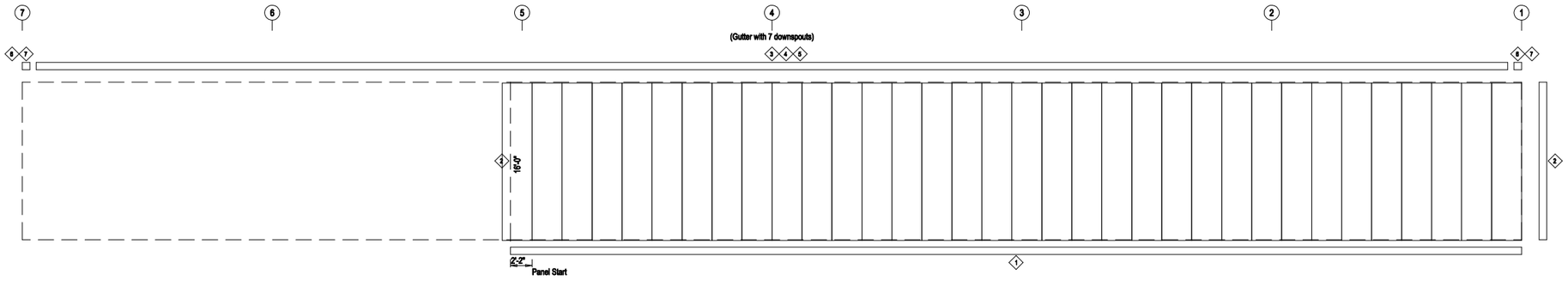
TRIM TABLE - THIS WALL ONLY FRAME LINE - A			
◇ID	PART	LENGTH	QTY
1	BT-101	10'-3"	10
2	CT-102	16'-0"	2
3	FL18C	15'-2"	1
4	FL18D	20'-2"	7
5	GC-10_4	10'-2"	15
6	FL16C4	8"	2
7	FL18A4	8"	2

MEMBER TABLE FRAME LINE A	
MARK	PART
EBR-1	BEAM
G-6	8X25Z16
G-7	8X25Z16
G-10	8X25Z14
G-11	8X25Z16
CB-4	0.38_CBL

CONNECTION PLATES FRAME LINE A	
□ID	MARK/PART
1	AK106



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. Super Span X - Cool White

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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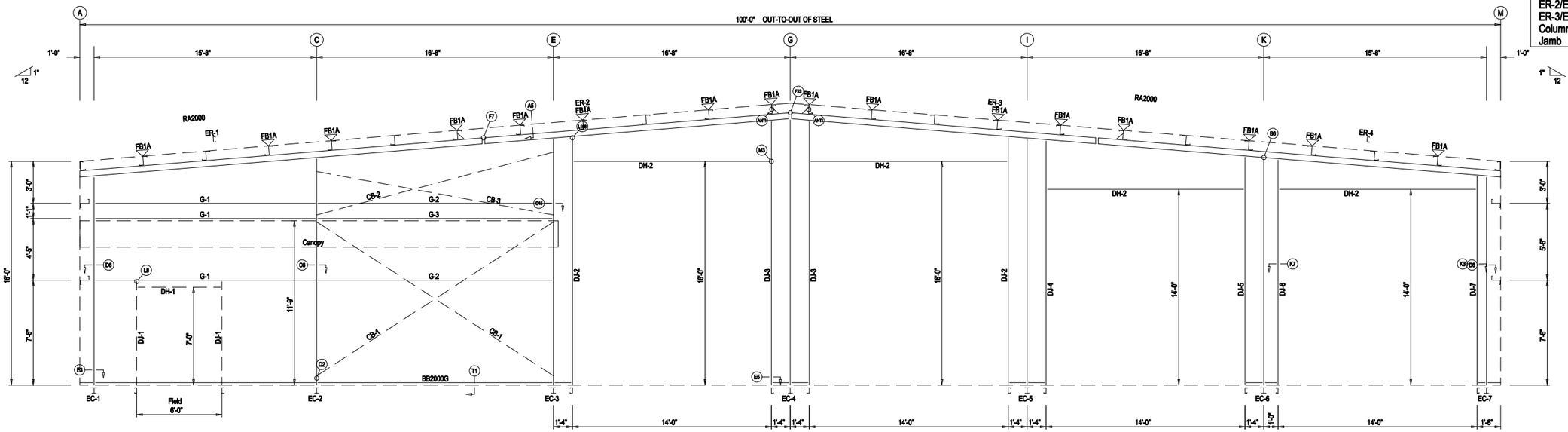
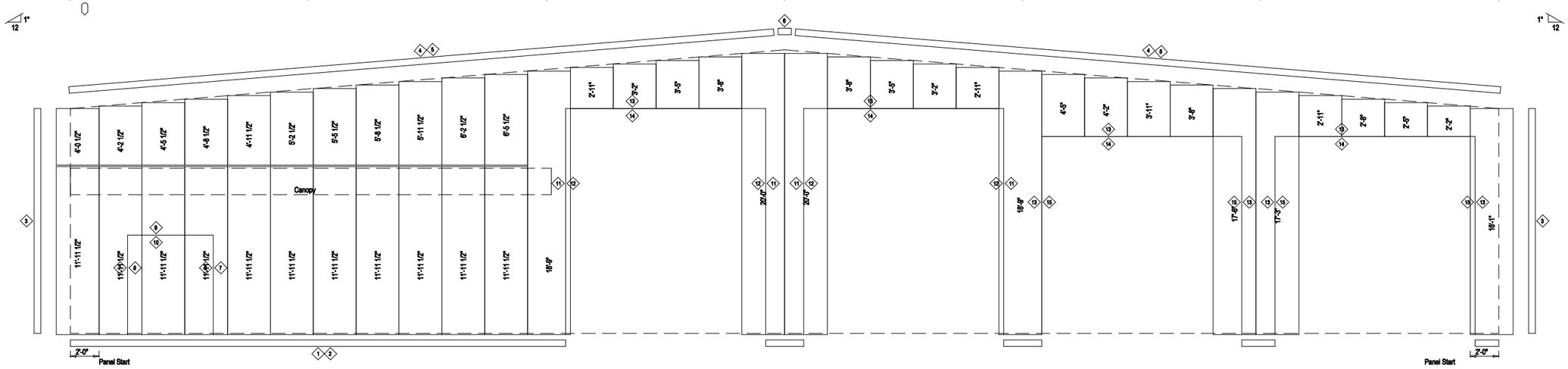
DESCRIPTION: SIDEWALL FRAMING						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

DOWNSPOUT LOCATIONS



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. Super Span X - Cool White

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	2"
ER-2/ER-3	4	A325	5/8"	2"
ER-3/ER-4	4	A325	5/8"	2"
Columns/Raf	4	A325	5/8"	1 1/2"
Jamb	2	A325	5/8"	1 1/2"

FLANGE BRACE TABLE
FRAME LINE 1

▽ID	MARK	LENGTH
1	FB1A	2'-5"

