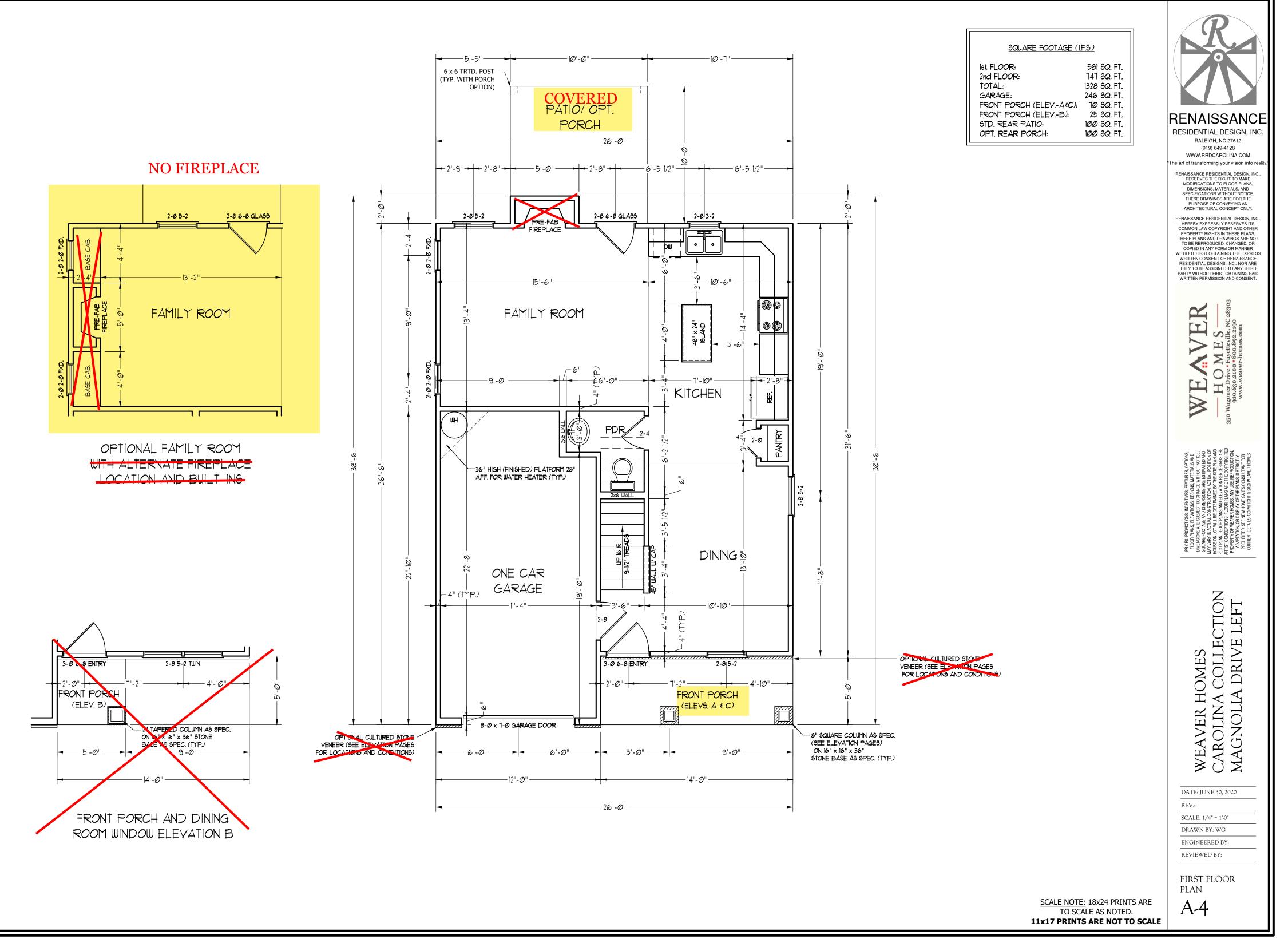
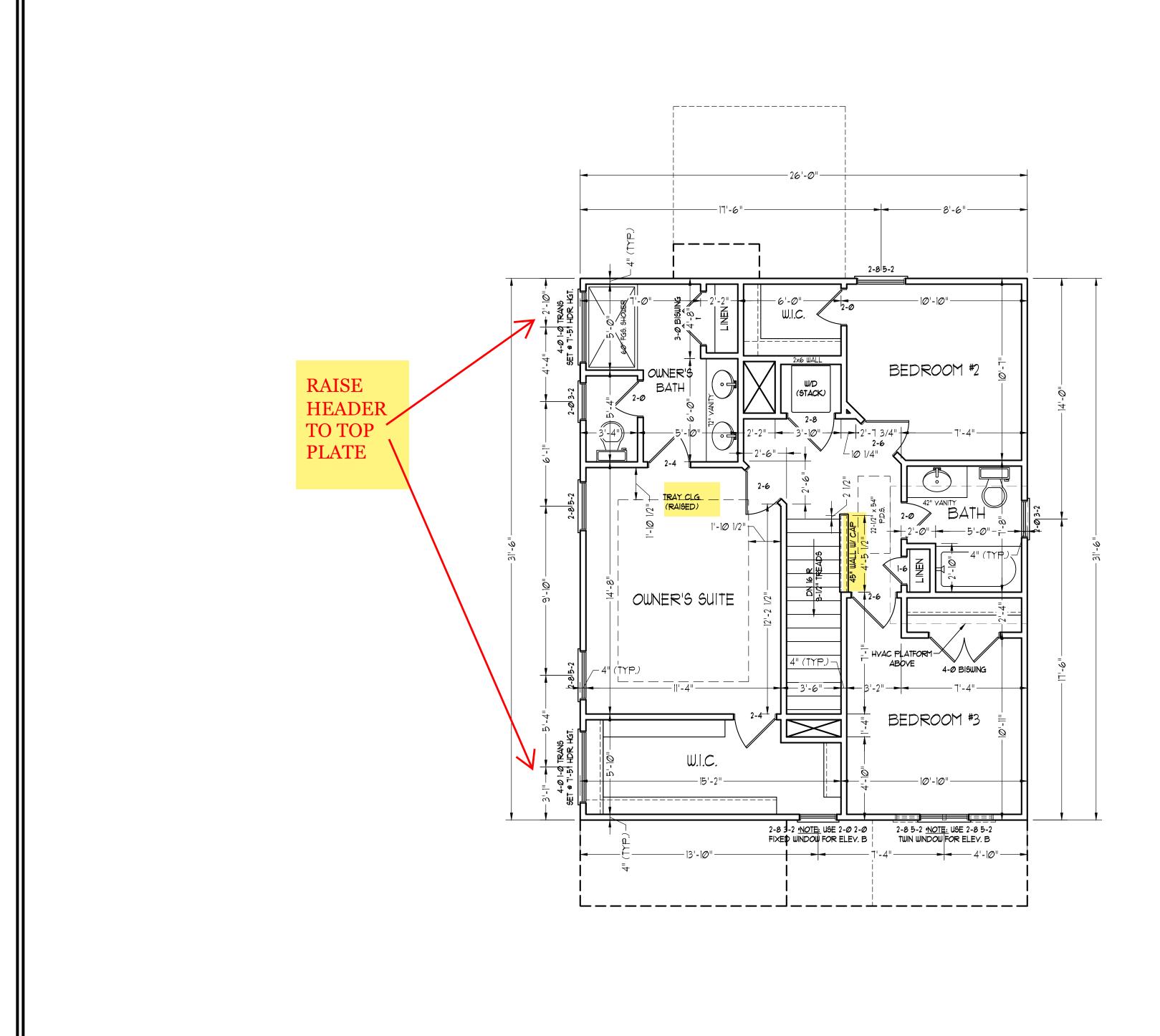




SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**

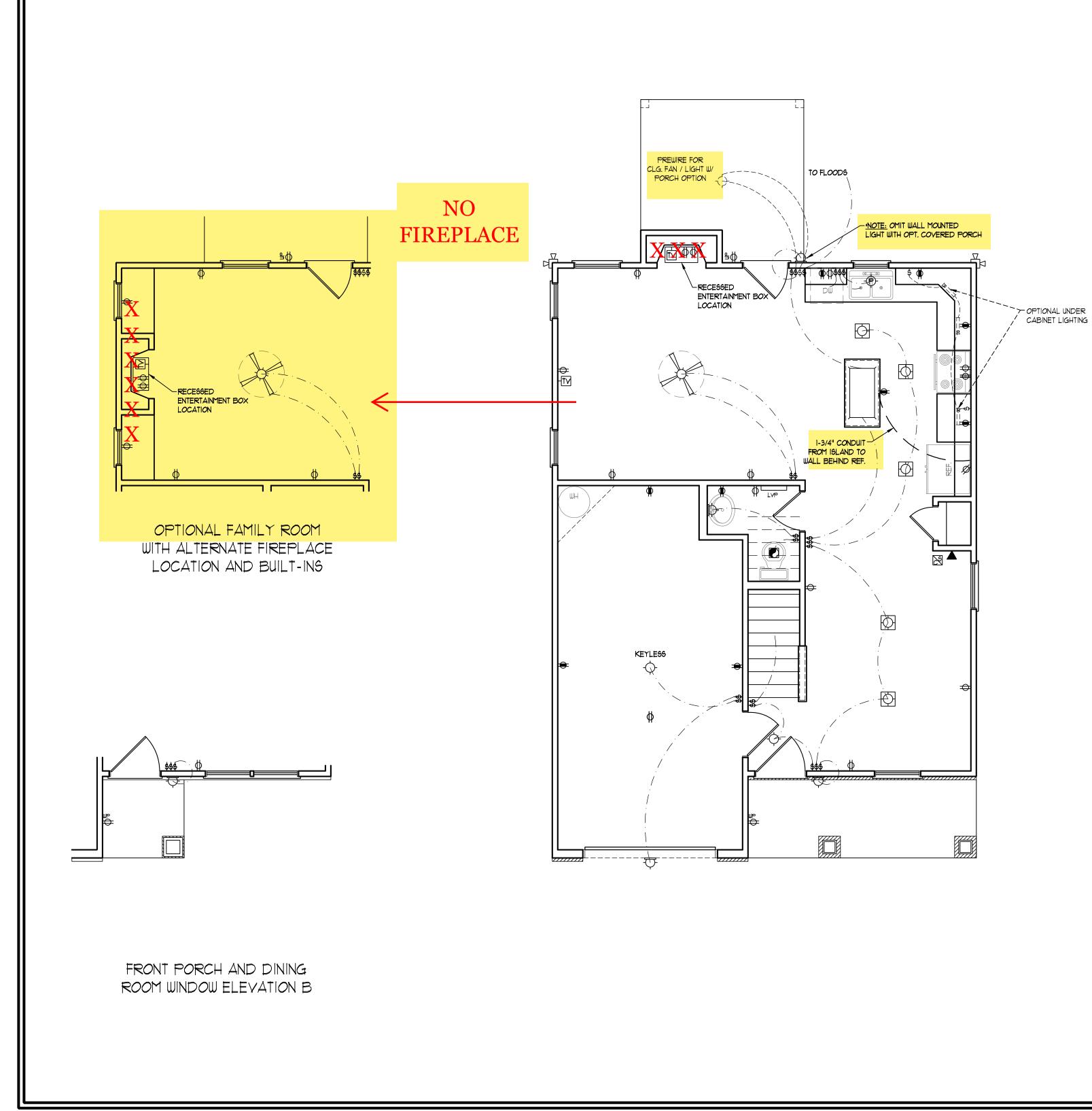
A-3







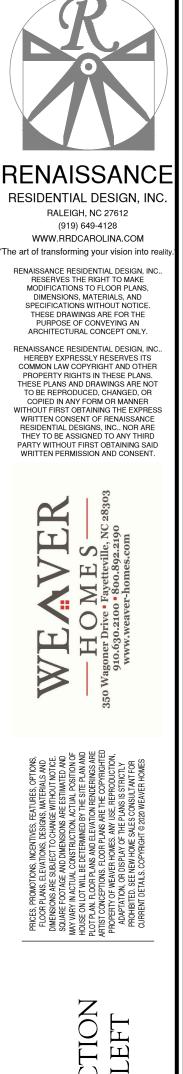
SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



ELECTRICAL LAYOUT NOTES: 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN. 2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.) 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN. 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS. ELECTRICAL LEGEND ↔ IIØ v OUTLET 😑 110 V GFI OUTLET BB + IIØ V BASEBOARD OUTLET 🚓 4-PLEX COUNTER OR FLOOR MOUNTED COUNTER OR FLOOR MOUNTED 110V GFI

- € 220 ∨ OUTLET
- Ø 10 V DEDICATED CIRCUIT
- Ø 220 V DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT UNDERCABINET LIGHT
- SWITCH
- DIMMER SWITCH \$₽_D
- TELEPHONE
- \triangle data
- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- SE 110 V SMOKE/ CM DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL ALARM ALARM PANEL
- - CEILING; FAN

 - CEILING FAN W/ LIGH1
- SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**



WEAVER HOME CAROLINA COL CAROLINA (MAGNOLIA DATE: JUNE 30, 2020 REV.: SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY: REVIEWED BY: FIRST FLOOR ELECTRICAL PLAN E-1

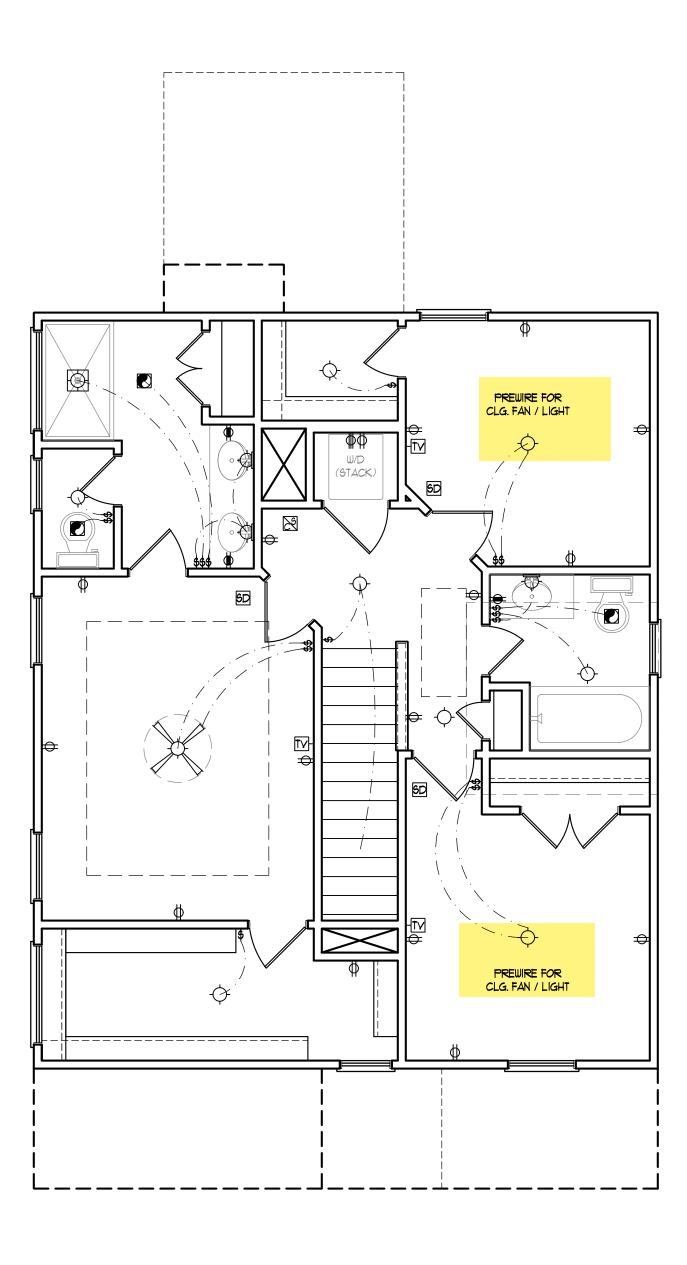
Ē

 \mathbf{O}

Ц

>

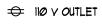
DRI





- 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 90" A.F.F. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

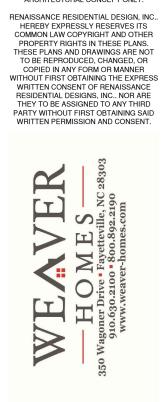
ELECTRICAL LEGEND



- 😑 110 y GFI OUTLET
- HIV Y SWITCHED OUTLET
- BB IIO V BASEBOARD OUTLET
- + ↓ 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI

