



# STRUCTURAL CALCULATIONS PREPARED FOR TARHEEL BASEMENT SYSTEMS FOR MILBOURN RESIDENCE FOUNDATION REPAIR 100 MARION DR ERWIN, NORTH CAROLINA

PROJECT NUMBER: 25.090.TBR

**DATE:** April 7, 2025

**PROJECT MANAGER:** Alicia Schumacher, P.E.

**COA:** C-4876





April 7, 2025 Project No.:25.090.TBR

Tarheel Basement Systems 3333 Air Park Rd Fuquay-Varina, North Carolina 27526

RE: Foundation Repair - 100 Marion Dr, Erwin, North Carolina

# PROJECT BACKGROUND

We understand the structure is a single-family residence and the owner wishes to provide additional framing support. It is our understanding (3) S4x7.7 supplemental beams and (6) Intellijack systems have been recommended by the contractor. A recent field sketch (attached) indicates the proposed locations of repair.



Image 1: Front Elevation

# **GEOLOGIC SETTING**

The existing structure is located in Erwin, North Carolina. Based on the information provided by the USDA Web Soil Survey, the general site condition in the area is comprised of sandy loam and the site is relatively flat.

# **SUMMARY**

Based on our scope of work, determing the design load, the design load requirement for the Intellijacks is 3000 lbs, and based on the technical manual for the Intellijack system, this is acceptable as the allowable load is 24100 lbs.

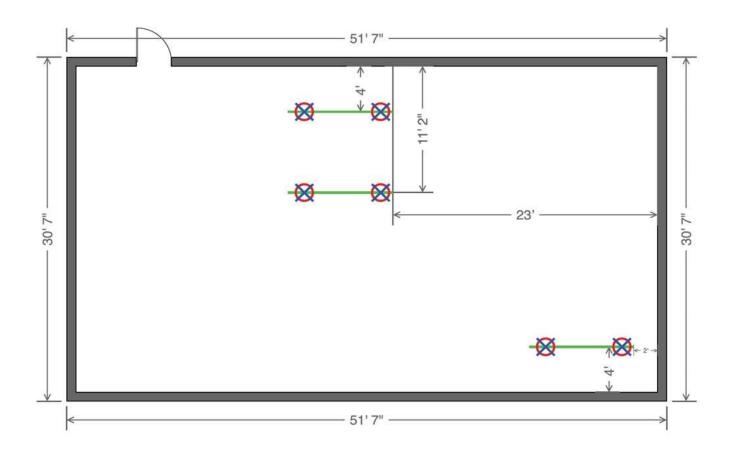
Regards,

Alicia Schumacher, P.E.

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Stark Foundations

# CONTRACTOR-PROVIDED FIELD SKETCH



**FRONT** 



Foundation Repair
100 Marion Dr
Erwin, North Carolina

Date: 7-Apr-25 Designed by: SHA

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# **Design Criteria**

Code(s): 2018 North Carolina State Building Code **ASCE 7-10** International Building Code (IBC) 2015 International Residential Code (IRC) 2015 Design Loads: Soil: Dead: Roof = 15 psf Allow Lateral Bearing Pressure = 100 psf/ft Chimney = 45 Active Pressure = 60 psf psf/ft Third Floor = psf Second Floor = psf First Floor = 15 psf Walls = 48 psf 8" Foundation Wall = 100 psf Soil = 110 psf Live: Roof (Snow) = 10 psf Roof Live Load = 20 psf governs Third Floor = 40 psf Second Floor = 40 psf First Floor = 40 psf Wind: (not applicable) Exposure = Risk Category = Ш Wind Speed, V = 119  $K_{zt} =$ 1 mph  $K_d = 0.85$ Gust Factor, G = 0.85 Int. Pressure Coefficient,  $GC_{pi} = -0.18$ 1 Ext. Pressure Coefficient, Cp = Height,  $h_z =$ 30 8.0 ft Design Wind Pressure: Design Load Combo = D + 0.6W  $p_w = q_z (GCp - GC_{pi})$ where:  $\omega = 0.6$  $q_z = 0.00256 K_z K_{zt} K_d V^2$ Therefore:  $q_z = 30.8 \text{ psf}$  $p_{w} = 26.5 \text{ psf}$ 

Factored Wind Pressure, p'<sub>w</sub> = 15.9 psf (say 16 psf)



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# Intellijack Design - Worst Case

Vertical Design Loads:

Tributary Widths:

Dead:

Roof = plf Third Floor = ft 0 plf Second Floor = ft 0 0 plf First Floor = 7.5 ft ----> 112.5 plf Walls = ft plf Foundation Wall (height) = 0 ft ----> 0 plf Soil (height) =

