



861-E N Dean Road
Auburn, AL 36830
P: 360.566.7343

STRUCTURAL CALCULATIONS
PREPARED FOR
TARHEEL BASEMENT SYSTEMS
FOR
GAISER RESIDENCE
FOUNDATION REPAIR
584 CP STEWART RD
LILLINGTON, NORTH CAROLINA

PROJECT NUMBER: 25.068.TBR

DATE: March 26, 2025

PROJECT MANAGER: Shane Adams, P.E.

COA: C-4876





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Tarheel Basement Systems
3333 Air Park Rd
Fuquay-Varina, North Carolina 27526

RE: Foundation Repair - 584 CP Stewart Rd, Lillington, North Carolina

PROJECT BACKGROUND

We understand the structure is a single-family residence and the owner wishes to provide additional framing support. It is our understanding a S4x7.7 supplemental beam and (5) Intellijack systems have been recommended by the contractor. A recent field sketch (attached) indicates the proposed locations of repair.



Image 1: Front Elevation

GEOLOGIC SETTING

The existing structure is located in Lillington, North Carolina. Based on the information provided by the USDA Web Soil Survey, the general site condition in the area is comprised of sandy loam and the site is relatively flat.

SUMMARY

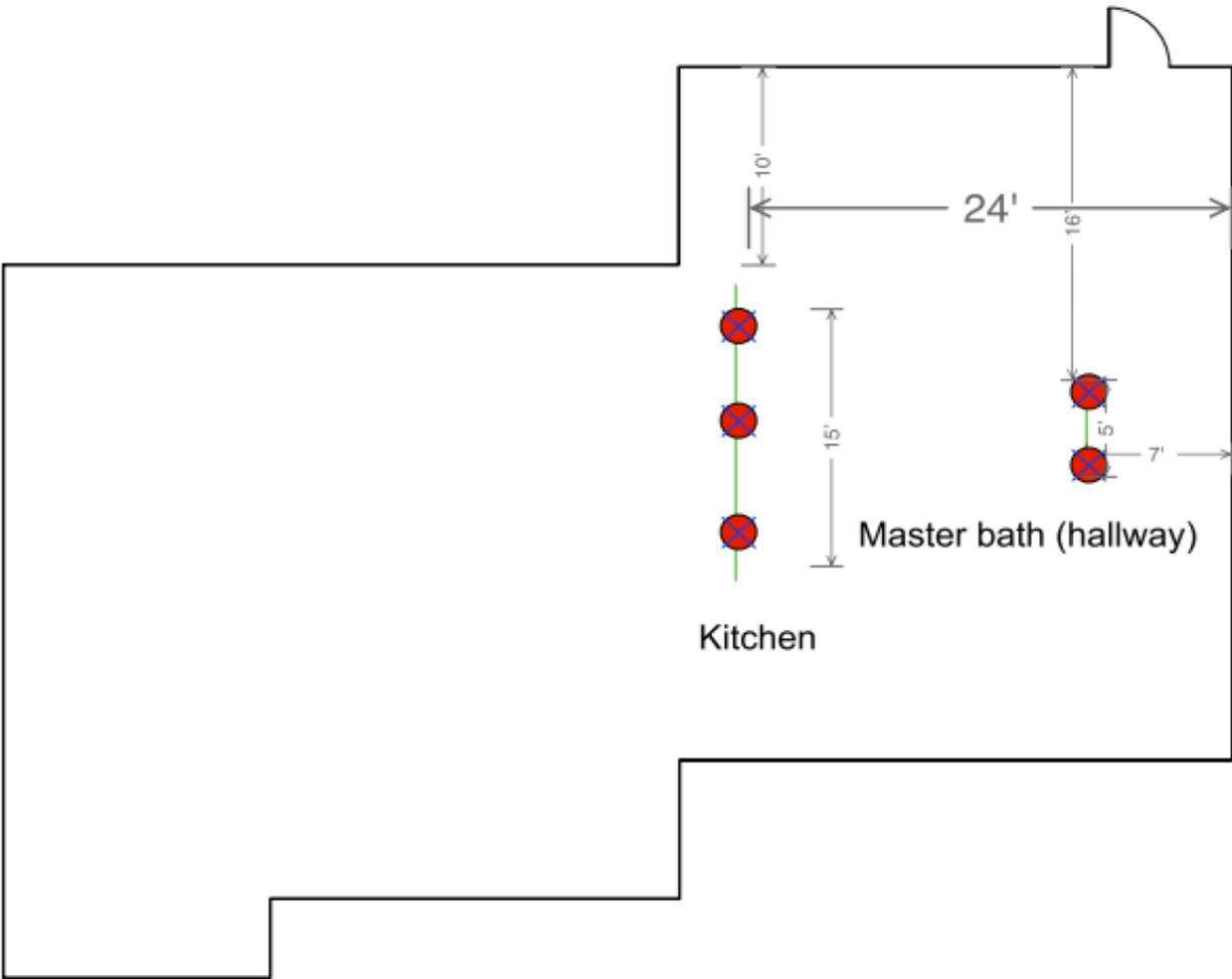
Based on our scope of work, determining the design load, the design load requirement for the Intellijacks is 3000 lbs, and based on the technical manual for the Intellijack system, this is acceptable as the allowable load is 24100 lbs.