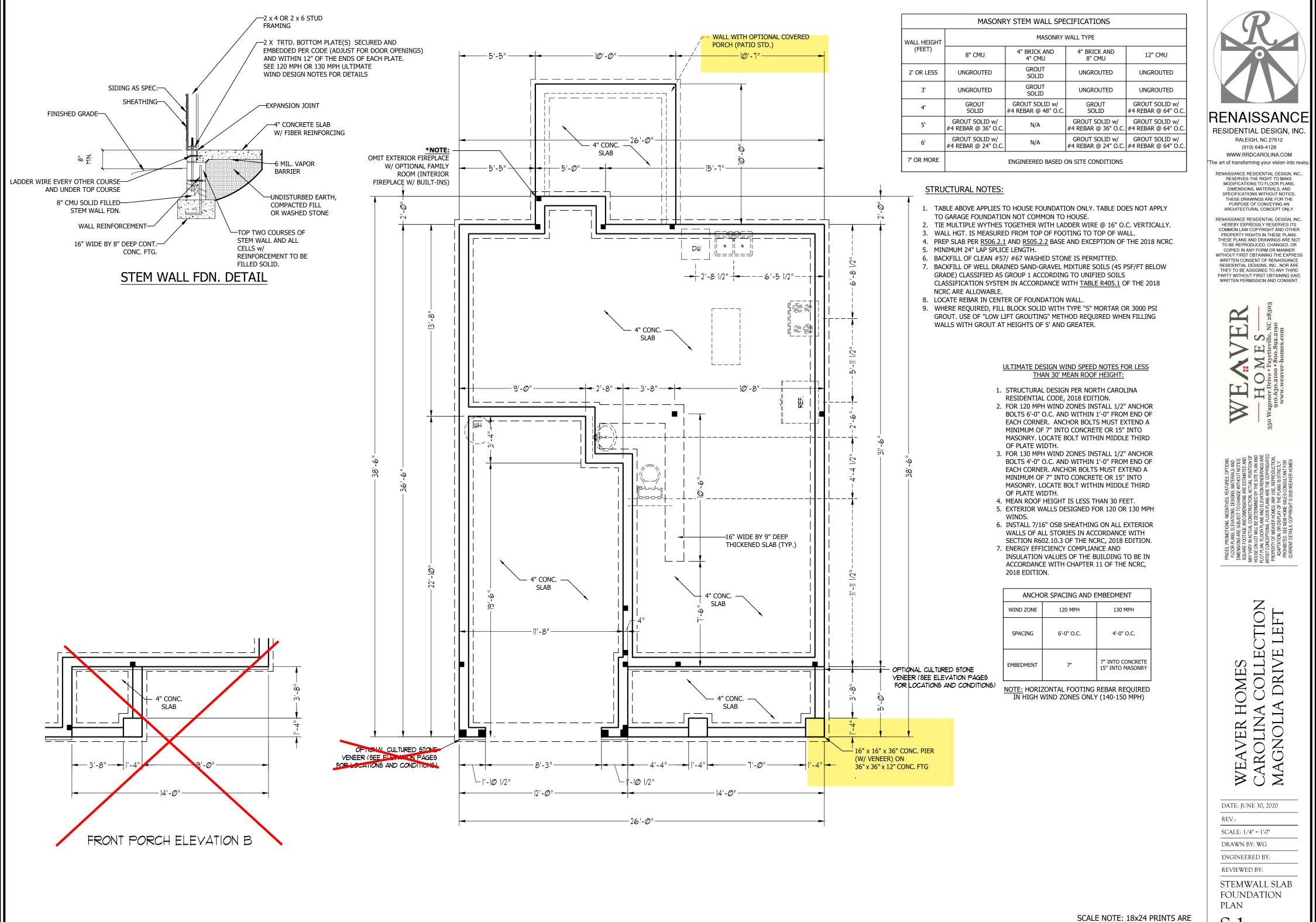
- 10 V DEDICATED CIRCUIT
- 120 V DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- MINI CAN LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT
- SWITCH \$
- \$_D DIMMER SWITCH
- \triangle data
- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 10 V SMOKE/ CO DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL ALARM ALARM PANEL
- CEILING FAN
- CEILING FAN W/ LIGHT

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE** RENAISSANCE RESIDENTIAL DESIGN, INC. RALEIGH, NC 27612 (919) 649-4128 WWW.RRDCAROLINA.COM ne art of transforming your vision into rea RENAISSANCE RESIDENTIAL DESIGN, INC.. RESERVES THE RIGHT TO MAKE MODIFICATIONS TO FLOOR PLANS, DIMENSIONS, MATERIALS, AND SPECIFICATIONS WITHOUT NOTICE. THISES DRAWINGS ADE FOR THE THESE DRAWINGS ARE FOR THE PURPOSE OF CONVEYING AN ARCHITECTURAL CONCEPT ONLY.



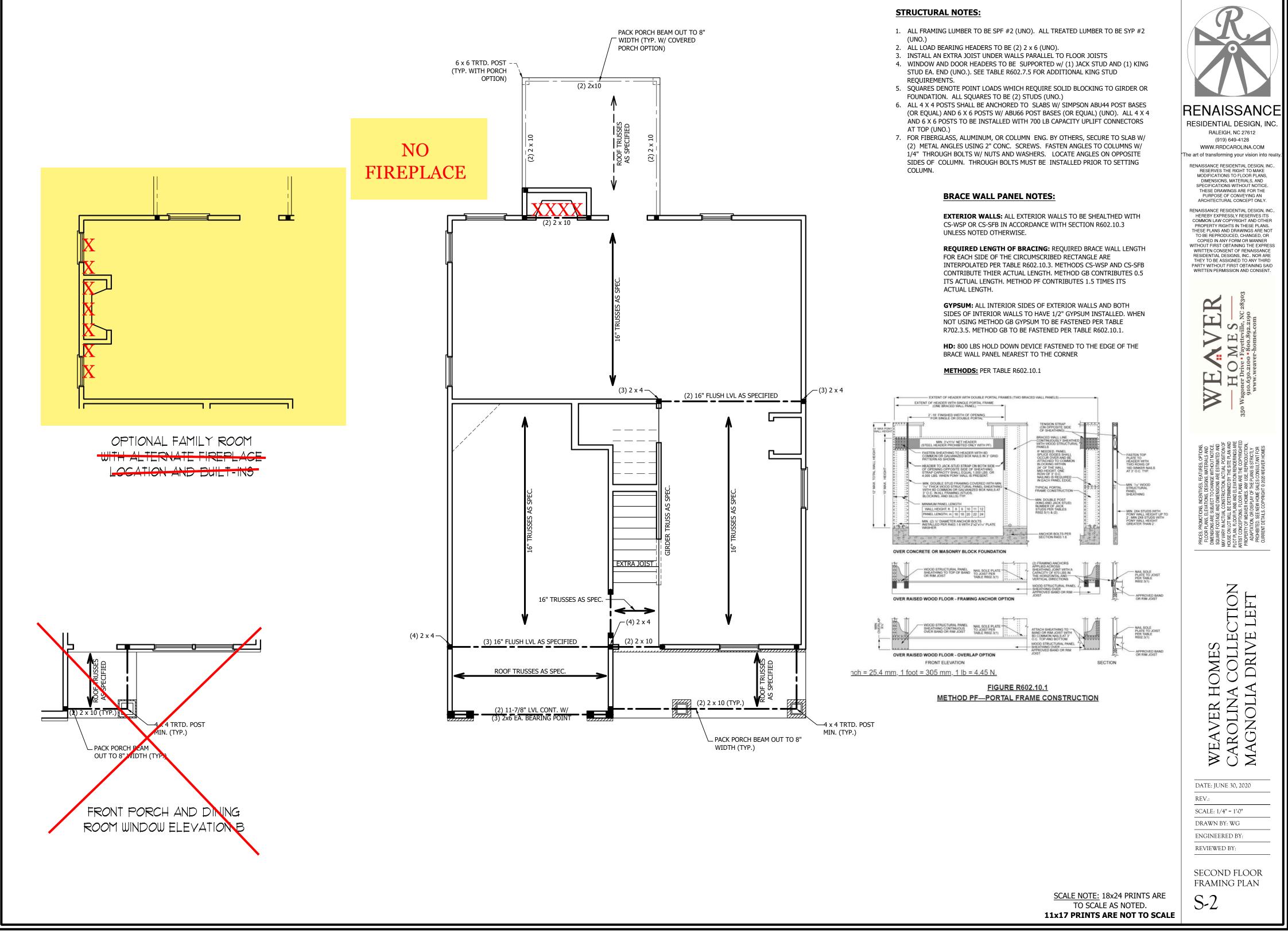
ALE A N SU A PRICES, PF FLOOR PU DIMENSIO SQUARE F MAY VARY MAY VARY PLOT PLA PLOT PLA PLOT PLA

> TION Ìц Ц Ц Ĕ > \mathcal{O} WEAVER HOMES CAROLINA COLL MAGNOLIA DRIV



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

S-1



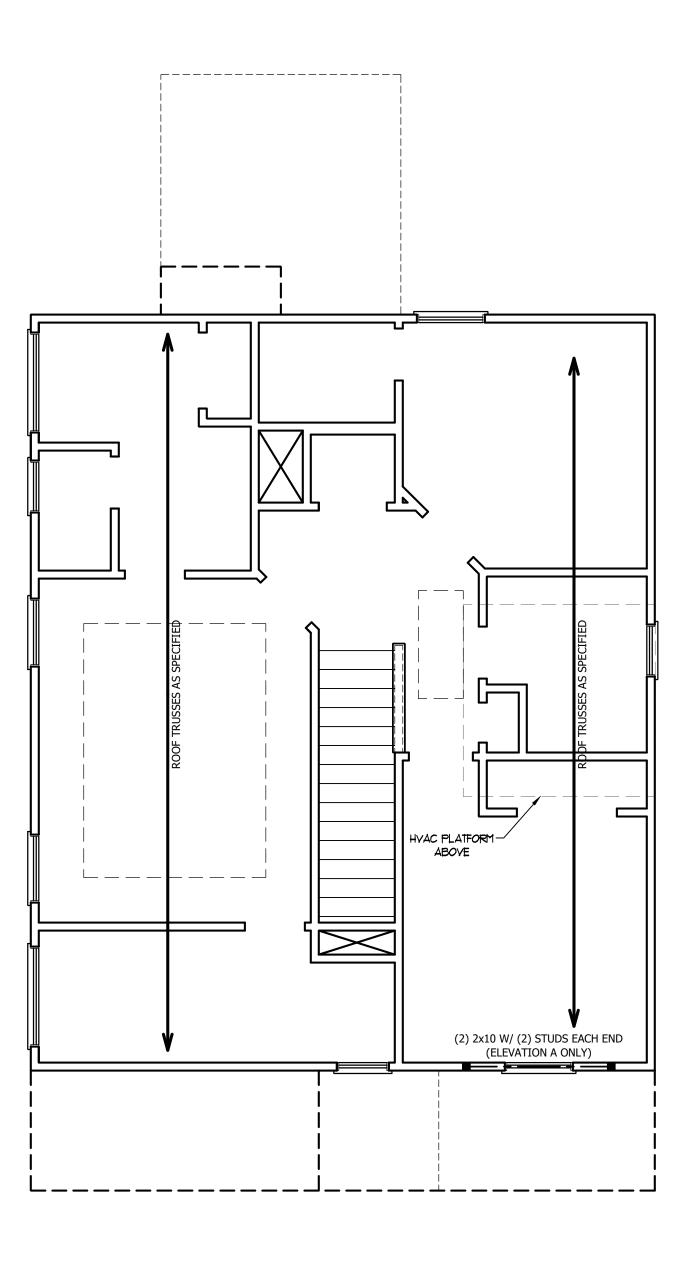


TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES (PER TABLE R602.3(5)				
(121)	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			

STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



RENAISSANCE RESIDENTIAL DESIGN, INC.



		-	
ENGIN	IEERED	BY:	
REVIE	WED BY	/ :	

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

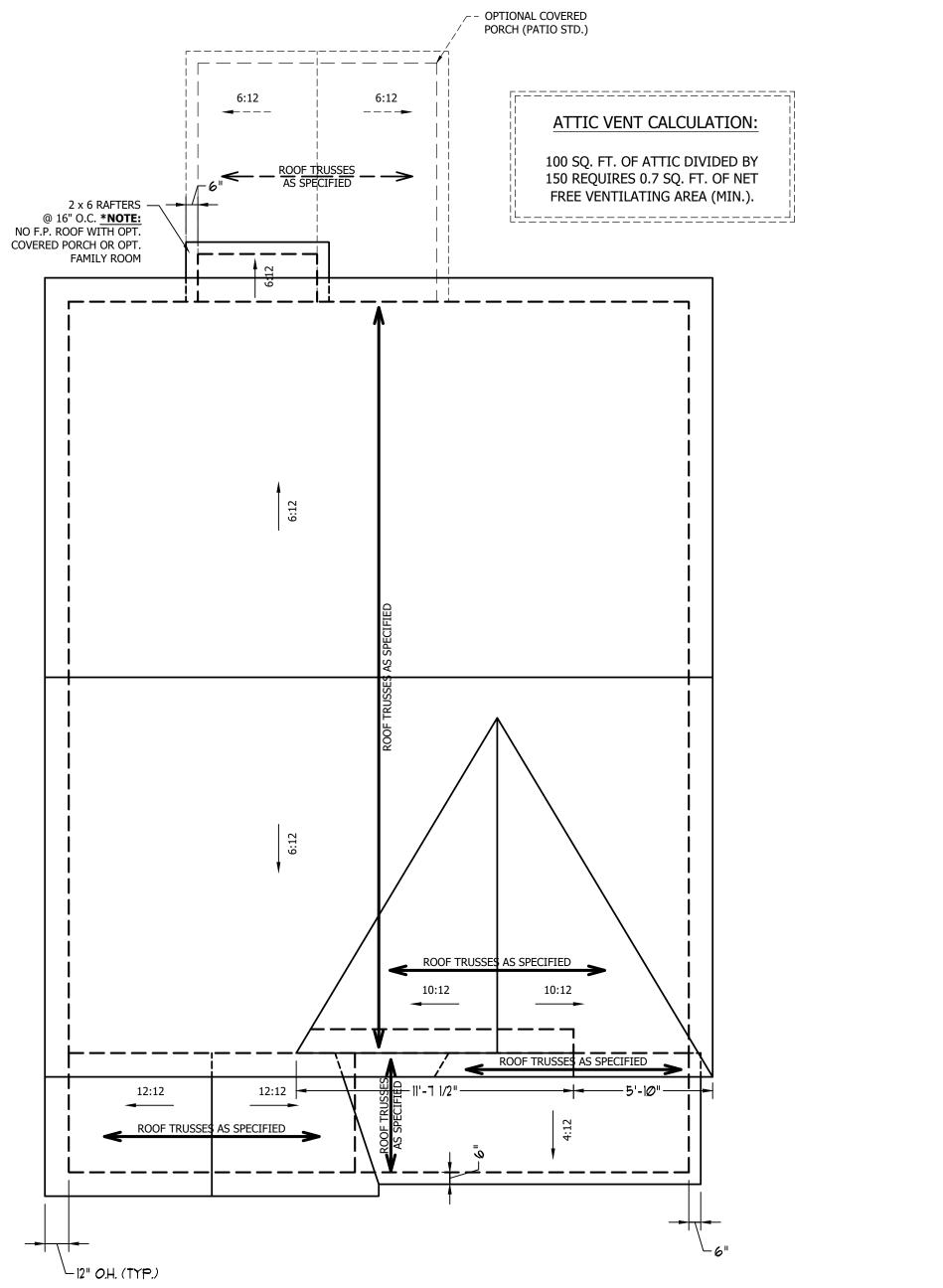
ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

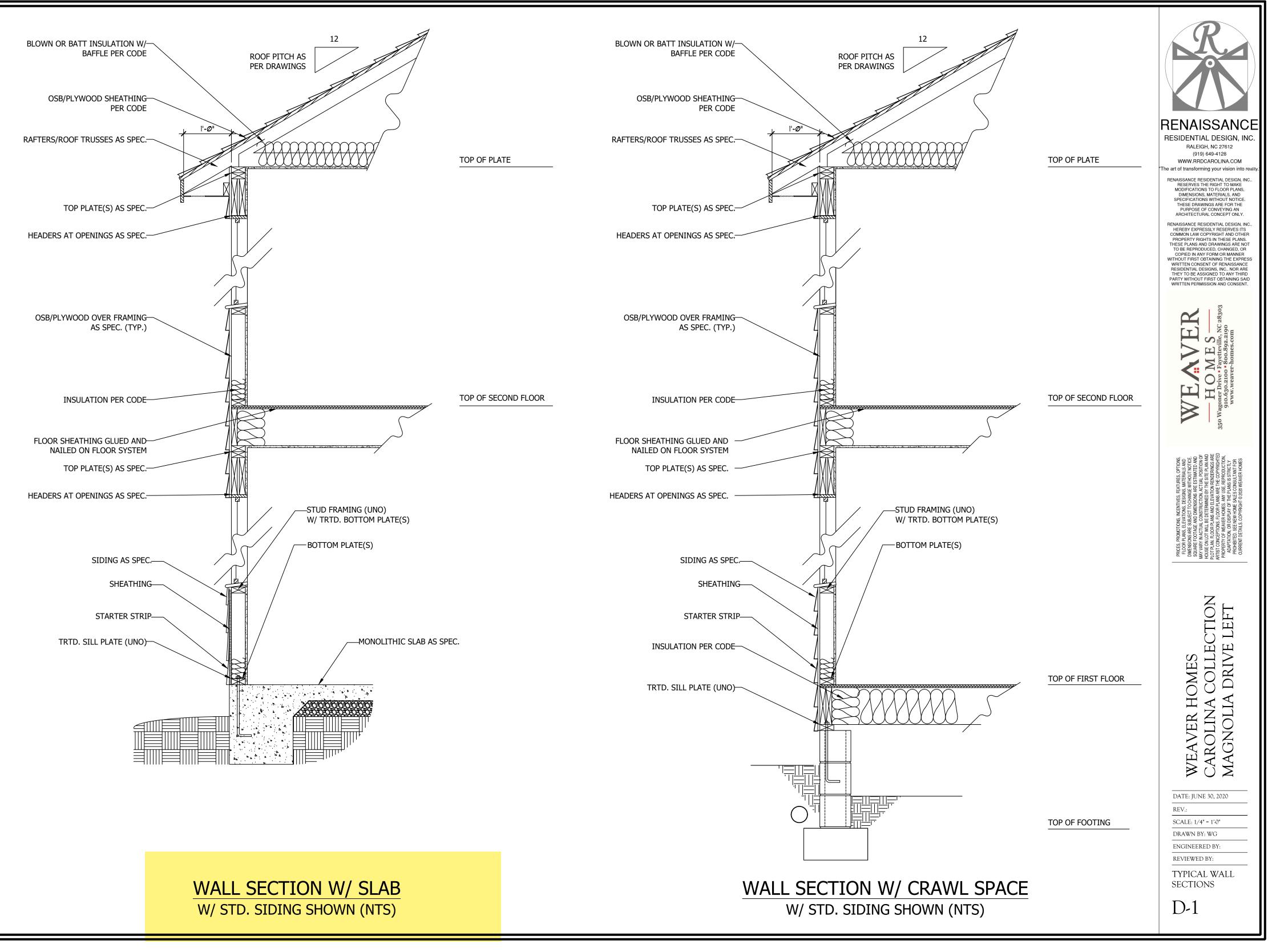
- ALL FRAMING LUMBER TO BE #2 SPF (UNO). 1. 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- REFER TO SECTION R802.11 OF THE 2018 NCRC 5. FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