TRIM TABLE - THIS WALL ONLY
FRAME LINE - 1

◇ID	PART	LENGTH	QTY
1	BT-101	10'-3"	5
2	BT-101	SCRAP	
3	CT-102	16'-0"	2
4	FL16	15'-2"	2
5	FL16D	20'-2"	4
6	FL16B	1'-10 1/2"	1
7	MT-116B	7'-0"	2
8	JT-101	7'-3"	2
9	MT-116B	6'-0"	1
10	FL26	6'-4"	1
11	MT-116B	16'-0"	4
12	JT-101	16'-3"	4
13	MT-116B	14'-0"	8
14	FL26	14'-4"	4
15	JT-101	14'-3"	4

MEMBER TABLE
FRAME LINE 1

MARK	PART
EC-1	W8X24
EC-2	W8X24
EC-3	W8X24
EC-4	W8X18
EC-5	W8X18
EC-6	W8X18
EC-7	W8X18
ER-1	8M35C12
ER-2	8M35C12
ER-3	8M35C12
ER-4	8M35C12
DJ-1	8M35C14
DJ-2	8M35C14
DJ-3	8M35C14
DJ-4	8M35C14
DJ-5	8M35C14
DJ-6	8M35C14
DJ-7	8M35C14
DH-1	8M25C14
DH-2	8M25C14
G-1	8X25Z16
G-2	8X25Z16
G-3	8X25Z12
CB-1	0.25_CBL
CB-2	0.25_CBL
CB-3	0.25_CBL

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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MAVERICK STEEL BUILDINGS INC.
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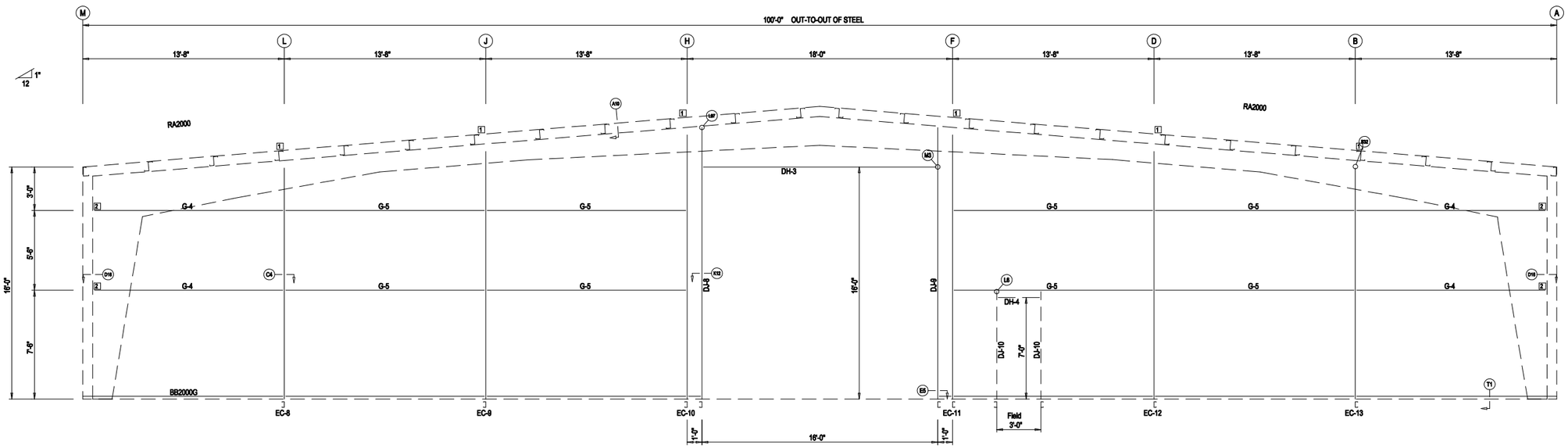
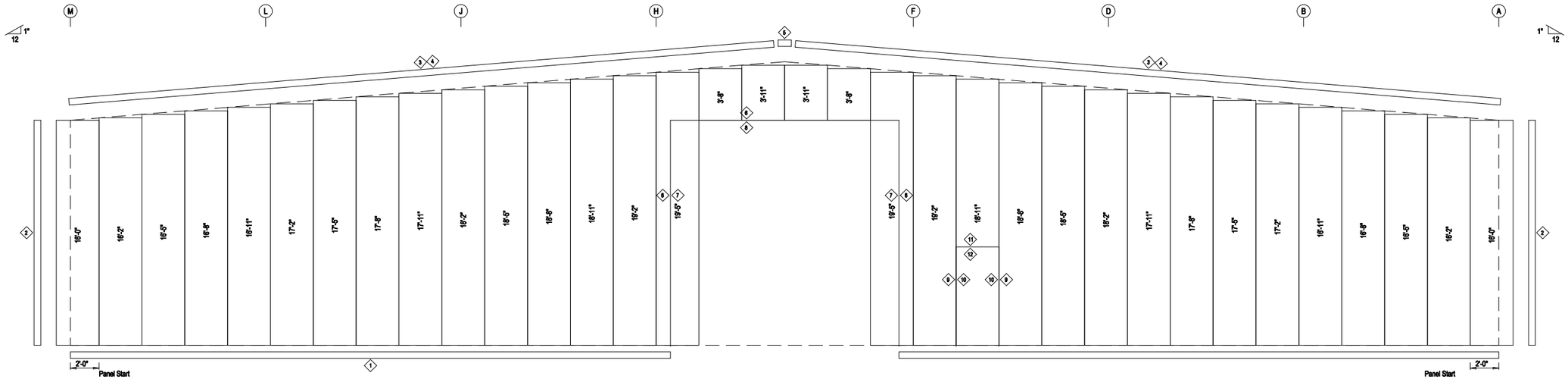
52 Apex Dr Jefferson, GA 30549
Ph. (888) 346-2426 Maverickssteelbuildings.com

DESCRIPTION: ENDWALL FRAMING						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF



ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. Super Span X - Cool White

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	4	A325	5/8"	1 1/2"
Jamb	2	A325	5/8"	1 1/2"

TRIM TABLE - THIS WALL ONLY
FRAME LINE - 5

◇ID	PART	LENGTH	QTY
1	BT-101	10'-3"	8
2	CT-102	16'-0"	2
3	FL16	15'-2"	2
4	FL16D	20'-2"	4
5	FL16B	1'-10 1/2"	1
6	MT-116B	16'-0"	3
7	JT-101	16'-3"	2
8	FL26	16'-4"	1
9	MT-116B	7'-0"	2
10	JT-101	7'-3"	2
11	MT-116B	3'-0"	1
12	FL26	3'-4"	1

MEMBER TABLE
FRAME LINE 5

MARK	PART
EC-8	12M35C14
EC-9	12M35C14
EC-10	12M35C14
EC-11	12M35C14
EC-12	12M35C14
EC-13	12M35C14
DJ-8	8M25C14
DJ-9	8M25C14
DJ-10	8M25C14
DH-3	8M25C14
DH-4	8M25C14
G-4	8X25Z16
G-5	8X25Z16