Regards,

A handwritten signature in cursive script that reads "Shane Adams".

Shane Adams, P.E.
Stark Foundations

CONTRACTOR-PROVIDED FIELD SKETCH





PROJECT

Foundation Repair
584 CP Stewart Rd
Lillington, North Carolina

Date: 26-Mar-25
Designed by: MSY

Project No.: 25.068.TBR

Design Criteria

Code(s):

2018 North Carolina State Building Code
International Building Code (IBC) 2015
International Residential Code (IRC) 2015

ASCE 7-10

Design Loads:

Dead:

Roof = 15 psf
Chimney = 45 psf
Third Floor = 15 psf
Second Floor = 15 psf
First Floor = 15 psf
Walls = 30 psf
8" Foundation Wall = 100 psf
Soil = 110 psf

Soil:

Allow Lateral Bearing Pressure = 100 psf/ft
Active Pressure = 60 psf/ft

Live:

Roof (Snow) = 15 psf
Roof Live Load = 20 psf governs
Third Floor = 40 psf
Second Floor = 40 psf
First Floor = 40 psf

Wind: (not applicable)

Exposure = C Risk Category = II
Wind Speed, V = 117 mph $K_{zt} = 1$
Gust Factor, G = 0.85 $K_d = 0.85$
Int. Pressure Coefficient, $GC_{pi} = -0.18$ $K_z = 1$
Ext. Pressure Coefficient, $C_p = 0.8$ Height, $h_z = 30$ ft

Design Wind Pressure:

Design Load Combo = D + 0.6W

where: $p_w = q_z (GC_p - GC_{pi})$ $\omega = 0.6$
 $q_z = 0.00256 K_z K_{zt} K_d V^2$

Therefore:

$q_z = 29.8$ psf
 $p_w = 25.6$ psf
Factored Wind Pressure, $p'_w = 15.4$ psf (say 16 psf)



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Intelijack Design - Worst Case

Vertical Design Loads:

Tributary Widths:

Dead:

Roof =	0	ft	----->	0	plf
Third Floor =	0	ft	----->	0	plf
Second Floor =	0	ft	----->	0	plf
First Floor =	7	ft	----->	105	plf
Walls =	0	ft	----->	0	plf
Foundation Wall (height) =	0	ft	----->	0	plf
Soil (height) =	0	ft	----->	0	plf
				<u>ΣDL =</u>	105 plf

Live:

Roof (live) =	0	ft	----->	0	plf
Third Floor =	0	ft	----->	0	plf
Second Floor =	0	ft	----->	0	plf
First Floor =	7	ft	----->	280	plf
				<u>ΣLL =</u>	280 plf

Max Pier Spacing or Trib = 5.5 ft

Pier Working Loads:

$P_{DL} = 577.5$ lbs
 $P_{LL} = 1540$ lbs
 Working Load, $P_{TL} = 3000$ lbs (ASCE 7-16 Load Combo 2 Governs)

