

ENCLOSED GABLE END BUILDING

MAXIMUM 30'-0" WIDE X 16'-0" EAVE HEIGHT WITH BOX FRAME / (UP TO) 145 M.P.H. WIND ZONE / 30 P.S.F. SNOW LOAD (UP TO) SEISMIC CATEGORY D/D2 (NOTE: NOT APPLICABLE TO PARTIALLY OPEN / ENCLOSED OR OPEN BUILDINGS)

FOR:

N.C. CARPORTS & GARAGES
116 EAST MARKET STREET
ELKIN, NORTH CAROLINA 28621

ISSUE DATE: 01.23.24



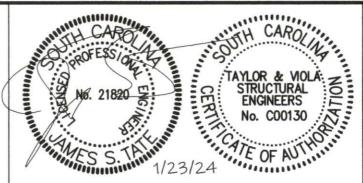




NOTE: THESE DRAWINGS ARE VALID FOR (1) CALENDAR YEAR AFTER THE ISSUE DATE LISTED ON THIS SHEET.



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 NC BUILDING CODE (2015 IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2021 SC BUILDING CODE (2021IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 VA CONSTRUCTION CODE (2018 IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2012 IBC

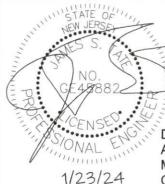


DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 IBC



1/23/24

AND DRAWN TO MEET THE 2020 BUILDING CODE OF NEW YORK STATE (2018 IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 20210 BUILDING **CODENEW JERSEY EDITION** (2021 IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 IBC



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CARPORTS NC GARAGES

116 EAST MARKET STREET ELKIN, NORTH CAROLINA 28621 TELE: 336-368-0668

PROJECT NO: 23-MISC
SHEET NO:



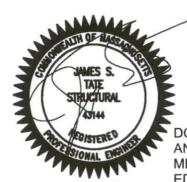
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DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2021 BUILDING CODE OF ALABAMA (2021 IBC)



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 IBC



1/23/24

DOCUMENTS DESIGNED AND DRAWN TO MEET THE NINETH EDITION OF THE MA STATE BUILIDING CODE



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 IBC



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2017 IBC



DOCUMENTS DESIGNED AND DRAWN TO MEET THE 2018 KENTUCKY BUILDING CODE (2015 IBC)

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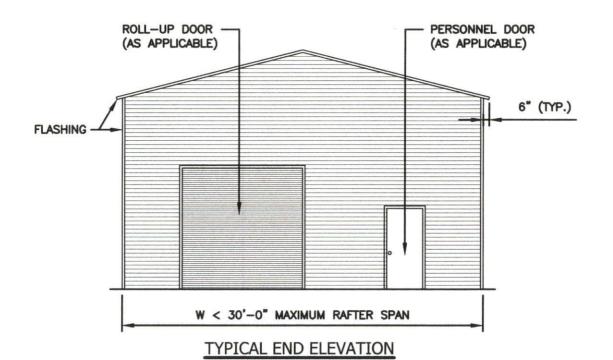
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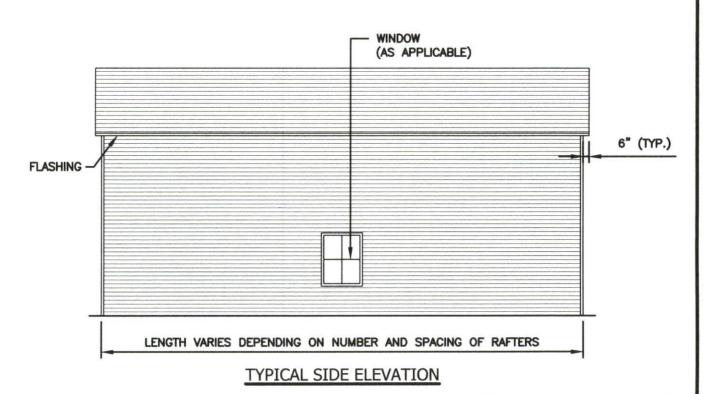
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	P.E. SEALS SHEET
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	& GARAGES	BKS	23-MISC
JCMT Associates, PLLC 211 Stone Drive, Pilot Mountain, NC 27041 Telephone: (336) 399-6277	116 EAST MARKET STREET ELKIN, NORTH CAROLINA 28621 TELE: 336–368–0668	DATE: 01.23.24	SHEET NO: S2

DESIGN LOADS:					
IMPORTANCE FACTORS WIND SNOW SEISM	(1s)	1.00			
DEAD LOADS ROOF ROOF	COLLATERAL	13 P.S.F. 0 P.S.F.			
LIVE LOADS ROOF		20 P.S.F.			
GROUND SNOW LOAD:		30 P.S.F.	DRIFT LOAD HAS NOT BE	EN CALCULATED	
	-	/ 145 M.P.H.	(ASCE 7-16)		
SEISMIC DESIGN CATAGORY A					
COMPLIANCE WITH SECTION 1616.49	[X NO	ÆS		_
SEISMIC DESIGN CATEGORY D/	D2				
PROVIDE THE FOLLOWING SEISMIC (ESIGN PARAMETER	S:			
OCCUPANCY CATEGORY					
SPECTRAL RESPONSE ACCELERATIO	N Ss 132.4	(MAX.) %g	S1 38.9 (MAX.) %g		
SITE CLASSIFICATION X	. FIELD TEST	X PRESUMP	TIVE . HISTORICAL DAT	r A	
BASIC STRUCTURAL SYSTEM (CHEC	K ONE)				
BEARING WALL BUILDING FRAME MOMENT FRAME	DUAL W/ SP DUAL W/ IN' INVERTED PE	termediate R/	IT FRAME C OR SPECIAL STEEL		
ANALYSIS PROCEDURE SIM	IPLIFIED X	EQUIVALANT LA	TERAL FORCE MC	DDAL	
LATERAL DESIGN CONTROL? X	_ EARTHQUAKE (U	P TO 145 M.P.I	H.)X WIND (UP TO	0 145 M.P.H.)	
SOIL BEARING CAPACITIES: PRESUMPTIVE BEARING CAPACITIES:		2,000 P.S.F.			
GENERAL NOTES:					
1. MAX FRAME SPACING SHALL E	E 60"oc UNLESS N	IOTED OTHERWIS	SE.		
2. MAX. END-WALL COLUMN SPA	CING SHALL BE 60	oc UNLESS NO	TED OTHERWISE.		
3. TUBE MATERIAL SHALL BE 2-	1/2" x 2-1/2" x 1	4 GA. 50 KSI	MIN. UNLESS NOTED OTHER	RWISE.	
4. ALL FASTENERS SHALL BE (2)	#12 SELF TAPPING	G AT 9"o.c. UN	LESS NOTED OTHERWISE.		
5. 2,000 PSF ASSUMED BEARING	CAPACITY UNLESS	NOTED OTHER	MSE.		
6. THESE DRAWINGS ARE NOT AF	PLICABLE TO PART	TALLY OPEN /	ENCLOSED OR OPEN BUILD	INGS.	
TAYLOR & VIOLA STRUCTURAL ENGINEERS P.O.B. 2616 HICKORY NORTH CAROLINA TELE: 828-328-6331 FAX: 828-322-1801 WWW TAYLORVIOLA . COM	N C &	2000 000	P O R T S A G E S	DRAWN BY: BKS	PROJECT NO: 23-MISC
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BOX EVE FRAME RAFTER STURCTURE







