

# **EAGLE METAL**

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Toza Truss**, **LLC**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: Baxter Addition 24 - 1218340 T1G, T1

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (NC, 31344) My license expiration date for the state of NC is 12/31/2025.

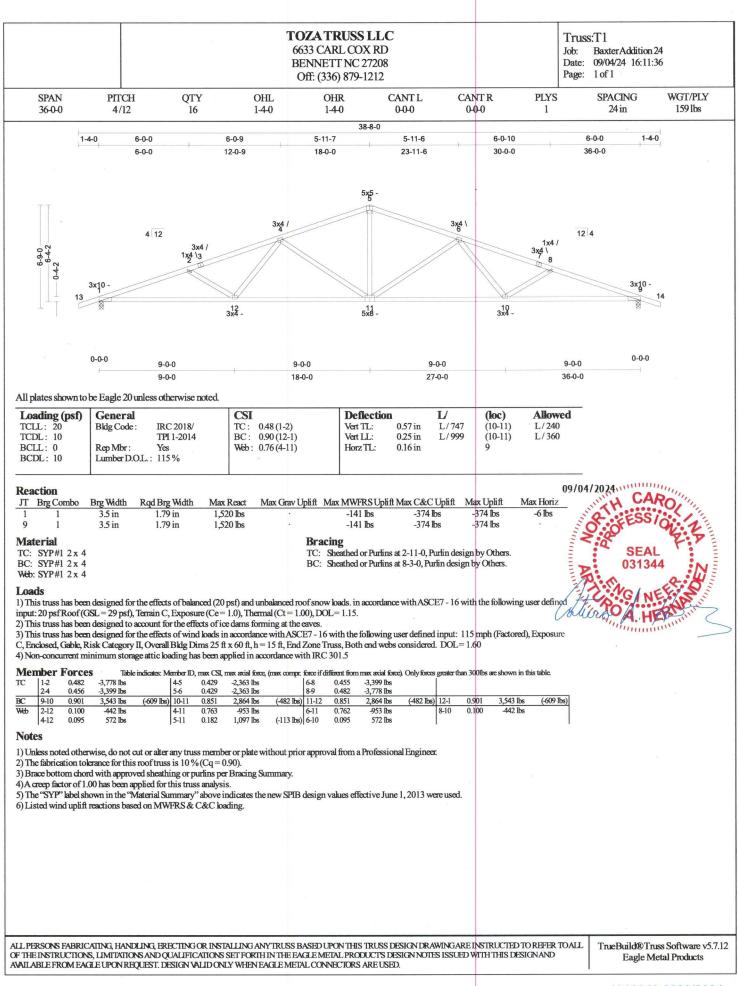
IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Empowering great component manufacturers.

