COLEMAN

HARRINGTON PLACE LOT 1

PLAN ID 060121.1201



110 VILLAGE TRAIL SUITE 215 WOODSTOCK, GA. 30188

DRAWING INDEX

A0.0	COVER SHEET
A1.1	FRONT ELEVATIONS
A2.1	SIDE & REAR ELEVATIONS
A3.1	SLAB FOUNDATION
A5.1	FIRST FLOOR PLANS & DETAILS
A5.2	SECOND FLOOR PLANS & DETAILS
A6.1	ROOF PLANS
A7.2-A7.3	ELECTRICAL PLANS
A8.1	TRIM LOCATION LAYOUTS

AREA TABULATION	
FIRST FLOOR	838
SECOND FLOOR	1215
TOTAL	2053
GARAGE	702
FRONT PORCH (COVERED)	84
REAR PATIO (COVERED)	120



Sales center only. Interior HC restroom

GOVERNMENTAL CODES & STANDARDS

HOME TO BE BUILT TO CONFORM TO ALL APPLICABLE LOCAL CODES, PRACTICES AND STANDARDS

BUILDING CODE ANALYSIS / DESIGN CRITERIA

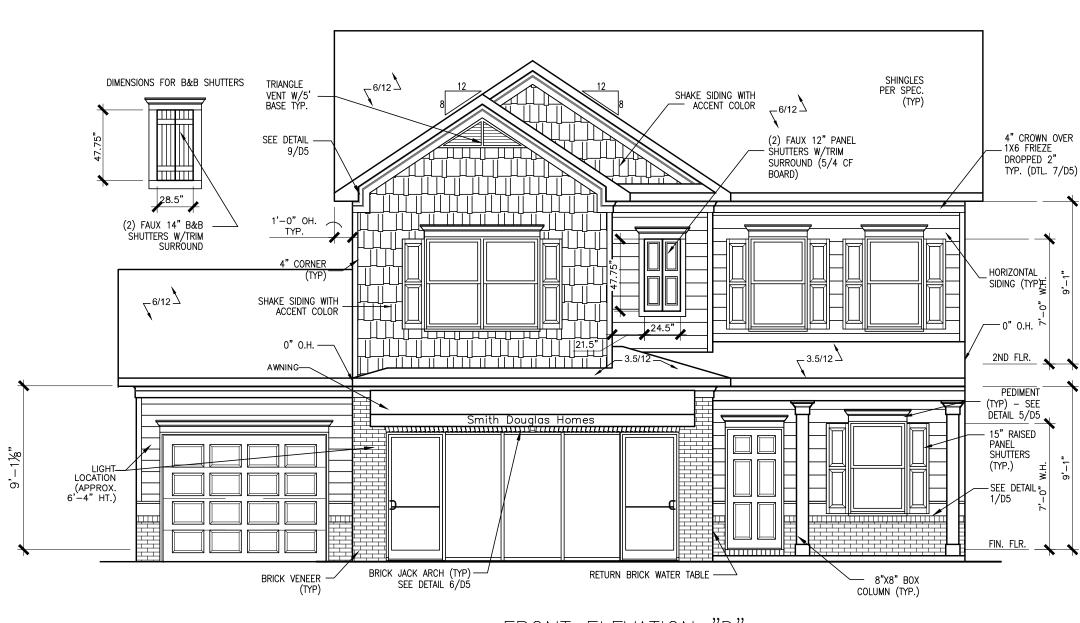
HOME TO BE BUILT TO MEET OR EXCEED ALL LOCAL CODES AND DESIGN CRITERIA

		PLAN REVISIONS	
DATE	BY	REVISION	PAGE #
10/30/2021	AW	Prototype walk revisions - see revision sheet	ALL
4/1/2022	AW	Final walk revisions - see revision sheet	A5.2, A5.2, A7.3
11/1/2022	AW	PCR #4985 Change 2x6 wall in laundry to 2-2x4s - takes 1.5" out of hall/linen	A5.2, A7.3
12/1/2022	AW	PCR #5030 Added 8" in depth to kitchen (pantry & around island) - reduced Dining/Study 8" in depth	A3.1, A5.1, A7.2, A8.1

ALL NON-MASONRY RETURNS TO BE HORIZONTAL SIDING

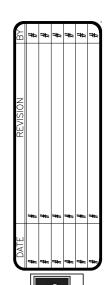
SEE SHEET D3 OF SDH TYPICAL DETAILS FOR SOFFIT DETAILS PER SOFFIT MATERIAL

HARRINGTON PLACE LOT 1



FRONT ELEVATION "D"

SCALE: 3/16"=1'-0"



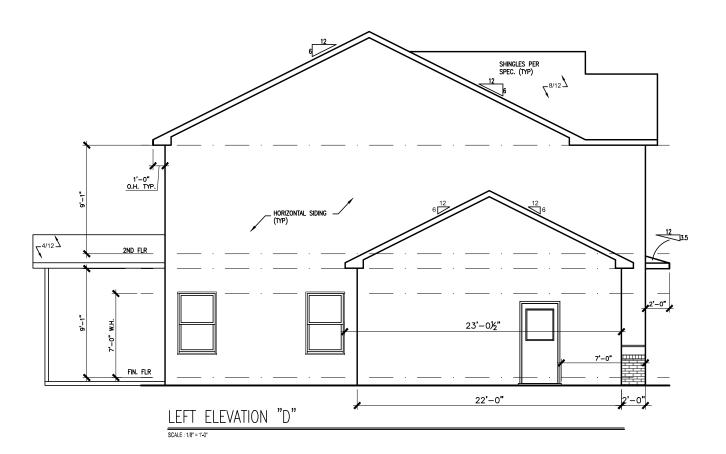
SMITH DOUGLAS HOMES GUALITY | VALUE

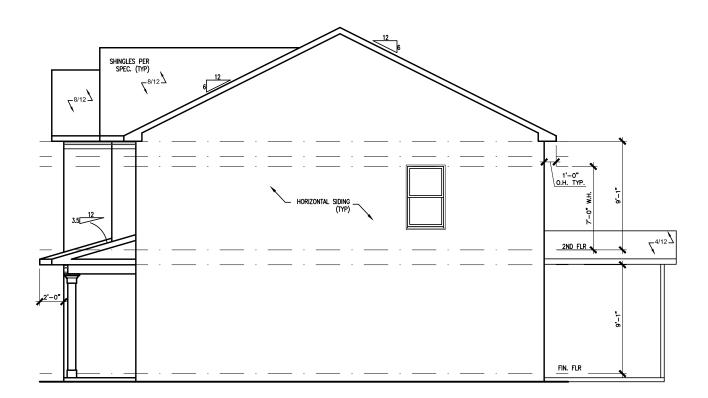
ELEVATIONS FRONT ELEVATION COLEMAN

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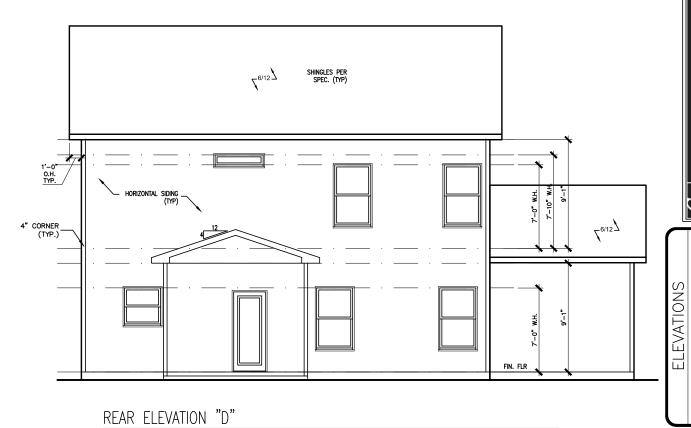






RIGHT ELEVATION "D"

HARRINGTON PLACE LOT 1



AND REAR COURTY INTERITY I VALUE

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SIDES

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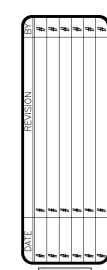


6'-6" DROP 4" BELOW HOUSE SLAB PROVIDE ELECTRICAL CONDUIT TO O 18'-5½" DROP 4" BELOW HOUSE SLAB DROP 4" BELOW HOUSE SLAB SEE FLOORPLAN FOR UTILITY DOOR LOCATION START AT THIS CORNER TO LAY OUT PLATES 9' X 7' OHGD (R.O. 9'-3" X 7'-1 1/2") DROP 4" BELOW HOUSE SLAB 5'W.X3'D. PAD POURED AT TIME OF GARAGE SLAB TO SUPPORT GLASS DOOR HINGES 1'-10½" 1'-10½" 16'-3" 20'-0" 46'-0" SLAB PLAN SCALE : 1/8" = 1'-0"

HARRINGTON PLACE LOT 1

*RADON VENT PROVIDED PER LOCAL CODE

REFER TO DETAIL 3/D1
FOR BRICK LEDGE
DETAIL WHEN BRICK
VENEER IS CHOSEN



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FOUNDATION PLAN SLAB PLAN COLEMAN

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COVERED 10'X12' PATIO 15'-7" 18'-5" 10'-3" 5'-1" LOC. TBD PER SITE FAMILY ROOM 9'-0" CLG. (2) 14"X16" HEADER ® 7' BREAKFAST \$\frac{1}{4}\$ VERIFY LOC. STORAGE 2'-6"W. OPNG. -HDR. @ 7'-10" GARAGE LOC. FOR ELEC. PANEL (2) 1.75"X16" LVL DROPPED HEADER WITH (5) 2X4 JACK STUD COLLUMNS WITH (2) 2X4 KING STUDS AT EACH END FOR FUTURE 5"W. OPENING INTO 2-CAR GARAGE. TOP OF HDR. AT 7"-10" (FRAME IN OPENING FOR SALES CENTER) 9'-7" GRAB BARS STUDY 9'-0" CLG. FOYER ₽ 2868 H.L. STEEL SCUTTLE HOLE 22 1/2"X30" R.O. GARAGE/SALES CENTER PROVIDE LATERAL & SHEAR WALL BRACING AT CORNERS 9'-0" CLG. COVERED PORCH 8X8 BOX COLUMN 9'x7' GARAGE DOOR 3'W.X7'H. GLS. (3) PANELS OF 7'H. TEMPERED FIXED GLASS. 3'W.X7'H. GLS. TEMP. DR. 5'W.X3'D. PAD POURED AT TIME OF GARAGE SLAB TO SUPPORT GLASS DOOR HINGES 16'-0" 20'-0" FIRST FLOOR PLAN SCALE : 1/8" = 1'-0" BASE TRIM 1. SALES CENTER FLOORING TO BE CARPET SQUARES (ALTERNATING SQUARES PAINTED CURB-TO BREAK UP THE PATTERN) - FLOOR TO HAVE STANDARD GARAGE SLOPE 2. CONCRETE GARAGE CURB TO BE PAINTED WITH BASE TRIM 3. DO NOT CREATE A WEATHER LIP FOR FUTURE OVERHEAD GARAGE DOOR GARAGE CURB 4. INTERIOR TRIM AROUND STOREFRONT DOORS/FIXED GLASS DETAIL 5. ADD BLOCKING OR BE SURE KIOSK MONITOR WALL MOUNT IS SCREWED INTO A STUD 6. ADD BLOCKING FOR CABINET DISPLAY RACK AND FLOATING SHELVES (REFER TO SALES

CENTER CABINET DRAWINGS)

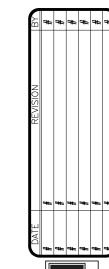
EQUIPMENT)

STOREFRONT GLASS

7. ELECTRICAL PANEL TO BE HIDDEN WITH WHITE TRIM AND DOOR WITH HANDLE 8. SEE LAYOUT FOR CLOSET LOCATION TO BE USED FOR STORING HELIUM TANK, LAWN SIGNS, BALLOONS AND STRING (DO NOT STORE IN CLOSET DESIGNATED FOR IT

9. INSULATE CEILING & ALL WALLS OF SALES CENTER AND USE 3M FILM TO TINT

HARRINGTON PLACE LOT 1



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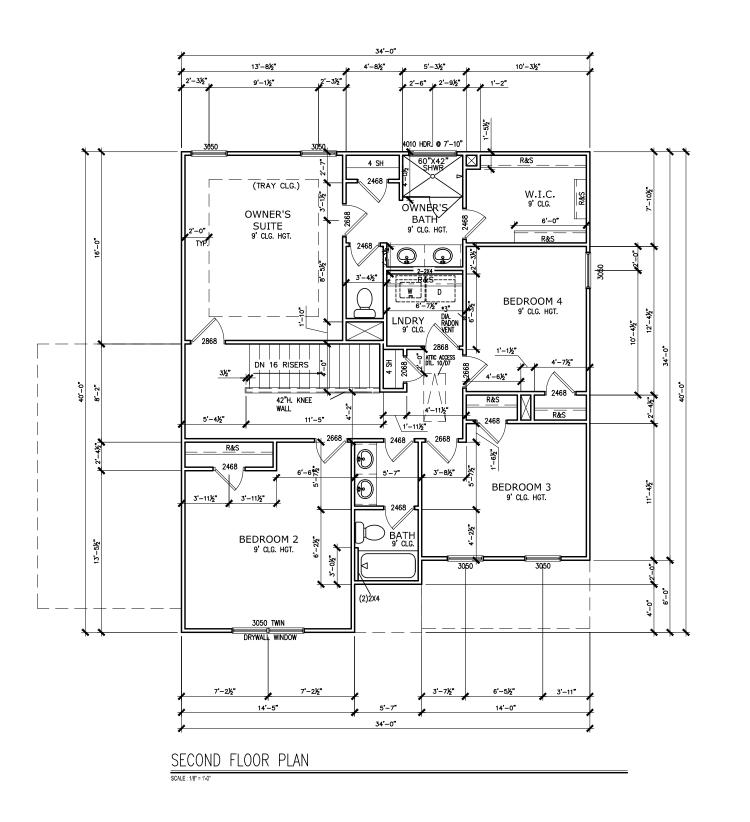
FLOOR PLAN FIRST FLOOR COLEMAN

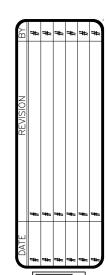
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SECOND FLOOR COLEMAN

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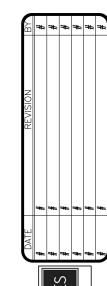
REFER TO MANUFACTURER'S SPECS. FOR DRAIN LOCATIONS ON DETAIL SHEETS D12, D12.1, & D12.2

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₹ 4/12 4/12 //1'-0" 1'-0" //1'-0" RIDGE VENTS 8'X8' HVAC PLATFORM 8/12 € 8/12 ROOF PLAN "D" SCALE : 1/8" = 1'-0"