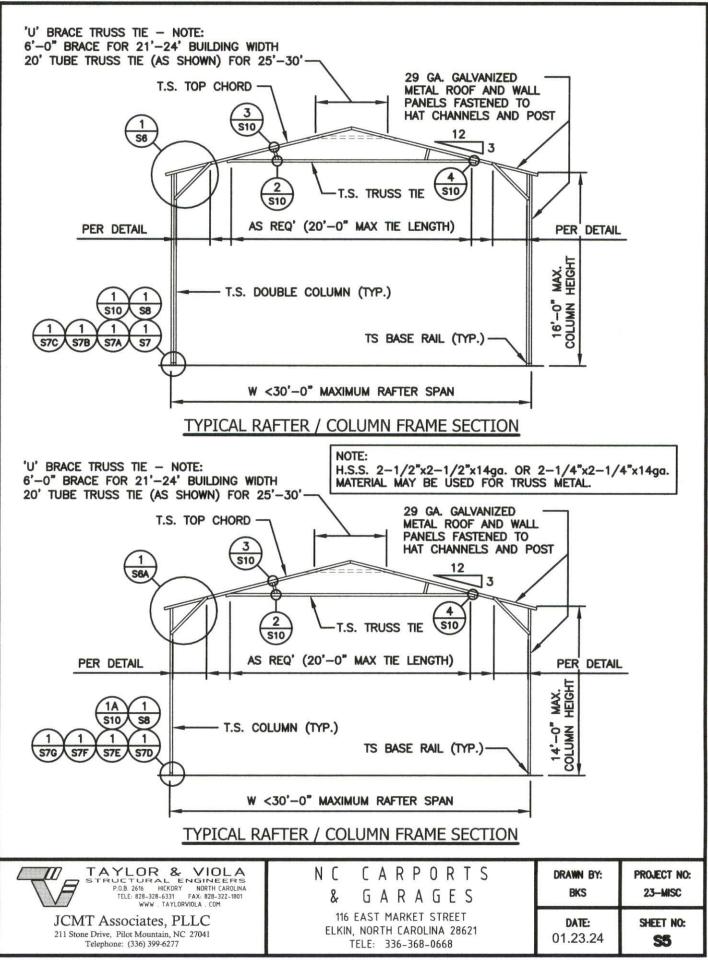
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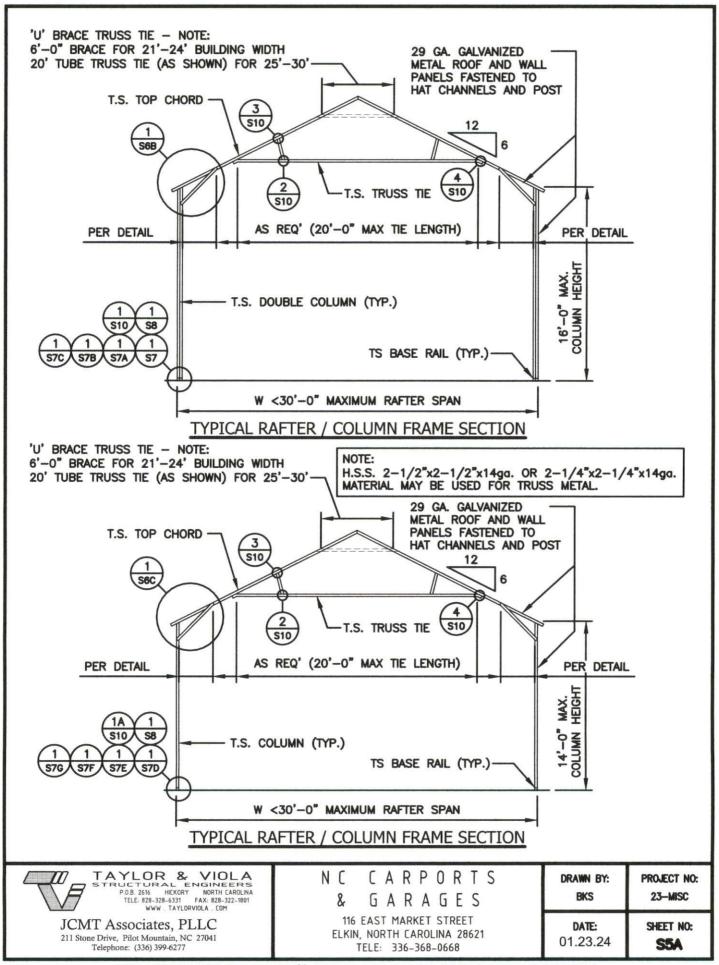
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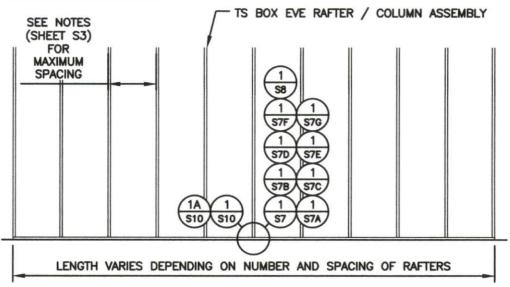
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NOTE: H.S.S. 2-1/2"x2-1/2"x14ga. OR 2-1/4"x2-1/4"x14ga. MATERIAL MAY BE USED FOR TRUSS METAL.