= 0 ft -----> 0 plf  $\Sigma DL = 112.5$  plf

Live:

Roof (live) = ft plf Third Floor = ft 0 plf Second Floor = 0 ft ----> 0 plf First Floor = 7.5 ft 300 ----> plf  $\Sigma LL =$ 300 plf

Max Pier Spacing or Trib = 5 ft

Pier Working Loads:

 $P_{DL} = 562.5$  lbs  $P_{LL} = 1500$  lbs

Working Load, P<sub>TL</sub> = 3000 lbs (ASCE 7-16 Load Combo 2 Governs)

Pier Type: Intellijack

Intellijack Column: Allowable Compression for system heights up to 9ft = 24.1 kips

Footing Check:

Allowable Bearing Pressure = 1500 psf

ok

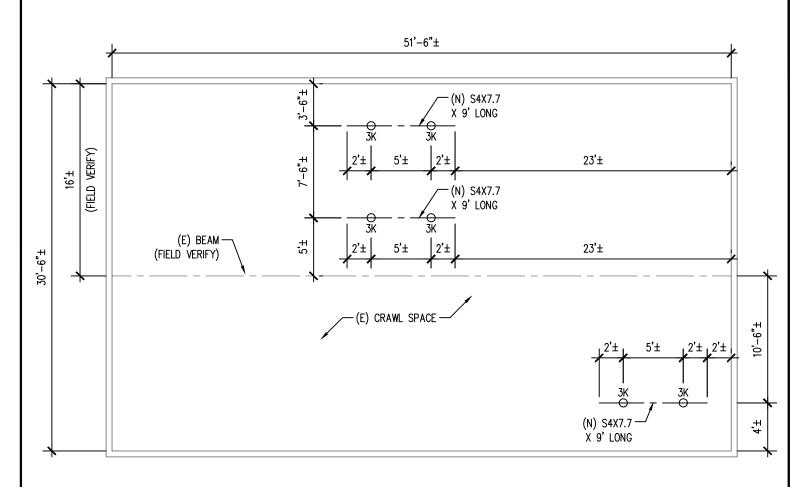
	_					
Footing Type	Length	Width	Depth	$A_{FTG}$	Soil Pressure	
Footing Type	(in)	(in)	(in)	(ft²)	(psf)	
CIP Concrete	18	18	10	2.25	1333	<1500 psf OK
Gravel	24	24	12	4	750	<1500 psf OK
Endurocrete IJ-IC	24	24	12	4	750	<1500 psf OK
-	-	-	-	-	-	

Beam: **\$4x7.7** 

Analysis of Section: Max. Span = 5 ft Allowable Dist. Load = 2070 pl

DL + LL =  $\frac{412.5}{2}$  plf ok Max. Cantilever =  $\frac{2}{2}$  ft Span / 2 =  $\frac{2.5}{1}$  ft





FRONT OF HOME

LEGEND
O INDICATES INTELLIJACK &

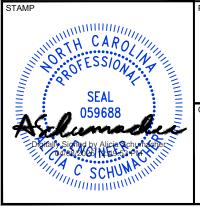
10K UNFACTORED DESIGN LOAD (KIPS)
SEE DETAIL 1/S2.0

# FOUNDATION REPAIR PLAN SCALE: 1/8"=1'-0"

### PLAN NOTES:

1. FIELD VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. NOTIFY STARK FOUNDATIONS OF ANY DISCREPANCIES.

2. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE STRUCTURAL STABILITY OF ALL NEW AND EXISTING STRUCTURES DURING CONSTRUCTION. THIS INCLUDES, BUT NOT LIMITED TO, EXCAVATIONS, COLUMNS, EQUIPMENT LOADS, MATERIAL LOADS, AND OTHERS. BRACING AND SHORING IS TO BE INSTALLED PER THE LATEST OSHA STANDARDS. THE DESIGN AND OBSERVATIONS BY STARK FOUNDATIONS DO NOT INCLUDE INSPECTIONS OF TEMPORARY LOADING AND STABILITY DURING CONSTRUCTION.



FOUNDATION REPAIR
MILBOURN RESIDENCE
100 MARION DR
ERWIN, NC

CLIENT

TAR HEEL
BASEMENT SYSTEMS
3333 AIR PARK RD
FUQUAY-VARINA, NC



FRONT ELEVATION

200						
	NO		REVISIONS	3	BY	DATE
1	ISSUED:		04.07.25	PROJ NO	: 25.090.TBR	
	DRAWN BY:		SHA	CHECKE	BY:	ACS

SHEET TITLE

FDN REPAIR PLAN

SHEET NUMBER

S1.0