Pier Type: Intelijack

Intelijack Column: Allowable Compression for system heights up to 9ft = 24.1 kips

Footing Check:

Allowable Bearing Pressure = 1500 psf

Footing Type	Length (in)	Width (in)	Depth (in)	A_{FTG} (ft ²)	Soil Pressure (psf)
CIP Concrete	18	18	10	2.25	1333
Gravel	24	24	12	4	750
Endurocrete IJ-IC	24	24	12	4	750
-	-	-	-	-	-

<1500 psf OK
 <1500 psf OK
 <1500 psf OK

Beam : **S4x7.7**

Analysis of Section:

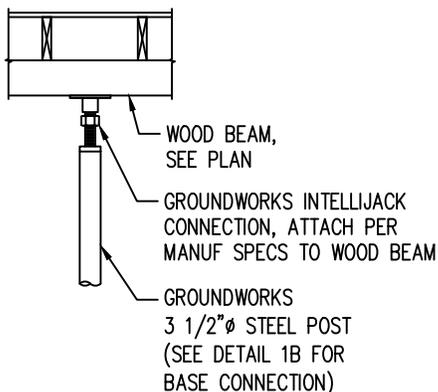
Max. Span = 6 ft Allowable Dist. Load = 1200 plf
 DL + LL = 385 plf **ok**
 Max. Cantilever = 2 ft Span / 2 = 3 ft **ok**



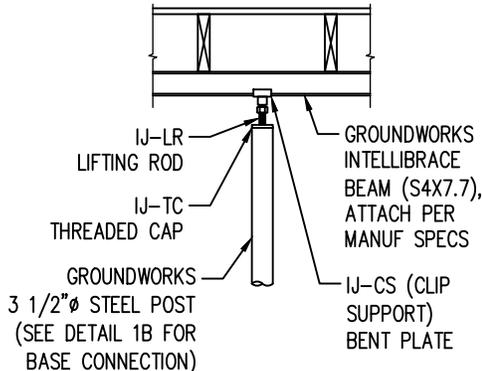
STARK FOUNDATIONS
 861-E N DEAN RD
 AUBURN, AL 36830
 P: 360.566.7343
 www.starkfoundations.com

NOTES:

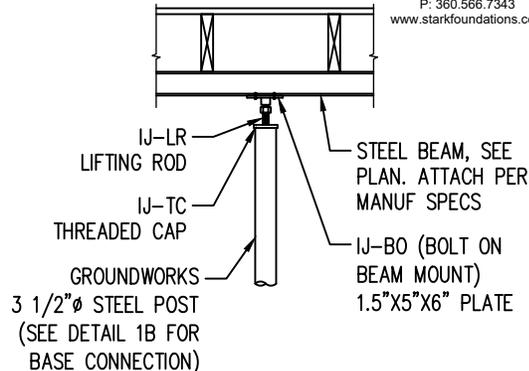
- HOUSE TO REMAIN UNOCCUPIED DURING THE REPLACEMENT OF EXISTING POSTS W/ NEW INTELLIJACK POSTS.
- SUPPLEMENTAL/REPLACEMENT WOOD TO BE SOUTHERN PINE/DOUGLAS FIR, NO.2.
- CAPACITY ASSUMES AN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF.
- CONCRETE COMPRESSIVE STRENGTH, $F_c' = 2,500$ PSI.
- BOTTOM OF FOOTING SHALL NOT BE LESS THAN THE FROST DEPTH.