TYPICAL RAFTER / COLUMN SIDE FRAME SECTION

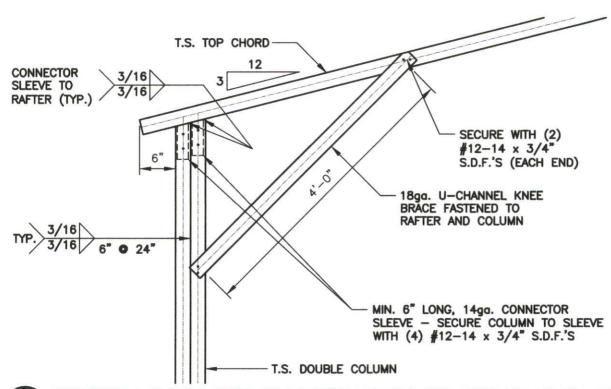


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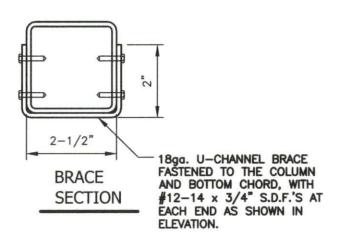
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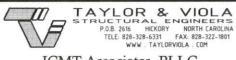
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BOX EAVE / CORNER POST CONNECTION DETAIL FOR HEIGHTS 14'-1" < TO < 16'-0"

S6 | SCALE: 3/4" = 1'-0"





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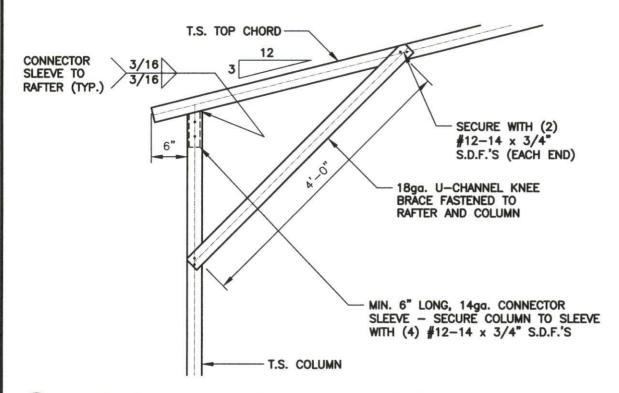
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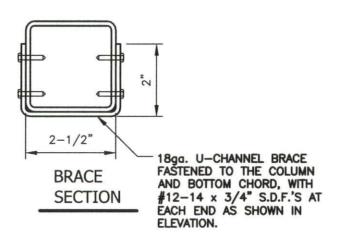
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S6

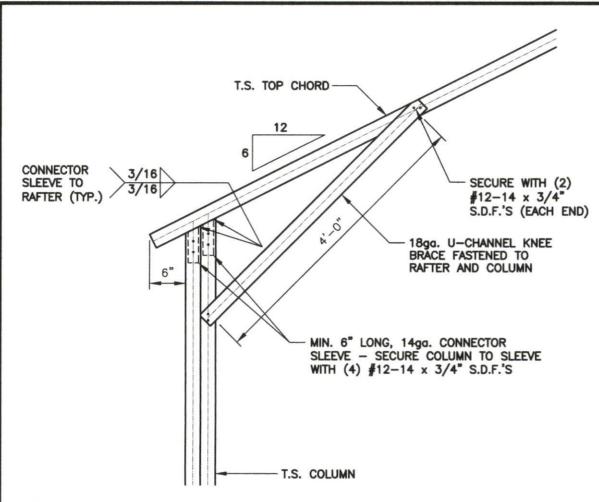


BOX EAVE / CORNER POST CONNECTION DETAIL FOR HEIGHTS < 14'-0"

S6A SCALE: 3/4" = 1'-0"



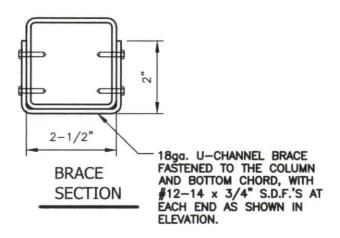




BOX EAVE / CORNER POST CONNECTION DETAIL FOR HEIGHTS 14'-1" < TO <16'-0"

S6C

SCALE: 3/4" = 1'-0"





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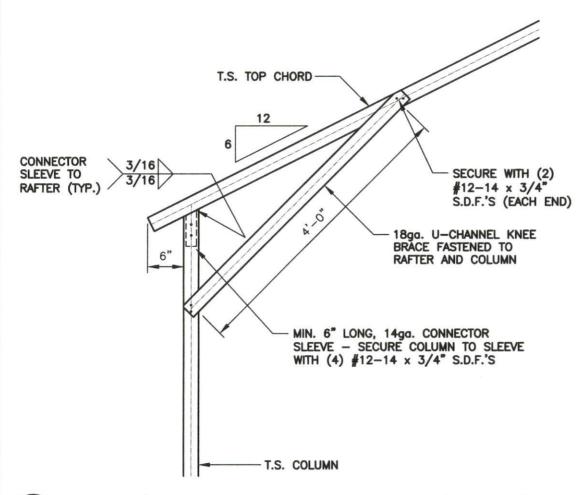
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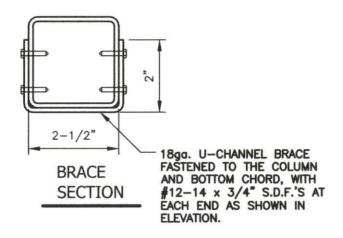
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BOX EAVE / CORNER POST CONNECTION DETAIL FOR HEIGHTS < 14'-0"

S6C

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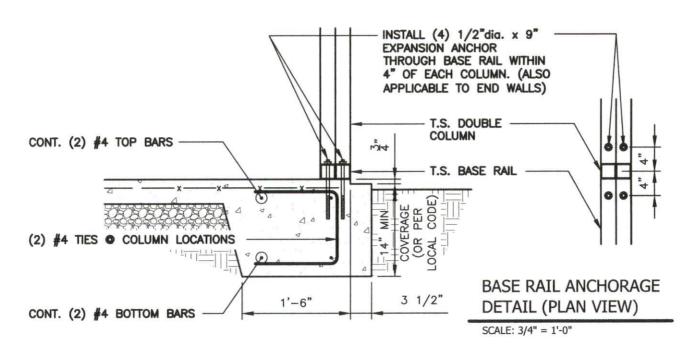
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DATE: 01.23.24

SHEET NO: S6C





BASE RAIL ANCHORAGE DETAIL

SCALE: 3/4" = 1'-0"

GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 PSF

CONCRETE:

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS

COVER OVER REINFORCING STEEL:

FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-318:

3" IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH AND WEATHER AND 1-1/2" ELSEWHERE.