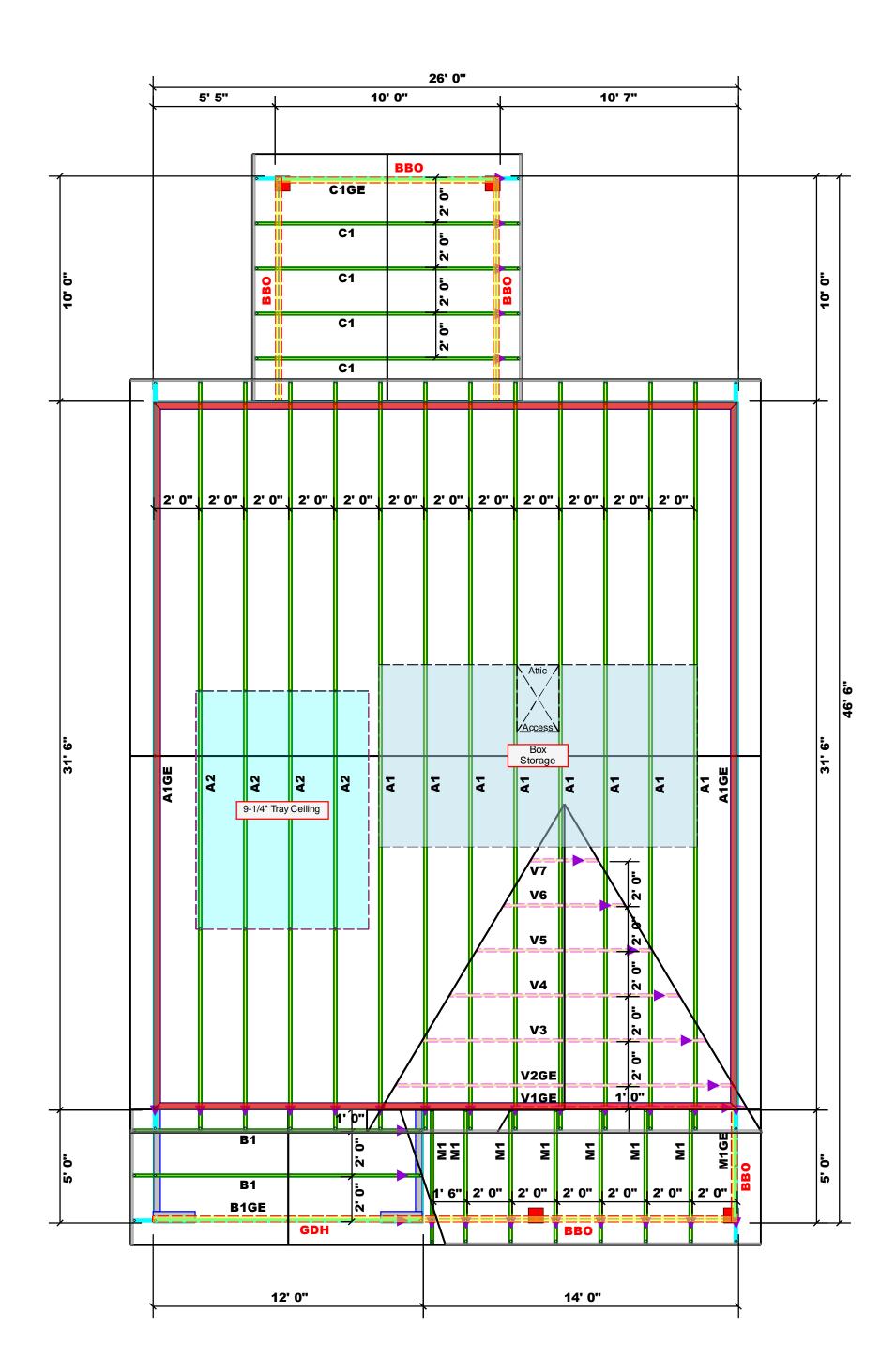
C:\Users\Wade\Documents\Projects\Westan-Weaver\Magnolia_Magnolia_GL_6-30-20.dwg, 7/22/2020 6:48:02 AM

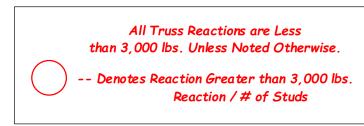


ELEVATION C









All Walls Shown Are Considered Load Bearing

Roof Area	= 1438.33 sq.ft.
Ridge Line	= 59.7 ft.
Hip Line	= 1.32 ft.
Horiz. OH	= 72.13 ft.
Raked OH	= 108.84 ft.
Decking	= 49 sheets

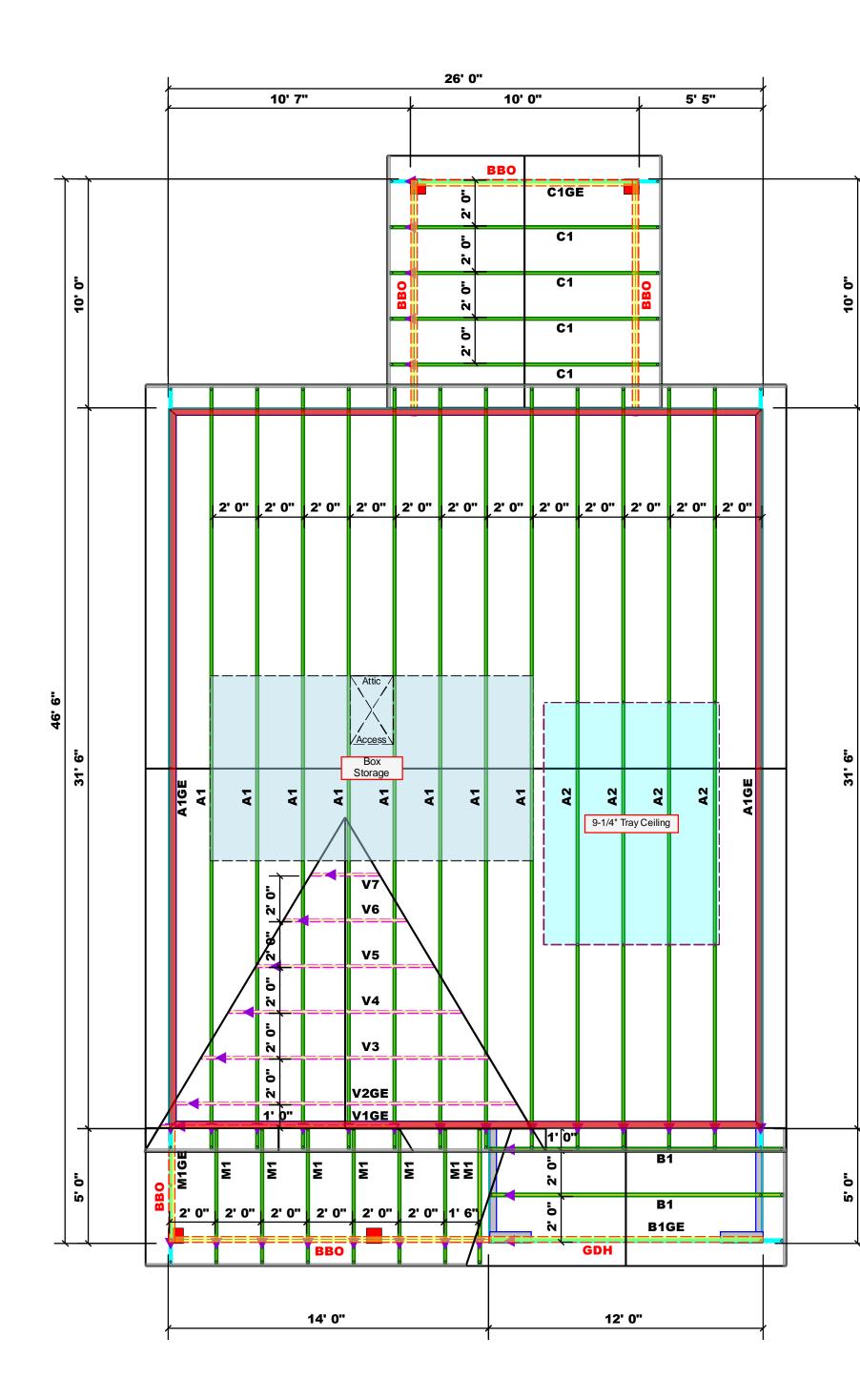
Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend
Second Floor Walls
Tray Ceiling
Box Storage
Flush Beam
Drop Beam

- (1)	Truss Placement Plan
	Scale: 1/4"=1'

COMTECH ROOF & FLOOR ROOF & FLOOR RUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444								
LOA				CK STU				
NUN	ABER OF JA		REQUIRED	1) & (b)) b @ EA END	a			
1700 3400 5100 6800 8500 10200 11900 13600	6 8 2 9 2 4 2 7 1 REQ'D STUDS FOR	2550 5100 7650 10200 12750	2 3 3 4 5	340 680 1020 1360	00 2 00 3 00 4			
CITY / CO . Lillington / Harnett	4166 Darroch Road	Roof	12/06/24	DRAWN BY Jonathan Landry	SALES REP. Lenny Norris			
CITY / CO .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.			
Weaver Homes	Lot 5 Maple Hill	Magnolia "C" / GLF, CP	N/A		J1224-6500			
BUILDER	JOB NAME	PLAN	SEAL DATE N/A	QUOTE #	JOB #			
These t comport design See ind identifie designe perman for the support and col designe consult	ImageImageImageImageImageImageTHIS IS A TRUSS PLACEMENT DIAGRAM ONLY.These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer.See individual design sheets for each truss design identified on the pacement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com							

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

> All Walls Shown Are Considered Load Bearing

Roof Area	= 1438.33 sq.ft. = 59.7 ft.
Ridge Line	= 59.7 ft.
Hip Line	= 1.32 ft.
Horiz. OH	= 72.13 ft.
Raked OH	= 108.84 ft.
Decking	= 49 sheets

Dimension Notes

 Dimension Notes

 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise

 2. All interior wall dimensions are to face of stud unless noted otherwise

 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend
Second Floor Walls
Tray Ceiling
Box Storage
Flush Beam
Drop Beam

Truss Placement Plan
Scale: 1/4"=1'

These t comport design See ind identifie designe perman for the support and col designe consult	BUILDER	Weaver Homes C	:ITY / CO .	CITY / CO . Lillington / Harnett		Signatu	deemed requiren attached requiren size and reaction retained reaction Tables. retained		
A TRUSS trusses ar nents to b at the sp lividual de ed on the er is resp ent braci overall st t structure lumns is t BCSI-B1 elivery pa	JOB NAME	Lot 5 Maple Hill A	ADDRESS	4166 Darroch Road	(BASED (BASED NBER OF JA NBSR OF	re	reactions to complements. The I Tables (nents) to I number s greater A register to design that exce A register to design s that exce	RUS eilly R Fayet Phon	
e designo ecification esign she placeme onsible fo ng of the ructure. 1 e includir the respo neral guid and BCS	PLAN	Magnolia "C" / GLF, CP	MODEL	Roof	ON TABL		y with the e contract derived f determin of wood s than 3000 red design the sup eeds thos red design the sup	OF & SES load Ir teville e: (910	
ed as ind orated int n of the b ets for ea nt drawin or tempor roof and The desig ng header nsibility o dance reg B-B3 prov	SEAL DATE	D D	DATE REV.	12/06/24	ES R502.5(1) REQUIRED VOIRDER VOIRDER VOI SQDLS Q VOI VOIRDER VOI SQDLS Q VOI VOIRDER VOI SQDLS Q VOI VOIRDER VOI SQDLS Q VOIR VOIRDER VOI SQDLS Q VOIR VOIRDER VOIR		e prescrip tor shall r rom the p to the min studs req D# but no n profess port syste e specifie n profess port syste	& FL & B	
ividual bu o the buil ouilding de ach truss g. The bu ary and floor sys n of the to s, beams of the buil parding br rided with	QUOTE #	Δ	DRAWN BY	DRAWN BY Jonathan Landry	l) & (b))	L <i>and</i> .andr	tive Code refer to the prescription imum fou uired to s t greater sional sha em for an ed in the a ional sha	ial Pai 28309 -8787	
ilding ding esigner. design ilding tem and russ , walls, ding acing, the	JOB #	J1224-6500 S.	SALES REP.	SALES REP. Lenny Norris	0 OF 80 SLUPS FOR 101 AU 100 0 100 0 100000000		e ve Code indation upport than II be y attached	ן א אS יk	

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J1224-6500 Lot 5 Maple Hill

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I70036744 thru I70036759

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



December 7,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the design for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

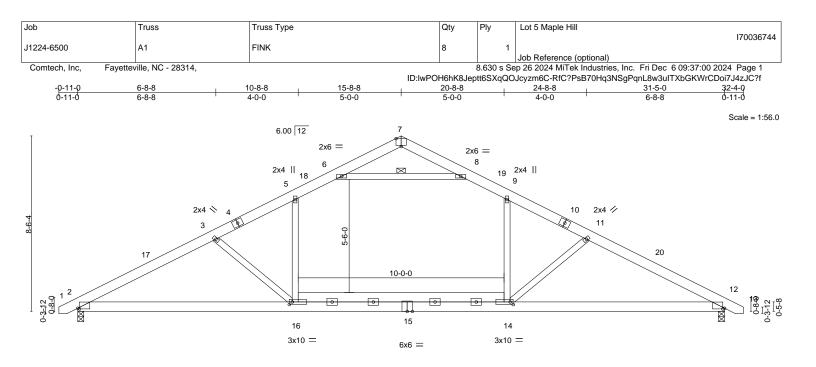


Plate Offsets (X,Y)	10-8-8 10-8-8 [2:0-1-0,0-0-7], [7:0-3-0,Edge], [12:0-1-1	,	20-8-8 10-0-0 6:0-1-12,0-1-8]				31-5-0 10-8-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.60 BC 0.54 WB 0.40 Matrix-S	Vert(CT) -0 Horz(CT) 0	in (loc) 0.24 12-14 0.35 12-14 0.06 12 0.18 2-16	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 223 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x6 SF WEBS 2x4 SF	TOP CHORD2x6 SP No.1TOP CHORDStructural wood sheathing directly applied or 4-6-8 oc purlins.BOT CHORD2x6 SP No.1BOT CHORDRigid ceiling directly applied or 9-10-6 oc bracing.WEBS2x4 SP No.2WEBS1 Row at midpt6-8REACTIONS.							
Max Horz 2=-142(LC 10) Max Uplift 2=-243(LC 12), 12=-243(LC 13) Max Grav 2=1364(LC 2), 12=1364(LC 2)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2405/866, 3-5=-2109/763, 5-6=-1688/732, 6-7=0/269, 7-8=0/269, 8-9=-1688/732, 9-11=-2109/763, 11-12=-2406/866								
BOT CHORD 2-16=-624/2126, 14-16=-380/1759, 12-14=-617/2079 WEBS 3-16=-532/324, 5-16=-97/742, 9-14=-97/742, 11-14=-532/324, 6-8=-2012/704								
2) Wind: ASCE 7-10; V	e loads have been considered for this de /ult=150mph Vasd=119mph; TCDL=6.0 -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-1	osf; BCDL=6.0psf; h=15ft; Ca						

for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x6 MT20 unless otherwise indicated.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