CONNECTION PLATES
FRAME LINE 5

□ID	MARK/PART
1	n1
2	AK244

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

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LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-8 1/4"
SP-2	4	4	0	A325	3/4"	2"	6"	1/2"	2'-1"
SP-3	4	4	2	A325	3/4"	2"	6"	1/2"	2'-10"

FLANGE BRACE TABLE

A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 2 3 4

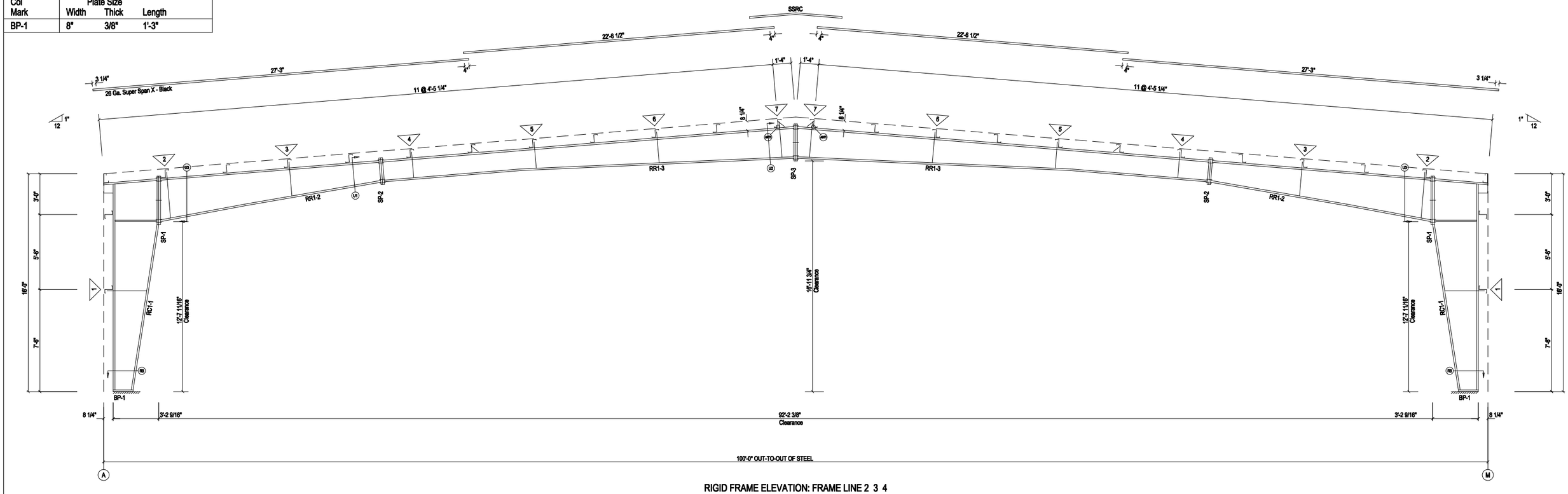
▽ ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB22B	3'-4 3/4"	2'-4"		
2	2	FB27A	4'-2 1/8"	3'-0"		
3	1	FB18B	3'-1"	2'-4"		
4	1	FB5A	2'-8 1/2"	2'-4"		
5	1	FB6A	2'-8 7/8"	2'-4"		
6	1	FB12A	2'-11 1/4"	2'-4"		
7	2	FB20A	3'-2"	2'-4"		

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	1'-3"

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RC1-1	14.0/38.0	0.250	147.1	6 x 1/4" x 183.5	6 x 5/16" x 149.0
	38.0/38.0	0.313	39.6	6 x 1/2" x 46.6	
RR1-2	35.0/27.0	0.250	75.6	6 x 1/4" x 192.6	6 x 3/8" x 76.0
	27.0/16.0	0.184	120.0		6 x 1/2" x 120.5
RR1-3	16.0/16.0	0.133	120.0	6 x 3/8" x 120.0	6 x 1/2" x 120.0
	16.0/25.0	0.133	240.0	6 x 1/2" x 240.0	6 x 1/4" x 238.1



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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DESCRIPTION: RIGID FRAME ELEVATION						
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-9 1/4"
SP-2	4	4	0	A325	3/4"	2"	8"	1/2"	2'-0"
SP-3	4	4	2	A325	3/4"	2"	8"	1/2"	2'-8"

FLANGE BRACE TABLE

A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 5

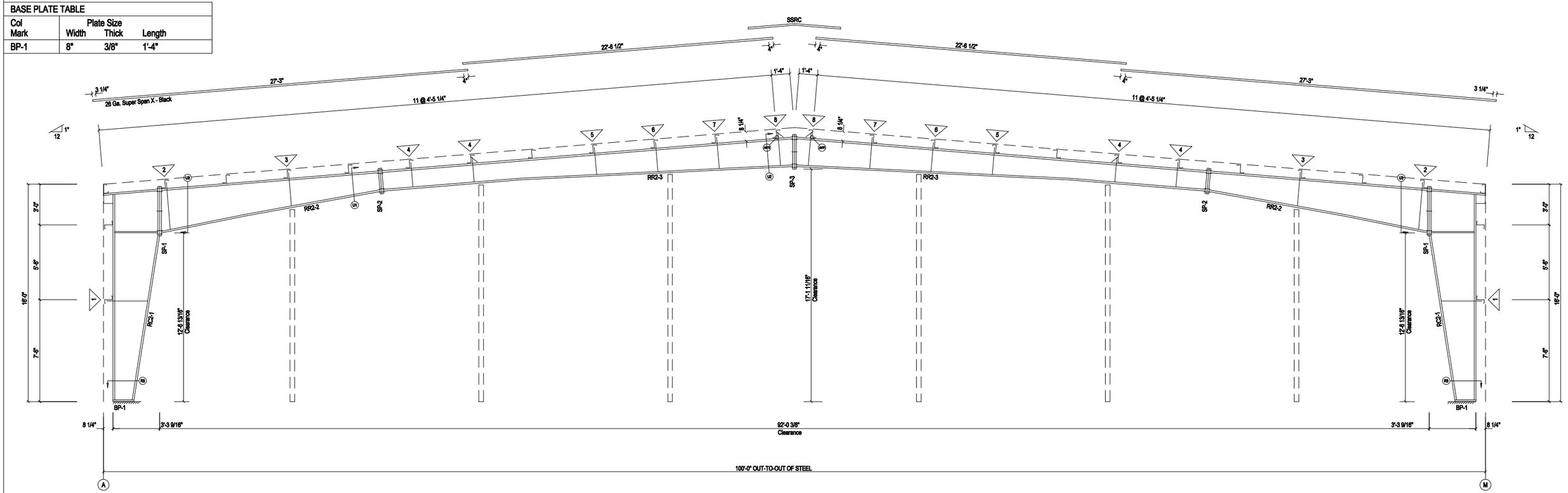
▽ ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB24B	3'-5 1/2"	2'-4"		
2	2	FB28A	4'-2 7/8"	3'-0"		
3	1	FB25B	3'-7"	3'-0"		
4	1	FB4A	2'-8"	2'-4"		
5	1	FB7A	2'-9 1/4"	2'-4"		
6	1	FB11A	2'-10 1/4"	2'-4"		
7	1	FB13A	2'-11 3/8"	2'-4"		
8	2	FB16A	3'-0 1/2"	2'-4"		