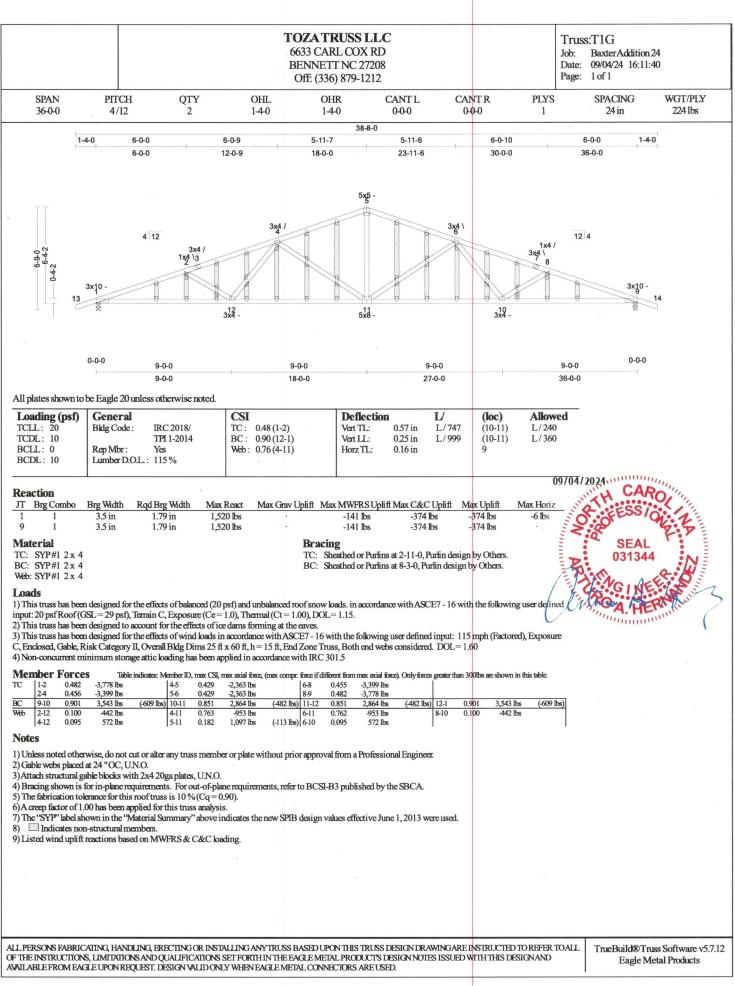
h.	EAGLE METAL		ANNING: Failure to follow m	$\widehat{\Lambda}$ WARNING: Failure to follow may result in property damage or personal injury.	
	<b>DESIGN NOTES</b>			SYMBOLS	
1.	The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1. Copies of each Truss Design Drawing shall be furnished to the	11. B 12. S Vi ir	Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.	<b>PLATE SIZE</b> <b>3X4</b> - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.	
	installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses. DESIGN LIMITATIONS	<b>13.</b> N <b>14.</b> O	Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI. Concentration of construction loads greater than the design loads	-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.	
3	The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy	12. × ×	shall not be applied to the trusses at any time; refer to BCSI. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection. MATERIALS & FABRICATION	20 Ga Gr40 connectors required <b>3X10-20HS</b> - 20 Ga Gr60 connectors required <b>8X10-18HS</b> - 18 Ga Gr60 connectors required	
4.	of specifications provided by the Building Designer. The Building Designer is solely responsible for the suitability	<b>16.</b>	Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.	LATERAL BRACING	
	based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.		Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.	When this symbol shown, continuous lateral bracing is	
3.	Each Truss Design Drawing is for the individual building	18. U	Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.	BEARING	
	indicates acceptance of professional engineering responsibility solely for the individual truss.	19. 9	Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.	Indicates location where bearings (supports) occur.	
6.	Each Truss Design Drawing assumes trusses will be suitably protected from the environment.	20. F	For a specified plate gauge and grade, the specified size is a minimum.	The plate shall be centered on joint and/or placed in accordance	
2	HANDLING, INSTALLING, & BRACING Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association www.shrindustry.com	21. O	Connections not shown are the responsibility of others. Adequate support shall be provided to resist gravity, lateral and uplift loads. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the	with the design drawing/QC full by scale details.	
ò	Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral	24. D	eage of the trues. Fabrication of trues shall be in accordance with ANSI / TPI 1. <b>OTHER NOTES</b>	•ANSI / TPI 1: National Design Standard for Metal Plate Connected Wood Trusses •RCs1• Building Commonent & Safety	
	load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.		Camber is a non-structural consideration and is the responsibility of truss fabricator.	Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of	
9.	Eagle is not responsible for improper truss fabrication, handling, erection or bracing. Compression chords shall be laterally braced by the roof or floor	20. L	uo not cut or atter any truss member or plate without prior approval from a professional engineer. Lumber design values are in accordance with ANSI / TPI 1; lumber	Metal Plate Connected Wood Trusses. •NDS: National Design Specification for Wood Construction	
	sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.	28.	design values are by others. Install specified hangers per manufacturer recommendations.	•ESR: 1082 published by the International Code Council, www.icc-es.org	

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1218340 0002/0064



1218340 0003/0004



1218340 0004/0004



### Toza Truss

### 6633 Carl Cox Rd Bennett, NC 27208 Phone: (336) 879 - 1212 Fax:

Email:sales@tozatruss.com Web Site:tozatruss.com

# Quote Date09/04/2024Sales PersonDesignerAP Number

Quote # Q13-Baxter Addition Rev

### QUOTE

Customer:	A Cash Customer	Project:	Baxter Addition 24
Contact:	Prj Descriptio		
Phone:		Tkt Description:	
Bill To:		Ship to:	
Name	A Cash Customer	Name	
Address		Address	
City, State, Zip		City, State, Zip	
Phone		Phone	

Roof Trusses Loading: 20 - 10 - 0 - 10 - Building Code: IRC 2018 - Wind Speed: 115