REINFORCING STEEL:

THE TURNDOWN REINFORCING STEEL SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT.

REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

REINFORCEMENT IS BENT COLD.

THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.

3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.



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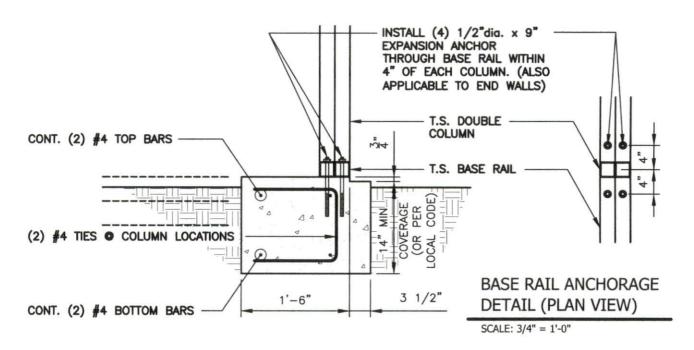
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PROJECT NO: DRAWN BY: 23-MISC BKS DATE: SHEET NO: 01.23.24

S7





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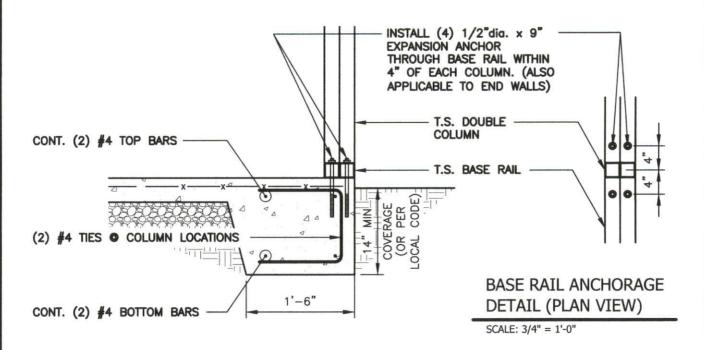
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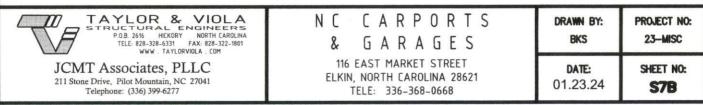
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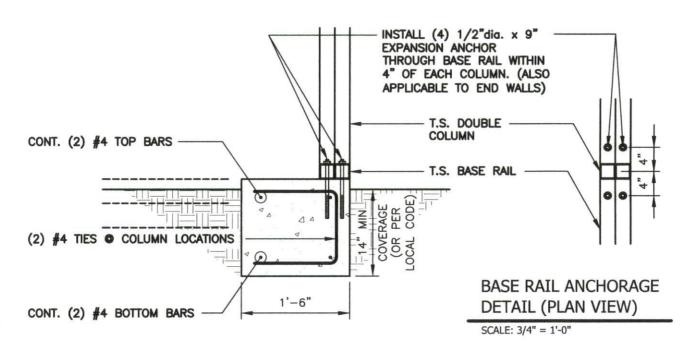
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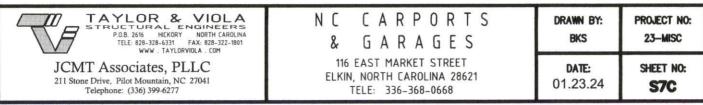
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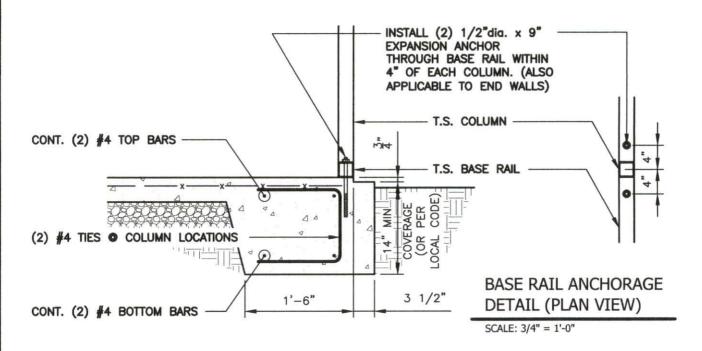
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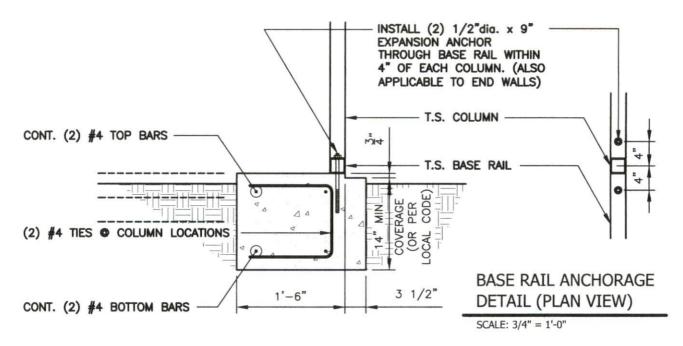
N C C A R P O R T S

& G A R A G E S

116 EAST MARKET STREET
ELKIN, NORTH CAROLINA 28621
TELE: 336-368-0668

DRAWN BY: PROJECT NO: 23-MISC

DATE: SHEET NO: 01.23.24 \$7D





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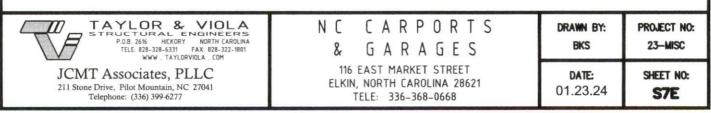
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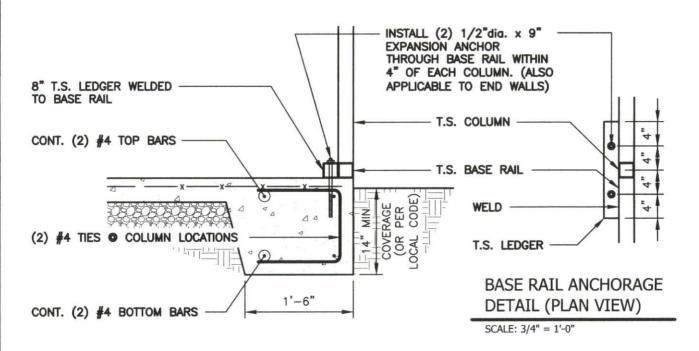
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- THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.
- 3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.