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	1'-4"

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RC2-1	15.0/39.0	0.250	146.2	6 x 1/4" x 183.5	6 x 5/16" x 148.1
	39.0/39.0	0.313	40.6	6 x 1/2" x 47.6	
RR2-2	36.0/27.0	0.250	74.7	8 x 1/4" x 191.6	8 x 3/8" x 75.2
	27.0/15.1	0.184	120.0		8 x 1/2" x 120.5
RR2-3	15.0/15.0	0.133	120.0	8 x 5/16" x 120.0	8 x 1/2" x 120.0
	15.0/23.0	0.133	240.0	8 x 3/8" x 240.0	8 x 1/4" x 238.2



RIGID FRAME ELEVATION: FRAME LINE 5

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-7 1/4"
SP-2	4	4	0	A325	3/4"	2"	6"	1/2"	2'-4"
SP-3	4	4	2	A325	3/4"	2"	6"	1/2"	2'-9"
SP-4	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-9 1/4"

FLANGE BRACE TABLE

A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 6

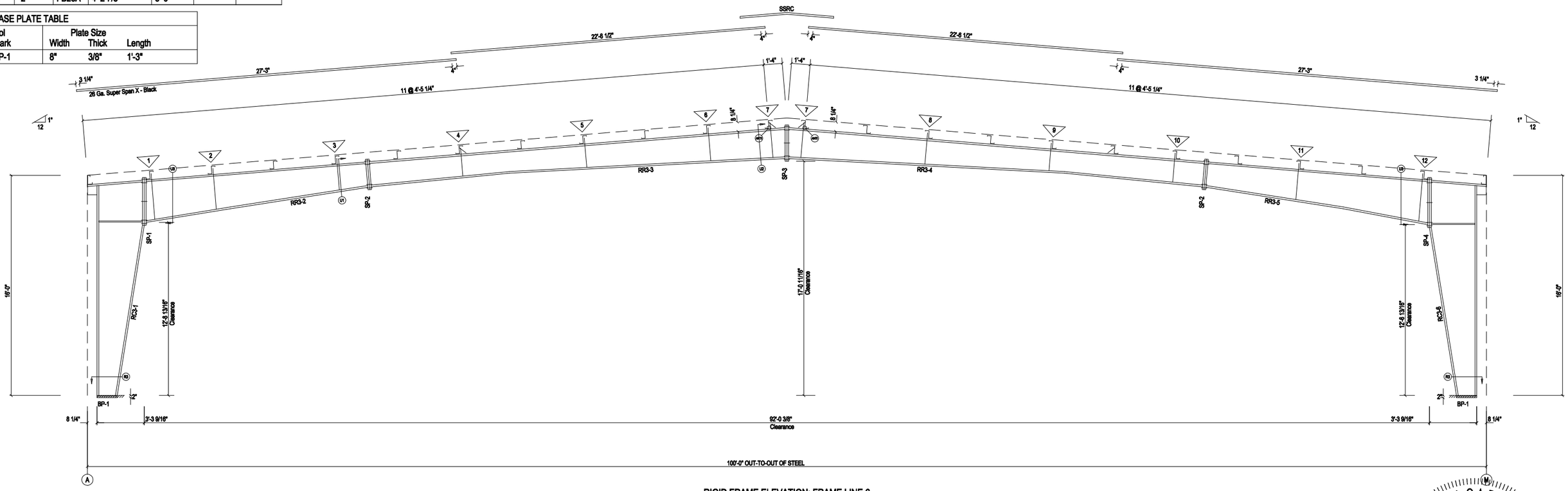
▽ ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	2	FB26A	4'-1 5/8"	3'-0"		
2	1	FB23B	3'-5"	2'-4"		
3	1	FB14A	2'-11 1/2"	2'-4"		
4	1	FB8A	2'-9 1/2"	2'-4"		
5	1	FB11A	2'-10 1/4"	2'-4"		
6	1	FB15A	3'-0 1/4"	2'-4"		
7	2	FB19A	3'-1 1/4"	2'-4"		
8	1	FB12A	2'-11 1/4"	2'-4"		
9	1	FB7A	2'-9 1/4"	2'-4"		
10	1	FB10A	2'-10"	2'-4"		
11	1	FB21B	3'-2 1/8"	2'-4"		
12	2	FB28A	4'-2 7/8"	3'-0"		

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	1'-3"

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RC3-1	14.0/39.0	0.250	150.2	8 x 1/4" x 185.5	8 x 5/16" x 152.2
	39.0/39.0	0.313	38.6	8 x 1/2" x 47.6	
RR3-2	34.0/28.0	0.250	74.5	6 x 1/4" x 191.6	6 x 3/8" x 74.7
	28.0/19.0	0.183	120.0		6 x 3/8" x 120.3
RR3-3	19.0/17.0	0.161	120.0	6 x 5/16" x 120.0	6 x 5/16" x 120.0
	17.0/24.0	0.133	240.0	6 x 3/8" x 240.0	6 x 1/4" x 238.1
RR3-4	24.0/17.0	0.133	240.0	6 x 3/8" x 240.0	6 x 1/4" x 238.1
	17.0/19.0	0.161	120.0	6 x 5/16" x 120.0	6 x 5/16" x 120.0
RR3-5	19.0/28.0	0.183	120.0	6 x 1/4" x 191.6	6 x 3/8" x 120.3
	28.0/36.0	0.250	74.7		6 x 3/8" x 75.1
RC3-6	39.0/39.0	0.313	40.6	8 x 1/2" x 47.6	8 x 5/16" x 150.2
	39.0/14.0	0.250	148.2	8 x 1/4" x 185.5	



RIGID FRAME ELEVATION: FRAME LINE 6

GENERAL NOTES:

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NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

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Ph. (888) 346-2426 Maverickssteelbuildings.com

DESCRIPTION: RIGID FRAME ELEVATION

CUSTOMER: MAVERICK STEEL BUILDINGS | PROJECT: Ricky Earnhardt

LOCATION: Lillington, NC 27504

DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2 1/2"	6"	3/4"	2'-9 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11"
SP-3	4	4	0	A325	3/4"	2"	6"	1/2"	2'-3"