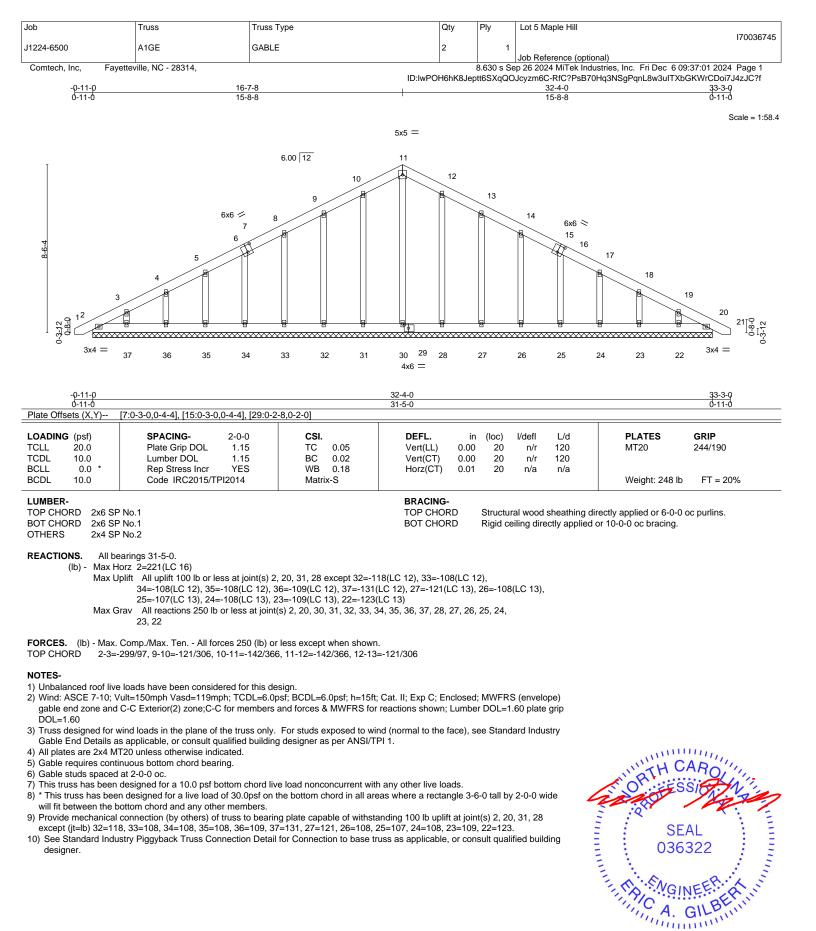
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2 and 243 lb uplift at joint 12.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scietur Information**. Building from the Structure Building Component Advance interpretented and the properties of th and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

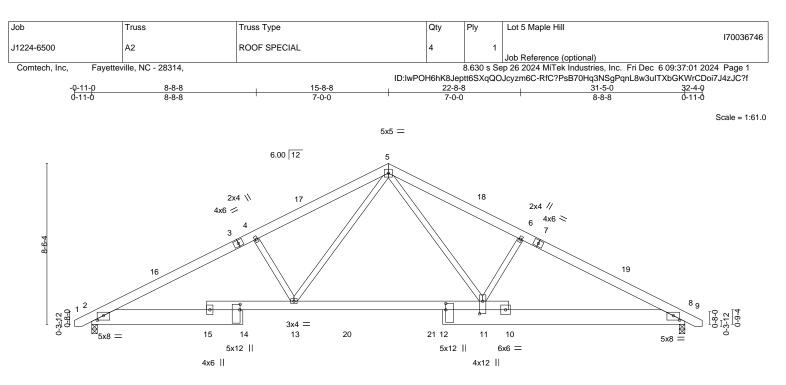


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek A 818 Soundside Road

Edenton, NC 27932

December 7,2024



ŀ	<u>8-0-0</u> 8-0-0	10-8-8	<u>18-7-0</u> 7-10-8	20-8-8 22-8-8	<u>31-5-0</u> 8-8-8	
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [8:0-4-0,	0-2-14], [11:0-8-0,0-2-0], [12:	:0-4-0,0-0-0], [14:0-3-8,0-0	-4]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	1.15 BC 0 YES WB 0	DEFL. 0.24 Vert(LL) 0.63 Vert(CT) 0.48 Horz(CT S Wind(LL) 0.07 8 n/a	L/d PLATES 360 MT20 240 n/a 240 Weight: 243 lb	GRIP 244/190 FT = 20%
	P No.1 *Except* 2x6 SP No.1		BRACIN TOP CH BOT CH	ORD Structural wood	sheathing directly applied or 4-9-13 ctly applied or 9-7-8 oc bracing.	oc purlins.
REACTIONS. (size Max H	e) 2=0-3-8, 8=0-3-8 orz 2=142(LC 11)					

Max Horz 2=142(LC 11) Max Uplift 2=-243(LC 12), 8=-243(LC 13) Max Grav 2=1297(LC 1), 8=1297(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2430/922, 4-5=-2182/940, 5-6=-2124/882, 6-8=-2325/855

BOT CHORD 2-13=-650/2096, 11-13=-277/1352, 8-11=-608/1980

WEBS 4-13=-429/393, 5-13=-356/993, 5-11=-271/919, 6-11=-430/396

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 32-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

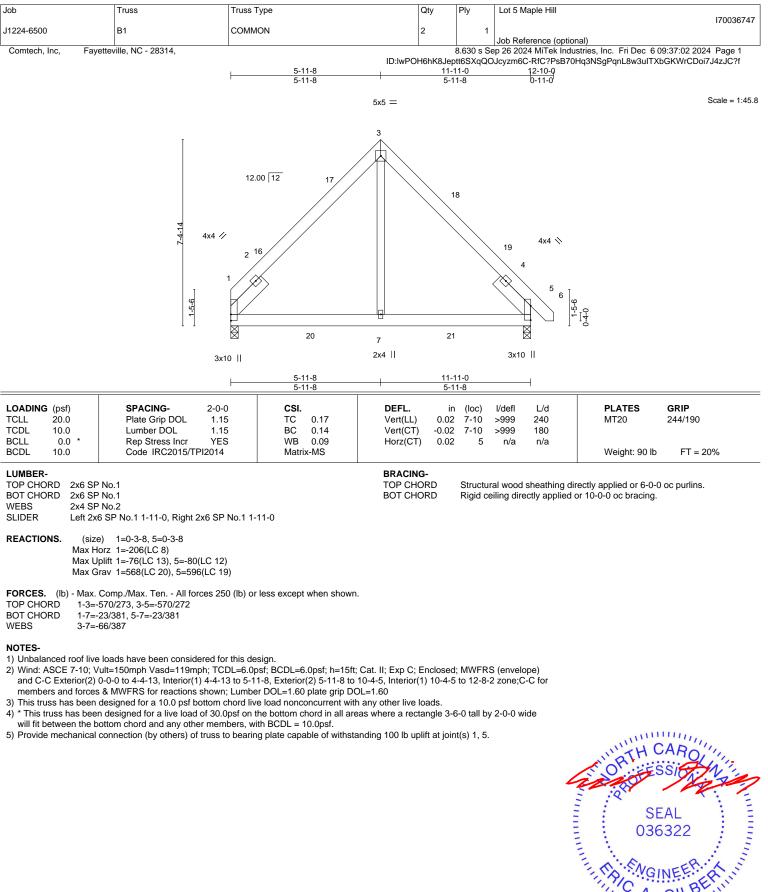
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=243, 8=243.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

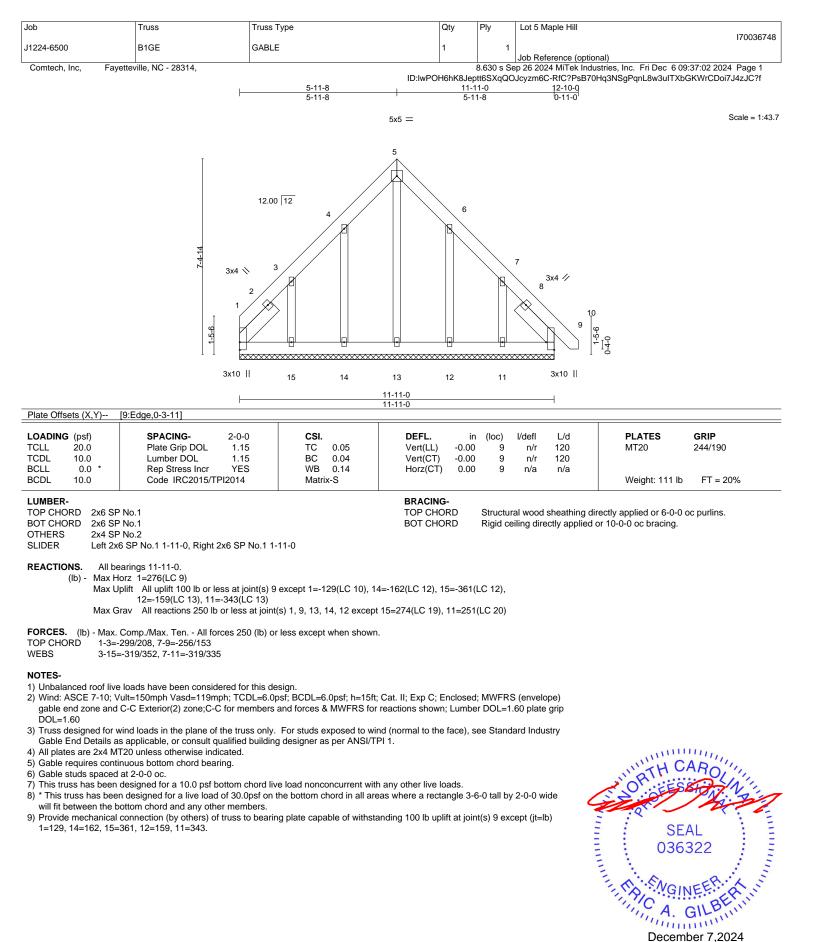


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



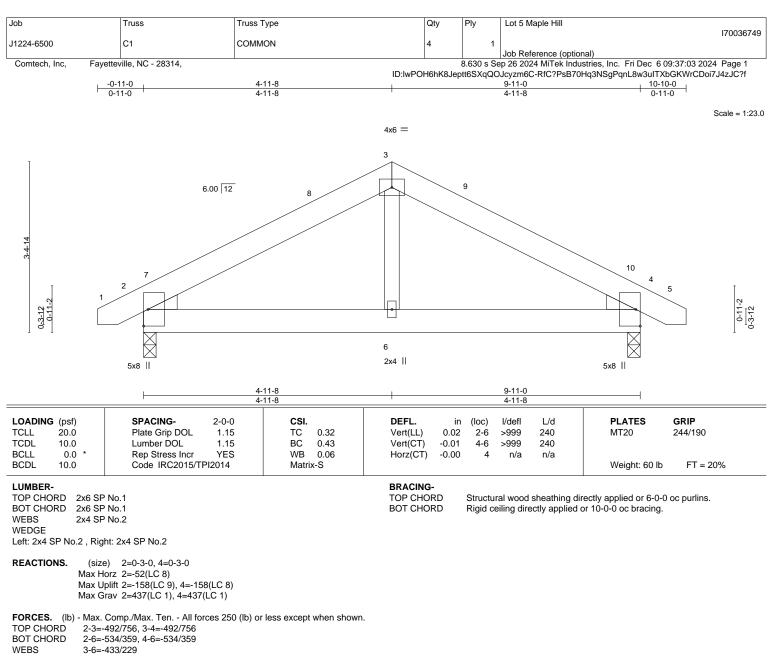
GI (1111111) December 7,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Aff



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 4-11-8, Exterior(2) 4-11-8 to 9-4-5, Interior(1) 9-4-5 to 10-7-10 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=158, 4=158.

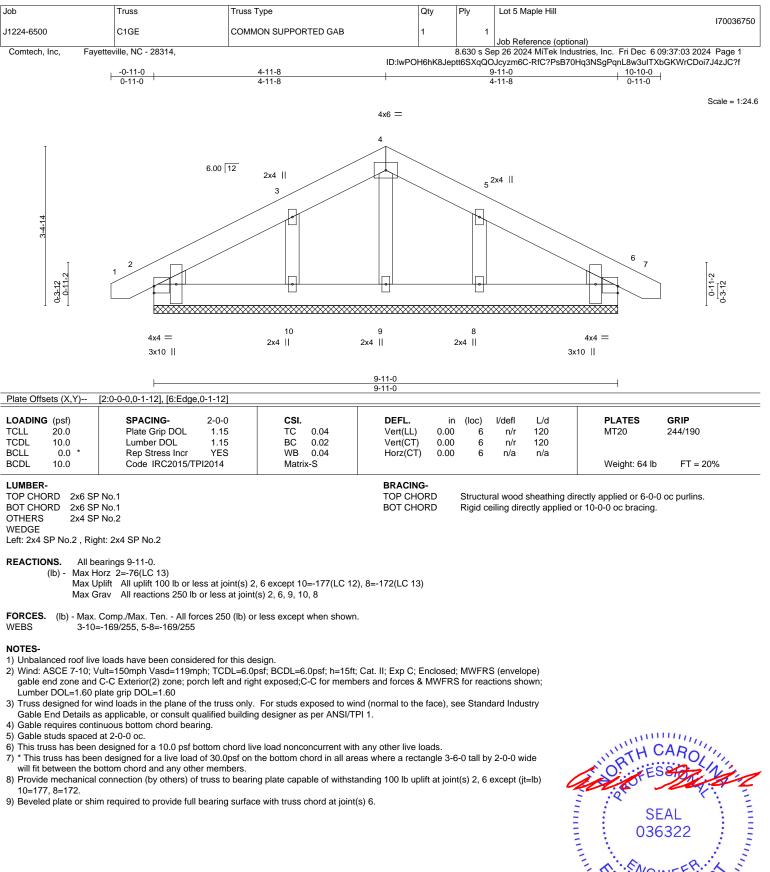


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



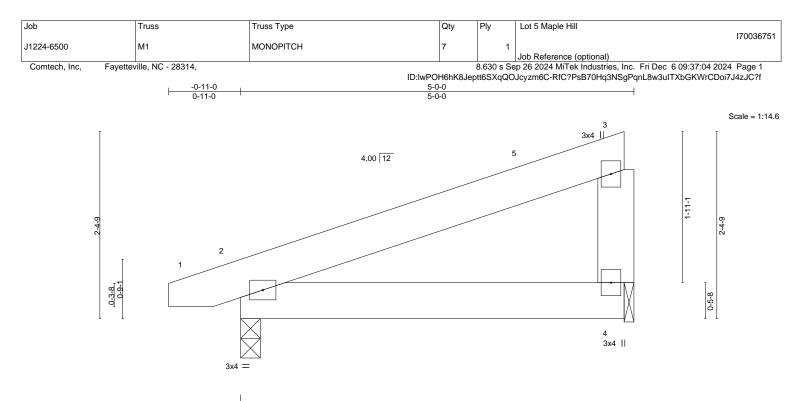
818 Soundside Road

Edenton, NC 27932





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	()	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) 0.02	2-4	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.01	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 29 lb	FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=79(LC 12) Max Uplift 2=-140(LC 8), 4=-127(LC 8) Max Grav 2=235(LC 1), 4=182(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

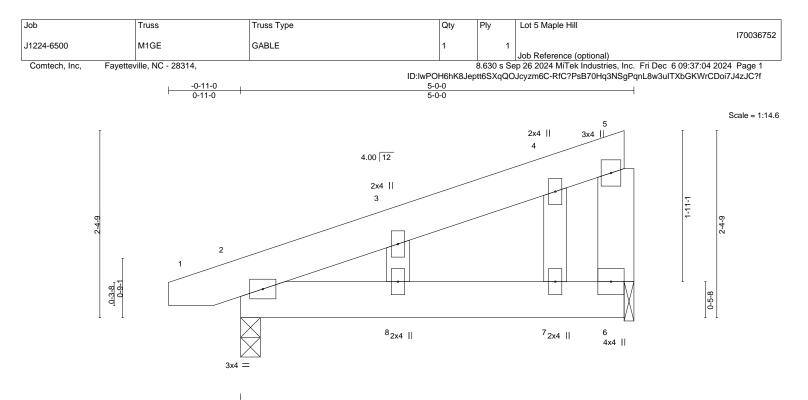
 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=140, 4=127.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 32 lb FT = 20%
--

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=113(LC 12) Max Uplift 2=-192(LC 8), 6=-177(LC 8) Max Grav 2=235(LC 1), 6=182(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=192, 6=177.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

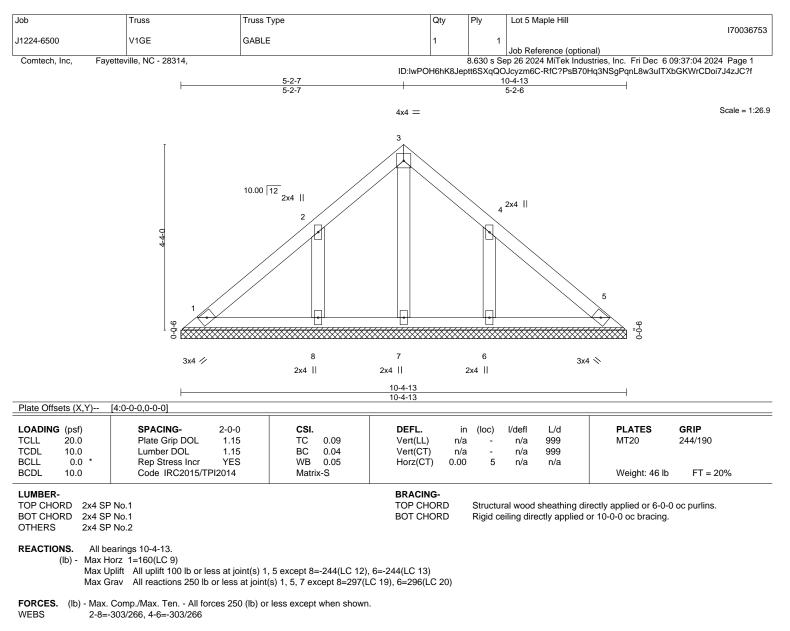
except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design that the operating of the second se and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road

Edenton, NC 27932



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

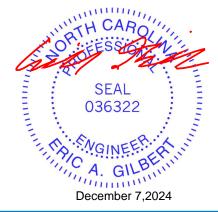
4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=244, 6=244.