BASE RAIL ANCHORAGE DETAIL

SCALE: 3/4" = 1'-0"

GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 PSF

CONCRETE:

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS

COVER OVER REINFORCING STEEL:

FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-318:

3" IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH AND WEATHER AND 1-1/2" ELSEWHERE.

REINFORCING STEEL:

THE TURNDOWN REINFORCING STEEL SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT.

REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

1. REINFORCEMENT IS BENT COLD.

THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.

NC

3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.



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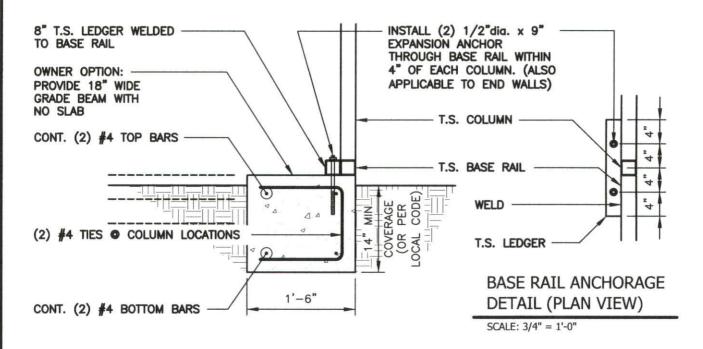
& GARAGES

116 EAST MARKET STREET
ELKIN, NORTH CAROLINA 28621
TELE: 336-368-0668

CARPORTS

DRAWN BY: PROJECT NO: 23-MISC

DATE: SHEET NO: 57F





BASE RAIL ANCHORAGE DETAIL

SCALE: 3/4" = 1'-0"

GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 PSF

CONCRETE:

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS

COVER OVER REINFORCING STEEL:

FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-318:

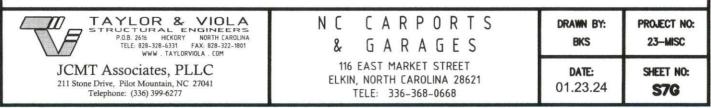
3" IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH AND WEATHER AND 1-1/2" ELSEWHERE.

REINFORCING STEEL:

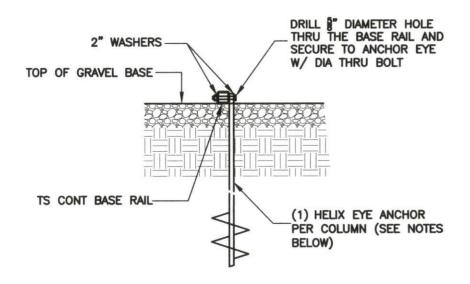
THE TURNDOWN REINFORCING STEEL SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT.

REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

- REINFORCEMENT IS BENT COLD.
- THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.
- REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.



SOIL NAIL BASE RAIL ANCHORAGE



SCALE: 3/4" = 1'-0"

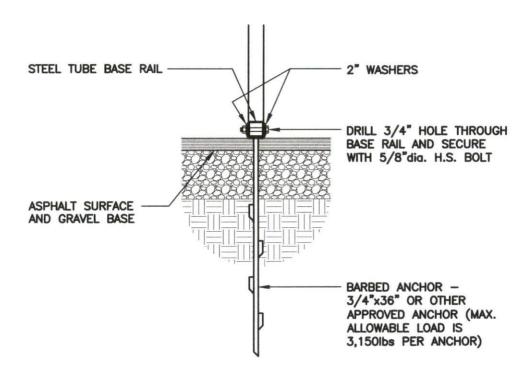
HELIX EMBEDMENT NOTE:

CONSULT GEOTECHNICAL ENGINEER FOR SOILS INFORMATION PRIOR TO ANCHORING BUILDING WITH THIS METHOD.

BASE RAIL ANCHORAGE DETAIL



ASPHALT ANCHOR BASE RAIL ANCHORAGE



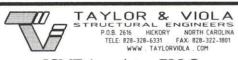


ASPHALT / BASE RAIL ANCHORAGE DETAIL

SCALE: 3/4" = 1'-0"

EMBEDMENT NOTE:

CONSULT GEOTECHNICAL ENGINEER FOR SOILS INFORMATION PRIOR TO ANCHORING BUILDING WITH THIS METHOD.



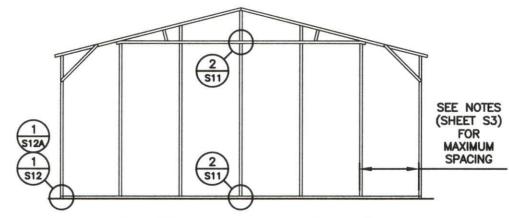
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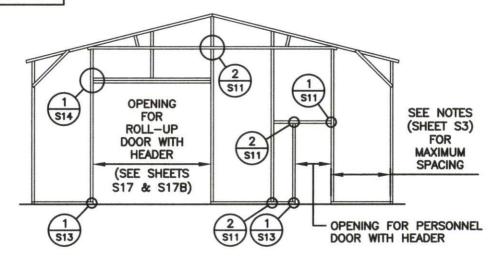
DATE: SHEET NO: 01.23.24 SBA

BOX EVE RAFTER END WALL OPENINGS



TYPICAL BOX EVE RAFTER / END WALL COLUMN FRAME SECTION

NOTE:
ROLL UP DOORS SHALL
HAVE ONE JAMB EACH
SIDE + ONE FRAME
COLUMN BETWEEN OR SITE
SPECIFIC SHALL BE REQ'D.