HARRINGTON PLACE LOT 1



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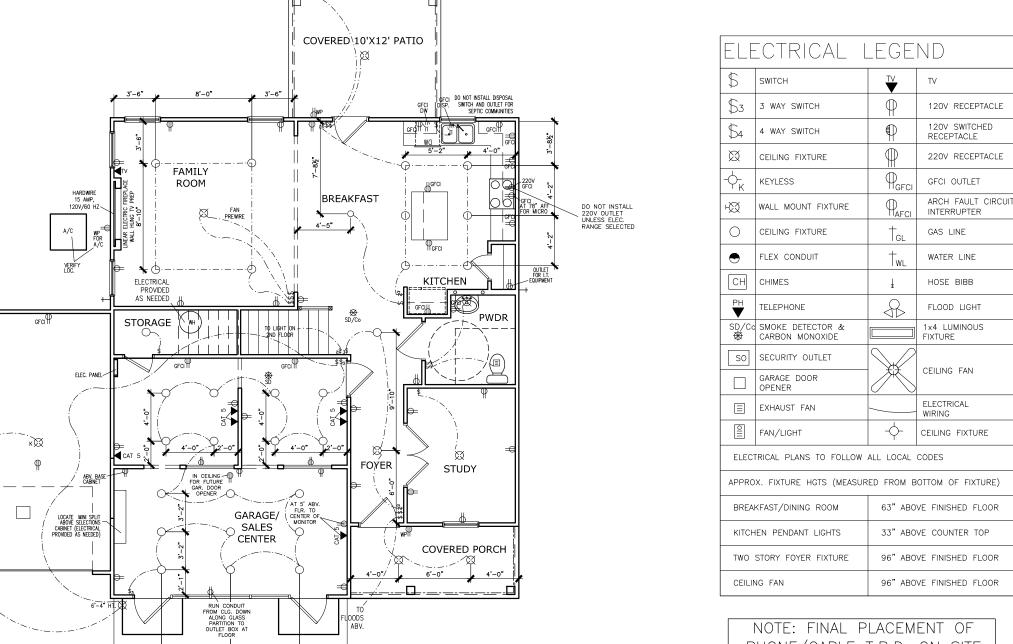
ROOF PLAN
COLEMAN

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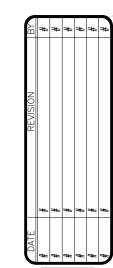
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HARRINGTON PLACE LOT 1



PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



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 \mathcal{L} FLOOF OLEMAN ELECTRICAL \mathcal{O} 딦

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FIRST FLOOR ELECTRICAL PLAN

SCALE : 1/8" = 1'-0"

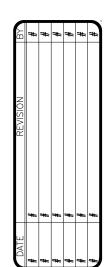
6'-4" HT. ◯◯

W.I.C. OWNER'S® OWNER'S BATH SUITE BEDROOM 4 HALL BATH BEDROOM 3 BEDROOM 2 `. ⁄O

HARRINGTON PLACE LOT 1

	ECTRICAL L		NID	
	LUTRICAL L		IND	
\$	SWITCH	TV.	TV	
\$3	3 WAY SWITCH	Ф	120V RECEPTACLE	
\$4	4 WAY SWITCH	\bigcirc	120V SWITCHED RECEPTACLE	
Ø	CEILING FIXTURE	0	220V RECEPTACLE	
- ∳ _K	KEYLESS	PGFCI	GFCI OUTLET	
+XX	WALL MOUNT FIXTURE	PAFCI	ARCH FAULT CIRCUIT INTERRUPTER	
0	CEILING FIXTURE	† _{GL}	GAS LINE	
•	FLEX CONDUIT	†wL	WATER LINE	
СН	CHIMES	¥	HOSE BIBB	
PH	TELEPHONE	\$	FLOOD LIGHT	
SD/Co	SMOKE DETECTOR & CARBON MONOXIDE		1x4 LUMINOUS FIXTURE	
SO	SECURITY OUTLET			
	GARAGE DOOR OPENER		CEILING FAN	
≡	EXHAUST FAN		ELECTRICAL WIRING	
OIII	FAN/LIGHT	-	CEILING FIXTURE	
ELECT	RICAL PLANS TO FOLLOW	ALL LOCAL	CODES	
APPRO:	APPROX. FIXTURE HGTS (MEASURED FROM BOTTOM OF FIXTURE)			
BREAK	BREAKFAST/DINING ROOM		63" ABOVE FINISHED FLOOR	
KITCH	EN PENDANT LIGHTS	33" ABOVE COUNTER TOP		
TWO STORY FOYER FIXTURE		96" ABOVE FINISHED FLOOR		
CEILIN	IG FAN	96" ABOVE FINISHED FLOOR		

NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



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SECOND FLOOR
COLEMAN

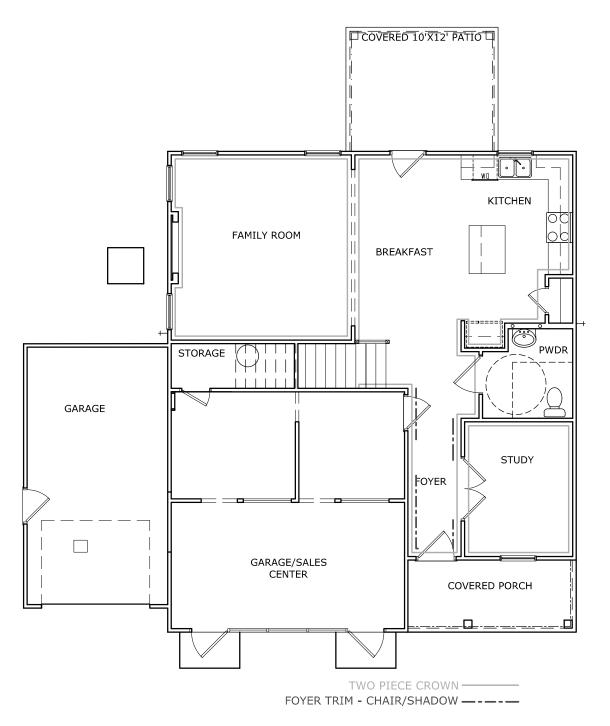
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SCALE : 1/8" = 1'-0"

HARRINGTON PLACE LOT 1



FIRST FLOOR PLAN

TRIM LAYOUT FIRST FLOOR PLAN

SCALE : 1/8" = 1'-0"

ONE PIECE CROWN

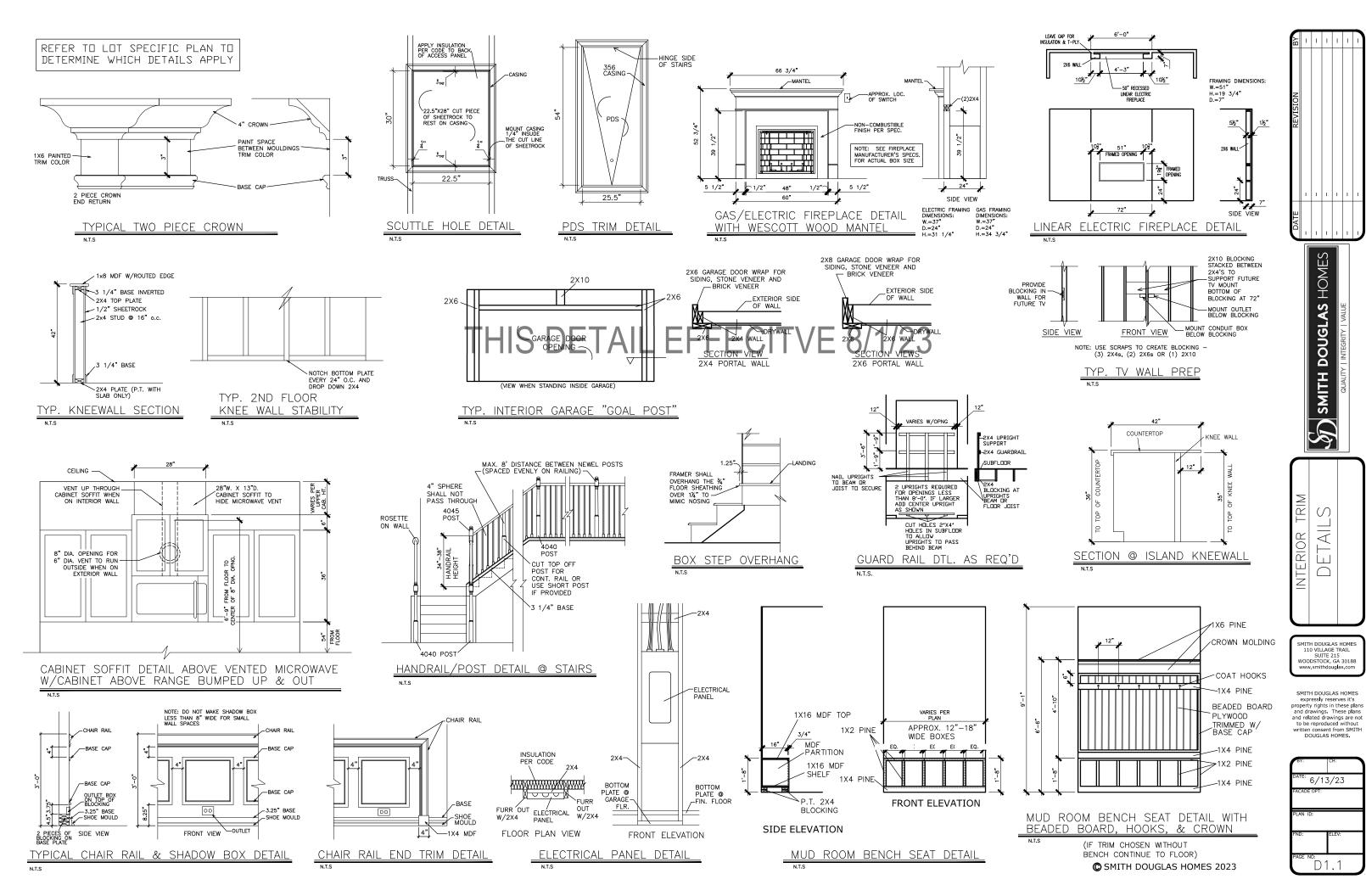
OWNER'S SUITE

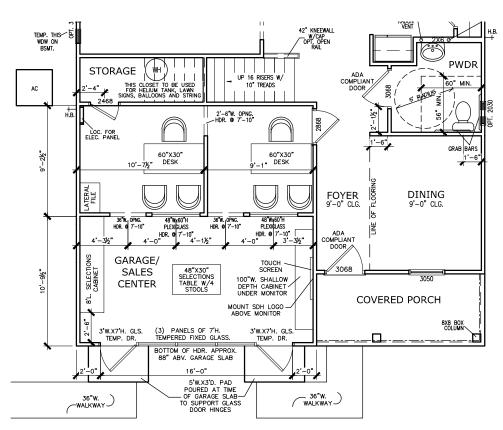


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BASE TRIM-

PAINTED CURB-

FLOOR-

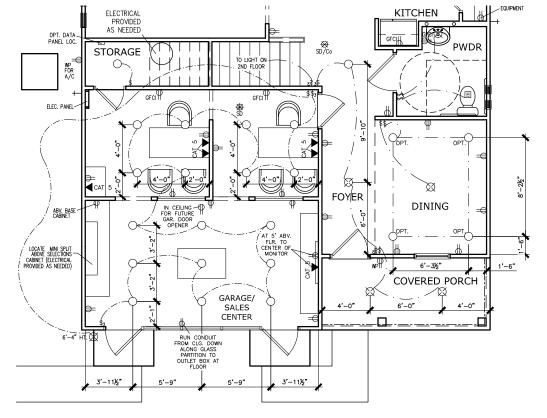
GARAGE CURB DETAIL

COLEMAN 2-OFFICE STOREFRONT SALES CENTER W/ADA BATHROOM FLOOR PLAN

NOTES: 1. SALES CENTER FLOORING TO BE CARPET SQUARES (ALTERNATING SQUARES TO BREAK UP THE PATTERN) - FLOOR TO HAVE STANDARD GARAGE SLOPE 2. CONCRETE GARAGE CURB TO BE PAINTED WITH BASE TRIM

- 3. DO NOT CREATE A WEATHER LIP FOR FUTURE OVERHEAD GARAGE DOOR

- 4. INTERIOR TRIM AROUND STOREFRONT DOORS/FIXED GLASS
 5. ADD BLOCKING OR BE SURE KIOSK MONITOR WALL MOUNT IS SCREWED INTO A STUD 6. ADD BLOCKING FOR CABINET DISPLAY RACK AND FLOATING SHELVES (REFER TO SALES
- CENTER CABINET DRAWINGS) 7. ELECTRICAL PANEL TO BE HIDDEN WITH WHITE TRIM AND DOOR WITH HANDLE
- 8. SEE LAYOUT FOR CLOSET LOCATION TO BE USED FOR STORING HELIUM TANK, LAWN SIGNS, BALLOONS AND STRING (DO NOT STORE IN CLOSET DESIGNATED FOR IT
- 9. INSULATE CEILING & ALL WALLS OF SALES CENTER AND USE 3M FILM TO TINT STOREFRONT GLASS



COLEMAN 2-OFFICE STOREFRONT SALES CENTER W/ADA BATHROOM ELECTRICAL PLAN

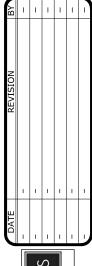
NOTES:

1. CONSIDER LOCATION OF ELECTRICAL PANEL AS IT RELATES TO LAYOUT — EXACT

LOCATION T.B.D. BY CM & MARKETING
2. INSTALL A DUPLEX OUTLET IN THE I.T. EQUIPMENT CLOSET — LOCATION OF EQUIPMENT CLOSET NOTED ON LAYOUT

3. PROVIDE ELECTRICAL AS REQUIRED FOR MINI SPLIT - LOCATION NOTED ON LAYOUT

	ECTRICAL		
\$	SWITCH	TV TV	TV
\$3	3 WAY SWITCH	φ	120V RECEPTACLE
\$4	4 WAY SWITCH	•	120V SWITCHED RECEPTACLE
Ø	CEILING FIXTURE	Φ	220V RECEPTACLE
-ф _к	KEYLESS	P _{GFCI}	GFCI OUTLET
μØ	WALL MOUNT FIXTURE	PAFCI	ARCH FAULT CIRCU
0	CEILING FIXTURE	† _{GL}	GAS LINE
•	FLEX CONDUIT	† _{wL}	WATER LINE
СН	CHIMES	¥	HOSE BIBB
PH	TELEPHONE	\$	FLOOD LIGHT
SD/Cd ₩	SMOKE DETECTOR & CARBON MONOXIDE		1x4 LUMINOUS FIXTURE
SO	SECURITY OUTLET		OFILINO FAN
	GARAGE DOOR OPENER		CEILING FAN
≣	EXHAUST FAN		ELECTRICAL WIRING
9	FAN/LIGHT	-	CEILING FIXTURE
ELEC.	TRICAL PLANS TO FOLLOW	ALL LOCAL	CODES
APPRO	IX. FIXTURE HGTS (MEASU	RED FROM B	OTTOM OF FIXTURE)
BREA	KFAST/DINING ROOM	63" ABO	VE FINISHED FLOOR
KITCH	HEN PENDANT LIGHTS	33" ABO	VE COUNTER TOP
TWO	STORY FOYER FIXTURE	96" ABO	VE FINISHED FLOOR
CEILI	NG FAN	96" ABO	VE FINISHED FLOOR



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PLANS EMAN \bigcirc () \bigcirc Ш SAL

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CONNECTION SPECIFICATIONS (TYP. U.N.O.)