FLANGE BRACE TABLE

A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 7

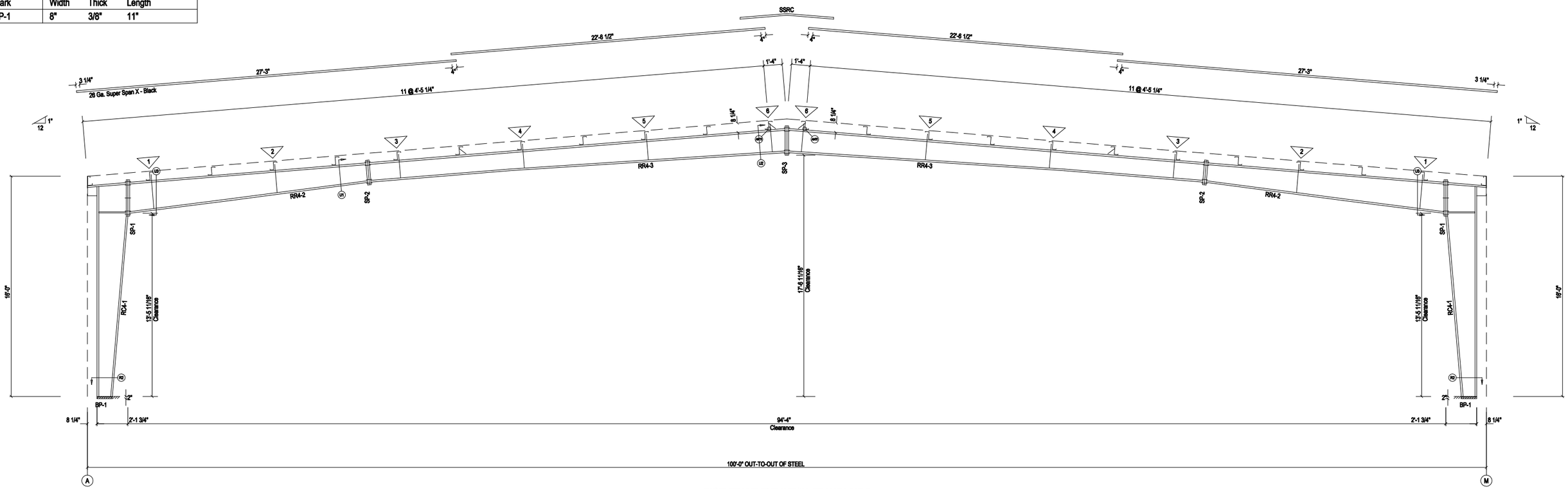
▽ ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB17B	3'-0 3/4"	2'-4"		
2	1	FB9A	2'-9 5/8"	2'-4"		
3	1	FB2A	2'-7 1/2"	2'-4"		
4	1	FB3A	2'-7 5/8"	2'-4"		
5	1	FB5A	2'-8 1/2"	2'-4"		
6	1	FB8A	2'-9 1/2"	2'-4"		

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	11"

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RC4-1	10.0/25.0	0.161	159.0	6 x 1/4" x 185.5	6 x 1/2" x 159.7
	25.0/25.0	0.250	28.6	6 x 1/2" x 33.6	
RR4-2	24.0/20.0	0.161	87.7	6 x 5/16" x 85.7	6 x 3/8" x 87.8
	20.0/14.0	0.133	120.0	6 x 1/4" x 120.0	6 x 5/16" x 120.1
RR4-3	14.0/14.0	0.133	120.0	6 x 1/4" x 120.0	6 x 1/4" x 120.0
	14.0/18.0	0.133	240.0	6 x 5/16" x 240.0	6 x 1/4" x 238.5



RIGID FRAME ELEVATION: FRAME LINE 7

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough

10-21-2025

GENERAL NOTES:

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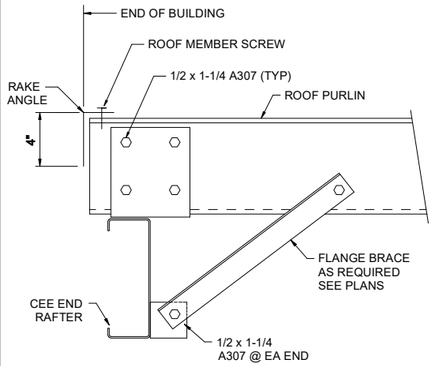
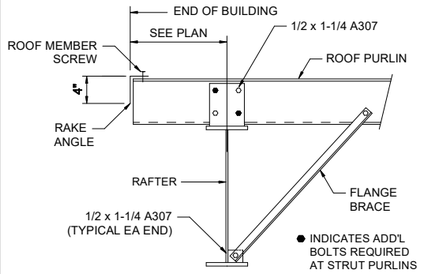
CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF



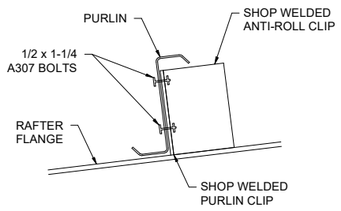
A5 CEE RAFTER TO PURLIN CONNECTION



(2) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(4) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

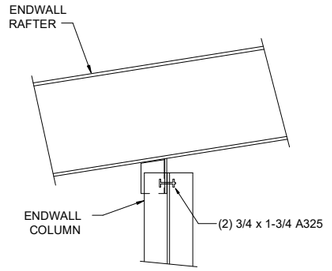
A STRUT PURLIN IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

A10 ROOF PURLIN CONNECTION AT MAIN FRAME ENDWALL

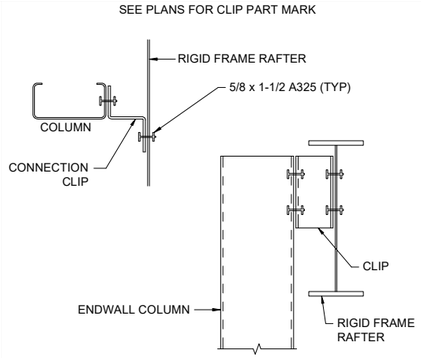


SOME PURLIN CLIPS WILL HAVE AN ADDITIONAL ANTI-ROLL CLIP ATTACHED. THE QUANTITY AND SPACING OF THESE CLIPS IS DETERMINED BY THE DESIGN FOR EACH SPECIFIC BUILDING.