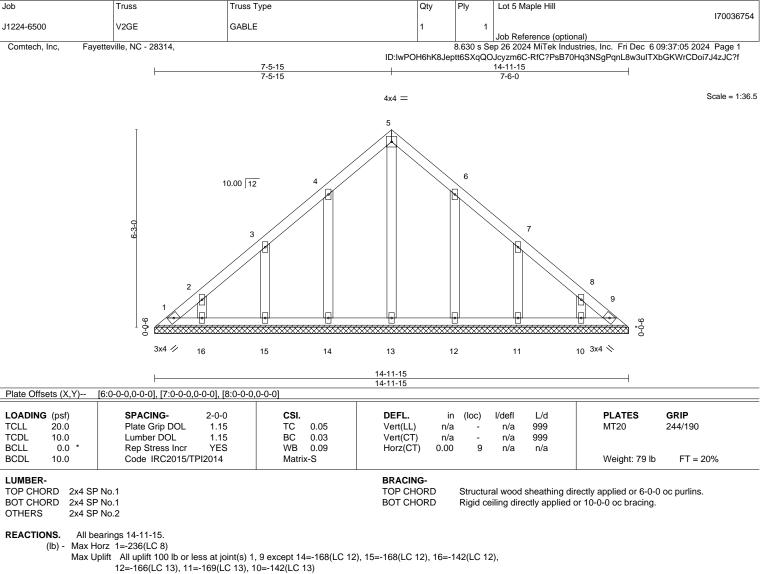


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



818 Soundside Road

Edenton, NC 27932



Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

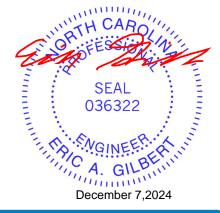
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD

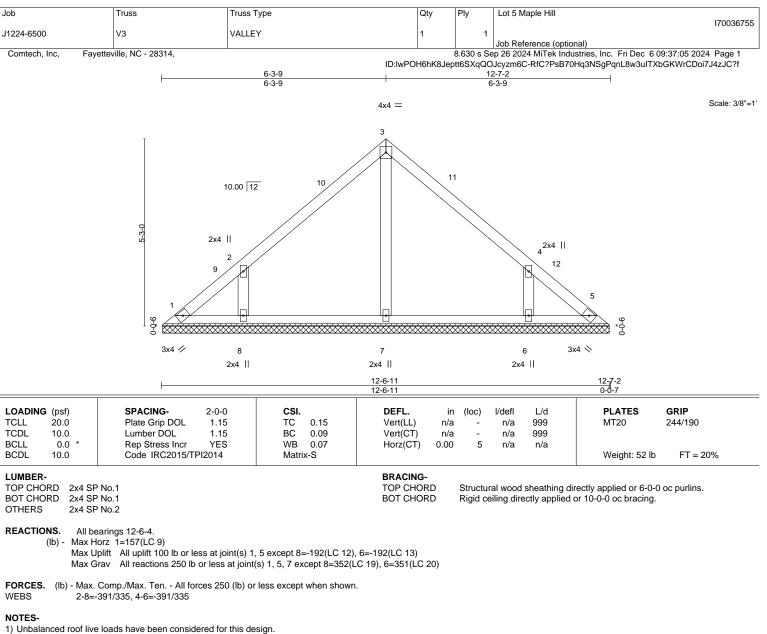
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

1-2=-275/189

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 14=168, 15=168, 16=142, 12=166, 11=169, 10=142.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-3-9, Exterior(2) 6-3-9 to 10-8-6, Interior(1) 10-8-6 to 12-2-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

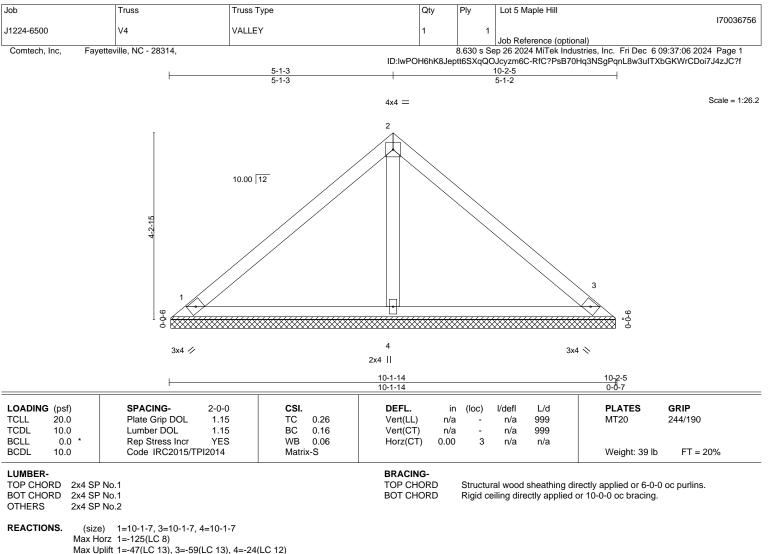
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=192, 6=192.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





Max Grav 1=201(LC 1), 3=201(LC 1), 4=350(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

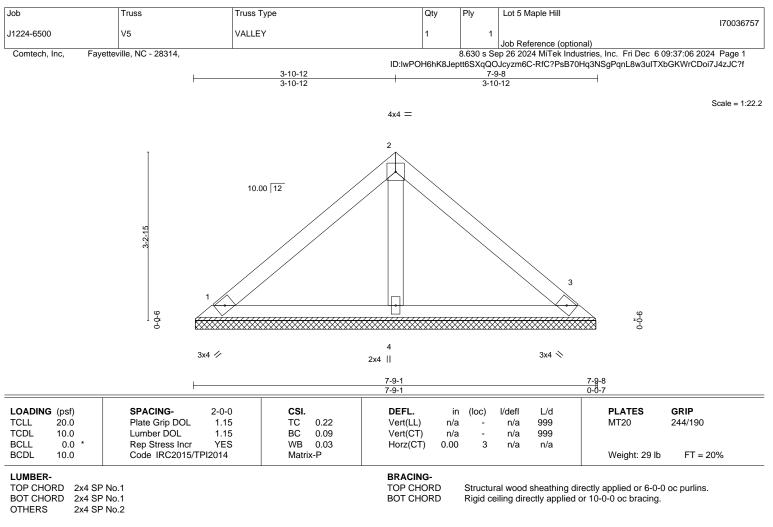
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



REACTIONS. (size) 1=7-8-10, 3=7-8-10, 4=7-8-10 Max Horz 1=-93(LC 8) Max Uplift 1=-47(LC 13), 3=-55(LC 13) Max Grav 1=162(LC 1), 3=162(LC 1), 4=236(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

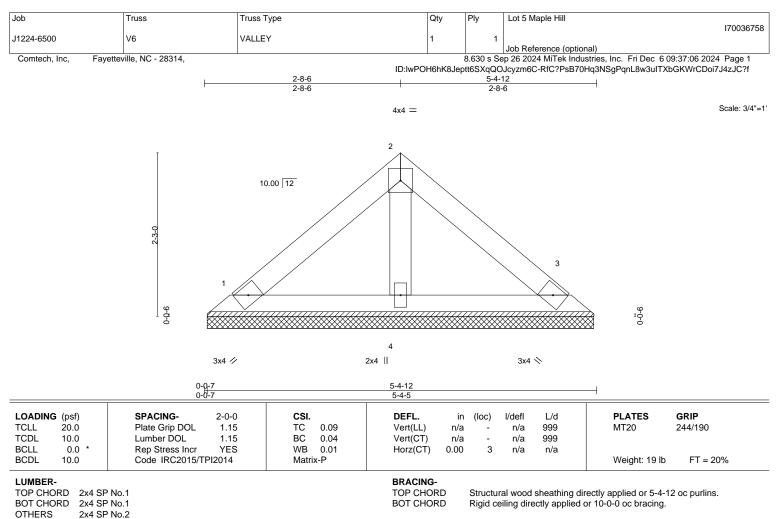
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

A MITEK Affiliat 818 Soundside Road

Edenton, NC 27932



01112110

REACTIONS. (size) 1=5-3-13, 3=5-3-13, 4=5-3-13 Max Horz 1=-61(LC 8) Max Uplift 1=-31(LC 13), 3=-36(LC 13) Max Grav 1=106(LC 1), 3=106(LC 1), 4=155(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

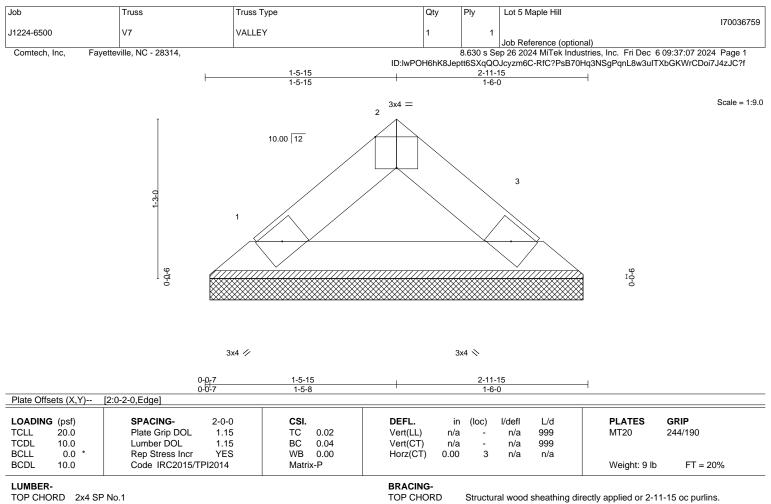
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. 1=2-11-0, 3=2-11-0 (size) Max Horz 1=29(LC 11) Max Uplift 1=-14(LC 12), 3=-14(LC 13) Max Grav 1=88(LC 1), 3=88(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

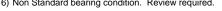
2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

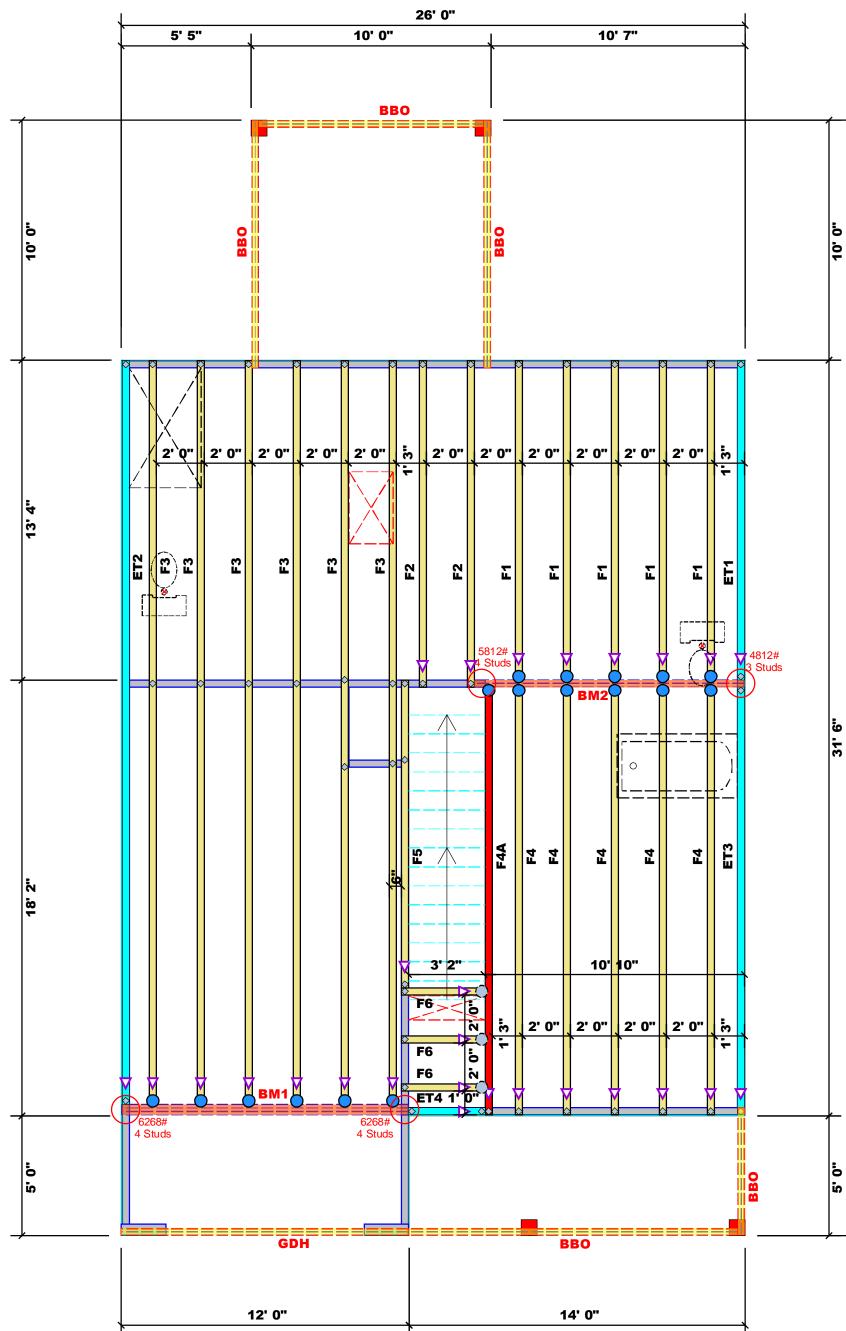
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) Non Standard bearing condition. Review required.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design that the operating of the second se and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes
1 Plumbing drop locations shown are NOT exa

- Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop
- locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 24"oc.
- **Dimension Notes**

 All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend
Second Floor Walls
Tray Ceiling
Box Storage
Flush Beam
Drop Beam

	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	17	NA	16d/3-1/2"	16d/3-1/2"
\bigcirc	MSH422	USP	3	Varies	10d/3"	10d/3"

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH	12' 0"	2x12 SPF No.1	2	2	FF

▲= Denotes Left End of Truss

(Reference Engineered Truss Drawing)

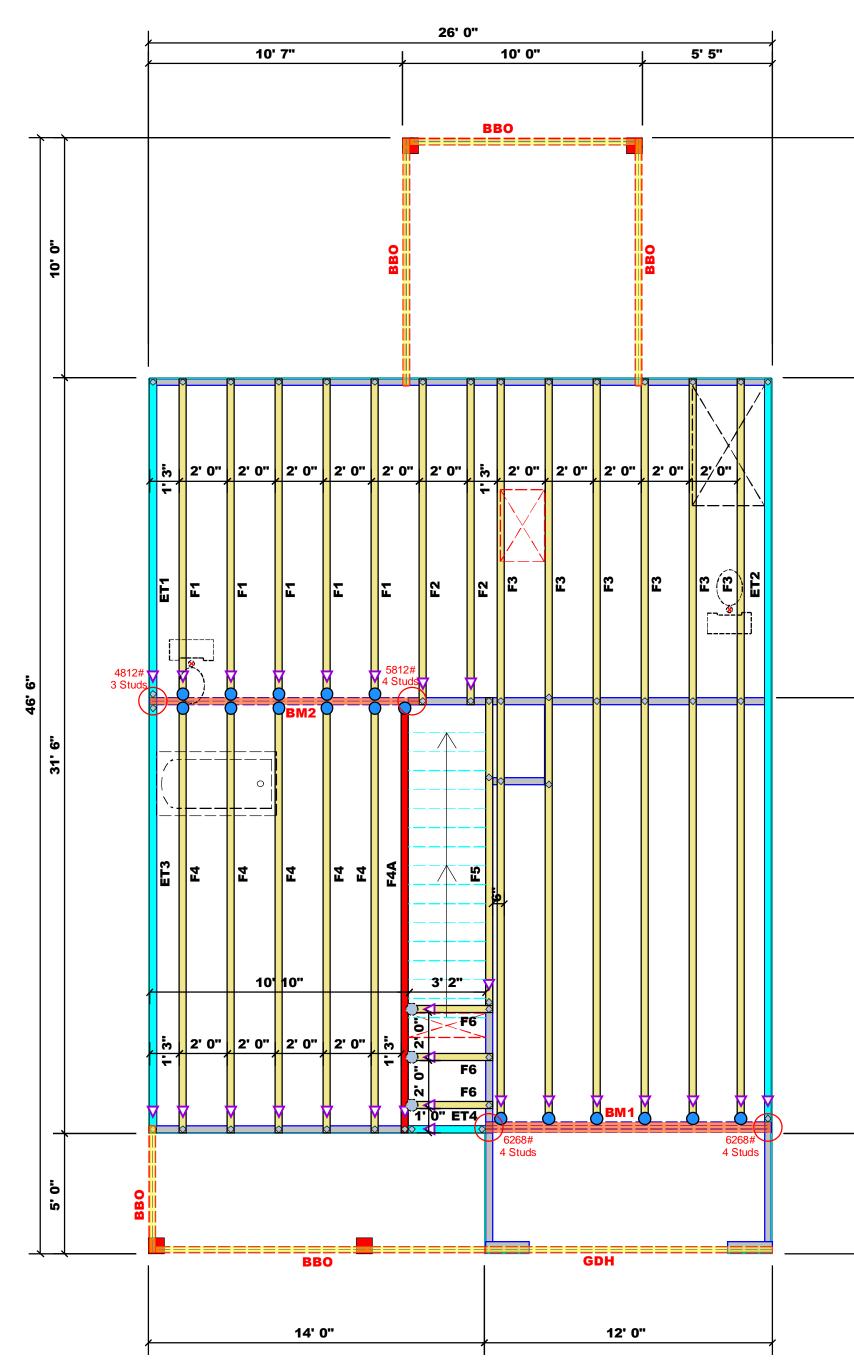
1) Truss Placement Plan Scale: 1/4"=1'