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PL. TO JOIST/RIM OR BLK'G	NAILS @ 4" o.c.	NAILS @ 4" o.c.
STUD TO PLATE	(4) TOENAILS/ (3)END NAILS	(4) TOENAILS/ (4)END NAILS*
RIM TO TOP PLATE	TOENAILS @ 6" o.c.	TOENAILS @ 4" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS EA. END	(3) TOENAILS EA. END*
DOUBLE STUD	NAILS @ 16" o.c.	NAILS @ 16" o.c.
DOUBLE TOP PLATE	NAILS @ 12" o.c.	NAILS @ 8" o.c.
DOUBLE TOP PLATE LAP SPLICE	(12) NAILS IN LAPPED AREA (24" MIN.)	(15) NAILS IN LAPPED AREA (24" MIN.)
TOP PLATE LAP @ CORNERS & INTERSECTING WALLS	(3) NAILS	(3) NAILS
RAFTER/TRUSS TO TOP PLATE	(4) TOENAILS + (1) SIMPSON H2.5T	(4) TOENAILS + (1) SIMPSON H2.5T
GAB. END TRUSS TO DBL. TOP PL.	TOENAILS @ 8" O.C.	TOENAILS @ 6" o.c.
R.T. w/ HEEL HT. 91/4" TO 12"	2xIO BLK EVERY 3RD BAY FASTENED TO DBL. TOP PLATE w/ TOENAILS @ 6" O.C.	2xIO BLK EVERY 3RD BAY FASTENED TO DBL. TOP PLATE w/ TOENAILS @ 4" O.C.
R.T. w/ HEEL HT. 12" TO 16"	2xI2 BLK EVERY 3RD BAY FASTENED TO DBL. TOP PLATE W/ TOENAILS @ 6" O.C.	2xI2 BLK EVERY 3RD BAY FASTENED TO DBL. TOP PLATE w/ TOENAILS @ 4" O.C.
R.T. w/ HEEL HT. UP TO 24"	LAP WALL SHTG. W/ DBL. TOP PL. & INSTALL ON TRUSS VERT FASTEN W/ NAILS @ 6" O.C.	LAP WALL SHTG. W/ DBL. TOP PL. & INSTALL ON TRUSS VERT FASTEN W/ NAILS @ 6" O.C.*
R.T. w/ HEEL HT. 24" TO 48"	LAP WALL SHTG, W/ DBL. TOP PL. & INSTALL ON TRUSS VERT FASTEN W/ NAILS @ 6" O.C. PROVIDE 2x BLK @ EA. BAY AT TOP OF HEEL	LAP WALL SHTG, W DBL, TOP PL. & INSTALL ON TRUSS VERT FASTEN W NAILS @ 6" O.C. PROVIDE 2x BLK @ EA. BAY AT TOP OF HEEL*
WALL TO FOUNDATION	WALL SHTG. LAP W/ SILL PL. & FASTENED PER SHEAR WALL FASTENING SPEC.	

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"X0.120", SAME SPACING OR NUMBER OF NAILS. ONLY ACCEPTABLE WHERE * ARE SHOWN)

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DEFLECTION CRITERIA BELOW UNLESS NOTED OTHERWISE ON PLAN. MULHERN & KULP CANNOT BE HELD RESPONSIBLE FOR ANY STRUCTURAL ISSUES RELATED TO ANY BUILDING COMPONENT IF COMPONENT SHOP DRAWINGS ARE NOT SUBMITTED TO M&K FOR REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUSH BEAMS DO NOT EXCEED THE FOLLOWING:

- ROOF TRUSSES: 1/4" DEAD LOAD
- ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD

ABSOLUTE DEAD LOAD DEFECTION OF ATTIC TRUSSES WHEN AD JACENT TO ELOOR FRAMING BY OTHERS SHALL BE LIMITED TO 3/16". (NOT DIFFERENTIAL DEFLECTION)

VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	STEEL ANGLE SIZE
3'-0"	20 FT. MAX	L3"x3"x¼"
	3 FT. MAX	L3"x3"x¼"
6'-0"	I2 FT. MAX	L4"x3"x¼"
	20 FT. MAX	L5"x3½"x¾"
8'-0"	3 FT. MAX	L4"x4"x¼" *
0-0	I2 FT. MAX	L5"x3½"x5%"
	l6 FT. MAX	L6"x3½"x¾"
9'-6"	I2 FT. MAX	L6"x3½"x5%"

- . LINTELS: HALL SUPPORT 2 % 3 ½ ' VENEER W 40 psf MAXIMM WEIGHT 6' SHALL HAVE 4' NIN BEARING 6' SHALL HAVE 5' MIN, BEARING 6' SHALL NOT BE FASTENED BACK TO HEADER.

- (4) SHALL BY TEE FASTENDE DACK TO HEADER IN MALL 048°02, M/3° DIA x 3 3/5° LONG 1A6 SCREPE BY ACT LONG VERTICALLY SLOTTED HOLES, M/3° DIA x 3 3/5° LONG 1A6 SCREPE IN 2° LONG VERTICALLY SLOTTED HOLES, MAX VEREER IN APPLIED TO ANY PORTION OF PROKE OVER THE OPENING, ALL INITIES SHALL BE LONG LEG VERTICAL.
 ALL INITIES SHALL BE LONG LEG VERTICAL.
 BY THE SHAPPLING TO THE THE THE THE THE THE PART OF THE HORIZONTAL LEG MAY BE CUT IN THE FIRLD TO BE 3/2° MIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTARY LONG THERMING.
 SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE AROUND PARAMETERS.
- R QUEEN VENEER USE L4x3x/4".

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NCSBC-RESIDENTIAL CODE \$ 2018 IRC WITH SOUTH CAROLINA AMENDMENTS
- FOOTING DESIGN 2,000 PSF NET ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO CONC FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, I2" MAX. FROM PLATE ENDS - UTILIZING
- I/2" DIA. ANCHOR BOLTS @ 6'-0" O.C.7" MIN. EMBEDMENT FA4 ANCHOR STRAPS @ 6'-0" O.C.
- FASTEN 2x10 SILL PLATES TO PRECAST BSMT WALLS WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX. FROM PLATE ENDS - UTILIZING: I/2" DIA, BOLTS @ 2'-0" O.C
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W/ PERIMETER FOUNDATION SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2
- BUILDER TO VERIFY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- FOUNDATION WALLS & FOOTINGS SHALL BE PLAIN CONCRETE, U.N.O.
- CONCRETE DESIGN BASED ON ACL 318, CONCRETE SHALL, ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.:
- f'c = 4,000 psi: FOUNDATION WALLS 3,000 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,500 psi: GARAGE & EXTERIOR SLABS ON GRADE eq 000,000 psi
- BASEMENT FOUNDATION WALL DESIGN BASED ON:
 - 8' OR 9' HEIGHT (AS NOTED ON PLANS) TALLER WALLS MUST BE ENGINEERED
- · BASEMENT WALL DESIGN IS BASED ON 30 OR 45 PCF BACKFILL SOIL TYPE CLASSIFICATIONS:
 - 30 PCF TYPE (GW GP GW SP) 45 PCF TYPE (GM, GC, SM, SM-SC, ML)
- IMPORTANT IF 60 PCF SOIL TYPE (SC, ML-CL, OR CL) IS UTILIZED FOR BACKFILL. CONTACT MULHERN & KULP FOR FURTHER EVALUATION OF FOUNDATION DESIGN.
- BASEMENT WALLS SHALL BE BRACED PRIOR TO BACKELLING BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT.
- ALL FOOTINGS SHALL BEAR BELOW FROST LINE (TYP.) OR 12" MIN IN REGIONS WHERE CODE FROST DEPTH IS NOT APPLICABLE. CONSULT SOILS REPORT OR BUILDING DEPT. FOR MINIMUM DEPTH BELOW
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP.
- JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" OC (MAXIMUM)
- JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I:I RATIO), WITH A MAXIMUM OF I:1.5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- TYPICAL REINFORCEMENT DETAILS: PROVIDE 3" MIN. CLEAR COVER WHERE CAST AGAINST FARTH, LI/2" MIN, CLEAR COVER AGAINST FORMS. LAP ALL REBAR 48 BAR DIAMETERS MIN. (24" FOR #4 BARS) & BEND BARS AND LAP AT CORNERS. PROVIDE 6

HOOK INTO SUPPORTING FOOTINGS WHEN FOOTINGS INTERSECT

• DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

LEGEND

R.T. NDICATES ROOF TRUSSES @ 24" O.C. PER ROOF. MANUE (TYP IINO)

SI ABS

GRADE

OF. INDICATES TRUSS OVERFRAMING @ 24" O.C. (TYP. U.N.O.)

F.J. NDICATES 14" DEEP FLOOR 1-JOISTS (24" O.C. MAX SPACING), JOIST SERIES AND SPACING SHALL BE THE RESPONSIBILITY OF THE JOIST MANUFACTURER D.J. NDICATES 2x8 P.T. DECK JOISTS @ 16" O.C. (MAX.)

- INDICATES LOCATIONS OF POTENTIAL TILE FLOOR. JOIST MANUFACTURER SHALL DESIGN FLOOR
 SYSTEM FOR ADD'L 10 PSF DEAD LOAD AT THESE LOCATIONS
- INTERIOR BEARING WALL
- □□□□□ BEARING WALL ABOVE (B.W.A.)
- JL METAL HANGER
- INDICATES POST ABOVE (P.A.) PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE

LATERAL/WALL BRACING & WALL SHEATHING SPECIFICATIONS

- THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: MPH WIND IN 2018 NCSBC:RO
- \$ 120MPH WIND IN 2018 IRC (120 MPH WIND SPEED IN ASCE 7 WIND MAP, PER IRC R301,2,1,1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.
- HE DESIGN WAS COMPLETED PER 2015 & 2018 IBC FCTION 1609) & ASCE 7, AS PERMITTED BY R30113 THE 2018 NCSBC:RC & 2018 IRC. ACCORDINGLY THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 1 (ACCEPTED) ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC & 2018 IRC SECTION R802.II.I.I. MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIET LOAD PATH PER SECTIONS R602.3.5¢ R802.II.

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W/ 2 3 "XO.II3 NAILS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE PANEL FIELD. (TYP, U.N.O.)
- ALL SHEATHING PANELS SHALL BE ORIENTED VERTICALLY (LONG DIRECTION PARALLEL TO STUDS, AND INSTALLED FULL HEIGHT OF SHEAR WALL - OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT, STAPLE CONNECTION SPEC: 1 3/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 2 3 × 0.113 NAILS @ 3" O.C. AND 12" O.C. IN THE PÄNEL FIELD NO STAPLE ALTERNATIVE AVAILABLE <u>AT THIS SPEC.</u> ALL SHEATHING PANELS SHALL BE ORIENTED VERTICALLY (LONG DIRECTION PARALLEI TO STUD) AND INSTALLED FULL HEIGHT OF SHEAR MALL - OR - 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

NOTES

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING. IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. T WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.
- PRE-MANUFACTURED PANELIZED WALLS: FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120 NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWALL, AND/OR 3" O.C. EDGE NAILING

INDICATES HOLDOWN

FLOOR FRAMING

- I-JOISTS SHALL BE DESIGNED BY MANUE TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA, (EXCLUDES STONE/MARBLE OR WET BED CONSTRUCTED FLOORS - CONTACT M&K FOR EXCLUDED FLOOR DESIGNS)
- PER THE GUIDELINES OF THE TILE COUNCIL OF NORTH AMERICA (TCNA HANDBOOK), IT SHALL BE THE FLOOR FINISH INSTALLER'S RESPONSIBILITY TO VERIFY THAT THE FINISHES TO BE INSTALLED MATCH THE DESIGN CRITERIA NOTED ABOVE (UNDER "DESIGN LOADS")
- FLOOR SYSTEMS & SHEATHING HAVE BEEN DESIGNED TO SUPPORT ADDITIONAL DEAD LOAD FROM CERAMIC TILE (EXCLUDING MARBLE OR STONE) HOWEVER IT SHALL BE THE FLOOR FINISH INSTALLER'S RESPONSIBILITY TO PROVIDE PROPER UNDERLAYMENT, UNCOUPLING MEMBRANE AND MORTAR/GROUT PER THE ASSEMBLY DESIGNATIONS IN THE TONA HANDBOOK (TILE COUNCIL OF NORTH AMERICA).
- AT I-JOIST FLOORS, PROVIDE I" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- REVIEW AND APPROVAL PRIOR TO FABRICATION OR DELIVERY. FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR'
- 24" O.C., EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W/ GLUE AND
- 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES \$ @ 12"o.c. FIELD. x 0.120" NAILS @ 4" O.C. @ PANEL EDGES & @ 8" O.C. FIELD
- 2 🖁 × 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.

ROOF FRAMING

- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS w/ 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12" O.C. FIELD.
- w/ 2 3 × 0.120 NAILS @ 4 O.C. @ PANEL EDGES & @ 8 O.C. FIELD. - w/ 2 3 × 0.113" NAILS @ 3"o.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.
- WITHIN 48" OF ALL ROOF EDGES, RIDGES, & HIPS FASTEN ROOF SHEATHING FIELD'S PER EDGE NAILING SPEC.
- FASTEN EACH ROOF TRUSS TO TOP PLATE W USP RTTA CLIP (OR APPROVED EQUAL) @ ALL BEARING POINTS. PROVIDE (2) RTTA CLIPS AT 2-PLY GIRDER TRUSSES, (3) RTTA CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.C
- ROOF TRUSS SHOP DWGS, SHALL BE SUBMITTED TO ARCH & ENG. FOR REVIEW AND APPROVAL PRIOR TO FABRICATION OR DELIVERY ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT SHORT SPAN ROOF TRUSSES W/2x4 LEDGER FASTENED TO FRAMING w/(2) 3" x 0.120" NAILS @ 16" O.C. (UP TO T' SPAN).

MEANS & METHODS NOTES

AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO THE ADDITION OF NECESSARY SHORING SHEETING TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO TABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF

TRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENTS IN CONTACT WITH LOOR FRAMING ARE LEVEL, INCLUDING, BUT NOT LIMITED FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIF LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY, OR WARRANTY TOLERANCES.

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NCSBC-RESIDENTIAL CODE & 2018 IRC
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.
- DESIGN LOADS: ROOF

LIVE = 20 PSF DEAD = 7 PSF T.C., 10 PSF B.C. LOAD DURATION FACTOR = 1.25

FLOOR LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS)

ADD'L IO PSF @ CERAMIC TILE IN BATHS & LAUND.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- CONNECTIONS TABLE (IRC TABLE R602.3(1)) OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL
- EXT. & INT. BEARING WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS) @ 16" O.C. SPF/SP "STUD" GRADE LUMBER, OR BETTER, U.N.O WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL INTERIOR BEARING WALLS ARE ASSUMED TO BE SHEATHED W/ GYP WALL BOARD (ONE SIDE MIN.) OR PROVIDE MID HT. BLOCKING
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRUCE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SP) LUMBER, OR BETTER. SUPPORT ALL HEADERS/ BEAMS W/ (1)2x JACK STUD & (1)2x
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 24" O.C. (MAX., U.N.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15)
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING: • 'LVL' - Fb=2600 psi; Fv=285 psi; E=2.0x10^6 psi

(I)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'.