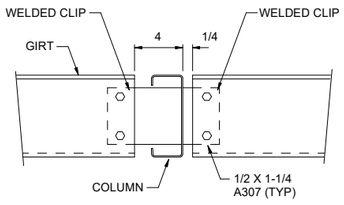
ANTI DETAIL AT ANTI-ROLL CLIP



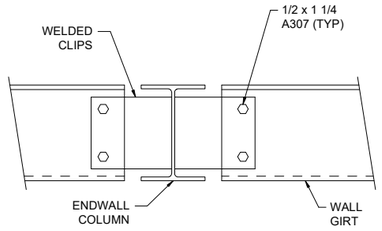
B6 ENDWALL RAFTER TO COLUMN



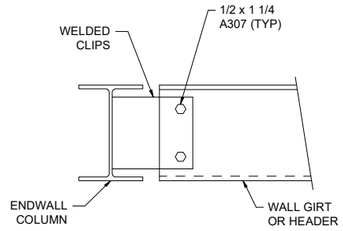
B32 CEE COLUMN TO RIGID FRAME RAFTER



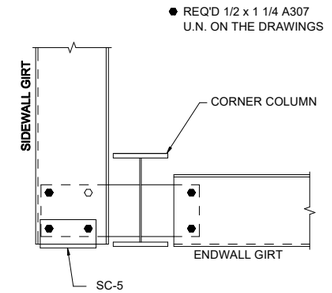
C4 GIRT TO COLUMN



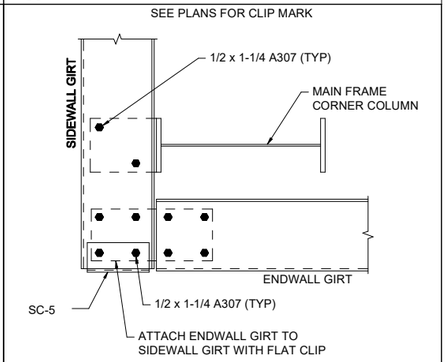
C6 ENDWALL GIRT TO COLUMN



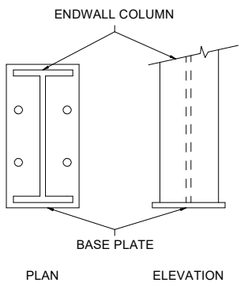
C15 GIRT/HEADER TO COLUMN



D6 CORNER COLUMN TO WALL GIRT

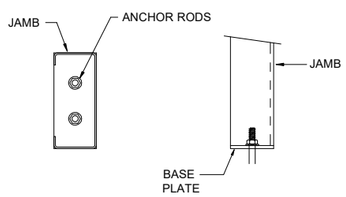


D16 GIRT CONNECTION AT CORNER SW & EW GIRTS AT SAME ELEVATION



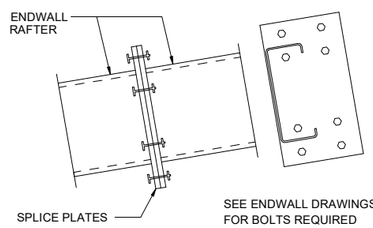
SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

E3 BASE PLATE FOR ENDWALL COLUMN

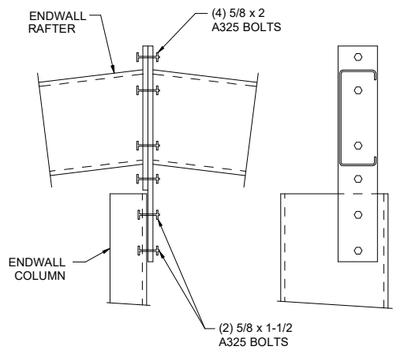


- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.

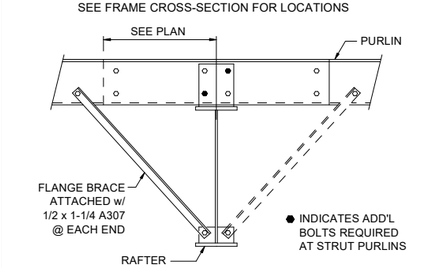
E5 BASE PLATE FOR DOOR JAMB



F7 RAFTER SPLICE ALONG SURFACE



F23 RAFTER SPLICE AT SURFACE CHANGE



(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTIONS TO SIZE & QTY OF BOLTS.

G2 ROOF PURLIN TO INTERIOR FRAME RAFTER

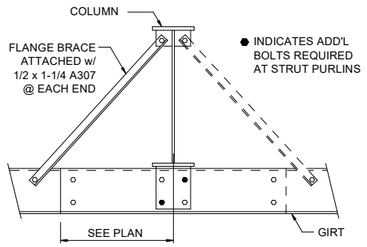
NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH
Kyle McDonough
10-21-2025

MAVERICK STEEL BUILDINGS INC.
WWW.MAVERICKSTEELBUILDINGS.COM
52 Apex Dr Jefferson, GA 30549
Ph. (888) 346-2426 Maverickssteelbuildings.com

DESCRIPTION: DETAIL DRAWINGS

CUSTOMER: MAVERICK STEEL BUILDINGS					PROJECT: Ricky Earnhardt	
LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
		10/20/25	N.T.S.	00	25-448	OF

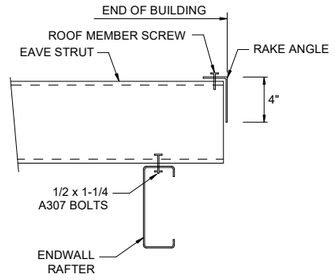
FLANGE BRACES MAY BE ON ONE OR BOTH SIDES
SEE FRAME CROSS-SECTION FOR LOCATIONS



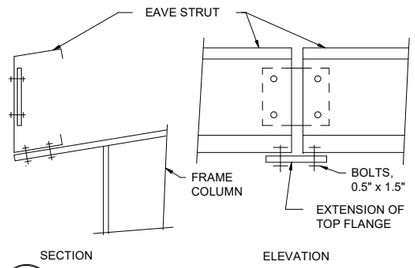
(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS.
SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