These compo design See ind identifi design permar for the suppor and co design consul	BUILDER	Weaver Homes	CITY / CO .	CITY / CO . Lillington / Harnett	Bearing deemed requirer attacher reaction 15000#. retainec reaction Tables. retainec reaction Signatu
S A TRUSS trusses an nents to b at the sp dividual d ed on the ed on the er is resp nent braci overall st t structur lumns is is er. For ge t BCSI-B1 lelivery pa	JOB NAME	Lot 5 Maple Hill	ADDRESS	4166 Darroch Road	Reilly R Fayet Phon Fax: reactions to compl ments. The d Tables (ments) to d number is greater A register that excc A register that exc A reg
e design be incorp ecification esign she placeme onsible for ng of the ructure. e includir the respon neral gui and BCS	PLAN	Magnolia "C" / GLF, CP	MODEL	Floor	OF & SES coad In teville e: (910) (9
ed as ind orated int n of the b tets for ea nt drawin or tempor roof and The desig ng header nsibility of dance reg B-B3 prov	SEAL DATE N/A	N/A	DATE REV.	12/06/24	hdustr a N.C. b) 864-4 864-4 n or equa a prescriptor shall r from the g be the ministree be the ministree be the ministree both system both system bo
ividual bu o the buil uilding do och truss g. The bu ary and floor sys n of the to s, beams of the buil parding br ided with	QUOTE #		DRAWN BY	Jonathan Landry	OOF EAN ial Par 28309 -8787 444 I to 3000¢ inum foo uired to st toreater to imum for an ed in the a ional shall more for all Land CK STU i 4 (b)) 0 @ EA END
ilding ding esigner. design ilding tem and russ , walls, ding acing, the	JOB #	J1224-6501	SALES REP.	SALES REP. Lenny Norris	AS k k k k k k k k k k k k k

lio

31.

10



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

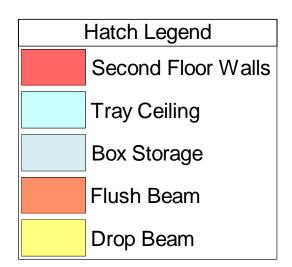
All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

hown are NOT exact.
hown are NOT exact. umbing drop
loor Trusses.
not to exceed 24"oc.

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	17	NA	16d/3-1/2"	16d/3-1/2"
\bigcirc	MSH422	USP	3	Varies	10d/3"	10d/3"

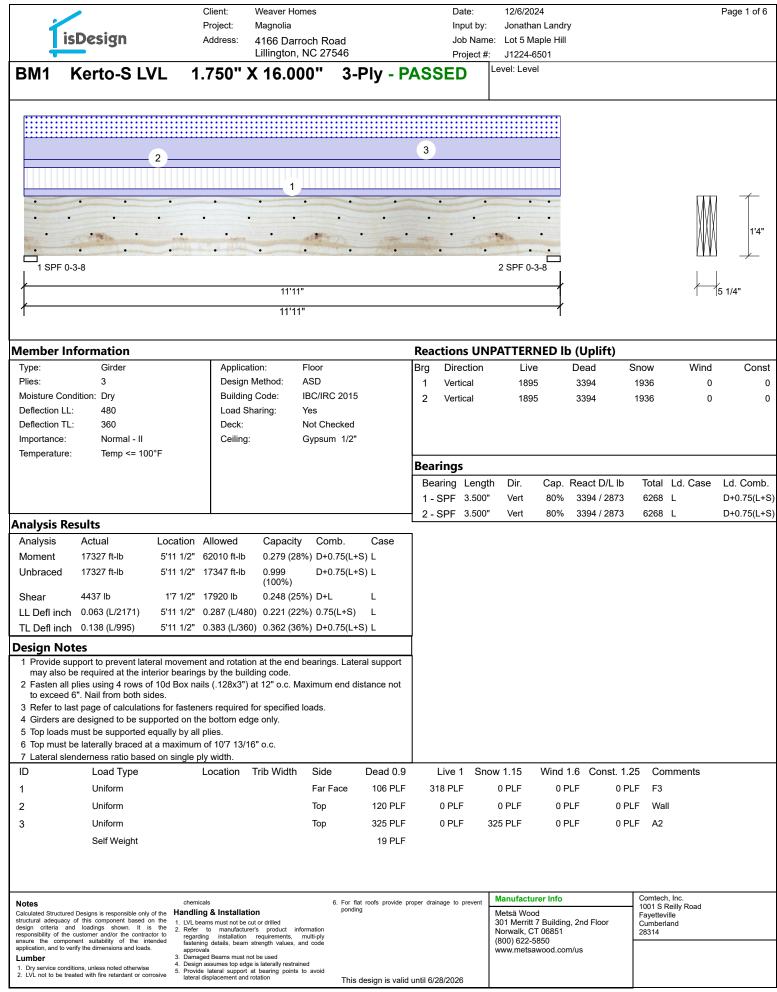
		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH	12' 0"	2x12 SPF No.1	2	2	FF

1) Truss Placement Plan Scale: 1/4"=1'

	co RO(Te			
	RUS eilly R Fayet Phon	SES load Ir teville e: (910	& B	EAN ial Par 28309 -8787	∕IS ⁺k	
deemed requirem attached requirem size and reaction 15000# retained reaction Tables retained	Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.					
LOA	AD CHA	ART FO		CK STU	<i>.</i>	
(0L d() 3400	1700 1 2550 1 3400 1 3400 2 5100 2 6800 2					
5100 6800 8500 10200 11900 13600 15300	3 4 5 6 7 8 9	7650 10200 12750 15300	04 05	1020 1360 1700	00 4	
CITY / CO . Lillington / Harnett	4166 Darroch Road	Floor	12/06/24	DRAWN BY Jonathan Landry	SALES REP. Lenny Norris	
CITY / CO .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP	
Weaver Homes	Lot 5 Maple Hill	Magnolia "C" / GLF, CP	N/A		J1224-6501	
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #	
These t compor design See ind identifie designe perman for the support and col designe consult	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com					

▲= Denotes Left End of Truss

(Reference Engineered Truss Drawing)

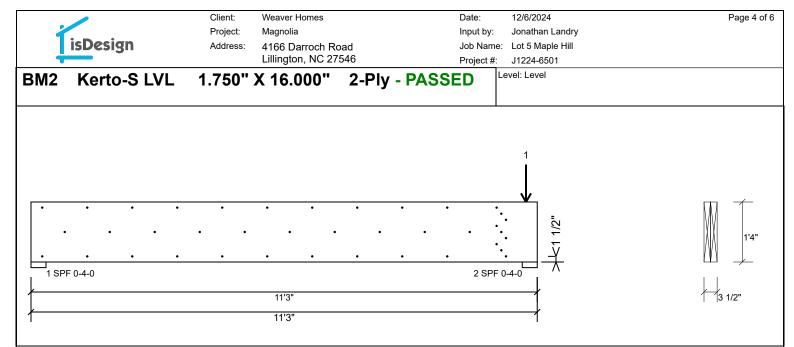


Project: Address: BM1 Kerto-S LVL 1.750" BM1 Kerto-S LVL 1.750" BM1 Fertility BM1 Kerto-S LVL 1.750 BM1 Fertility BM1 Fertility	Magnolia 4166 Darroch Road Lillington, NC 27546 * X 16.000" 3-Ply •	Projec - PASSED	Name: Lot 5 Maple Hill act #: J1224-6501 Level: Level 2 SPF 0-3-8	$ \begin{array}{c} $
BM1 Kerto-S LVL 1.750" Image: Second system Image: Second system 1 SPF 0-3-8 Image: Second system Image: Second system Multi-Ply Analysis Fasten all plies using 4 rows of 10d Box nails 6". Capacity 86.3 %	Lillington, NC 27546 X 16.000" 3-Ply 	Projec - PASSED	2 SPF 0-3-8	
Image: Spr 0-3-8 Image: Spr 0-3-8 <th>X 16.000" 3-Ply</th> <th>- PASSED</th> <th>Level: Level</th> <th></th>	X 16.000" 3-Ply	- PASSED	Level: Level	
Multi-Ply Analysis Fasten all plies using 4 rows of 10d Box nails 6". Capacity 86.3 %	- • • • • • • • • • • • • • • • •	· · · · · · · · · · · · · · · · · · ·	2 SPF 0-3-8	
Multi-Ply Analysis Fasten all plies using 4 rows of 10d Box nails 6". Capacity 86.3 %		· · · · · · · · · · · · · · · · · · ·	2 SPF 0-3-8	
Fasten all plies using 4 rows of 10d Box nails 6". Capacity 86.3 %				
Fasten all plies using 4 rows of 10d Box nails 6". Capacity 86.3 %				
6". Capacity 86.3 %	100 000 1000 NI 11 C			
Capacity 86.3 %	s (.128x3") at 12" o.c Nail fro	om both sides. M	Aaximum end distance	not to exceed
load 282 / PLE				
Yield Limit per Foot327.4 PLFYield Limit per Fastener81.9 lb.				
Cm 1				
Yield Mode IV				
Edge Distance 1 1/2"				
Min. End Distance 3"				
Load Combination D+L				
Duration Factor 1.00				

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc.
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	 I VI beams must not be cut or drilled 	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	1001 S Reilly Road Fayetteville Cumberland 28314