- ENGINEERED LUMBER POSTS TO MEET OR EXCEED THE FOLLOWING:
 'LVL' Fb=2400 psi; FcII=2500 psi; E=I.8×10^6 psi
- FOR 2 & 3 PLY BEAMS OF EQUAL 13/4" MAX, WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"XO.120" NAILS @ 8" O/C OR 2 ROWS USP WS35 SCREWS (OR 31/3" TRUSSLOK SCREWS) @ 16" O/C, USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER.

 APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE. SOLID 3 $\frac{1}{2}$ " OR 5 $\frac{1}{4}$ 4 BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 & 2x8
- FOR 4 PLY BEAMS OF EQUAL 13/4" MAX, WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF USP WS6 SCREWS (OR 6 3/4" TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID T" BEAM IS ACCEPTABLE.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND./BEARING. BLOCKING TO MATCH POST ABOVE.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE USP BCS22-4 CAP & PA44E BASE, U.N.O.
- CORROSION NOTES
 - BUILDER RESPONSIBLE TO DETERMINE CORROSION-RESISTANCE REQUIREMENTS AND COMPATIBILITY OF HARDWARE, FASTENERS AND CONNECTORS FOR ENVIRONMENTAL EXPOSURE AND IN CONTACT W PRESERVATIVE-TREATED WOOD OF ACTUAL FINA CONDITIONS AND SOURCED MATERIALS. CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- ALL FASTENERS AND CONNECTORS EXPOSED TO SALT WATER (WITHIN 300' OF SALT WATER SHORELINE, INCLUDING VENTED SPACES) SHALL BE STAINLESS STEEL.

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERINS C-3825



Mulhern+Kulp project numbe 256-21006

SMK ILM issue date: 10-21-202

REVISIONS

initial: JPP

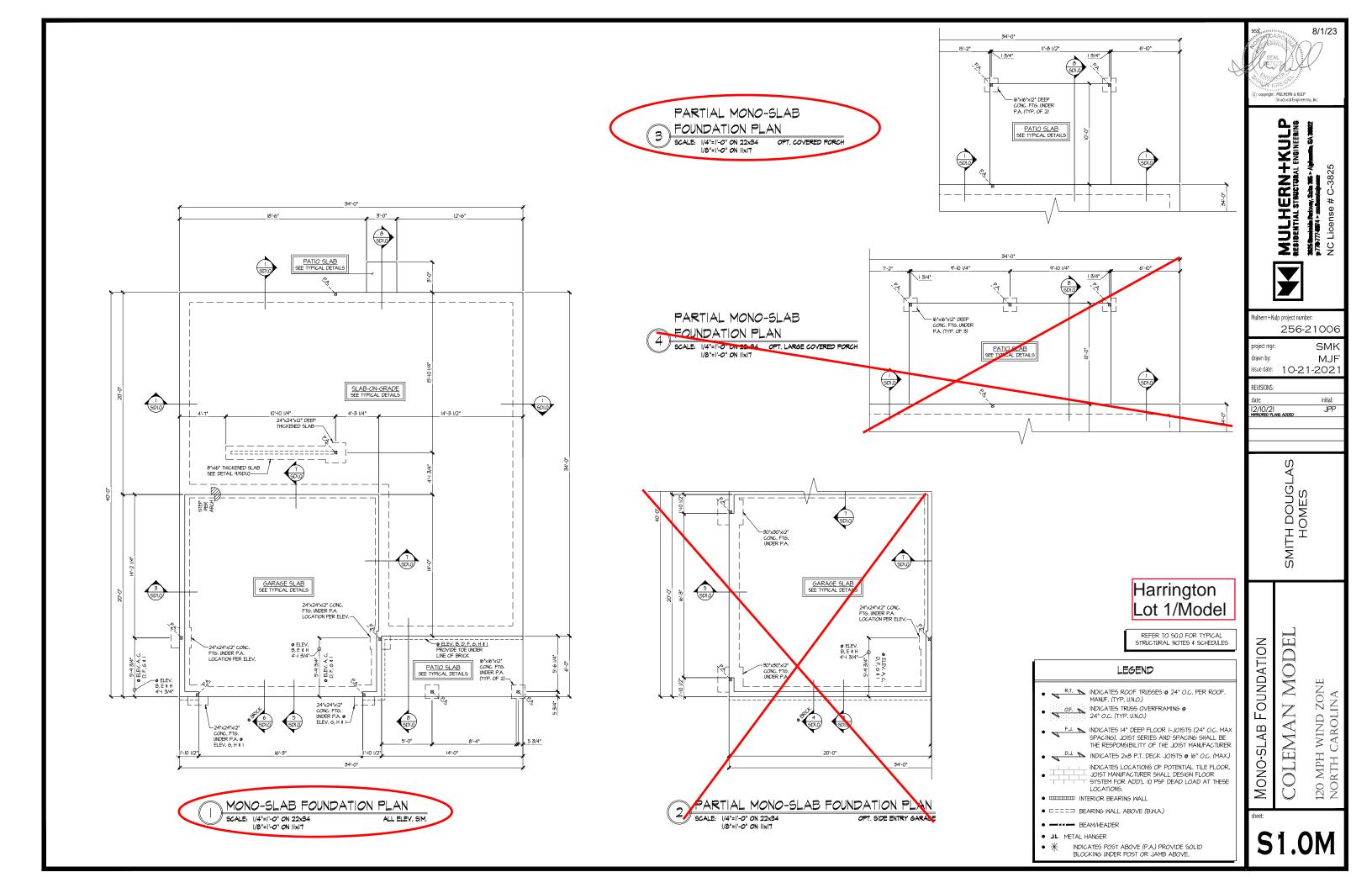
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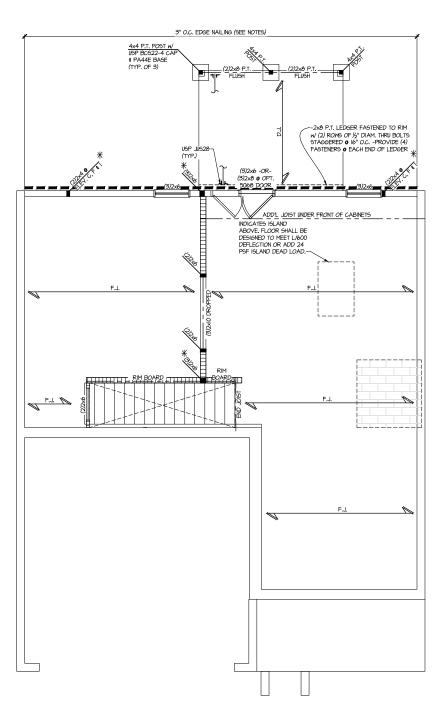
STRUCTURAL NOTES Ξ MODI

ZONI COLEMAN WIND 120 N

GENERAL

Harrington .ot 1/Model





3" O.C. EDGE NAILING (SEE NOTES) PARTIAL IST FLOOR SCALE: 1/4"=1"-O ON 20-44 OPT. EXT. DECK 1/8"=1"-O" ON 11x1T COVERED DECK SIM ADD'L JOIST UNDER FRONT OF CABINETS



THIS LEVEL HAS BEEN DESIGNED FOR 9'-I" PLATE HEIGHT

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

LEGEND

• RT. INDICATES ROOF TRUSSES @ 24" O.C. PER ROOF. MANUF. (TYP. U.N.O.)

• OF. INDICATES TRUSS OVERFRAMING • 24" O.C. (TYP. U.N.O.)

F.J. NDICATES 14" DEEP FLOOR I-JOISTS (24" O.C. MAX SPACING). JOIST SERIES AND SPACING SHALL BE THE RESPONSIBILITY OF THE JOIST MANUFACTURER

• J.J. NDICATES 2x8 P.T. DECK JOISTS • 16" O.C. (MAX.)

INDICATES LOCATIONS OF POTENTIAL TILE FLOOR.

JOIST MANUFACTURER SHALL DESIGN FLOOR

SYSTEM FOR ADD'L 10 PSF DEAD LOAD AT THESE LOCATIONS.

• IIIIIII INTERIOR BEARING WALL

• CTTT BEARING WALL ABOVE (B.W.A.)

• --- BEAM/HEADER

• JL METAL HANGER

INDICATES POST ABOVE (P.A.) PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

MUCHERNAL STRUCTURAL ENGINEERING
TESTINGHAMEN, SATE NO. ANTER SATE
TO THE SATE OF THE THE SATE

8/1/23

Mulhern+Kulp project number:

256-21006

SMK MJF issue date: 10-21-202

REVISIONS:

initial: JPP

SMITH DOUGLAS HOMES

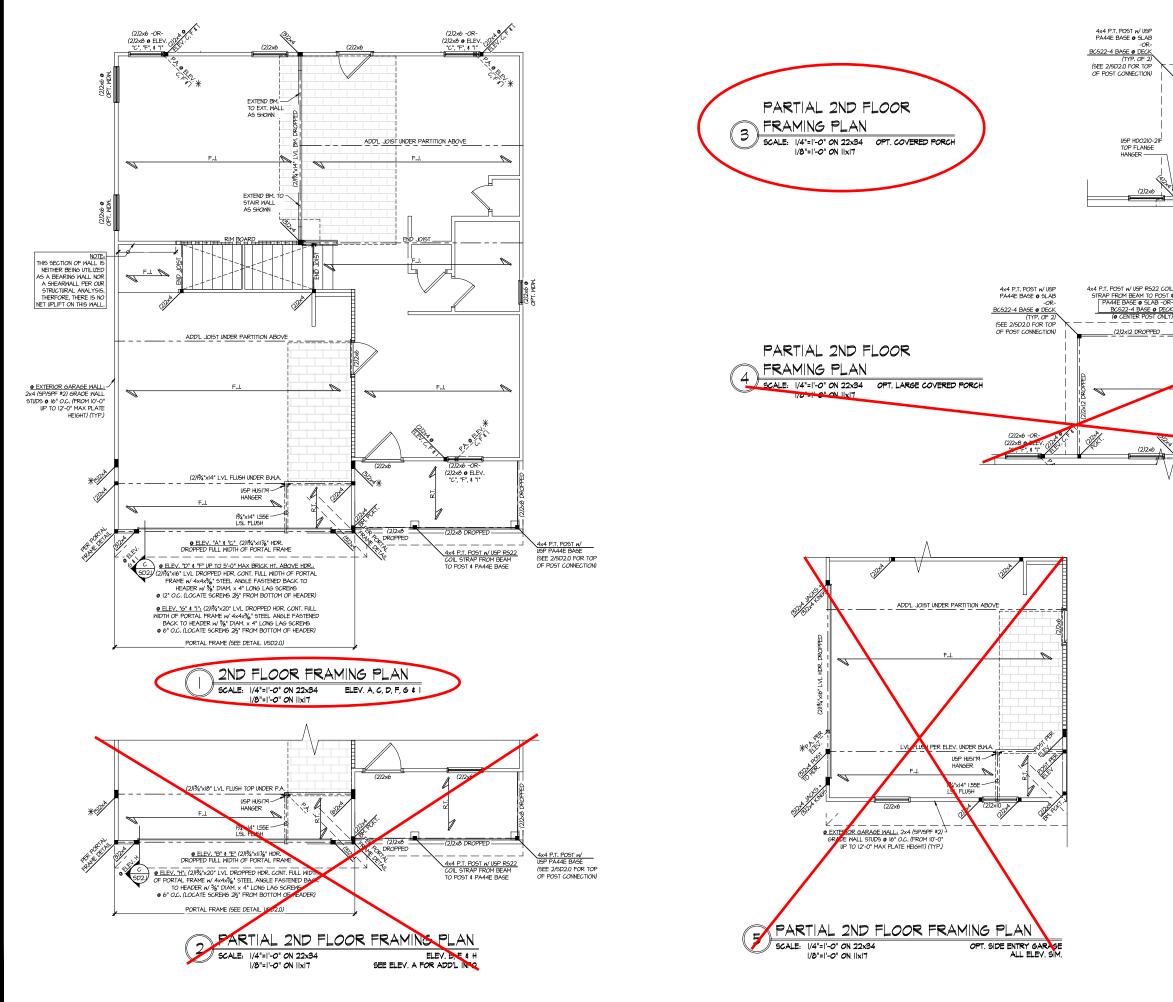
PLAN

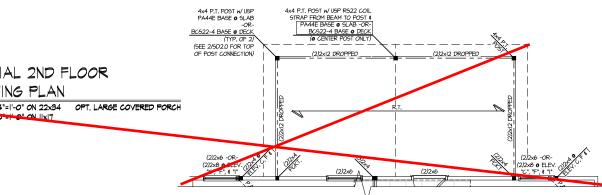
COLEMAN MODEL FLOOR FRAMING ST

120 MPH WIND ZONE NORTH CAROLINA

S2.0M

IST FLOOR FRAMING PLAN ALL ELEV. SIM. SCALE: |/4"=|'-0" ON 22x34 |/8"=|'-0" ON ||x|7





Harrington Lot 1/Model

THIS LEVEL HAS BEEN DESIGNED FOR 9'-1" PLATE HEIGHT

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

LEGEND

• R.T. INDICATES ROOF TRUSSES @ 24" O.C. PER ROOF. MANUF. (TYP. U.N.O.)

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D.J. NDICATES 2x8 P.T. DECK JOISTS @ 16" O.C. (MAX.) INDICATES LOCATIONS OF POTENTIAL TILE FLOOR. - JOIST MANUFACTURER SHALL DESIGN FLOOR SYSTEM FOR ADD'L 10 PSF DEAD LOAD AT THESE

LOCATIONS.

• IIIIIII INTERIOR BEARING WALL

• □□□□□ BEARING WALL ABOVE (B.W.A.)