Name Profile		QTY	Pitch	Unit WGT	Span Spacing	OH Left OH Right	HH Left HH Right
T1		16	4	160	36-0-0 2-0-0	1-2-8 1-2-8	0-4-2 0-4-2
T1G		2	4	225	36-0-0 2-0-0	1-2-8 1-2-8	0-4-2 0-4-2
Total Truss		Max Span		Total WGT	Max H	leight	Total Price
18		36-0-0		3010	6-8	-8	\$2,762.87

### **Delivery Charges**

Description	Туре	Dollars	Qty	Delivery Count	Price	
Delivery Flat Rate	FlatRate	\$150.00		1	\$150.00	

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### Summary:

Truss Price:		\$2,762.87
Wall Panels:		\$0.00
Additional Material:		\$0.00
Sub - Total		\$2,912.87
Tax: 7.00%	Desc: NC	\$203.90
Grand Total:		\$3,116.77

Notes: Disclaimer:

# BUYER IS RESPONSIBLE TO VERFIY TRUSS DESIGNS SPECS PRIOR TO FABRICATION!

### ALL ORDERS MUST BE PREPAID ...

Accepted by: \_\_\_\_\_ / \_\_/

## QUOTE GOOD FOR 7 DAYS FROM QUOTE DATE!

# **Baxter Addition Layout**

	<u>t، 0, </u>	.0 .81	
12' 0"			12' 0"
36' 0"	EXISTING BUILDING		36' 0"
10' 0"			10' 0"
	,		 ¥ 4

58' 0"