1A.A: FLAT PLATE W/ WOOD BEAM



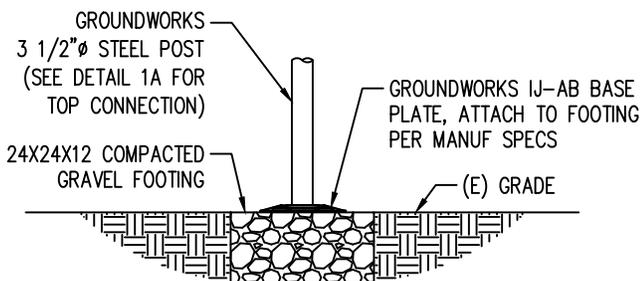
1A.B: BENT PLATE W/ STEEL BEAM



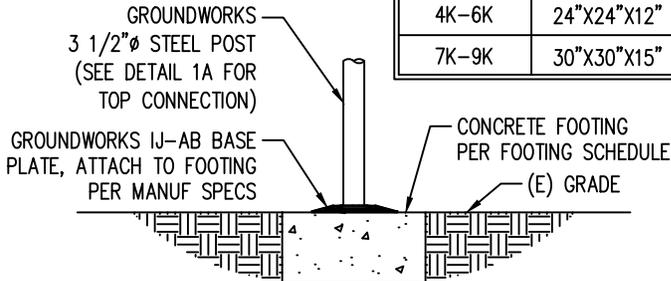
1A.C: FLAT PLATE W/ STEEL BEAM

1A: INTELLIJACK TOP CONNECTIONS

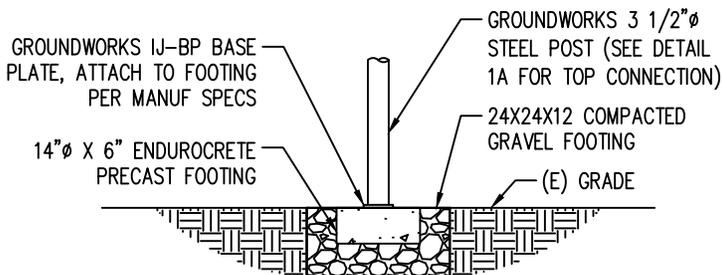
CONCRETE FOOTING SCHEDULE	
DESIGN LOAD	SIZE
1K-3K	18"X18"X10" THK
4K-6K	24"X24"X12" THK
7K-9K	30"X30"X15" THK



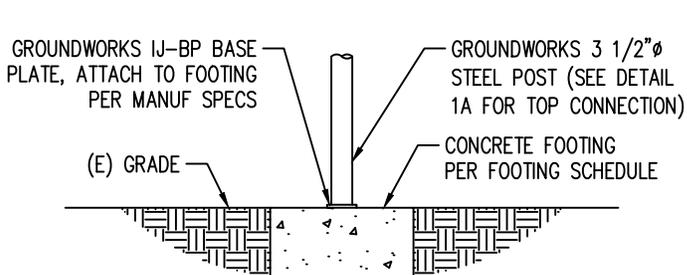
1B.A: BASE PLATE W/ GRAVEL FOOTING



1B.B: IJ-AB BASE PLATE W/ CONCRETE FOOTING



1B.C: ENDURO WHEEL W/ GRAVEL FOOTING



1B.D: IJ-BP BASE PLATE W/ CONCRETE FOOTING

1B: INTELLIJACK BASE CONNECTIONS

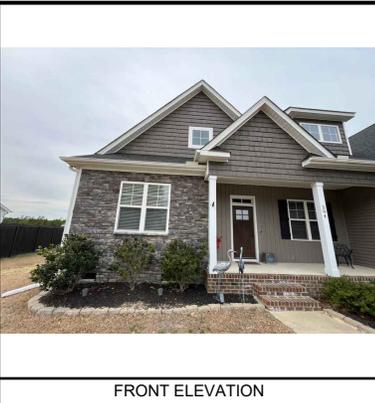
1 INTELLIJACK DETAIL
 SCALE: 3/8"=1'-0"

STAMP

Stane C. Adams
 Digitally signed by Stane Adams
 02/26/2023 2:43:28 PM

PROJECT
 FOUNDATION REPAIR
 GAISER RESIDENCE
 584 CP STEWART RD
 LILLINGTON, NC

CLIENT
 TAR HEEL
 BASEMENT SYSTEMS
 3333 AIR PARK RD
 FUQUAY-VARINA, NC



NO	REVISIONS	BY	DATE
ISSUED:	03.24.25	PROJ NO.:	25.068.TBR
DRAWN BY:	MSY	CHECKED BY:	ACS
SHEET TITLE			
DETAILS			
SHEET NUMBER			
S2.0			