● ■■■ BEAM/HEADER

• JL METAL HANGER

INDICATES POST ABOVE (P.A.) PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

MULHERN+KULP RESIDENTIAL STRUCTURAL ENGINEERINS

3855 Brounds & Autor 200 - Aut



Mulhern+Kulp project number: 256-21006

MJF issue date: 10-21-202

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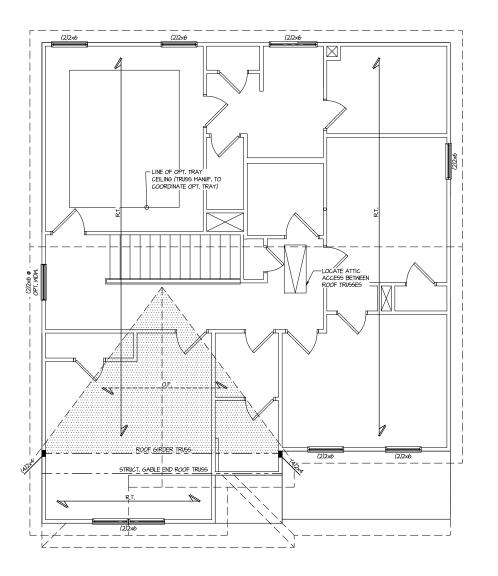
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PLAN MODEL FRAMING COLEMAN FLOOR 2ND

120 MPH WIND ZONE NORTH CAROLINA

S3.0M





MULHERN+KULP

RESIDENTIAL STRUCTURAL ENSINEERING

RESIDENTIAL ENSINEERING

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Mulhern+Kulp project number:

256-21006

MJF issue date: 10-21-202

JPP

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COLEMAN MODEL FRAMING PLAN

ROOF

S4.0M

120 MPH WIND ZONE NORTH CAROLINA

Harrington Lot 1/Model

THIS LEVEL HAS BEEN DESIGNED FOR 9'-I" PLATE HEIGHT

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

LEGEND

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SPACING), JOIST SERIES AND SPACING SHALL BE
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JOIST MANUFACTURER SHALL DESIGN FLOOR
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LOCATIONS.

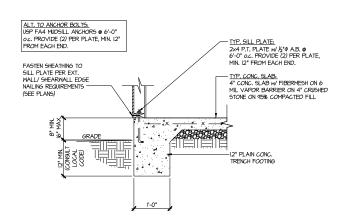
• IIIIIII INTERIOR BEARING WALL

● □□□□□ BEARING WALL ABOVE (B.W.A.)

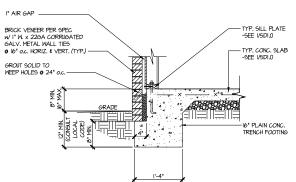
BEAM/HEADER

• JL METAL HANGER

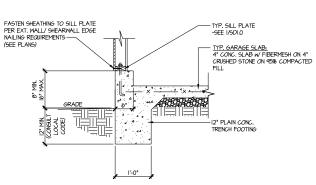
INDICATES POST ABOVE (P.A.) PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.



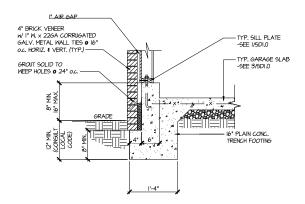
TYPICAL SLAB ON GRADE PERIMETER FOOTING



TYPICAL SLAB ON GRADE 2 PERIMETER FOOTING W/ BRICK VENEER



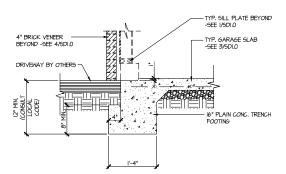
TYPICAL SLAB ON GRADE GARAGE 3 PERIMETER FOOTING



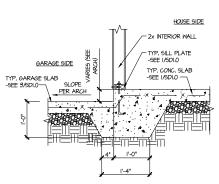
TYPICAL SLAB ON GRADE GARAGE 4 PERIMETER FOOTING

- TYP. SILL PLATE BEYOND -SEE I/SDI.0 - TYP. GARAGE SLAB -SEE 3/SDI.0 I'-0"

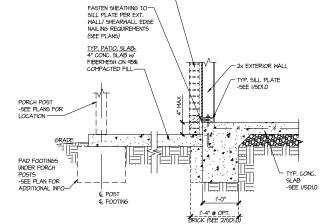
TYPICAL SLAB ON GRADE GARAGE (5) ENTRY @ PERIMETER FOOTING



TYPICAL SLAB ON GRADE GARAGE (6) ENTRY @ PERIMETER FOOTING

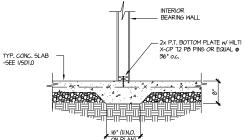


TYPICAL MONOLITHIC INTERIOR GARAGE FOOTING

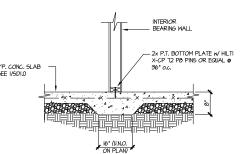


OPT. BRICK (SEE ARCH FOR LOCATIONS)

TYPICAL SLAB ON GRADE PERIMETER FOOTING @ PORCH/PATIO



TYPICAL THICKENED SLAB @ 9 INTERIOR BEARING WALL



Harrington Lot 1/Model

8/1/23

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERINS 265 Street city Parkey, Suite 255 - Agina 2-778-777-4874 - memberschapen NC License # C-3825

Mulhern+Kulp project number: 256-21006

SMK MJF issue date: 10-21-202

REVISIONS:

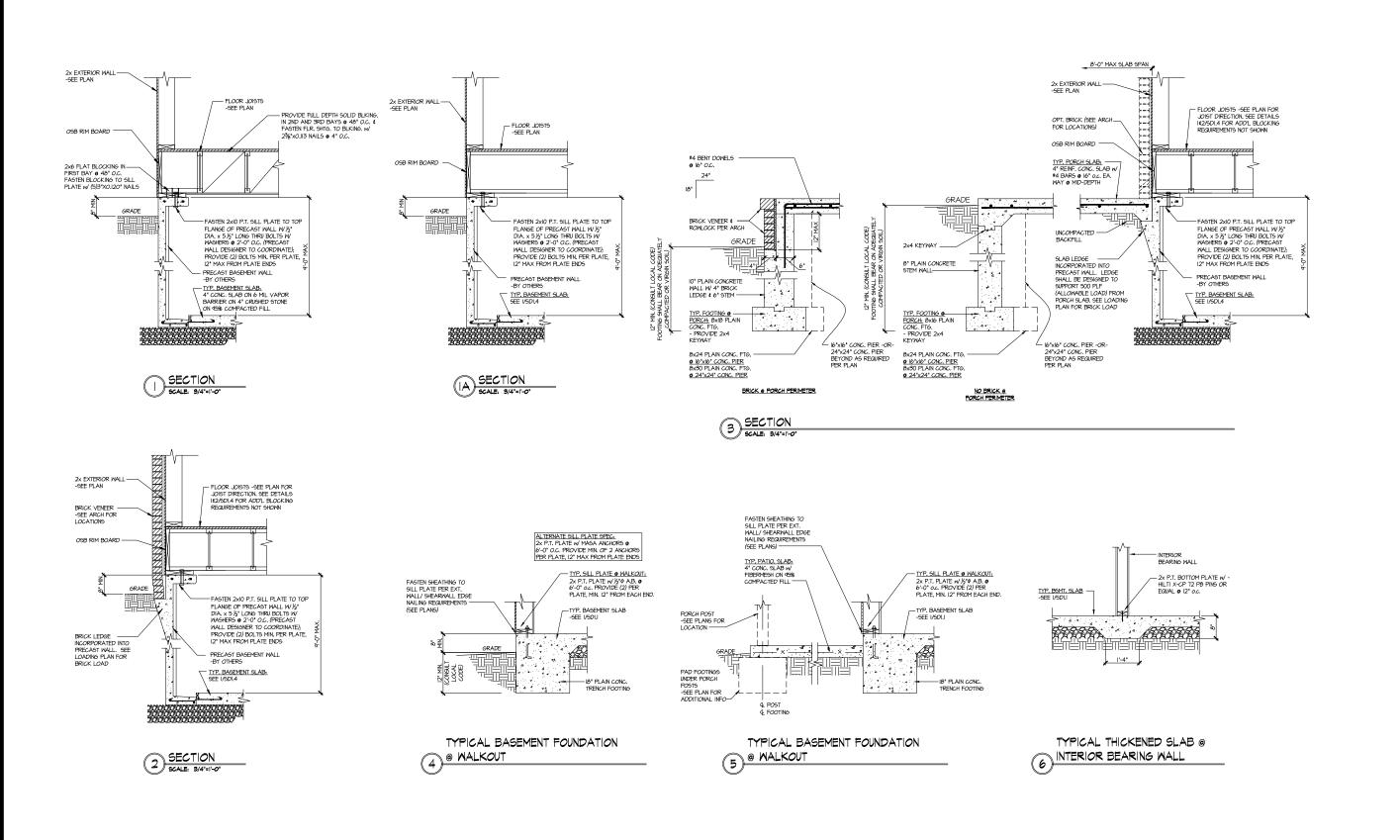
initial: JPP

SMITH DOUGLAS HOMES

MODEL FOUNDATION DETAILS

120 MPH WIND ZONE NORTH CAROLINA COLEMAN

SD1.0



Harrington Lot 1/Model

8/1/23

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING ₽77₽77₹874 ► material appears NC License # C-3825

Mulhern+Kulp project number: 256-21006

SMK MJF

> issue date: 10-21-202 REVISIONS:

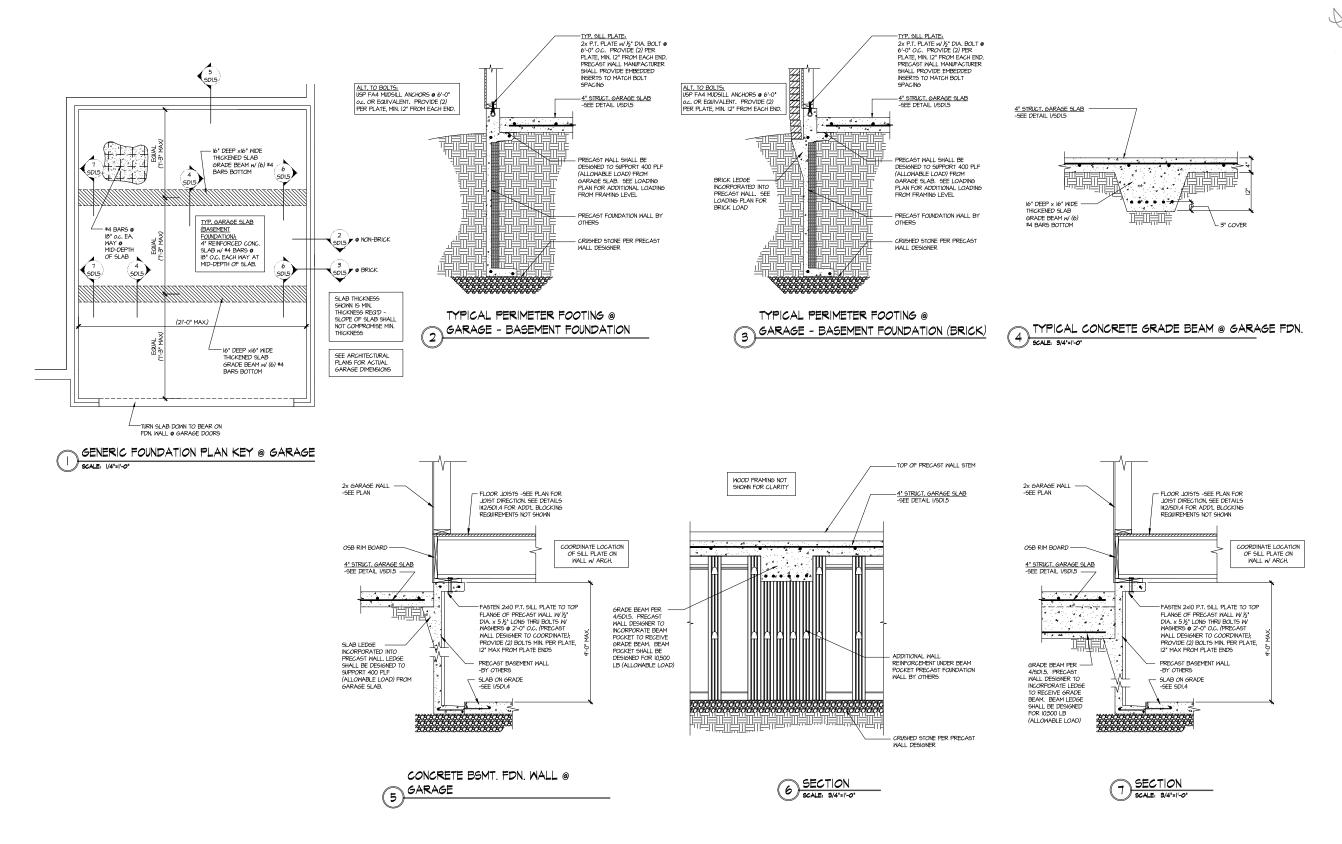
initial: JPP

SMITH DOUGLAS HOMES

MODEL FOUNDATION DETAILS

120 MPH WIND ZONE NORTH CAROLINA COLEMAN

SD1.4



Harrington Lot 1/Model 8/1/23

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERINS Filtradula Perkway, Soits 185 • Agin 1977-19874 • mathemical poses Chicense # C-3825

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Mulhern+Kulp project number: 256-21006

SMK drawn by: MJF

REVISIONS:

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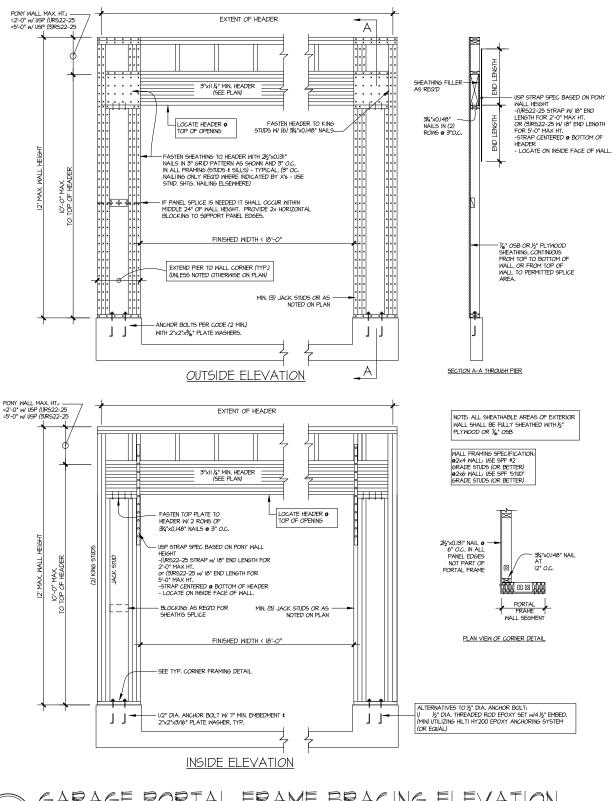
issue date: 10-21-202

SMITH DOUGLAS HOMES

MODI FOUNDATION DETAILS

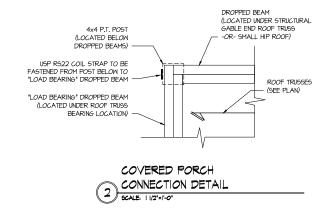
120 MPH WIND ZONE NORTH CAROLINA COLEMAN

SD1.5



SCALE: N.T.S.