TYPICAL BOX EVE RAFTER END WALL OPENINGS FRAMING SECTION



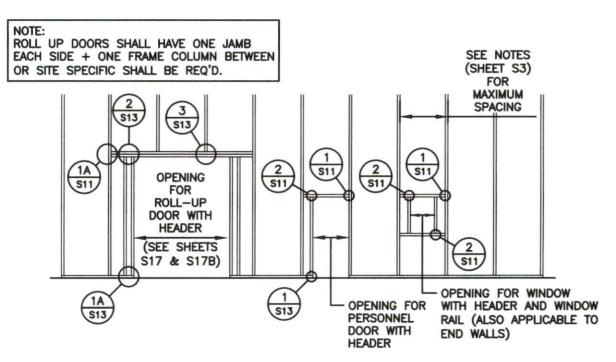
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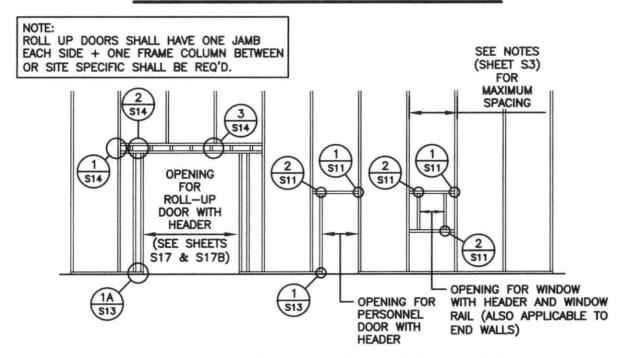
DATE: 01.23.24

SHEET NO:

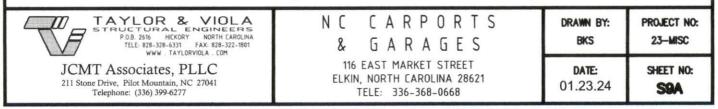
BOX EVE RAFTER END WALL AND SIDE WALL OPENINGS

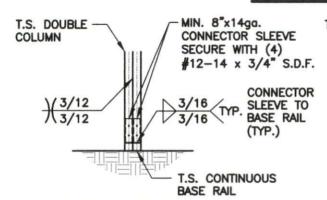


TYPICAL END WALL OPENING FRAMING SECTION

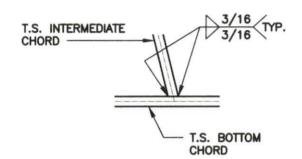


TYPICAL SIDE WALL OPENING FRAMING SECTION





POST / BASE RAIL
CONNECTION DETAIL
SCALE: 1/2" = 1'-0"



INTERMEDIATE CHORD TO BOTTOM CHORD CONNECTION DETAIL

SCALE: 1/2" = 1'-0"

T.S. COLUMN

MIN. 8"x14ga.

CONNECTOR SLEEVE

SECURE WITH (4)

#12-14 x 3/4" S.D.F.

CONNECTOR

SLEEVE TO

BASE RAIL

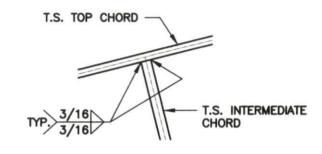
(TYP.)

T.S. CONTINUOUS

BASE RAIL

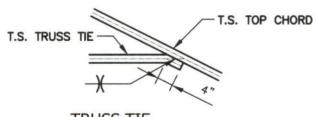
POST / BASE RAIL
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



INTERMEDIATE CHORD TO TOP CHORD CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



TRUSS TIE
CONNECTION DETAIL
SCALE: 1/2" = 1'-0"



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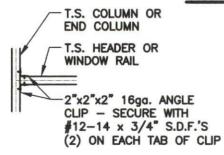
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DATE: SHEET NO: \$1.23.24

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T.S. COLUMN OR END COLUMN

T.S. DOUBLE HEADER

(3/12)
(3/12)
(3/12)

2"x2"x2" 16ga. ANGLE
CLIP - SECURE WITH
#12-14 x 3/4" S.D.F.'S
(2) ON EACH TAB OF CLIP

COLUMN / DOUBLE HEADER

CONNECTION DETAIL

COLUMN OR WINDOW RAIL / WALL GIRT TO POST CONNECTION DETAIL

1A S11

SCALE: 1/2" = 1'-0"

SCALE: 1/2" = 1'-0"

T.S. END POST OR CHORD OR HEADER

T.S. TRUSS RAFTER CHORD OR HEADER

T.S. TRUSS RAFTER CHORD OR HEADER

2"x2"x2" 16ga. ANGLE CLIP — SECURE WITH #12—14 x 3/4" S.D.F.'S

RAIL, OR (2) ON EACH TAB OF CLIP

DOOR OR WINDOW HEADER RAIL TO POST CONNECTION DETAIL

2 S11

WINDOW RAIL

SCALE: 1/2" = 1'-0"

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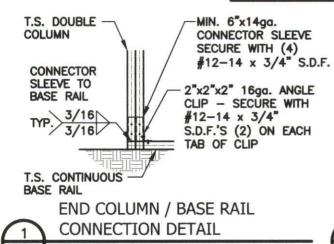
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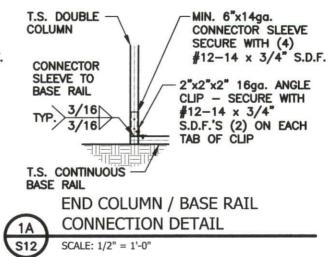
DATE: SHEET NO:

S11

01.23.24

NOTE: THESE DRAWINGS ARE VALID FOR (1) CALENDER YEAR AFTER THE ISSUE DATE LISTED ON THIS SHEET.





S12

SCALE: 1/2" = 1'-0"

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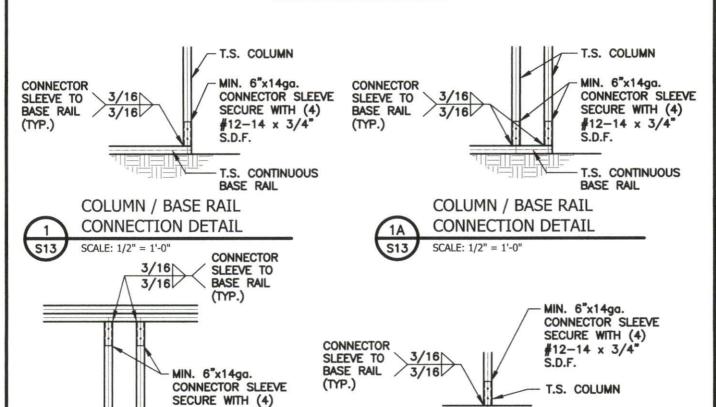
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 BKS
 23-MISC

 DATE:
 SHEET NO:

 01.23.24
 \$12



DOUBLE HEADER / COLUMN
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"

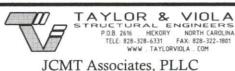
#12-14 x 3/4

- T.S. COLUMN

S.D.F.