Other         The Target-Theme R         Bodd24         Page 1 of 1           Address         Basker Additori U.V.L 2400005322         Basker Additori U.V.L 240000532         Basker Additori U.V.L 2400005322         Basker Additori U.V.L 240000532         Basker Additori U.V.L 240000532         Basker Additori U.V.L 240000532         Basker Additori U.V.L 24000053         Basker Additori U.V.L 240000532															
ISDES(I)         Advices         Exact rAddition (U.S 2020)005022         Just Name         Issue Name		-		Client: Project:			10906083622		Date:		9/6/2024 Dan Schreffler				Page 1 of 1
Bennett NC         Project #         <	lis	Design		-				3622			the second second second	LVLs			
Image: state in the s										_					
Importance information       Reactions UNPATTERNED Ib (Uplift)         Type       Gifter       Application:       Root       Bit       Bit       Direction       Live       Deads       Snow       Wind       Const         Deficient Li:       240       Deals       Method: ASD       Deals       Method: ASD       Deals       Not Ohicked       0       3532       3420       0       0         Importance:       Normal JI       Deals       Method: ASD       Deals       Not Ohicked       2       Verical       0       3532       3420       0	18ft Roo	of Bm	Kerto-S	LVL 1	.750" X	16.000"	2-Ply	- PA	SSED	Lev	vel: Level				
Importance information       Reactions UNPATTERNED Ib (Uplift)         Type       Gifter       Application:       Root       Bit       Bit       Direction       Live       Deads       Snow       Wind       Const         Deficient Li:       240       Deals       Method: ASD       Deals       Method: ASD       Deals       Not Ohicked       0       3532       3420       0       0         Importance:       Normal JI       Deals       Method: ASD       Deals       Not Ohicked       2       Verical       0       3532       3420       0									ж.						
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18'       Reactions UNPATTERNED Ib (Uplift)       Type:     Girder     Application:     Roof       Type:     2     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Dading Code:     INC Checked       Importance:     Normal-I       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC       Deci:     Not Checked       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC     Cap React D/L Ib     Total Ld. Case Ld. Comb.       Deci:     Not Checked     Top:     Stope:     Top:     Stope:       Analysis     Actual     Location Allowed     Capse:     Case       Shear     Stope:     116'     Oral Ld. Case     Ld. Combined       Shear     Stope:     110'     Location Allowed     Cap React D/L Ib     Total Ld. C						1									
18'       Reactions UNPATTERNED Ib (Uplift)       Type:     Girder     Application:     Roof       Type:     2     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Dading Code:     INC Checked       Importance:     Normal-I       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC       Deci:     Not Checked       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC     Cap React D/L Ib     Total Ld. Case Ld. Comb.       Deci:     Not Checked     Top:     Stope:     Top:     Stope:       Analysis     Actual     Location Allowed     Capse:     Case       Shear     Stope:     116'     Oral Ld. Case     Ld. Combined       Shear     Stope:     110'     Location Allowed     Cap React D/L Ib     Total Ld. C												41			
18'       Reactions UNPATTERNED Ib (Uplift)       Type:     Girder     Application:     Roof       Type:     2     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Dading Code:     INC Checked       Importance:     Normal-I       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC       Deci:     Not Checked       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC     Cap React D/L Ib     Total Ld. Case Ld. Comb.       Deci:     Not Checked     Top:     Stope:     Top:     Stope:       Analysis     Actual     Location Allowed     Capse:     Case       Shear     Stope:     116'     Oral Ld. Case     Ld. Combined       Shear     Stope:     110'     Location Allowed     Cap React D/L Ib     Total Ld. C															1'4"
18'       Reactions UNPATTERNED Ib (Uplift)       Type:     Girder     Application:     Roof       Type:     2     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Stope:     012       Method:     Dading Code:     INC Checked       Importance:     Normal-I       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC       Deci:     Not Checked       Tope:     OPE:     Deci:     Not Checked       Deci:     Not Checked     INC     Cap React D/L Ib     Total Ld. Case Ld. Comb.       Deci:     Not Checked     Top:     Stope:     Top:     Stope:       Analysis     Actual     Location Allowed     Capse:     Case       Shear     Stope:     116'     Oral Ld. Case     Ld. Combined       Shear     Stope:     110'     Location Allowed     Cap React D/L Ib     Total Ld. C															Ш
Member Information         Reactions UNP TTERNED Ib (Upliff)           Direction Line         2         Application:         Birg         Direction         Live         Data 2         Snow         Wind         Const           Molitaure Condition:         Day         Design Method:         ASD         Birg         Direction         0         3532         3420         0         0         0           Defector IL:         20         Building Code:         IRC 2018         Design Method:         ASD         Design Method:         Desid:         Desid:         Desid:	1 SP 0-3-8	5									·	2 5	P 0-3-8		3 1/2"
Member Information         Reactions UNP TTERNED Ib (Upliff)           Direction Line         2         Application:         Birg         Direction         Live         Data 2         Snow         Wind         Const           Molitaure Condition:         Day         Design Method:         ASD         Birg         Direction         0         3532         3420         0         0         0           Defector IL:         20         Building Code:         IRC 2018         Design Method:         ASD         Design Method:         Desid:         Desid:         Desid:															