GARAGE PORTAL FRAME BRACING ELEVATION BOTH SIDES OF GARAGE DOOR 120 MPH WIND SPEED (ULT)



8/1/23

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENSINEERINS

1905 Section Parkway, Suits 1905 • Aging 190

Mulhern+Kulp project number:

256-21006

MJF issue date: 10-21-202

REVISIONS:

initial: JPP

SMITH DOUGLAS HOMES

MODEL

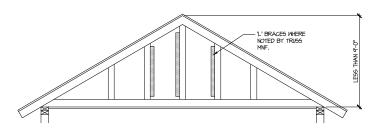
FRAMING DETAILS COLEMAN

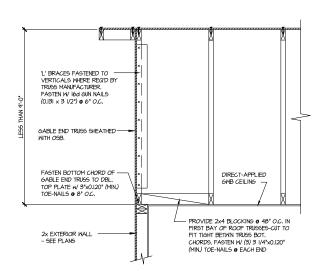
120 MPH WIND ZONE NORTH CAROLINA

Harrington

Lot 1/Model

SD2.0

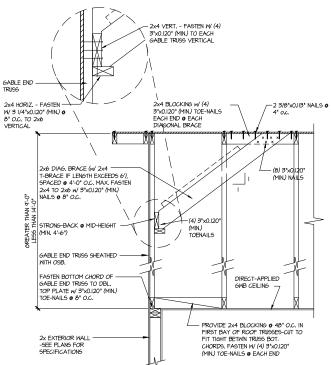




BRACE GABLE END TRUSGES PER ABOVE DETAIL WHEN GABLE HEIGHT IS LEGS THAN 9'-O'. L' BRACES REQUIRED WHERE NOTED BY TRUSS MANUFACTURER.

TYPICAL GABLE END BRACING DETAIL SCALE: NONE REGID & GABLE END TRUSS

- STRONG-BACK • MID-HEIGHT FOR DIAG. BRACES



TYPICAL GABLE END BRACING DETAIL SCALE. NONE REQUE 6 64BLE END TRUSS

BRACE GABLE END TRUSSES PER ABOVE DETAIL WHEN GABLE HEIGHT EXCEEDS 9'-0'. 'L' BRACES NOT REQUIRED.

- LAP ROOF RAFTERS AND WALL STUDS AND FASTEN W (5)3"x0.120" NAILS 2x SOLID BLOCKING @ EVERY BAY - FASTEN ROOF SHEATHING TO BLOCKING WITH 2 %"XO.II3" NAILS @ 3" O.C. - FLOOR I-JOISTS -SEE PLAN 2x6 ROOF RAFTERS @ 16" O.C. -USPJL24 HANGER TO-RIMBOARD 2x CEILING NAILER LAP CEILING JOISTS AND ROOF RAFTERS AND FASTEN W/ (3) 3'x0.120" NAILS BRICK VENEER— -SEE ARCH BRICK TO POCKET AROUND FRAMING MEMBERS - EXTERIOR WALL -SEE PLANS 2x4 CEILING JOISTS @ 16" O.C. PROVIDE I" CLEAR BETWEEN BRICK & WOOD FRAMING

DETAIL @ PENT ROOF SCALE. 3/4"=1"-0"

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

> Harrington Lot 1/Model

8/1/23

MUCHERNAL STRUCTURAL ENGINEERING
TESTICAL PRINCE, SER TO ANTICAL SATES
TO FITTER A TESTICAL SATES
TO F

Mulhern+Kulp project number: 256-21006

SMK MJF issue date: 10-21-202

REVISIONS:

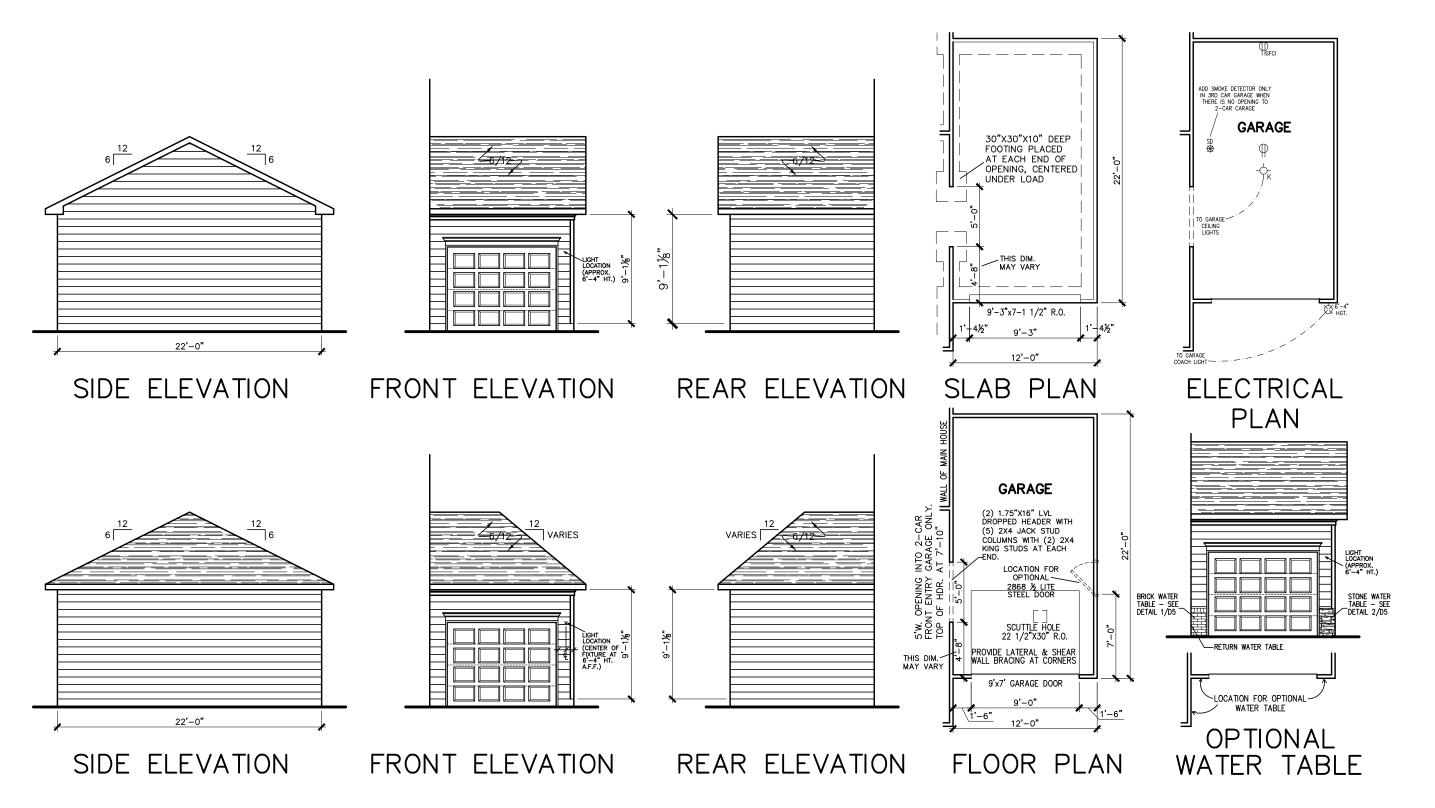
initial: JPP

SMITH DOUGLAS HOMES

MODEL

120 MPH WIND ZONE NORTH CAROLINA FRAMING DETAILS COLEMAN

SD2.1





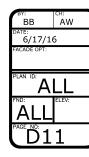


SMITH DOUGLAS HOMES

ADD ON GARAGE

SMITH DOUGLAS HOMES 110 VILLAGE TRAIL SUITE 115 WOODSTOCK, GA 30188 www.smithdouglas.com

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DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

- 2018 North Carolina Residential Building Code
- ASCE 1-10: Minimum Design Loads for Buildings and Other Structures

Design Loads:

_	l.	Roof

. 20 PSF
. 10 PSF
. 15 PSF
. 1.0
. 40 PSF
. 40 PSF . 30 PSF
40 PSF
.50 PSF
10 PSF
. 15 PSF
. 15 PSF
. 130 MPH
. B
.B .l.Ø

4.3.2 Vy =	
5. Component and Čladding (in PS	F

MEAN ROOF HT.	UP TO 30'	3@'1"-35'	35' "-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

6. Seismic

6.1 Site Class	D
62 Design Category	C
6.3 Importance Factor	10

6.4 Seismic Use Group.....

6.5 Spectral Response Acceleration

6.5.1 Sms = %q

6.5.2 Sml = %g 6.6 Seismic Base Shear

6.6.1 Vx =

6.62 Vy =

6.1 Basic Structural System (check one)

⊠ Bearing Wall

☐ Building Frame
☐ Moment Frame

□ Dual w/ Special Moment Frame

☐ Dual w/ Intermediate R/C or Special Steel

☐ Inverted Pendulum

osf



STRUCTURAL PLANS PREPARED FOR:

THIRD CAR ADD ON GARAGE (LH)

PROJECT ADDRESS:

TBD

OWNER:

Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 27539

ARCHITECT/DESIGNER:

Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineer of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AB ANCHOR BOLT PT PRESSURE TREATED AFF ABOVE FINISHED FLOOR RS ROOF SUPPORT CJ CEILING JOIST SC STUD COLUMN CLR CLEAR SJ SINGLE JOIST DJ DOUBLE JOIST SPF SPRUCE PINE FIR DSP DOUBLE STUD POCKET SST SIMPSON STRONG-TIE EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE TSP TRIPLE STUD POCKET OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE PSI POUNDS PER SQUARE INCH WUF WELDED WIRE FABRIC				
CJ CEILING JOIST SC STUD COLUMN CLR CLEAR SJ SINGLE JOIST DJ DOUBLE JOIST SPF SPRUCE PINE FIR DSP DOUBLE STUD POCKET SST SIMPSON STRONG-TIE EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE TSP TRIPLE STUD POCKET OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	AB	ANCHOR BOLT	Þ	PRESSURE TREATED
CLR CLEAR SJ SINGLE JOIST DJ DOUBLE JOIST DSP SPRUCE PINE FIR SST SIMPSON STRONG-TIE EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
DJ DOUBLE JOIST SPF SPRUCE PINE FIR DSP DOUBLE STUD POCKET SST SIMPSON STRONG-TIE EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE TSP TRIPLE STUD POCKET OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	CJ	CEILING JOIST	SC	STUD COLUMN
DSP DOUBLE STUD POCKET EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	CLR	CLEAR	SJ	SINGLE JOIST
EE EACH END SYP SOUTHERN YELLOW PINE EW EACH WAY TJ TRIPLE JOIST NTS NOT TO SCALE TSP TRIPLE STUD POCKET OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
EW EACH WAY NTS NOT TO SCALE OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
NTS NOT TO SCALE OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	EE	EACH END	SYP	SOUTHERN YELLOW PINE
OC ON CENTER TYP TYPICAL PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	ΕW	EACH WAY	TJ	TRIPLE JOIST
PSF POUNDS PER SQUARE FOOT UNO UNLESS NOTED OTHERWISE	NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
V 1	OC	ON CENTER	TYP	TYPICAL
PSI POUNDS PER SQUARE INCH WWF WELDED WIRE FABRIC		·		
	PSF	POUNDS PER SQUARE FOOT	uno	UNLESS NOTED OTHERWISE

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by SMITH DOUGLAS HOMES, Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

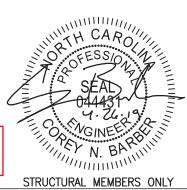
SHEET LIST:

Sheet No.	Description		
CSI	Cover Sheet, Specifications, Revisions		
C52	Specifications Continued		
S1.Ø	Foundation & First Floor Framing Plans		
SI.1	Roof Framing Plan		

REVISION LIST:

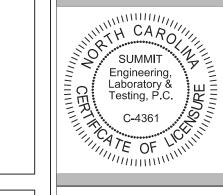
Revision No.	Date	Project No.	Description

Harrington Lot 1/Model





3070 Hammond Business Place Suite 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



THIRD CAR ADD ON GARAGE (LH)
Coversheet
client
Smith Douglas Homes - Raleigh
2520 Reliance Ave

CURRENT DRAWING

DATE: 4/26/2019

SCALE: 1/8"=1'-0"

PROJECT *: 3832265

DRAWN BY: CNB

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT * 10/21/2016 3832.21

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SUFFT

CS1

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entitu.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- 3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4
 of the 2018 NC Residential Building Code (Special
 consideration shall be given to Chapter 45 in wind zones
 above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- 4. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- 5. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- II. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter II of the 2018 NCRC

CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
- 3.1. Footings: 5%
- 3.2. Exterior Slabs: 5%
 4. No admixtures shall be added to any structural concrete
- without written permission of the SER
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint.
 Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of WWF.

CONCRETE REINFORCEMENT:

- I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM CIlle, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement.
 The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
 - 2.1. E = 1,900,000 psi
 - 2.2. Fb = 2600 psi
 - 2.3. Fv = 285 psi
 - 2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard BI82.1-1981.
 Lead holes for lag screws shall be in accordance with NDS specifications.
- Áll beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SPF*2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one IØd nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached wth (3) lod nails @ 24" O.C.
- IØ. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

WOOD TRUSSES:

- I. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- 2. All structurally required wood sheathing shall bear the mark of the $A\mathsf{P}A$.
- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"0/c at panel edges and at 12"0/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 5. Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (I)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

EXTERIOR WOOD FRAMED DECKS:

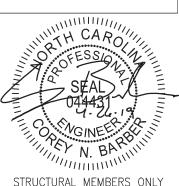
 Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

STRUCTURAL STEEL:

Harrington

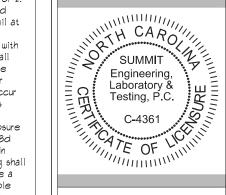
Lot 1/Model

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- . Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS DI.I. Electrodes for shopt and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above standards.





3070 Hammond Business Place Suite 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



THIRD CAR ADD ON GARAGE (LH)
Coversheet
client
Smith Douglas Homes - Raleigh
2520 Reliance Ave
Apex, NC 21539

CURRENT DRAWING

DATE: 4/26/2019

SCALE: 1/8"=1'-0"

PROJECT *: 3832,265

DRAWN BY: CNB

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT • 10/27/2016 3832.27

REFER TO COVER SHEET FOR A

SUFF

CS₂

FOUNDATION NOTES:

- 1. FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- AMENDMENTS. STRUCTURAL CONCRETE TO BE F $_{\rm c}$ = 3000 PSI, PREPARED AND PLACED IN
- STRADIGRAL COURTER IS TO BE 79, 30000 F91, FREFARED MAY FLAZED IN ACCORDANCE WITH ACI STANDARD 38.

 FOOTINGS TO BE FLACED ON INDISTURBED EARTH, BEARING A MINIMUM OF IZ TO SELOW ADJUSTED STRATE, BEARING A MINIMUM OF IZ TO SELOW ADJUSTED THE SELOW ADJUSTED THE THE CODE BYFORCEMENT OFFICIAL.

 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 F9F, CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY.
- OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
- FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS, PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF

- ELEMENTS, PROVIDE 2" MINIMM FOOTING PROJECTION FROM THE FACE OF MAGNEY

 MAXIMIT DEPTH OF UNBALANCED FILL AGAINST MASCHIRT WALLS TO BE AS SPECIFIED IN SECTION RAPAJ OF THE 20% NORTH CAROLINA RESIDENTIAL BUILDING CODE.

