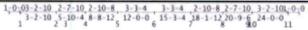
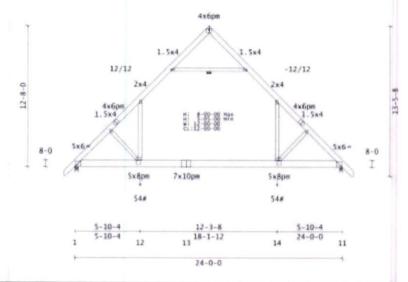


Project Name: QTREC0716111 Truss: GA1 Qty: 16 Customer: WALK IN SID TID Date: 01/31/22 Page: 1 of 1





Truss Weight = 198.0 lb

de/Design: IRC-2015/TF1-2 F Live Dead Dur Live 30.0 FSF Live Dead Dur Pactors
TC 30.0 7.0 Live Wind Snow
BC 0.0 10.0 Lum 1.15 1.60 1.15
Total 47.0 Fit 1.15 1.60 1.15
Bpacing: 2-05-00 o.c. Piles: 1
Bepetitive Member Increase: Yes
Green Lumber: No Met Service: No
Fab Tolerance: 156 Creep (Mort) = 2.0
OH Soffit Load: 1.0 psf - Seew Load Spece ASCE7-10 Spound Snow(Pg) - 40.0 paf Nisk Cat: II Terrain Cat: B Noof Expoure: Fully Exposed Thermal Condition: Cold Pentilated(1.1) Inobstructed Slippery Roof: No Low-Slope Minimumo(Pfmin): No

Unbalanced Snow Loads: Yes Main Surcharge: No Ice Dam Chk: Yes

M.R.K(n) - 15.0 ft Kgt - 1.0 Bldg Enclosure: Enclosed Wind DLCpsf;: TC = 4.2 BC - 6.0 End Vestical Exposed: L - Yes K - Yes Wind Oplift Reporting: ASCE? NWYKS C&C End Zone: 4-00-00

-Additional Design Checks Non-Concurrent BCLL: Teq 19 pel Non-Computent SCLL: 10 pel BC Limited Sterage: 100 ib BC Accessible Galling: 100 ib TC Maintenance Load: 100 ib TC dafe Load: 100 lb TC dafe Yes Yes

Material Summary

20	2x6	SP (ALSC6-2013)	61
	2×6	SF 2400/2.0 3-	6 6-9
BC	2×8	SP (ALSC4-2013)	63
Maba	2×4	OPF #3	

mber Forces Summary

		rees our		
	Hon	Ten	Comp	.CAI.
30		84	0	0.04
	1- 3	0	2052	0.24
	2- 3	D.	1957	0.18
	3- 4	0	1929	0.60
	4- 5	70	1119	0.57
	5- 6	144	217	0.35
	6- 7	144	217	0.35
	7- 8	7.0	1119	0.57
	8- 9	0	1929	0.60
	9-10	0	1957	0.18
	10-11	0	2052	0.14
	11-OH	8.4	0	0.00
BC.	1-12	1400	0	0.50
	11-14	1400	0	0.50
	12-13	1123	0	0.91
	13-14	1123	0	0,85
Web	2-12	86	413	0.11
	4-12	1102	0	0.31
	5- 7	92	1760	0.41
	8-14	11.02	0	0.91
	10-14	86	413	0.251

Reaction Summary

Jat	X-Loc-	React	-Up	-Midth-	-Roqd	-Mat	PSI
1	01-12					DFL	565
11	23-10-04	1580	- G	03-08	01-14	59%	568
Has	Horiz =	-177 /	+17	7 at Jo	int 1		

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [12-00-00] using a 1.00 Full and 0.00 Reduced load

Attic space centered at 12-00-00 is loaded with 40.0 psf live s 1.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection driteria 1/360.

See Loadcawe Report for loading combinations and additional details. Dead Loads may be slope adjusted: > 8.0/12 Loads based on maximum and minimum reactions from tie-in spane Domain Max Min Location Dir Description BC 54 32 5-10-04 Vert SidewallDL BC 54 32 18-01-12 Vert SidewallDL

Notes

Plates designed for Cq at 0.65 and Rotational Tolerance of 10.0

degrees:
Plates located at TC pitch breaks meet the prescriptive minimum are
requirement to transfer umblucked disphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user
modified; see Flate Offsets for any special positioning requirements,
Continuous Lateral Restraint (CLR) rows require diagonal bracing per
D-REBCLRBRACE. Alternatively, see D-REBELHFORCE.