COLUMN / DOUBLE HEADER
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



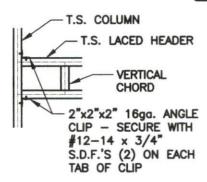
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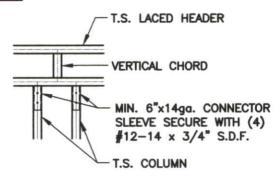
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DATE: SHEET NO: 91.23.24

S13

T.S. DOUBLE HEADER



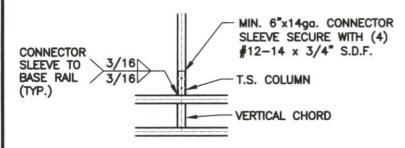


COLUMN / LACED HEADER
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"

LACED HEADER / COLUMN
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



COLUMN / LACED HEADER
CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



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	ELKIN	, NO	RTH	CA	ROL	INA	28	3621	

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DATE: SHEET NO: 01.23.24 \$14

TYPICAL LEAN-TO OPTIONS FRAMING SECTION (BOTH OPTIONS SHOWN)

<30'-0" MAXIMUM RAFTER SPAN

MAXIMUM WIDTH OF SINGLE MEMBER ROOF EXTENSION AND LEAN-TO OPTION IS 12'-0" MAXIMUM WIDTH OF DOUBLE MEMBER ROOF EXTENSION AND LEAN-TO IS 15'-0" 12'-0" MAX. LEAN-TO HEIGHT WITH SINGLE COLUMN 16'-0" MAX. LEAN-TO HEIGHT WITH DOUBLE COLUMN

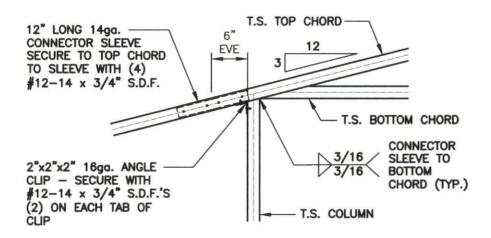


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DATE: 01.23.24	SHEET NO:

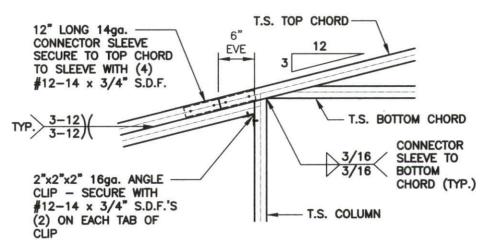
LEAN-TO CONNECTION DETAILS



ROOF EXTENSION RAFTER / CORNER POST DETAIL FOR WIDTHS < 12'-0"

S15A

SCALE: 3/4" = 1'-0"



ROOF EXTENSION RAFTER / CORNER POST DETAIL FOR WIDTHS < 15'-0"

(1A S15A)

SCALE: 3/4" = 1'-0"



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N	C	C	Α	R	Ρ	0	R	T	S
	&	G	Α	R	Α	G	Ε	S	
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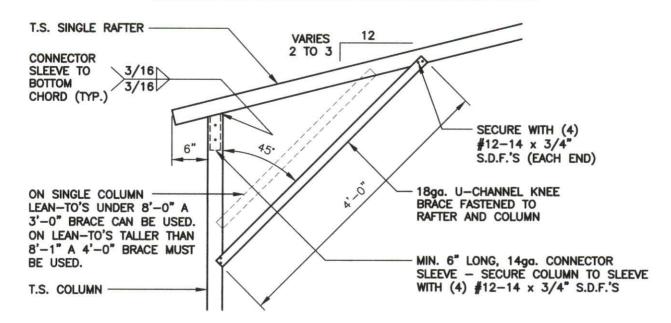
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DRAWN BY:	PROJECT NO:
BKS	23-MISC
DATE:	SHEET NO:

S15A

01.23.24

LEAN-TO CONNECTION DETAILS - SINGLE COLUMN



LEAN-TO SINGLE RAFTER / SINGLE COLUMN CONNECTION DETAIL

S15B SCALE: 3/4" = 1'-0"

T.S. DOUBLE RAFTER **VARIES** 2 TO 3 CONNECTOR SLEEVE TO 3/16 **BOTTOM** CHORD (TYP.) SECURE WITH (4) 6" #12-14 x 3/4" S.D.F.'S (EACH END) 18ga. U-CHANNEL KNEE BRACE FASTENED TO ON SINGLE COLUMN LEAN-TO'S UNDER 8'-0" A RAFTER AND COLUMN 3'-0" BRACE CAN BE USED. ON LEAN-TO'S TALLER THAN 8'-1" A 4'-0" BRACE MUST MIN. 6" LONG, 14ga. CONNECTOR SLEEVE — SECURE COLUMN TO SLEEVE BE USED. WITH (4) #12-14 x 3/4" S.D.F.'S

LEAN-TO DOUBLE RAFTER / SINGLE COLUMN CONNECTION DETAIL

NC



T.S. COLUMN -

SCALE: 3/4" = 1'-0"

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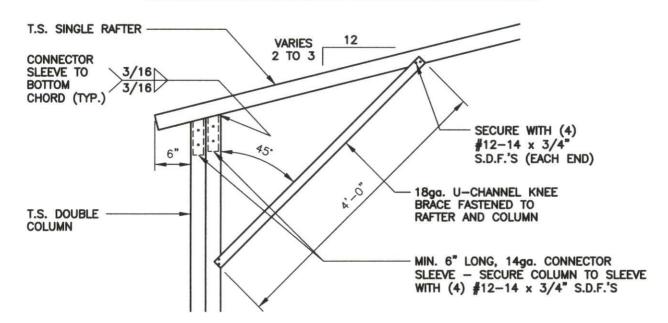
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DATE: SHEET NO: 01.23.24 S158