 PILLASTERS TO BE BONDED TO FERMITETER FOUNDATION WALL.

 FROVIDE FONDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OTHER 14 A REQUIRED BY SHE CONDITIONS.

 FROVIDED FERMITETER INBULATION FOR ALL FOUNDATIONS FER 20% NORTH CAROLINA RESIDENTIAL BUILDING CODE.

 CORREL FONDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEERS.

 CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.

 CROWN STATEMENT OF THE CONSTRUCTOR FER THE 20% NORTH CAROLINA RESIDENTIAL CODE SECTION RAPAJE MINIMM IN" DIA BOLTS SPACED AT 6 -6" OR CENTER WITH A "IMMINIMA PRESEDENTIAL CODE SECTION RAPAJE MINIMM IN" DIA BOLTS SPACED AT 6 -6" OR CENTER WITH A "IMMINIMA PRESEDENTIAL CODE SECTION RAPAJE MINIMM IN" DIA BOLTS SPACED AT 6 -6" OR CENTER WITH A "IMMINIMA PRESEDENTIAL TO MASCHEY OR CONCRETE. ANCHOR BOLTS SHALL BE IN" RECONT THE BID OF EACH PLATE OR CONCRETE. ANCHOR BOLTS SHALL BE IN" RECONT THE BID OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 9J = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER PL = POINT LOAD DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE
- 14. ALL PIERS TO BE 16"X16" MASONRY AND ALL PILASTERS TO BE 8"X16"
- 14. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONRY, TYPICAL. (UNIV) INJUST CONCRETE, SIZES PER STRUCTURAL PLAN B. A PONDATION EXCAVATION DESERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS GUALIFIED EXPRESSIONAL GEOTECHNICAL ENGINEER, OR HIS GUALIFIED EXPRESSIONAL GEOTECHNICAL ENGINEER, OR YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SHATT ENGINEERING, LABORATION'S AT THE TIME OF CONSTRUCTION, SHATT ENGINEERING, LABORATION'S AT THE TIME OF PROVIDED THE OPPORTUNITY TO REVIEW THE POOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
 17. ALL FOOTINGS 4 SLASS AND TO BEAR ON INDISTURBED SOIL OR 95% COMPACTED FILL, VERFIED BY ENGINEER OR CODE OFFICIAL.

- CONSTRUCTION SHALL CORPORT TO 2008 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL A*ENDYENTS.

 CODE WITH ALL LOCAL A*ENDYENTS.

 THE CONTRACTOR SHALL VERREY ALL DISENSIONS CONTRACTOR SHALL COMPLY WITH THE CONTRACTOR SHALL VERREY BUILDING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR SHAVING THE PROPERTY BUILDING TO RESIST ALL FORCES ENCONTERED DURING ERECTION.

 PROPERTIES USED IN THE DESIGN ARE AS FOR POIL 2: 1930° PSI

 PROPERTIES USED IN THE DESIGN ARE AS FOR SET 1930° PSI

 PARALLAM (15/1), \$= 1600 PSI, \$= 256 PSI, \$= 1930° PSI

 ALL WOOD TEMBERS SHALL BE \$\$ 95 FUNCES NOTED ON PLAN, ALL STUD COULTING AND JOINTS SHALL BE \$\$ 95 FUNCES.

- COLUMNS AND JOISTS SHALL BE 12 SPF (UNO)

- COLLINIS AND JOISTS SHALL BE 7 SFF (IMO).

 ALL BEAR'S SHALL BE SUPPORTED WITH A (7) 2x4 7 SFF STUD COLLIN'N AT EACH END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A65 AND SHALL HAVE A MINIMUM COVER OF 3?

 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED FER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION RASILE MINIMUM 12" DIA BOLTS SPACED AT 6-0" ON CENTER WITH A 1" MINIMUM EPBEDPIENT INTO MASONEY OR CONCRETE. ANCHOR BOLTS SHALL BE E" REOM THE BUD OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION ANCHOR BOLTS SHALL BE DESCRIPTION OF THE BUD OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- CONTRACTOR TO PROVIDED LOXCOITS WHEN CEILINS JOSTS 99-AN
 FERFENCIOLULAR TO RAFTERS.
 FLITCH BEANS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS 5-HALL BE BOLTED
 TOGETHER WITH I/2" DIA THRU BOLTS 9PACED AT 24" OC. (THAX) STAGGERED OR
 EQUIVALENT CONNECTIONS FER DETAIL IODS! MIN EDGE DISTANCE 9HALL BE 2"
 AND (2) BOLTS SHALL BE LOCATED MINITH 6" FROM EACH BOL OF THE BEAM
 II. ALL NON-LOAD BEARING HEADERS SHALL BE (1)" FLAT 2"A 95" "S, DROPPED FOR
 NON-LOAD BEARING HEADERS EXCEEDING 8"-6" NI WIDTH AND/OR WITH HONE
 THAN 2"-6" OF CRIPTLE WALL ABOVE, SHALL BE (2)" FLAT 2"A 95" "S, DROPPED.
- (UNLESS NOTED OTHERWISE) ABBREVIATIONS:
- DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER PL = POINT LOAD

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>SMITH DOUGLAS HOMES</u> COMPLETED/REVISED ON <u>89/23/16</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SMITHT ENSINEERING, LABORATORY & TESTING, P.C. IF ANY CHANGES

ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO

BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

ARE THADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCT SYMMIT ENGINEERING, LABORATORY (* TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN WESD WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE: LISTED ABOVE.

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I PER TABLE R4051

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFO. PER SECTION R602.10.4 AND

FIGURE R602.10.3(4) OF THE 2018 NCRC.

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.4.3 OF THE 2018 NCRC

DID D3b P10b/ Dlm/ \ D2m, 3000 PSI 4" CONCRETE SLAB W/ 6"x6" W.4xW.4 WWR OR FIBERMESH REINFORCEMENT OVER 6 MIL. (D100) 3000 PSI 4" CONCRETE SLAB W/ 6"x6" WI.4xWI.4 WWR OR FIBERMESH REINFORCEMENT AND *4 REBAR * 24" O.C. RUNNING PERPENDICULAR 30"x30"x10" DP CONC. FTG. (W/ OPT 5'-0" OPENING) OPT. BRICK NOTE: FOOTINGS TO BE ´4` CENTERED UNDER POINT LOADS FROM 2 (Dlm) HEADER ABOVE, SEE PLAN FOR SPECIFIC (Dep/Dep/ \ D2m, (D10b)

MONOSLAB FOUNDATION PLAN

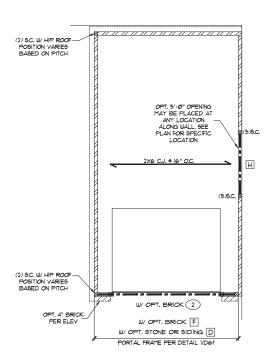
NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

FIRST FLOOR BRACING (FT)					
CONTINUOUS SHI	CONTINUOUS SHEATHING METHOD: ELEV A.D.G.J. & C.F.J.L.				
	REQUIRED PROVIDED				
FRONT SIDE	4.0	4.5			
LEFT SIDE	4.0	8.5			
REAR SIDE	4.0	12.0			
RIGHT SIDE	4.0	22.00			

NOTE: SEE PLAN FOR SPECIFIC LOCATION NOTE: REFER TO BASE PLAN FOR REQ'D FND FRAMING @ HOUSE



1ST FLOOR FRAMING PLAN

BASEMENT FOUNDATION PLAN

REQ	REQUIRED BRACED WALL PANEL CONNECTIONS					
		MIN.	REQUIRED CONNECTION			
METHOD	MATERIAL	THICKNESS	@ PANEL EDGES	# INTERMEDIATE SUPPORTS		
C9-W6P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS # 12" O.C.		
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ® 7" O.C.	5d COOLER NAILS** ⊕ 7" O.C.		
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 6 ° O.C.	6d COMMON NAILS 12" O.C.		
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1		
"OR EQUIVALENT PER TABLE RT@235						

BRACED WALL NOTES:

REAR HOUSE

FRONT

WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 WALLS SHALL BE USEIGNED IN ACCOMPANCE WITH SECTION 1860/180 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE.

WALLS ARE DESIGNED FOR SEISHIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 180 MPH.

REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING SIZES.

BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE REGOJOJ.

ACCOMPANCE WITH TABLE REPORTED.

ALL BRACED WALL PANELS SHALL BE RULL WALL HEIGHT AND SHALL

NOT EXCEED TO FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR

CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING

CONTINUOUS SHEATHING TELHOU WILDOW DOUT DOUBLE ENGINEERING CALCULATION, ENGTH SHALL BE FER TABLE REØX.10.1. THE NTERORS OIDS OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 12" GYPSUM BOARD (UNO).
FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE

SUMMIT Engineering, Laboratory & Testing, P.C.

C-4361 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR UALLS SHALL BE SHEATHED ON ALL SHEATHABLE SHEATES FORLIDING NIFILL AREAS BETWEEN BRACED UALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE FIND WALLS.
FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FORDATION OF BEARSH WALL BELOW WITHOUT ADDITIONAL BIGNEEPING CALCILLATIONS.
A BRACED WALL NEW SHALL BE LOCATED WITHIN 12 FEET OF EACH BIO OF A BRACED WALL INSE.
THE MACHINE BOOK DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.

C-4361

C-4361

C-4361 NOT EXCEED 21 FIET.

NASONEY OR CONCRETE STEM WALLS WITH A LENGTH OF 49° OR LESS SUPPORTING A BRACED WALL PAYEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE RESOLUCIA'S OF THE 20'8 NORD.

BRACED WALL PANEL CONNECTIONS TO FLOORICELING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION RESOLUCIAL PANEL OF CONSTRUCTED IN ACCORDANCE WITH SECTION RESOLUCIAS SO CONSTRUCTED IN ACCORDANCE WITH SECTION RESOLUCIAS SO CONSTRUCTED IN ACCORDANCE WITH SECTION RESOLUCIAS DE DESIGNED IN ACCORDANCE WITH SECTION RESOLUCIAS WITH SECTION RESOLUCIAS WITH FIGURE RESOLUCIA WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION RESOLUCIA WALLS SHALL BE WALLD WALLS SHALL BE WALL BE WALL

ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

HEADER/BEAM SCHEDULE

STUD COLUMN (SC.) CALL OUTS ON PLAN BEAM/HEADER SCHEDULE. KING STUDS TO BE INSTALLED PER APPLICABLE BUILDING CODE

WALL STUD SCHEDULE

16T & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" O.C. OR 2x6 STUDS @ 24" O.C. IST FLOOR LOAD BEARING STUDS W/WALK-UP ATTIC: 2×4 STUDS © 12" O.C. OR 2×6 STUDS © 16" O.C. 244 STUDS 6 12" O.C. ON 266 STUDS 6 16" O.C. BALLOON

244 STUDS 6 12" O.C. ON 266 STUDS 6 16" O.C. NON-LOAD BEARING STUDS 6 16" O.C. NON-LOAD BEARING STUDS (ALL FL.CORS).

244 STUDS 6 24" O.C. ON 266 STUDS 6 16" O.C. BALLOON FRAMED W/ CROSS BRACING . 6'-0" O.C. VERTICALLY

KING STUD REQUIREMENTS				
OPENING WIDTH	KINGS (EACH END)			
LESS THAN 3'-0"	(D)			
3'-Ø TO 4'-Ø"	(2)			
4'-0" TO 8'-0"	(3)			
8'-0" TO 12'-0"	(5)			
12'-@" TO 16'-@"	(6)			

KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS

		HEADER TAG	BEAM TAG	SIZE	JACKS (EACH END)
		-	BI	(1) 14" FLOOR JOIST	(2)
Į		-	B2	(2) 14" FLOOR JOIST	(2)
_		A	B3	(2) 2x6	(1)
ı		В	B4	(2) 2x8	(2)
I		С	B5	(2) 2xlØ	(2)
I		D	B6	(2) 2x12	(2)
I		E	B1	(2) 9-1/4" LVL	(3)
I		F	B8	(2) 11-7/8" LVL	(3)
I		G	B9	(2) 14" LVL	(3)
I		H	BIØ	(2) 16" LVL	(3)
1		1	BII	(2) 18" LVL	(3)
H	1	,	m10	(2) 23 1 4	(4)

(3) II-7/8" LVL (3) I4" LVL (3) I6" LVL PIS 1 (37 AF LVI. (4))

PIS 1 (37 AF LVI. (4))

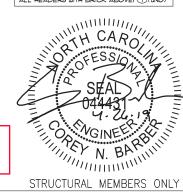
READERBEAM SIZES SHOUND OF ILANS ARE MINITUMS, GREATER
HEADERBEAM SIZES MAY BE USED FOR EASE OF CONSTRUCTION
ALL HEADERS TO BE PROPPED UNLESS NOTED OTHERWISE. ALL
BEAMS TO BE FLUSH UNLESS NOTED OTHERWISE.

LINTEL SCHEDULE					
TAG	TAG SIZE OPENING SIZE				
0	L3x3x1/4"	LESS THAN 6'-0"			
2	L5x3x1/4"	6'-0" TO 10'-0"			
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"			
4	ROLLED OR ALL ARCH EQUIV.				
SECURE LINTEL TO HEADER W/ (2) 1/2"					

DIAMETER LAG SCREWS STAGGERED # 16"

O.C. (TYP FOR (3)) ALL HEADERS WITH BRICK ABOVE: ()(UNO)

CHECKED BY: WAJ



S1.0

DATE PROJECT *

REFER TO COVER SHEET FOR A

COMPLETE LIST OF REVISIONS

3832.27

3070 Hammond Business Place

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CURRENT DRAWING DATE: 4/26/2019 SCALE: 1/8"=1'-@"

PROJECT *: 3832.265

DRAWN BY: CNB

10/27/2016

ORIGINAL DRAWING

SHEET

 ∇ F

Slab

Monolithic

Harrington

Lot 1/Model

FOUNDATION & FIRST FLOOR FRAMING PLANS

TRUSS UPLIFT CONNECTOR SCHEDULE					
MAX. UPLIFT ROOF TO WALL FLOOR TO FLOOR FLOOR TO			FLOOR TO FND		
535 LB6 H2.5A PER WALL SHEATHING & FASTENERS					
IØ7Ø LBS (2) H2.5A		C6I6 (END = 13")	DTT2Z		
1245 LBS	HT52Ø	CSI6 (END = 13")	DTT2Z		
249Ø LBS	(2) HT52Ø	(2) CSI6 (END = I3")	HTT4		
2365 LBS	LGT3-9D52.5	(2) CSI6 (END = 13")	HTT4		
I ALL DECOLICTS LISTED ARE SIMPLEAN STRONG TIE EQUIVALENT DECOLICTS					

I. ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE EQUIVALENT IMPOULUS MAY BE USED PER MANAFACTURER'S SPECIFICATIONS

2. UPLIFT VALUES LISTED ARE FOR SPF 12 GRADE HEMBERS.

3. REPER TO TRUSS LAYOUT PER MAUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTIONS SPECIFIED BY TRUSS MANAFACTURER.