Detiec	tion aumin	aummary				
TrussS	pan Limit	Actual (in)	Location			
Vert L	L L/240	1/660 (=0.43)	12-14			
Vert D	L L/90	1/999(-0.10)	12-14			
Vert C	R L/180	1/465(-0.61)	12-14			
Mora L	1. 0.751	h (0.01)	0.21.11			
Bors C	N 1.251	6 (0,02)	8-71.11			
Ohne C	R 21/180	21/999(-0.00)	1- 1			
		ZL/999(-0,00)				

Bracing Data Summary

Chords; continuous except where shown attic tie beam (TB) & walls; bracing indicated or sigid eheathing. TB 3-05-00 8-07-00 15-05-00 2 Single: ontinuous Restraint Bracing Req'd

Plate offsets (X, Y):

indicated below)

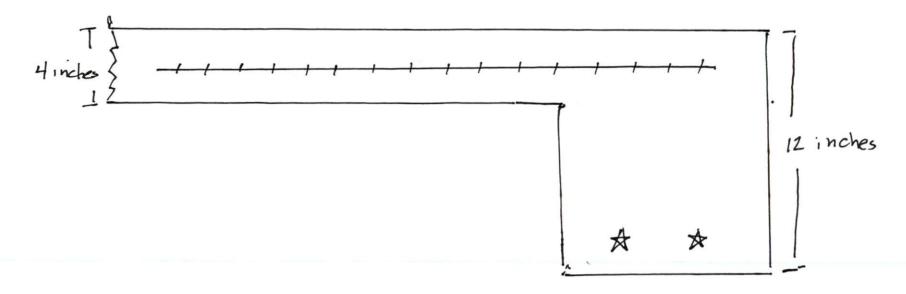
Joint Stress Index (JSI):

Jnt1(0.36). Jnt2(0.55). Jnt3(0.56). Jnt4(0.85). Jnt5(0.91). Jnt6(0.33). Jnt7(0.91). Jnt8(0.85). Jnt9(0.56), Jnt10(0.55). Jnt11(0.36). Jnt12(0.44). Jnt13(0.94). Jnt14(0.44)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and reflex upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates a coeptance of professional engineering responsibility sokely for the truss component design shown. See the cover page and the "important information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-The Company, inc in accordance with ESR-2782. All connector plates are 20 gauge, unless the specified plate size is followed by a "-16" which indicates an 18 gauge plate, or "54" 18", which indicates a high tension 18 gauge plate.



Midwest Manufacturing 5311 Kane Road Eau Claire, WI 54703 (715) 876-5555 midwestmanufacturing.com Footing Plan for 2-Car Garage 58 Plain field Kane Lillington, N.C. 27546 6/2/2025



not to scale

A - Rebar

D - Welded Wire

Michelle F. Myatt

Central Permitting Division

Richard Peppe <pepperwjr@aol.com> Thursday, June 19, 2025 10:05 AM Michelle F. Myatt Dick Peppe; Maggie Peppe Re: Business License Number for Site Built Sheds Dwner Affidavit.pdf</pepperwjr@aol.com>
to the engineers questions concerning the garage bill. Been out of tow te transactions.
on the second floor space of the garage? e stored on the second floor will be my wife's Christmas light containers ranging from Christmas, easter, July 4th, etc. r will be a stair way = Three rails installed under each step with a hand ack of the garage-Approximately 16 steps. No windows will be installe for storage only. Electrical lighting will be installed later in the year and that will require another work permit. eeting up with the roof will be 8 feet tall, 16 inches on center. The soffit inches. Eventually gutters will be installed later in the year.
n please do not hesitate to contact me.
55 PM EDT, Michelle F. Myatt <mmyatt@harnett.org> wrote:</mmyatt@harnett.org>
eneral contractor if you are doing the work yourself.
te the attached notarized affidavit.