Type:         Cittler         Application:         Roof         Big         Direction         Live         Dead         Snow         Wind         Const           Plies:         2         Dashing Condition: Dry         Dasking Method:         ASD         1         Vertical         0         3532         3420         0         0           Deflection L:         240         Dualing Code:         IRC 2018         Dead:         0         3532         3420         0         0           Importance:         Normal -11         Temperature:         Temperature:         Not Checked         Bearing Length         Dir.         Cappet Condition: Dry         Dead:         0         3532         3420         0         0           Dead:         20 PSF         Dead:         Not Checked         Bearings         Dir.         Cappet Condition: Dry         Dead:         Dead:         Des	1					18'							1		
Type:         Cittler         Application:         Roof         Big         Direction         Live         Dead         Snow         Wind         Const           Plies:         2         Dashing Condition: Dry         Dasking Method:         ASD         1         Vertical         0         3532         3420         0         0           Deflection L:         240         Dualing Code:         IRC 2018         Dead:         0         3532         3420         0         0           Importance:         Normal -11         Temperature:         Temperature:         Not Checked         Bearing Length         Dir.         Cappet Condition: Dry         Dead:         0         3532         3420         0         0           Dead:         20 PSF         Dead:         Not Checked         Bearings         Dir.         Cappet Condition: Dry         Dead:         Dead:         Des															
Pine:       2       Stope:       012       1       Vertical       0       3532       3420       0       0         Moisture Condition:       Design Method:       ASD       Dualing Code:       RC 2018       0       3532       3420       0       0         Deflection TL:       160       Load Sharing:       No       No<				,				-	ions UN	PA	TTERNED I	b (Uplift)			
Model condition: Dry Deflection TL:       240       Design Method: ASD Building Code:       IRC 2018 IRC 2018 Load Sharing: No Temperature:       0       3532       3420       0       0         Importance:       Normal -11 Temperature:       Temperature:       Normal -10 Temperature:       Temperature:	and the second se														
Deficition LL:       240       Building Code:       RC 2018       Load Sharing:       No         Deficition TL:       180       Importance:       Normai - II       Load Sharing:       No         Deck:       Not Checked       Importance:       PSF       Deck:       Not Checked         Bearing Length       Deck:       Not Checked       Deck:       Not Checked         Analysis       Actual       Location Allowed Capacity Comb.       Case       Deck:       No         Analysis       Actual       Location Allowed Capacity Comb.       Case       L       Vert       100%       3532 / 3420       6952 L       D+S         Analysis       Actual       Location Allowed Capacity Comb.       Case       L       L       Vert       100%       3532 / 3420       6952 L       D+S         Lib Definich 0.371 (U569)       9' 116* 0.878 (U240)       42% S       L       L       L       Design Notes       L       L         2 Griders are designed to be supported on the bottom edge only.       3 Multipe pilores machineurity for 24 filor 1.100 (M4K)       D+S       L       L       No       Nind 1.6       Const. 1.25       Comments         1 Dotad must be laterally braned at end bearings.       12 PLF       20 PSF       0 PSF								1							
Importance:       Normal - II       Deck:       Not Checked       Bearing       Length       Sector       Sector <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>2</td><td>ventical</td><td></td><td>0</td><td>3332</td><td>3420</td><td>0</td><td>0</td></t<>								2	ventical		0	3332	3420	0	0
Tempseture:       Temp <= 100°F       Bearings       Searings         General Load       20 PSF       20 PSF       Bearing Length       Dir.       Cap. React D/L Ib       Total Ld. Case       Ld. Comb.         Analysis Results       Analysis Results       Analysis P372 Lhb       9 3570 Ab       75% Drs.       Case       Drs.       Case       Drs.       Case       Drs.       Drs.       L       Drs.       Drs.       Drs.       L       Drs.       Drs.       L       Drs.       Drs. </td <td>Deflection TL:</td> <td>180</td> <td></td> <td>Load S</td> <td>haring:</td> <td>No ,</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Deflection TL:	180		Load S	haring:	No ,									
General Load       Bearing s       Hearing Length       Bearing S         Pioc Live:       0 PSF       3.60°       Vert       100%       3532 / 3420       6962       L       D+S         Analysis       Actual       Location       Allowed       Capacity       Comb.       Case       Case       6962       L       D+S         Analysis       Actual       Location       Allowed       Capacity       Comb.       Case       Case       D       S52 / 3420       6962       L       D+S         Analysis       Actual       Location       Allowed       Capacity       Comb.       Case       Case       D       S52 / 3420       6962       L       D+S         Unbraced       23782 /hb       9       39750 /hb       176%       D+S       L       D       D       Case       D				Deck:		Not Checked									
Productive:       0 PSF       Desiring       Length       Dir.       Gap. React D/L Ib.       Total       Ld. Case       Ld. Comb.         Snow:       20 PSF		Temp	<= 100°F					Dent							