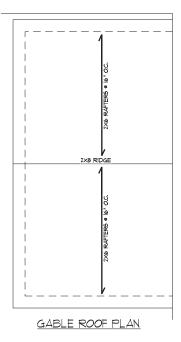
TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.

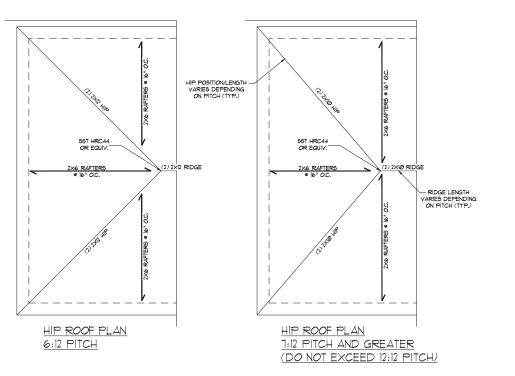
4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (1YP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: TRUSS UPLFT LOADS SHALL BE DETERMINED PER TRUSS MANIFACTURER IN ACCORDANCE WITH SECTION RESSIT. WALLS SHEATHING AND FASTENERS HAVE BEED DESIGNED TO RESIST THE WIDD UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION RESIDED THE 2018 NCRC, REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.





THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED REVISED ON \$23236. If IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SWITHING THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SWITHING, P.C. PANY CLAMAGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WEND WISH MEAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/8"=1"



Harrington

ON GINER BUTTON OF TURAL METERS OF TURAL METERS OF TURAL METERS OF THE PROPERTY OF THE PROPERT STRUCTURAL MEMBERS ONLY



3070 Hammond Business Place Suite 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM

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C-4361

C-4361

9 ON GARAGE <u>@</u> 0<u>7</u> Fnd client Smith Douglas Homes 2520 Reliance Ave Apex, NC 21539 Slab ADD CAR Monolithic TH RD

CURRENT DRAWING

DATE: 4/26/2019

SCALE: 1/8"=1'-@"

PROJECT #: 3832.265

DRAWN BY: CNB

CHECKED BY: WAJ

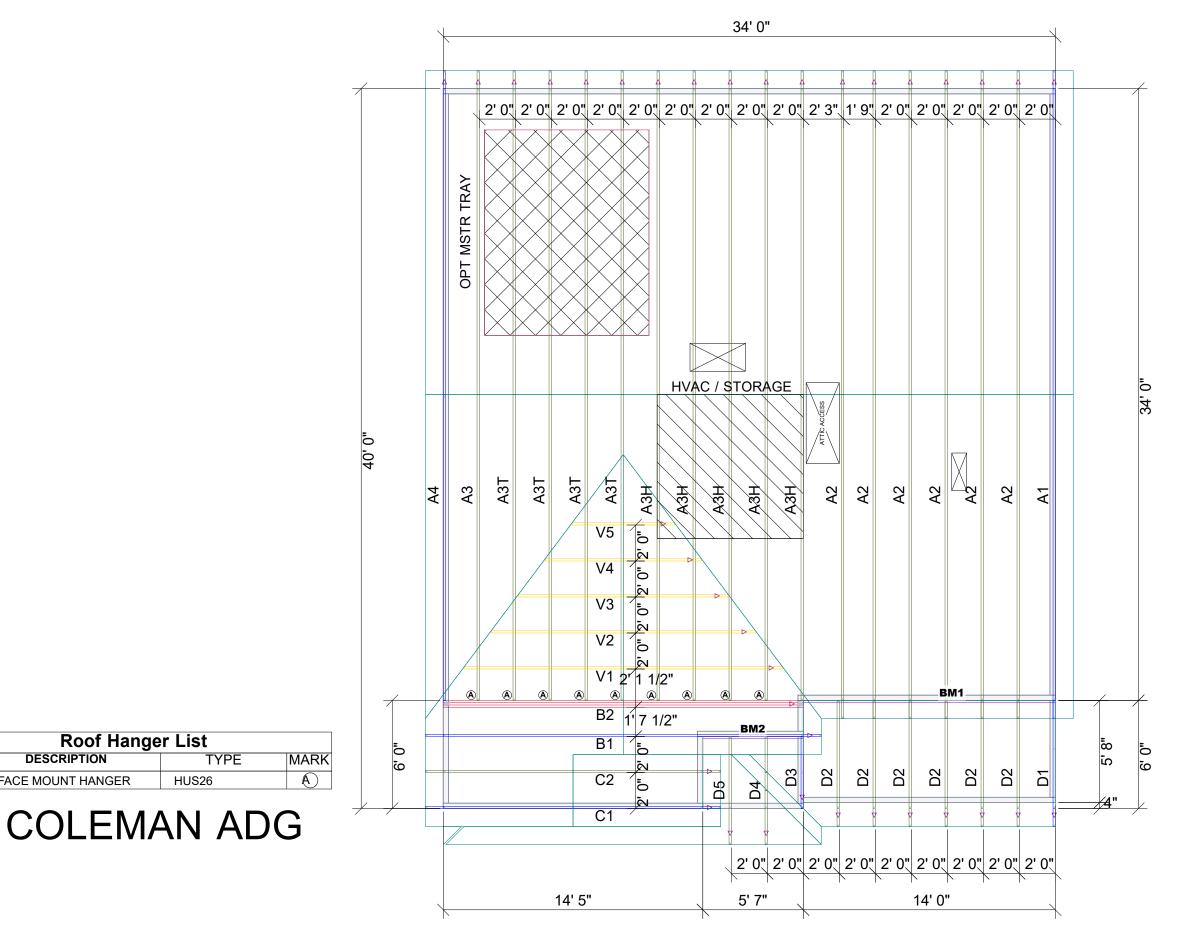
ORIGINAL DRAWING

DATE PROJECT * 10/27/2016 3832.27

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S1.1



PLACEMENT PLAN

Roof Hanger List

TYPE

HUS26

DESCRIPTION

FACE MOUNT HANGER

UFP SITE BUILT
A UPP INDUSTRIES COMPANY

DOUGLAS

DESIGNER JNN LAYOUT DATE 8/1/23 ARCH DATE -

JOB #: MASTER

12' 0" |2' 0" |2' 0" |2' 0" |2' 0" |2' 0" 3CG2 3CG2 3CG2 **G2** 3CG2 22' 0" 3CG1 3C 12' 0" PLACEMENT PLAN

UFP SITE BUILT A UPP INDUSTRIES COMPANY

CAR (**COLEMAN 3RD**

DOUGLAS

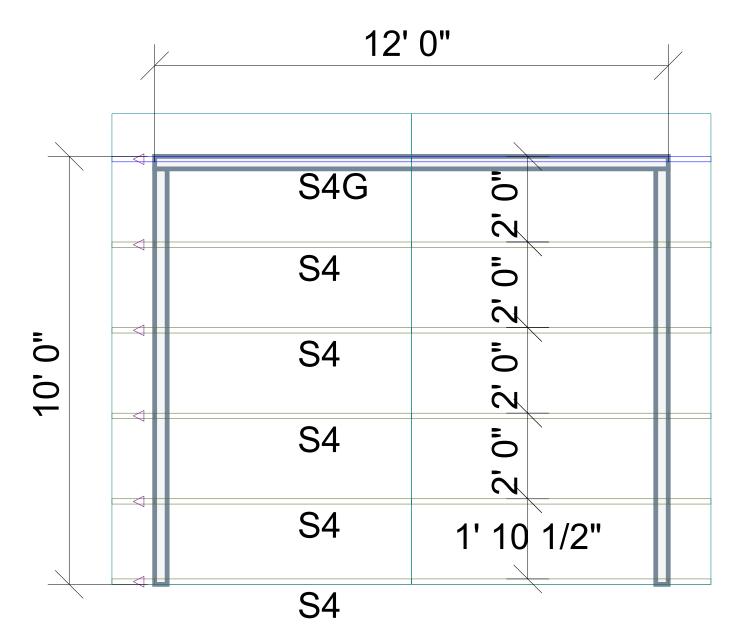
SMITH

DSN

DESIGNER JNN LAYOUT DATE 8/1/23 ARCH DATE -STRUC DATE

JOB #: MASTER

 \triangle indicates left end of truss Scale: N.T.S



COLEMAN 10x12 PORCH

 Δ Indicates Left End of Truss VALLEY LINES: 0 ROOF AREA: 162.33 ft²_RIDGE LINE: 11 ft

UFP MID-ATLANTIC, LLC
A UNIVERSAL FOREST PRODUCTS COMPANY

COLEMAN 10 X 12 PORCH SD COMMUNITIES

CP PROPOSED COLEMAN D SLAB 3 CAR - LEFT PO SW SW DW INSET SCALE: 1"=20"

LOT INFORMATION:

PIN:9680-69-5816 REFERENCE: DB4093 PG 784 TOTAL LOT AREA = 0.460 AC = 20,079 SF HOUSE = 1,540 SF PORCH = 84 SF SIDEWALKS = 100 SF DRIVEWAY = 808 SF COVERED PATIO = 120 SF PROPOSED IMPERVIOUS = 2,544 SF PERCENT IMPERVIOUS = 12 67%

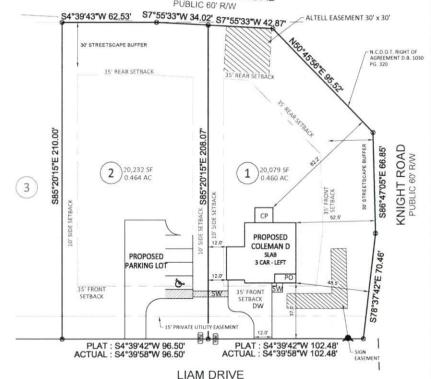
LOT INFORMATION:

PIN:9680-69-5936 REFERENCE: DB4093 PG 784 TOTAL LOT AREA = 0.464 AC = 20,232 SF PARKING LOT = 2,619 SF PROPOSED IMPERVIOUS = 2,619 SF PERCENT IMPERVIOUS = 12 95%

BUILDING SETBACKS FRONT - 35' REAR - 35' SIDE - 10' CORNER SIDE - 20"



ROSSER PITTMAN ROAD



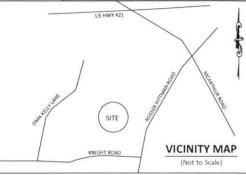
PUBLIC 60' R/W



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Engineers • Surveyors • Planners

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LEGEND

PO = PORCH WD = WOOD DECK WD = WOOD DECK
SW = SIDEWALK
DW = CONC DRIVEWAY
P = CONC PATIO
&= COMPUTED POINT
X = MAG NAIL FOUND

O= IRON PIPE FOUND ●= IRON PIPE SET = IRON PIPE SET
 = DRILL HOLE FOUND
 | WATER METER
 CO = CLEAN OUT

AC = AIR CONDITIONER

S = SEWER MANHOLE

EB = ELECTRIC BOX O = CABLE BOX

T = TELEPHONE PEDESTAL
CB = CATCH BASIN
IC = IRRIGATION CONTROLLER ☼ = LIGHT POLE

0 = UTILITY POLE = FIRE HYDRANT

DI = DRAIN INLET

WY = WATER VALVE

= STREET SIGN

YI = YARD INLET E = ELECTRIC METER

I, STEVEN P. CARSON, CERTIFY THAT THIS PLAT WAS DRAWN UNDER MY DIRECT SUPERVISION FROM A SP = SCREENED PORCH/PATIO SURVEY MADE UNDER MY SUPERVISION (PLAT BOOK CP = COVERED PORCH/PATIO REFERENCED IN TITLE BLOCK); THAT THE BOUNDARIE NOT SURVEYED ARE CLEARLY INDICATED AS DIAWN FROM INFORMATION LISTED UNDER REFERENCES; THAT THE RATIO OF PRECISION AS CALCULATED IS 1:10,000+; AND THAT THIS MAP MEETS . JE REQUIREMENTS OF THE STANDARD OF RECTICE FOR LAND SURVEYING IN NORTH CAROLIF A. L-4752 DATED:

> This map is of an existing parcel of land and is only intended for the parties and purposes shown. This map not for recordation. No title report provided.

BUILDER TO VERIFY HOUSE LOCATION DIMENSIONS AND REVIEW TOTAL IMPERVIOUS NOTED ON THIS PLOT PLAN

PRELIMINARY PLOT PLAN FOR

SMITH DOUGLAS HOMES

HARRINGTON PLACE - LOTS 1-2

20, 40 LIAM DRIVE, BROADWAY, NC UPPER LITTLE RIVER TOWNSHIP, HARNETT COUNTY

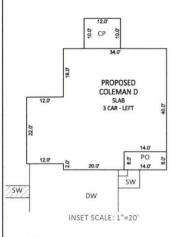
DATE: 7/25/23 DRAWN BY: AHB CHECKED BY: SPC

REFERENCE: BK2023 PG335

1" = 40 ft.

BCS# 230119

SCALE: 1" = 40"



NOTES:

- THIS SURVEY WAS PREPARED BY BATEMAN CIVIL SURVEY CO., UNDER THE SUPERVISION OF STEVEN P. CARSON, PLS.
- THIS PLAN HAS BEEN PREPARED FOR LAYOUT AND PERMITTING
- PROPERTY LINES SHOWN WERE TAKEN FROM EXISTING FIELD EVIDENCE, EXISTING DEEDS AND PLATS OF PUBLIC RECORD, AND NFORMATION SUPPLIED TO THE SURVEYOR BY THE CLIENT.
- ALL DISTANCES ARE HORIZONTAL GROUND DISTANCES AND ALL BEARINGS ARE NORTH CAROLINA STATE PLANE COORDINATE SYSTEM UNLESS OTHERWISE SHOWN.
- THIS MAP IS NOT FOR RECORDATION AND SHOULD BE REVIEWED BY A OCAL GOVERNMENT AGENCY FOR COMPLIANCE WITH ANY APPLICABLE LAND DEVELOPMENT REGULATIONS.
- THE BASIS OF NORTH AND ALL EASEMENTS, RIGHTS-OF-WAYS, BUFFERS, SETBACKS AND ADJOINERS, ETC, REFERENCED IN TITLE
- NO INVESTIGATION INTO THE EXISTENCE OF JURISDICTIONAL WETLANDS OR RIPARIAN BUFFERS PERFORMED BY THIS FIRM.
- SURVEYOR HAS MADE NO INVESTIGATION OR INDEPENDENT SEARCH FOR EASEMENTS OF RECORD, ENCUMBRANCES, RESTRICTIVE COVENANTS, OWNERSHIP TITLE EVIDENCE OR ANY OTHER FACTS THAT AN ACCURATE AND CURRENT TITLE SEARCH MAY DISCLOSE
- THE PROPERTY IS LOCATED IN FEMA FLOOD ZONE "X" MAP # 371096800J DATED 10/3/2006.
- 10. ZONING IS RA-20.
- 11. PROPERTY OWNER: SMITH DOUGLAS HOMES 2520 RELIANCE AVENUE