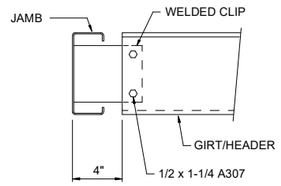
H2 WALL GIRT TO FRAME COLUMN



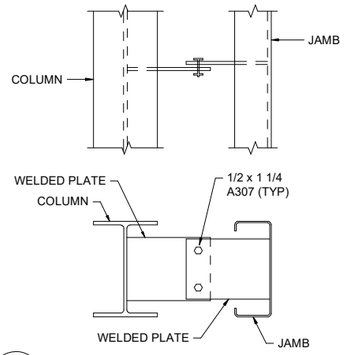
I6 EAVE STRUT TO ENDWALL RAFTER



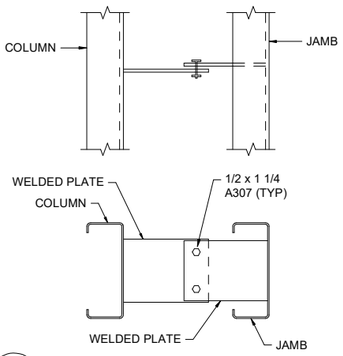
J4 EAVE STRUT TO RIGID FRAME



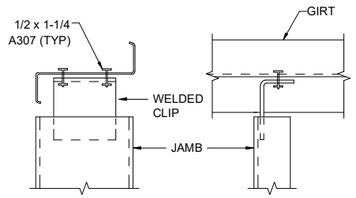
K3 WALL GIRT TO DOOR JAMB



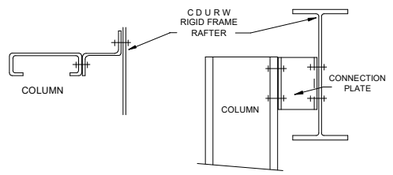
K7 ENDWALL CONNECTION TO JAMB



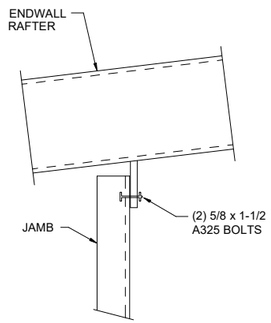
K12 ENDWALL CONNECTION TO JAMB



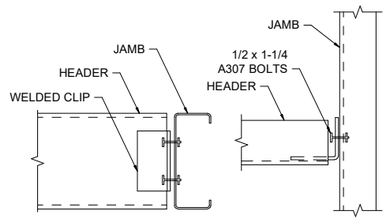
L8 DOOR JAMB TO WALL GIRT



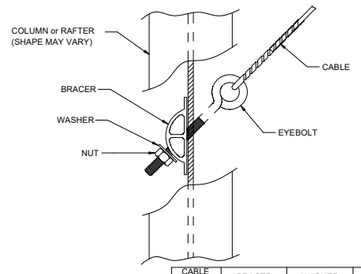
L97 C/U COLUMN TO RIGID FRAME RAFTER



L108 ENDWALL RAFTER TO JAMB

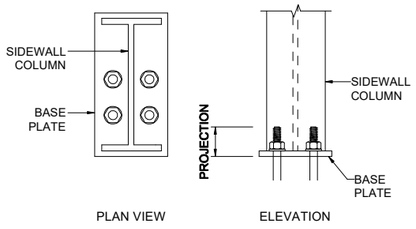


M3 HEADER TO CEE JAMB



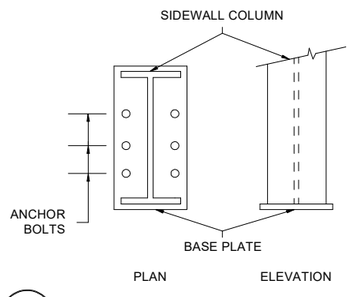
CABLE SIZE	BRACER	WASHER	NUT
1/4"	BRACER #1	F844 1/2"	A563 1/2"
5/16"	BRACER #1	F844 5/8"	A563 5/8"
3/8"	BRACER #2	F844 3/4"	A563 3/4"
1/2"	BRACER #2	F844 7/8"	A563 7/8"

Q2 DIAGONAL CABLE BRACING INSTALLATION

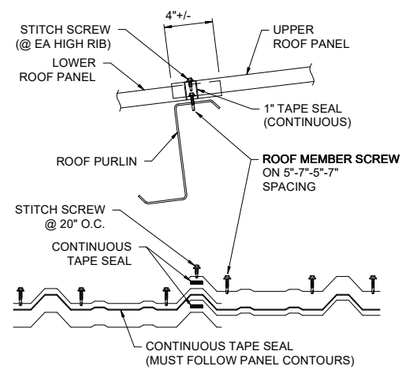


- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

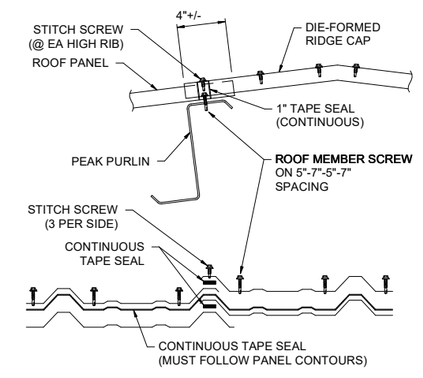
R2 ANCHOR RODS AT SIDEWALL COLUMN



R3 ANCHOR BOLTS AT SIDEWALL COLUMN



SCREW_2 TYPICAL ROOF PANEL ENDLAP SUPER SPAN X ROOF



SCREW_8 TYPICAL DIE-FORMED RIDGECAP ENDLAP - SUPER SPAN X ROOF

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH
Kyle McDonough
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LOCATION: Lillington, NC 27504						
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
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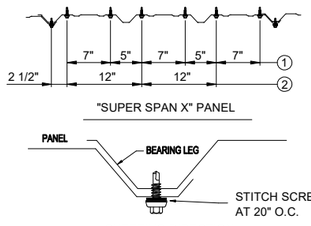
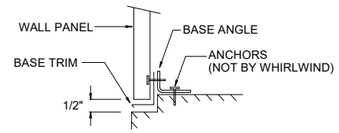
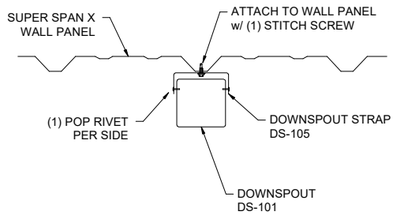


SCREW_25 FASTENER PATTERN FOR WALL PANEL (SUPER SPAN X)



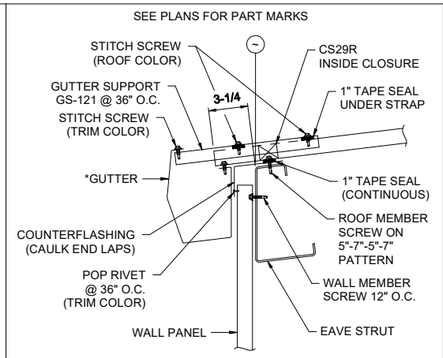
— PANELS MUST NOT REST ON CONCRETE OR PANEL WARRANTY WILL BE VOID.
— ANCHORS MUST BE ADEQUATE TO SAFELY RESIST THE LOADS IN THE REACTION REPORT.
— WALL MEMBER SCREWS ARE AT 6" O.C AT THE BASE MEMBER.

T1 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION



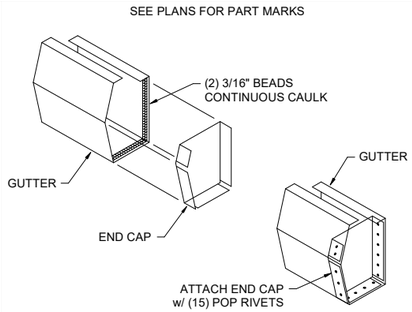
ATTACH ONE DS-105 STRAP AT THE BASE OF THE DOWNSPOUT THEN APPROXIMATELY 5'-0" CENTERS (8'-0" MAX.)

TRIM_7 DOWNSPOUT STRAP ATTACHMENT



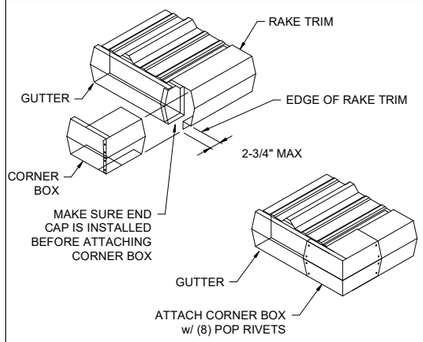
*LAP GUTTER 2" w/ (2) CONTINUOUS BEADS OF CAULK

TRIM_8 TYPICAL GUTTER DETAIL SUPER SPAN X ROOF & WALLS



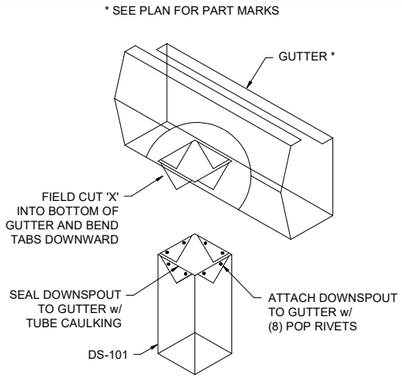
MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL AND THERE ARE NO BREAKS IN THE BEADS OF CAULK