Ispecien Advert Isite Damoch Road Hullington X27568 How Nume: Lot Studye Hill Program BM2 Kerto-S LVL 1.750" X 16.000" 2-PIy - PASSED State State
BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED Lowit Lowal Image: Lowal Lowa
Image: Angle and Angle
Image: Control of the second
Image: Control of the second
Image: Set of the set
Image: Second and the interformation is in addition is in
Image: Production of the second sec
Image: Production of the second sec
Hember Information Type: 2 Original Construction Application: Floor Beagin Method: Application: Ploor Plote: 2 Delegin Method: Application: No Delegin Method: Application: Delegin Method: Application: No Delegin Method: Application: Delegin Method:
Hember Information Type: 2 Original Construction Application: Floor Beagin Method: Application: Ploor Plote: 2 Delegin Method: Application: No Delegin Method: Application: Delegin Method: Application: No Delegin Method: Application: Delegin Method:
Benchican Universities Reactions UNPATTERNED Ib (Uplift) Type:: Circle Paglication: Place Design Method: ASD Mosture Contilion: Dry Design Method: ASD Duration: No 0 0 Defection L1:: 300 Design Method: ASD Duration: No 0 0 Defection T1:: 300 Deck: No Checked Deck: No Checked Paglication: No Deck: No Checked Deck: No Checked Deck: No Checked Deck: No Checked Decked
Type: Girder Application: Floor Big Direction Live Dead Snow Wind Construction Ples: 2 Design Method: ASD Building Code:: Big Direction Live To assist 1257 0 0 Deflection LL: 430 Design Method: ASD Building Code:: Big Direction Live train 3355 1567 0 0 Deflection LL: 430 Deflection T: No (Decked Caling:: Cypsum 1/2" Eearings Bearing Length Direction Allowed Capacity Comb. Case Location Allowed Capacity Comb. Case Analysis Actual Location Allowed Capacity Comb. Case Det. L Location Allowed Capacity Comb. Case Unbraced 12309 ft-b 57 1/2" 236 (400) 0.346 (305) C1" 2.5FF 4.000" Vent 81% 1507 / 4.805 5812 L Det. 11 Unbraced 1230 ft-b 57 1/2" 236 (400) 0.346 (305) 1.12 C.5FF 4.000
Prines 2 Design Method: ASD Design Method: ASD Moisture Condition: Provide Status Moisture Condition: Status <
Mosture Condition: Dry Deflection L1: 480 Deflection L2: 480 Importance: Normal - II Temperature: Temp <= 100°F
Deletion LL: 490 Deletion LL: And 300 Deletion LL: Load Sharing: Bearing No Deck: Not Not Checked Deck: Normal - II Bearing Deck: Not Not Checked Deck: Normal - II Bearing Deck: Not Not Normal Deck: Not Normal Analysis Actual Location Allowed Capacity Comb. Case Normal Deck: Not Normal Deck: Deck:<
Importance: Normal - II Ceiling: Gypsum 1/2" Bearings: Bearings: Bearing Length Dir. Cap. React D/L Ib Total Ld. Case Ld. Comit 1SPF 4.000° Vert 81% 12577.3565 4812 L D+L Analysis Actual Location Allowed Capacity Comb. Case Moment 12309 R-Ib 57 1/2" 34565 R-Ib 0.356 (6%) D+L L Unbraced 13% 11947 Ib 0.356 (6%) D+L L L Unbraced 2030 R-Ib 57 1/2" 2358 (J480) 0.348 (J580) D+L L Unbraced 13% 11947 Ib 0.356 (6%) D+L L L L Detail Detail 0.000 (J1220) 57 1/2" 0.358 (J480) 0.348 (J480) 0.488 (J480) 0.4
Temperature: Temp <= 100°F Bearings Images in the intervence of the intervence
Bearings Bearings Bearings Comparison of the second DL bit is the seco
Bearing Length Dir. Cap. React DI. Lb Total Ld. Case Ld. Cam Analysis Actual Location Allowed Capacity Comb Case Moment 12309 R-lb 57 1/2 34565 ft/b 0.356 (36%) D+L L Unbraced 12309 R-lb 57 1/2 1230 ft/b 0.356 (36%) D+L L Unbraced 12309 R-lb 57 1/2 1230 ft/b 0.356 (36%) D+L L LL Deft Inch 0.098 (L/1320) 57 1/2 0.256 (L/360) 0.346 (36%) L L The Deft Inch 0.398 (L/360) 0.356 (36%) D+L L L Design Notes 1 1000 Str. 122 (L975) 57 1/2 0.356 (L/360) 0.346 (36%) L L 1 Contrast Line Torong Line Line Line Line Line Line Line Line
Image: sevents 1 - SPF 4.000° Vert 81% 1257/3555 4812 L D+L Analysis Actual Location Allowed Capacity Comb. Case Moment 12309 ft-lb 57 1/2 3456 ft-lb 0.356 (36%) D+L L Unbraced 1000° Vert 96% 1507 / 4305 5812 L D+L Unbraced 12309 ft-lb 57 1/2 3456 (14%) 0.337 (36%) D+L L L L L L Dett 1000° Vert 96% 1507 / 4305 5812 L D+L Unbraced 12309 ft-lb 57 1/2 0.358 (1/360) 0.344 (36%) L L L Dett D+L
2 - SPF 4.00° Vert 98% 1507/4305 5812 L D+L Analysis Actual Location Allowed Capacity Comb. Case Moment 12309 ft-lb 57 1/2° 34565 ft-lb 0.356 (36%) D+L L Unbraced 12309 ft-lb 57 1/2° 12310 ft-lb 1.000 D+L L (100%) D+L L LL Deft inch 0.098 (J/1320) 57 1/2° 0.268 (J/480) 0.369 (37%) D+L L Shear 4527 lb 11° 11947 lb 0.379 (38%) D+L L LL Deft inch 0.132 (J/37) 57 1/2° 0.268 (J/480) 0.369 (37%) D+L L Design Notes 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 1 2 Concentrated load fisteners specification is in a features required for specified loads. 4 4 Concentrated load fisteners specification is in and restateners if a hanger is present. 5 5 Girders are designed to be supported on the bottom edge onty. 5 1 Duiform Far Face 90 PLF 0 PLF 0 PLF 0 PLF 0 PLF 2 Uniform Near Face 120 PLF 0 PLF 0 PLF 0 PLF 0 PLF Fat Mining & Statulation hanger is prevised in the end the end the many is a statulation hanger is prevised in the end bearings. Lateral support 0 ib 0 ib 0 ib F4A 10 Lateral stenders designed to be suported on the bottom edge ontyl. 12 PLF </td
analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 12309 ft-lb 57 1/2" 3465 ft-lb 0.356 (38%) D+L L Unbraced 12309 ft-lb 57 1/2" 12310 ft-lb 1000 D+L L LL Deflinch 0.098 (U1320) 57 112" 0.258 (U480) 0.364 (38%) L L TL Deflinch 0.132 (U375) 57 112" 0.258 (U480) 0.384 (38%) D+L L Motes Thread of the function of the function bearings by the building code. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support nay also be required at the interior bearings by the building code. Provide support to prevent lateral movement and rotation to hanger fasteners if a hanger is present. Set of the location on single ply width. Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments Constant delad fastener specifications is in addition to hanger fasteners if a hanger is present. 1 Uniform Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Location Trib Width Side Dead 0.9
Analysis Actual Location Allowed Capacity Comb Case Moment 12309 ft-lb 57 1/2' 128 456 ft-lb 0.356 (36%) D-L L Unbraced 12309 ft-lb 57 1/2' 128 '104 ft-lb 0.379 (38%) D-L L Lib Defl inch 0.98 (L/1320) 57 7 1/2' 0.286 (2480) 0.364 (36%) L L TL Defl inch 0.98 (L/1320) 57 7 1/2' 0.288 (L/480) 0.364 (36%) L L The Defl inch 0.98 (L/1320) 57 7 1/2' 0.288 (L/480) 0.364 (36%) L L The Defl inch 0.98 (L/1320) 57 7 1/2' 0.288 (L/480) 0.364 (36%) L L The Defl inch 0.98 (L/1320) 57 7 1/2' 0.288 (L/480) 0.364 (36%) L L The Defl inch 0.132 (L/975) 57 7 1/2' 0.288 (L/480) 0.364 (36%) L L The Provide support to prevent lateral movement and rotation at the end bearings. Lateral support 12 Value Second ft-100 (Second ft-100 (Sec
Moment 12309 ft-lb 57 1/2 34565 ft-lb 0.356 (36%) D+L L Unbraced 12309 ft-lb 57 1/2 12310 ft-lb 1,000 D+L L Shear 4527 lb 138* 11947 lb 0.379 (38%) D+L L L LLD defineh 0.038 (L/1320) 57 1/2* 0.258 (L/360) 0.368 (37%) D+L L Design Notes
Unbraced 12309 ft-lb 57 1/2 12310 ft-lb 1000 D+L L (100%) Shear 4527 lb 178 11947 lb 0.378 (28%) D+L L LL Defl inch 0.988 (L/1320) 57 1/2 0.268 (L/480) 0.364 (36%) L L T. Defl inch 0.132 (L/975) 57 1/2 0.268 (L/480) 0.364 (36%) D+L L Design Notes I Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the inferior bearings by the building code. 2 Fasten all plies using 3 rows of 10d Box nalls (.128x3') at 12' o.c. Maximum end distance not to exceed 0'. 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fasteners secification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16' o.c. 7 Lateral slenderness ratio based on single ply width. 1 Uniform 1 Uniform 1 Uniform 2 optimit be 110-0 Near Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated based on single ply width. 1 Uniform 4 Optimit 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb 0 lb 74A 5 eff Weight 12 PLF 4 For fasteners ratio based on single ply width. 4 Denit 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb 74A 5 eff Weight 12 PLF 4 For fasteners fraction provide proper drainage b prever Manual distance for the other provide proper drainage b prever Manual distance for the other provide proper drainage b prever 4 Prestar face 12 PLF 4 Prestar fac
(100%) Shear 4527 lb 18' 11947 lb 0.379 (38%) D+L L Lip Deflinch 0.098 (L/1320) 57 1/2' 0.268 (L/480) 0.364 (38%) L L Tip Deflinch 0.132 (L/975) 57 1/2'' 0.368 (37%) D+L L Design Notes
LLDefl inch 0.098 (L/1320) 57 1/2* 0.268 (L/480) 0.364 (38%) L L TL Defl inch 0.132 (L/975) 57 1/2* 0.258 (L/360) 0.369 (37%) D+L L Design Notes 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 Fasten all plies using 3 rows of 10 Box nails (.128x3*) at 12* o.c. Maximum end distance not to exceed 6 ¹ . 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Torp must be laterally braced at a maximum of 99 3/16* o.c. 7 Lateral stenderness ratio based on single ply width. 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F1 3 Point 11-0-0 Near Face 250 Ib 750 Ib 0 Ib 0 Ib 0 Ib F4A Self Weight 1 10* 12 PLF 1 Purcent Manual Construction of the ordination of a direct of the method provide support direct of the ordination of the addition of a direct of the method 1 Point 11-0-0 Near Face 250 Ib 750 Ib 0 Ib 0 Ib 0 Ib F4A Self Weight 1 2 PLF
TL Deflinch 0.132 (L/975) 57 1/2" 0.368 (L/360) 0.369 (37%) D+L L Design Notes
Design Notes 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 2 Fasten all piles using 3 rows of 10d Box nails (128x3)" at 12" o.c. Maximum end distance not to exceed 6". 3 3 Refer to last page of calculations for fasteners required for specified loads. 4 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 5 Girders are designed to be supported on the bottom edge only. 6 6 Top must be laterally braced at a maximum of 99 3/16" o.c. 7 7 Lateral slenderness ratio based on single pty width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 12 PLF 0 Fb 0 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 Ib 750 Ib 0 Ib 0 Ib F4A Self Weight 12 PLF F1 Defad Sem
1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 Fastern all piles using 3 rows of 100 Box nails (.128x3") at 12" o.c. Maximum end distance not to exoceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16" o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb F4A Self Weight 12 PLF 12 PLF 12 PLF Manufacturer Info 101 S Reily Road 101 S Reily Road 201 Merrit 7 Suilding, 2nd Floor 101 S Reily Road 201 Merrit 7 Suilding, 2nd Floor
may also be required at the interior bearings by the building code. 2 Faster all plies using 3 rows of 10d Box nails (128x3") at 12" o.c. Maximum end distance not to exceede". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16" o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform 2 Uniform 3 Point 1 10-0 Near Face 2 50 lb 750 lb 0 lb 0 lb 0 lb 0 lb 0 lb 750 lb 0 lb 0 lb 0 lb 750 lb 0 lb
2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16" o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform I10-0 Near Face 121 PLF 363 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 Ib 750 Ib 0 Ib 0 Ib 0 Ib 0 Ib F4A Self Weight 12 PLF
to exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 9'9 3/16" o.c. 7 Lateral silenderness ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Par Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F4 56 OPLF 750 Ib 0 Ib 0 Ib 0 Ib 750 Ib 0 Ib 750 Ib 0 Ib 0 Ib 750 Ib 0 Ib 750 Ib 0 Ib 0 Ib 750 Ib 0 Ib
 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16° o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Location Trib Width Side Pear Face 90 PLF 269 PLF 0 PLF
present. 5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 99 3/16° o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Location 1 Uniform 2 Uniform 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 12 PLF -<
5 Girders are designed to be supported on the bottom edge only. 6 Top must be laterally braced at a maximum of 9'9 3/16" o.c. 7 Lateral slendermess ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb 0 lb F4A Self Weight 12 PLF Neter Structure Designs is responsible only of the intension of the manual curver information approvals. Comment subally of the intension and loss. The intension of the curver information and to verify the dimensions and based. Lumber 1. Un verify the intension and based. Lumber 1. Unifers mark to the curver of the intension and based. 1. Uniform Structure Designs is responsible only of the intension and based. Lumber 1. Design assumes to get gis laterally restrained 1. Uniform Structure Designs is responsible only of the intension and be curver intension and be curver of the curver intension and based. Lumber 1. Uniform Structure Designs is responsible only of the intension and be curver intension and be curver intension be used. 1. Dry watche conflictors unders and curver intension and be curver intension and be curver intension and be used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the structure and curver intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and bus used. 1. Dry watche conflictors and the intension and the intension and bus used. 1. Dry watche intension and the intension watched interview. 1. Dry watche conflictors and the intension watched interview. 1. Dry
7 Lateral slenderness ratio based on single ply width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 12 PLF 6. For flat roots provide proper drainage to prover Manufacturer Info 101 S Reilly Road Fayetterville 101 S Reilly Road Fayetterville Commetch, Inc. 101 S Reilly Road Fayetterville Commetch, Inc. 101 S Reilly Road Fayetterville Commetch, Inc. 101 S Rei
ID Locad Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF 0 PLF 12 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF 74 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb 10 b 74A Self Weight 12 PLF 0 Face 6. For flat roofs provide proper drainage to prevent product information reporting addition show at beard or drilled Manufacturer Info Comtech, Inc. 1001 S Reilly Road Face 28314 1. Ub beams must not be cut or drilled 1. Worker to answerth contractor to regarding installation requirements, multipopt approvals 6. For flat roofs provide proper drainage to prevent (800 dign) (622-5850 www.metsawood.com/us 201 Metril 7 Building, 2nd Floor Norwark, CT 06851 (800) (622-5850 www.metsawood.com/us 28314 2. Unit to the tradead with fire drandard or coursele 10 being asumestop degie is laterally re
1 Uniform Far Face 90 PLF 269 PLF 0 PLF 0 PLF 0 PLF F1 2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 12 PLF 12 PLF 12 PLF 12 PLF 101 S Reilly Road And the customer andor the contrast must not be cut of mensions and loadings shown. It is the responsibility of the intended sprilotation, and to verify the dimensions and loadings chemicals 6. For flat roofs provide proper drainage to prever Manufacturer Info Comtech, Inc. 1. Uny service conditions, unless noted otherwise Numan facturers product information requirements, multiphy appril atilation requirements, multiphy appril installation requirements, multiphy appril installation requirements, multiphy appril installation requirements, multiphy appril installation stop edge is laterally restrained 6. Ser flat roofs provide proper drainage to prever Manufacturer Info Comtech, Inc. 1001 S Reilly Road 1. Uny to the customer and roof the customer and protect cord merge is laterally restrained 9. Ser flat roofs provide proper drainage to prever Mergits 250 Wood 301 Merrits 7 Building, 2nd Floor Norwalk, CT 06851
2 Uniform Near Face 121 PLF 363 PLF 0 PLF 0 PLF 0 PLF F4 3 Point 11-0-0 Near Face 250 lb 750 lb 0 lb 0 lb 0 lb F4A Self Weight 12 PLF 12 PLF 12 PLF Manufacturer Info Comtech, Inc. Notes Calculated Structured Designs is responsible only of the interdage indensions and loadings shown. It is the sponsability of the customer addor the contomer addor the
And the self Weight 11-0-0 Near Face 250 lb 750 lb 0 lb
Self Weight 12 PLF Notes Chemicals Chemicals Comtech, Inc. Calculated Structured Designs is responsible only of the segonsibility of the customer and/or the control or and/or the order and/or the control or and/or the related with the fraged with the fraged with the fraged with the related and the relative segonsibility of the intended application, and to verify the dimensions and loads. Chemicals 6. For flat roofs provide proper drainage to prover Manufacturer Info Comtech, Inc. 1. Ury service conditions, unless noted otherwise papication, and to verify the dimensions and loads. 1. Where the related with the relatived and interguing installation requirements, multi-points to avoid 6. For flat roofs provide proper drainage to prove for points or avoid Metsä Wood 301 Merritit 7 Building, 2nd Floor Norwalk, ICT 06851 (800) 622-5850 www.metsawood.com/us Comtech, Inc. 1. Ury pervice conditions, unless noted otherwise 2. Uvi not to be trafed with the relatived mit the relatived provide is laterally restarted. 0. Damaged Beams must not be used Metsä Wood 301 Merritit 7 Building, 2nd Floor Norwalk, ICT 06851 (800) 622-5850 www.metsawood.com/us Comtech, Inc.
Notes chemicals 6. For flat roofs provide proper drainage to prevent ponding Manufacturer Info Comtech, Inc. Calculated Structured Designs is responsible only of the segnoriteria and loadings shown. It is the segnoriteria and loadings shown. It is the segnoritieria and loadings shown shown. It is the segnoritieria and loadings shown shown. It is the segnoritieria and loadings shown shown and loadings shown shown and loadings shown at hearing points to avoid Manufacturer Info Comtech, Inc. Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us 1. Dys service conditions, unless noted otherwise 2. Uving the the tradel with fire relarded not the relarded with fire relarded not the relarded with fire relarded not the relarged with fire relarded not the relarged with the relared with the shown asuport at hearing points to avoid
Notes Chemicals Chemicals Chemicals Chemicals Chemicals Design sprovide proper drainage to provide proper drainage to provide Metsä Wood 1001 S Reilly Road Calculated Structured Designs is responsible only of the score and/or the contractor to esponsibility of the customer and/or the contractor to seponsibility of the dimensions and loads. I. UL beams must not be cut or drilled 1. UL beams must not be cut or drilled 301 Merritt 7 Building, 2nd Floor Yourwalk, CT 06851 2 Refer to serify addition sublex on to be used 3. Damaged Beams must not be used 8. Design assumes to p edge is laterally restrained 9. Down all kerail support at bearing points to avoid 9. Damaged Beams must not be used 9. Down all kerail support at bearing points to avoid 9. Damaged Beams must not be used 9. Design assumes top edge is laterally restrained 9. Design assumes top
Notes Chemicals Chemicals Chemicals Chemicals Chemicals Design sprovide proper drainage to provide proper drainage to provide Metsä Wood 1001 S Reilly Road Calculated Structured Designs is responsible only of the score and/or the contractor to esponsibility of the customer and/or the contractor to seponsibility of the dimensions and loads. I. UL beams must not be cut or drilled 1. UL beams must not be cut or drilled 301 Merritt 7 Building, 2nd Floor Yourwalk, CT 06851 2 Refer to serify addition sublex on to be used 3. Damaged Beams must not be used 8. Design assumes to p edge is laterally restrained 9. Down all kerail support at bearing points to avoid 9. Damaged Beams must not be used 9. Down all kerail support at bearing points to avoid 9. Damaged Beams must not be used 9. Design assumes top edge is laterally restrained 9. Design assumes top
Laculates Structured Designs is responsible only of the function get instantiation Fagettaville Fagettaville Lumber 1. Dry service conditions, unless noted otherwise 3. Damaged Beams must not be used 1. Design assumes to pedge is laterally restrained 1. Dry service conditions, unless noted otherwise 1. Dry service conditions, unless noted otherwise Design assumes to pedge is laterally restrained
Jumpber 2. Refer to manufacturer's product information Norwalk, CT 06851 28314 Production and to verify the dimensions and loads astening details, beam strength values, and code (800) 622-5850 www.metsawood.com/us Lumber 3. Damaged Beams must not be used besign assumes top edge is laterally restrained besign assumes top edge is laterally restrained Provide lateral support at bearing points to avoid
application, and to verify the dimensions and loads. approvals approvals www.mets3awood.com/us Lumber 3. Damaged Beams must not be used Design assume top edge is laterally restrained 1. Dry service conditions, unless noted otherwise 4. Design assume top edge is laterally restrained 5. Provide lateral support at bearing points to avoid
1. Dry service conditions, unless noted otherwise 5. Provide laterally restrained 5. Provide lateral support at bearing points to avoid

Version 23.40.705 Powered by iStruct[™] Dataset: 24070801.3993



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6".

Capacity	98.6 %	
Load	242.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination	D+L	
Duration Factor	1.00	

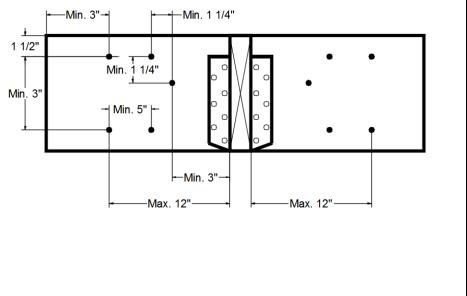
Concentrated Load

Fasten at concentrated side load at 11-0-0 with a minimum of (9) – 10d Box nails (.128x3") in the

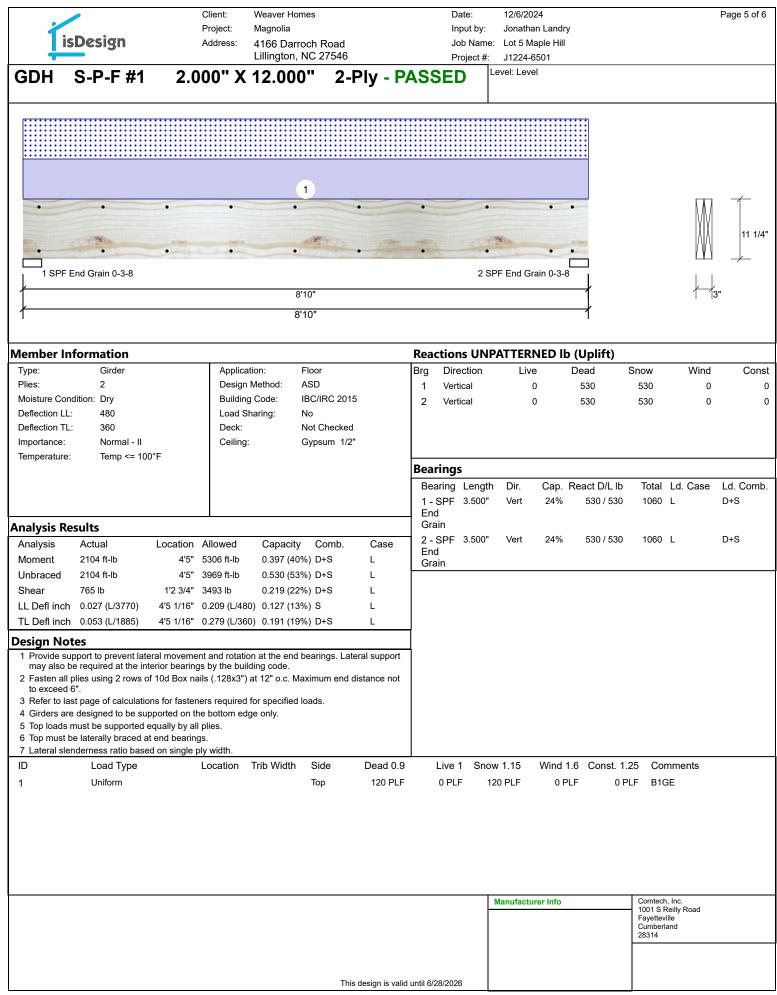
pattern shown.