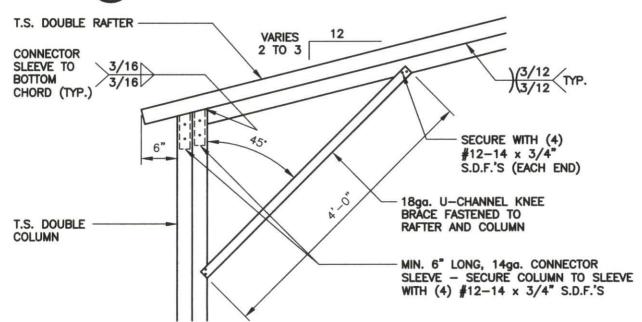
LEAN-TO CONNECTION DETAILS - DOUBLE COLUMN



LEAN-TO SINGLE RAFTER / DOUBLE COLUMN CONNECTION DETAIL

1 S15C

SCALE: 3/4" = 1'-0"



LEAN-TO DOUBLE RAFTER / DOUBLE COLUMN CONNECTION DETAIL



SCALE: 3/4" = 1'-0"

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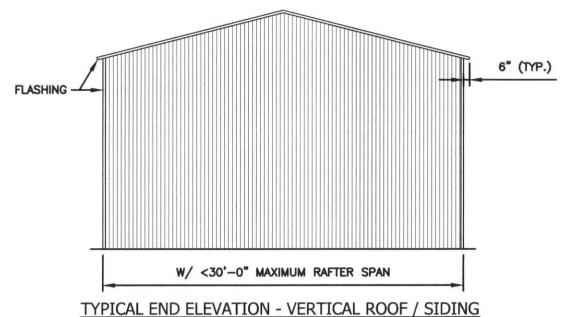
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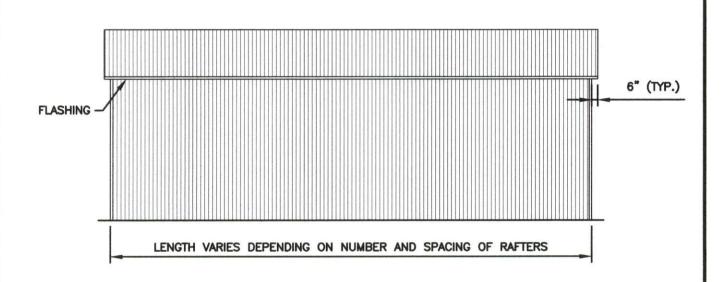
DATE: SHEET NO:

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SHEET NO: S15C

VERTICAL ROOF / SIDING OPTION





TYPICAL SIDE ELEVATION - VERTICAL ROOF / SIDING



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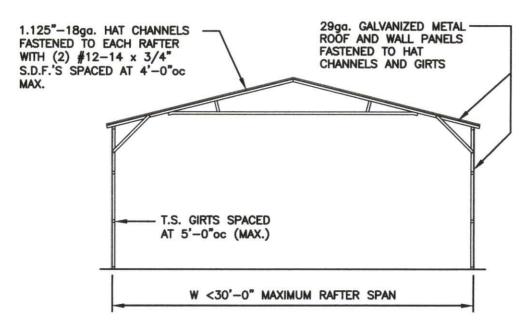
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SHEET NO:

VERTICAL ROOF / SIDING OPTION



TYPICAL SECTION VERTICAL ROOF / SIDING OPTION

NOTE:

H.S.S. 2-1/2"x2-1/2"x14ga. OR 2-1/4"x2-1/4"x14ga. MATERIAL MAY BE USED FOR TRUSS METAL.



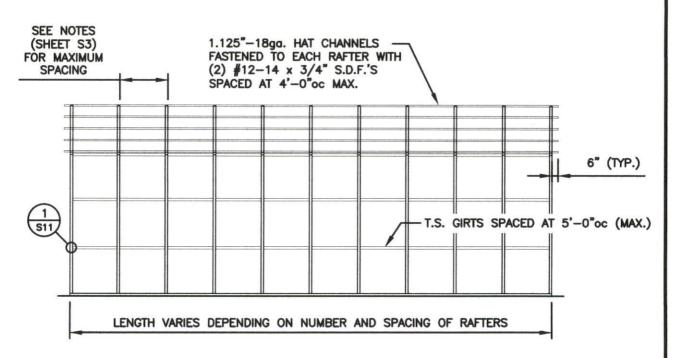
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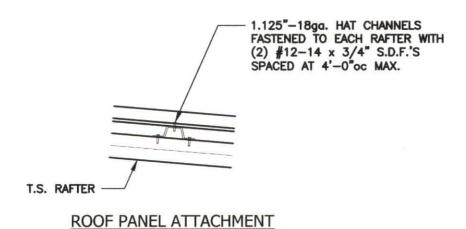
DRAWN BY: PROJECT NO: 23-MISC BKS

DATE: SHEET NO: 01.23.24 **S16A**

VERTICAL ROOF / SIDING OPTION

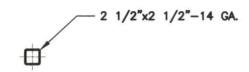


TYPICAL SIDE FRAMING SECTION VERTICAL ROOF / SIDING OPTION

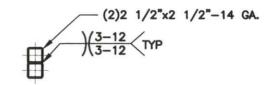




SIDE WALL HEADER OPTIONS



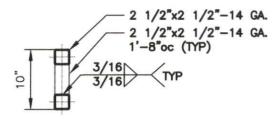
HEADER DETAIL FOR DOOR OPENINGS < 4'-0" NTS



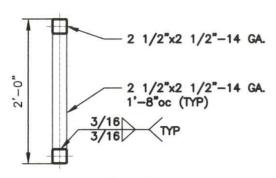
HEADER DETAIL FOR DOOR

OPENINGS 4'-1" < TO < 12'-0"

NTS



LACED HEADER DETAIL FOR DOOR OPENINGS 12'-1" < TO < 15'-0" NTS



LACED HEADER DETAIL FOR DOOR OPENINGS 15'-1" < TO < 20'-0"



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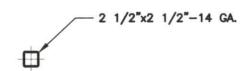
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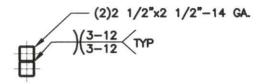
DATE: 01.23.24

SHEET NO:

END WALL HEADER OPTIONS

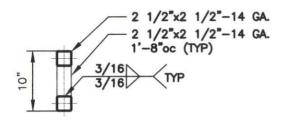


HEADER DETAIL FOR DOOR OPENINGS < 7'-0" NTS



HEADER DETAIL FOR DOOR

OPENINGS 7'-1" < TO < 16'-0"
NTS



LACED HEADER DETAIL FOR DOOR OPENINGS 16'-1" < TO < 20'-0" NTS

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