Dead:       20 PSF       1SP       3.500*       Vert       100%       3532/3420       6952       L       D+S         Analysis       Actual       Location       Allowed       Capacity       Comb       Case         Moment       29782       PSF       Universe       9       39750.7b       D*S       L         Unbraced       29782.7b       9       9       97572.7b       D*S       L         Shear       5697 lb       164/12*       13739 lb       41%       D*S       L         Design Abore       9       9716*       1.17 (U180)       64%       D+S       L         Design Abore       2       Circles are designed to be supported on the boltom edge only.       2       3 biologie proteore       2       Circles are designed to be supported on the boltom edge only.       2         2       Circles are designed to be supported on the boltom edge only.       2       Dead mate be laterally braced at end bearings.       7       Diverse       2       DPSF       0 PSF       0 PSF<		0.000							-						
Snow:       20 PSF       2 - SP       3.500*       Vert       100%       3532 / 3420       6952 L       D+S         Analysis Results       Analysis Actual       Location Allowed Capacity Comb.       Case       Analysis Actual       00%       3532 / 3420       6952 L       D+S         Analysis Actual       2 - SP       3.500*       Vert       100%       3532 / 3420       6952 L       D+S         Analysis Actual       20782 R-b       9       29782 R-b       0       S       L         Design Notes       1       1004 0.578 (U280)       9'1/16'       1.171 (U180) 64%       D-S       L       D <td>Contraction of the Contract</td> <td></td> <td></td> <td>2</td> <td></td>	Contraction of the Contract			2											
Analysis       Actual       Location       Allowed       Capacity       Comb.       Case         Moment       28782 ft-b       9'       39750 ft-b       75%       D+S       L         Unbraced       29782 ft-b       9'       29792 ft-b       100%       D+S       L         Shear       5697 lb       166 412'       13739 lb       41%       D+S       L         LLDefinich       0.751 (L/589)       9' 11/6'       1.71 (L/180)       64%       D+S       L         Design Notes       -       -       -       -       -       -       -         2 Circles are designed to be supported on the bottom dege onty.       -       -       -       -       -         3 Multiple ples must be alterally braced at a maximum of 37 68' co.       6 abtom must be alterally braced at a maximum of 37 68' co.       6 btom must be alterally braced at a maximum of 37 68' co.       6 btom must be alterally braced at a maximum of 37 68' co.       6 btom must be alterally braced at a maximum of 37 68' co.       6 btom must be alterally braced at a maximum of 37 68' co.       9 co PSF       0 PSF											Carloren andread				
Analysis       Actual       Location       Allowed       Capacity       Comb.       Case         Moment       29782 A-lb       9'       39750 ftb       75%       D+S       L         Unbraced       29782 A-lb       9'       39750 ftb       75%       D+S       L         LL Definich       0.371 (U569)       9' 1/16'       0.378 (U280)       9' 1/16'       0.378 (U200)       2%       S       L         Design Notes       1       170:045 support to prevent lateral movement and rotation at the end bearings.       2       0       175 (0'/280)       9' 1/16'       0.377 (U280)       9' 1/16'       0.376 (U280)       0.578 (U	Analysis Re	sults		<b>L</b>			9		5.500		10070	00027042	0002	-	0.0
Unbraced       29782 ft-lb       9       29792 ft-lb       100%       D+S       L         Shear       5697 lb       164 i12*       13738 lb       41%       D+S       L         LL Define       0.371 (L/569)       9' 1/16*       0.878 (L/240)       42%       S       L         Design Notes			Locatio	n Allowed	Capacity	Comb.	Case	]							
Shear       5697 lb       16'4'1/2'       13739 lb       41%       D+S       L         LL Defineh       0.371 (U/569)       9'1/16'       0.878 (U/240)       42%       S       L         Design Notes	Moment	29782 ft-lb			75%	D+S	L								
LL Define       0.371 (L/569)       9'1/16"       0.878 (L/240)       42%       S       L         Design Notes															
TL Define       0.753 (U28)       9' 1/16'       1.171 (U180)       64%       D+S       L         Design Notes         1       Provide support to prevent lateral movement and rotation at the end bearings.       2       Griders are designed to be supported on the bottom edge only.       3       3       Multiple piles must be laterally braced at an aximum of 37 5/8' o.c.       6       Bottom must be laterally braced at an aximum of 37 5/8' o.c.       6       Bottom must be laterally braced at an aximum of 37 5/8' o.c.       6       Dottom must be laterally braced at an aximum of 37 5/8' o.c.       6       Dottom must be laterally braced at an aximum of 37 5/8' o.c.       6       Dottom must be laterally braced at an aximum of 37 5/8' o.c.       6       Dottom must be laterally braced at an aximum of 37 5/8' o.c.       0       D       Load Type       Location       The your of							L								
Design Notes         1 Provide support to prevent lateral movement and rotation at the end bearings.         2 Girders are designed to be supported on the bottom edge only.         3 Multiple piles musts be fastered together as per manufacturer's details.         4 Top loads must be supported equally by all piles.         5 Top musts be laterally braced at a maximum of 37 5/6° o.c.         6 Bottom must be laterally braced at an aximum of 37 5/6° o.c.         6 Bottom must be laterally braced at an aximum of 37 5/6° o.c.         1 Load Type       Load Type         1 Uniform       19-0-0         1 Uniform       19-0-0         Self Weight       12 PLF         Vertice in design in the strength or provide strength or provide proper dialings to provide grouper diali		Contraction of the second					L								
1 Provide support to prevent lateral movement and rotation at the end bearings.         2 Girders are designed to be supported on the bottom edge only.         3 Multiple piles must be fastened together as per manufacturer's details.         4 Top loads must be supported equally by all piles.         5 Top must be laterally braced at an eakinum of 37 5/8° o.c.         6 Bottom must be laterally braced at an eakinum of 37 5/8° o.c.         7 Lateral slenderness ratio based on single ply width.         ID       Load Type         1       Uniform         1       Uniform         1       Uniform         Self Weight       12 PLF			9.1/10	5" 1.171 (L/18	0) 64%	D+S	L	-							
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