TRIM_9 GUTTER END CAP ATTACHMENT

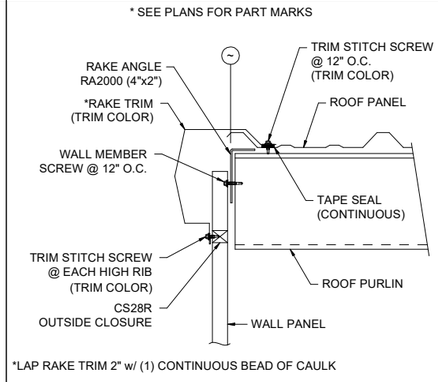


MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL

TRIM_10 CORNER BOX ATTACHMENT

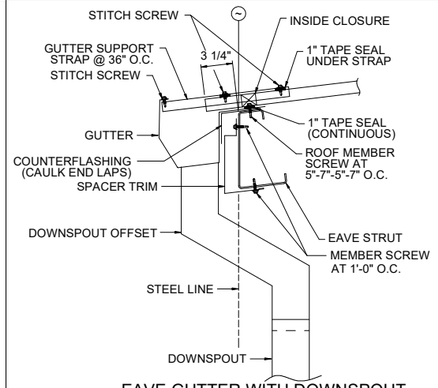


TRIM_11 DOWNSPOUT ATTACHMENT @ GUTTER

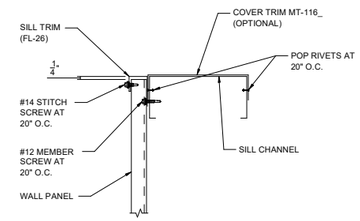


*LAP RAKE TRIM 2" w/ (1) CONTINUOUS BEAD OF CAULK

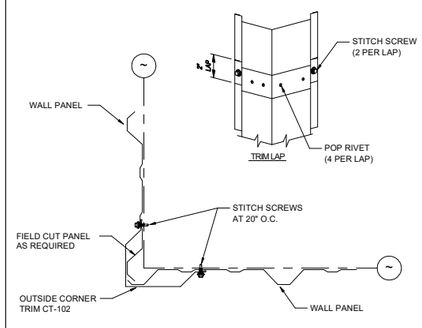
TRIM_12 TYPICAL RAKE DETAIL SUPER SPAN X ROOF & WALLS



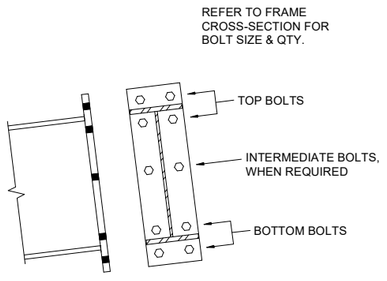
TRIM_151 EAVE GUTTER WITH DOWNSPOUT AT OPEN WALLS - BY-PASS (FL)



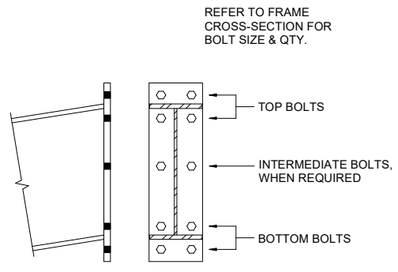
TRIM_180 SILL TRIM DETAIL (SUPER SPAN X)



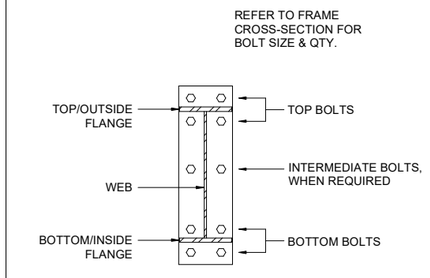
TRIM_203 CORNER TRIM INSTALLATION (SUPER SPAN X)



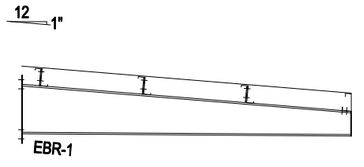
U1 BOLTED END PLATE RAFTER SPLICE



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK



U3 BOLTS FOR RAFTER TO COLUMN CONNECTION



X1

NORTH CAROLINA PROFESSIONAL SEAL 048363 ENGINEER KYLE MCDONOUGH

Kyle McDonough 10-21-2025

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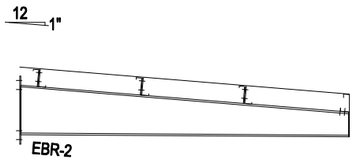

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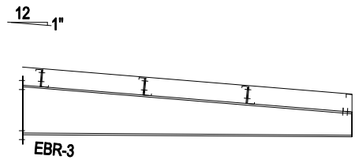

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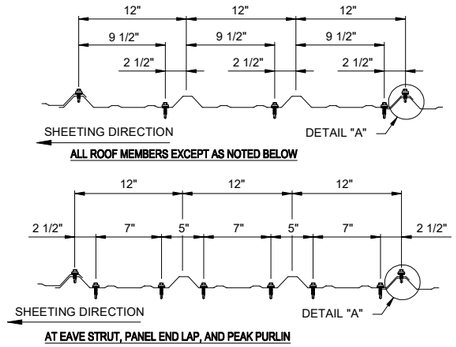
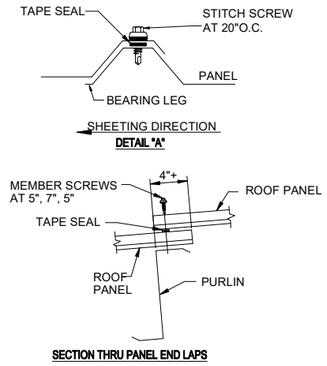
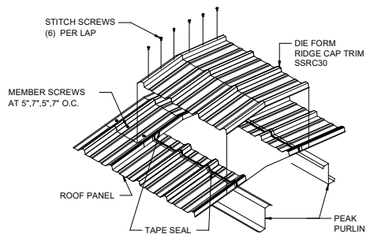




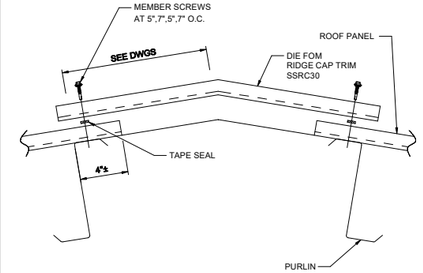
SCREW_30 FASTENER PATTERN FOR "SUPER SPAN X" ROOF PANEL

INSULATION NOT BY
METAL BUILDING PROVIDER



TRIM_54 DIE FORMED RIDGE CAP INSTALLATION (SUPER SPAN X)

INSULATION NOT BY
METAL BUILDING PROVIDER



TRIM_55 DIE FORM RIDGE CAP INSTALLATION (SUPER SPAN X)

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10-21-2025

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