P	
Capacity	67.9 %
Load	500.0lb.
Total Yield Limit	736.5 lb.
Cg См	0.9998
См	1
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Load Combination	D+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S Reilly Road
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	1. IVI beams must not be cut or drilled	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Fayetteville Cumberland 28314



		Client:	Weaver Homes		Date:		Page 6 of 6
1	icDecign	Project:	Magnolia		Input I		
	isDesign	Address:	4166 Darroch R			ame: Lot 5 Maple Hill	
			Lillington, NC 2		Projec		
GDH	S-P-F #1	2.000" X	12.000"	2-Ply - F	PASSED	Level: Level	
				-			
						·	
•	•	• •	•	•	•	• • •	
						1/2"	MM
							11 1/4"
•	•	• •	•	•	•	• • • Y	- ////
1 SP	F End Grain 0-3-8					2 SPF End Grain 0-3-8	
			8'10"				1-13"
∤			8'10"				
			0.10			1	
Multi-Ply	y Analysis						
Fasten all	plies using 2 rows	s of 10d Box nails (.128x3") at 12"	o.c Maximun	n end distance	not to exceed 6".	
Capacity	1 5	0.0 %					
Load		0.0 PLF					
Yield Limit pe	er Foot	157.4 PLF					
Yield Limit pe	er Fastener	78.7 lb.					
CM Viold Mode		1 IV					
Yield Mode Edge Distanc	`	1 1/2"					
Min. End Dist		3"					
Load Combin		0					
Duration Fact		1.00					
							-
						Manufacturer Info	Comtech, Inc. 1001 S Reilly Road
							Fayetteville Cumberland 28314
							Cumberland 28314



RE: J1224-6501 Lot 5 Maple Hill Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: Weaver HomesProject Name: J1224-6501Lot/Block: 5Model: MagnoliaAddress: 4166 Darroch RoadSubdivision: Maple HillCity: LillingtonState: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.6 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 11 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	170033970	ET1	12/6/2024
2	170033971	ET2	12/6/2024
3	170033972	ET3	12/6/2024
4	170033973	ET4	12/6/2024
5	170033974	F1	12/6/2024
6	170033975	F2	12/6/2024
7	170033976	F3	12/6/2024
8	170033977	F4	12/6/2024
9	170033978	F4A	12/6/2024
10	170033979	F5	12/6/2024
11	170033980	F6	12/6/2024

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		Truss Type		Qty	Ply	Lot 5 Maple Hill			1700	
224-6501	ET1	GABLE		1	1				1700	03397
		GABEE				Job Reference (c				
comtech, Inc, Fayettev	ville, NC - 28314,					ep 26 2024 MiTek				
			I	D:lwPOH6hK8	Jeptt6SXqQ0	OJcyzm6C-RfC?Ps	B70Hq3NSgP	qnL8w3ulTXbGl		
									0 ₁ 1	1 ₁ 8
									Scale =	= 1:22
3x4			0.4 -							
1 2	3	4	$5^{3x4} = 6$	-	7	8	9	10	11	
64 64	•	•			•	0	0	•		2
22 21	20	19	18 17		<u>*************************************</u>	**************************************	14	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	12	
22 21 3x4	20	19			10	15	14	13		
5.4 11			3x4 =	-					3x4 =	_
, 1-4-0 _i	<u>2-8-0</u> 4- 1-4-0 1-	<u>-0-0 5-4-0</u> 4-0 1-4-0	<u> </u>	<u>8-0-0</u> 1-4-0		-0 10-8 -0 1-4-	3-0	<u>12-0-0</u>	<u>13-3-8</u> 1-3-8	

LOADING (psf) TCLL 40.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00	CSI. TC 0.06 BC 0.01	Vert(CT)	n/a n/a	oc) l/defl - n/a - n/a	L/d 999 999	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.03 Matrix-S	Horz(CT) 0	.00	12 n/a	n/a	Weight: 62 lb	FT = 20%F, 11%E
	P No.1(flat) P No.1(flat)		BRACING- TOP CHORD		ructural wood		ectly applied or 6-0-0) oc purlins,

WEBS 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 13-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

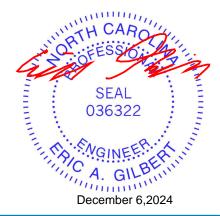
3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scietur Information**. Building from the Structure Building Component Advance interpretented and the properties of th and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job		Truss				Truss T	ype					Qty	/	Ply	Lot 5	Maple Hil	I						
																						1700	033971
J1224-6501		ET2				GABLE						1		1									
Oriente als Jure	F		2 0004	4										0.000 - 0		eference			F # D -	- 0.00.0	22.00.00	04 0	
Comtech, Inc,	Fayette	eville, ind	C - 2831	4,							ID:IV	vPOH6h				024 MiTe 6C-RfC?I							
											10.1		Robep	lioonqac	Jocyzin	00-110-1	307011	quitogi	queowo		I WIOD		
																						0-1-	8
																						Scale	= 1:51.8
																						Scale	- 1.51.0
3x4			3x4 =	=							3x6 FP	=						3x4 =					
1 2	3	4	5	6	7	8	9	10	11	12	13	14 15	16	17	18	19	20	21	22	23	24	25	
	-	-	R	-	-	9	8	-	-	9		-	8	0	e	-		Å	8	-	9		54 JC
1-4-0																							51 9
	XXXXXX	×××××		XXXXX	××××××	××××××	XXXXX	××××××	XXXXXX	XXXXX				×××××××		××××××	XXXXX	××××××		XXXXXX			1-
50 49	48	47	46	45	44	43	42	41	40	39	3837	36	35	34	33	32	31	30	29	28	27	26	
3x4				3x4 =						276	FP =						3x4 =	_				3x4 3	

 1-4-0
 2-8-0
 4-0-0
 5-4-0
 6-8-0
 9-4-0
 12-0-0
 13-4-0
 14-8-0
 16-0-0
 17-4-0
 18-8-0
 20-0-0
 21-4-0
 22-8-0
 24-0-0
 25-4-0
 26-8-0
 28-0-0
 29-4-0
 30-11-12

 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0
 1-4-0

				, <u></u>			j-	-, <u></u>]				
LOADING ((psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 4	40.0	Plate Grip DOL	1.00	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	-0.00	31	n/a	n/a		
BCDL	5.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 138 lb	FT = 20%F, 11%E
LUMBER-						BRACING-	•					
TOP CHOR	D 2x4 SF	P No.1(flat)				TOP CHOP	RD	Structu	iral wood	sheathing d	irectly applied or 6-0-0 o	oc purlins,
BOT CHOR	D 2x4 SF	P No.1(flat)						except	end verti	cals.		
WEBS	2x4 SF	P No.3(flat)				BOT CHOP	RD	Rigid c	eiling dire	ectly applied	or 6-0-0 oc bracing.	

REACTIONS. All bearings 30-11-12.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 50, 26, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scietur Information**. Building from the Structure Building Component Advance interpretented and the properties of th and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 5 Maple Hill	
			Qiy	FIY		170033972
J1224-6501	ET3	GABLE	1	1	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.630 s S	Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:26	2024 Page 1
		ID:IwP	OH6hK8Je	ptt6SXqQ0	OJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrC	
0- <mark>1</mark> -8						0- <mark>1-</mark> 8
						Scale = 1:29.6
		3x4 =	3x6 FP=			
1 2	3 4	5 6 7 8	9 10		11 12 13 14	15 16
						34 • • • • • • • • • • • • • • • • • • •
32 31	30 29	28 27 26 25 24	23		22 21 20 19	18 17
3x4 =		3x6 FP = 3x4 =				3x4 =
<u>1-4-0</u> 2-8 1-4-0 1-4 Plate Offsets (X,Y) [7:1		<u>6-8-0 8-0-0 9-4-0</u> 1-4-0 1-4-0 1-4-0	10-8-0 1-4-0	12-0-0 1-4-0	<u>13-4-0 14-8-0 16-0-0 17-4-0</u> 1-4-0 1-4-0 1-4-0 1-4-0	17-10-0 0-6-0

L OADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.06 BC 0.01 WB 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc n/a - n/a - 0.00 24	n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			f Il/a	n/a	Weight: 82 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI	P No.1(flat)	BRACING- TOP CHORD		tural wood		ectly applied or 6-0-0) oc purlins,	

2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. 2x4 SP No.3(flat)

REACTIONS.

All bearings 17-10-0.
 (lb) - Max Uplift All uplift 100 lb or less at joint(s) 17 Max Grav All reactions 250 lb or less at joint(s) 32, 31, 30, 29, 28, 26, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

WEBS

OTHERS

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17.

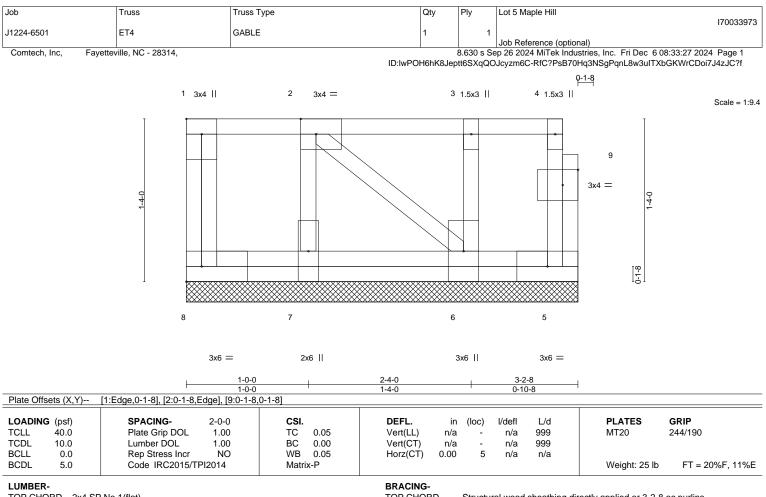
7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scietur Information**. Building from the Structure Building Component Advance interpretented and the properties of th and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



¹⁾ All plates are 1.5x3 MT20 unless otherwise indicated.



TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)OTHERS2x4 SP No.3(flat)

 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 3-2-8 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-2-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 4=-79 2=-72

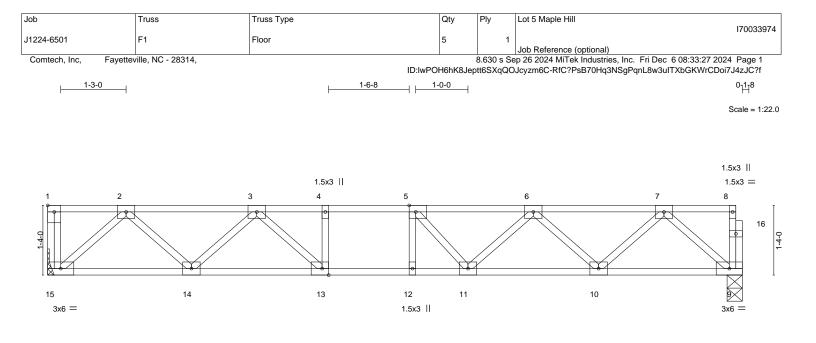


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road

Edenton, NC 27932



1			13-3-8			1
			13-3-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [13:0-1-4	3,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYES	CSI. TC 0.34 BC 0.60 WB 0.31	Vert(LL) -0.09	n (loc) l/defl L/d 9 11-12 >999 480 2 11-12 >999 360 3 9 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 71 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o) oc purlins,

REACTIONS. (size) 15=Mechanical, 9=0-3-8 Max Grav 15=717(LC 1), 9=711(LC 1)

Wax Grav 15=717(LC 1), 9=711(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1210/0, 3-4=-1904/0, 4-5=-1904/0, 5-6=-1819/0, 6-7=-1220/0

BOT CHORD 14-15=0/759, 13-14=0/1646, 12-13=0/1904, 11-12=0/1904, 10-11=0/1659, 9-10=0/754

WEBS 2-15=-1010/0, 2-14=0/628, 3-14=-606/0, 3-13=0/505, 7-9=-1001/0, 7-10=0/648,

6-10=-611/0, 6-11=0/305, 5-11=-304/60

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

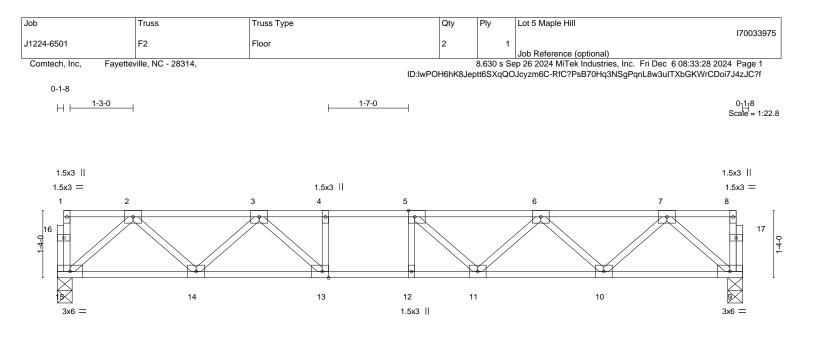
Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Affi 818 Soundside Road Edenton, NC 27932



L			13-7-0			
			13-7-0			1
Plate Offsets (X,Y)	[5:0-1-8,Edge], [13:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.39 BC 0.65 WB 0.32	Vert(LL) -0.10	n (loc) l/defl L/d 0 11-12 >999 480 4 11-12 >999 360 3 9 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	. (-)		Weight: 71 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	11	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	2 11) oc purlins,
REACTIONS. (size Max G	e) 15=0-3-8, 9=0-3-8 irav 15=727(LC 1), 9=727(LC 1)				Ŭ	

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1242/0, 3-4=-1987/0, 4-5=-1987/0, 5-6=-1882/0, 6-7=-1257/0

BOT CHORD 14-15=0/776, 13-14=0/1698, 12-13=0/1987, 11-12=0/1987, 10-11=0/1718, 9-10=0/770

WEBS 2-15=-1031/0, 2-14=0/649, 3-14=-634/0, 3-13=0/547, 7-9=-1022/0, 7-10=0/677,

6-10=-642/0, 6-11=0/302, 5-11=-320/48

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

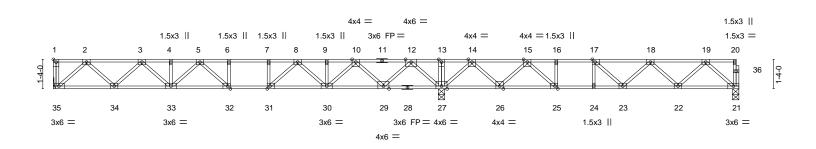


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 5 Maple Hill
					170033976
J1224-6501	F3	Floor	6	1	
					Job Reference (optional)
Comtech, Inc, Fayett	eville, NC - 28314,			8.630 s Se	ep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:28 2024 Page 1
		ID:IwPC	H6hK8Jep	tt6SXqQC	Jcyzm6C-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
1-3-0	<u>⊢_1-</u>	8-0			<u>1-6-12</u> 0- <u>1</u> -8
					Scale = 1:52.1



	17-6-8					30-11-		
Plate Offsets (X,	Y) [1:Edge,0-1-8], [17:0-1-8,Edge], [25:0-1	-8,Edge], [31:0-1-8,Edge], [32:0-1-8,Edge]			13-5-4	4	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.70 BC 0.81 WB 0.56 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.18 32-33 -0.25 32-33 0.04 21	l/defl >999 >824 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 163 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD	2x4 SP No.1(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat)	BRACING- TOP CHOF BOT CHOF	RD Structu except	t end vert	icals.	rectly applied or 6-0-0 o	oc purlins,	
REACTIONS.	(size) 35=Mechanical, 27=0-3-8, 21=0-3- Max Grav 35=847(LC 3), 27=2005(LC 1), 21=							
FORCES. (Ib) TOP CHORD	- Max. Comp./Max. Ten All forces 250 (b) o 2-3=-1496/0, 3-4=-2418/0, 4-5=-2418/0, 5-6 8-9=-1766/0, 9-10=-1766/0, 10-12=-385/291 14-15=-502/949, 15-16=-1487/329, 16-17=-1	=-2629/0, 6-7=-2629/0, 7- , 12-13=0/1961, 13-14=0/	8=-2629/0, 1961,					
BOT CHORD	34-35=0/906, 33-34=0/2063, 32-33=0/2634, 20 - 20 - 20 - 20 - 20 - 20 - 20 - 20 -	31-32=0/2629, 30-31=0/2	2229,					

- 29-30=-53/1172, 27-29=-790/0, 26-27=-1220/0, 25-26=-664/1055, 24-25=-329/1487, 23-24=-329/1487, 22-23=0/1460, 21-22=0/672

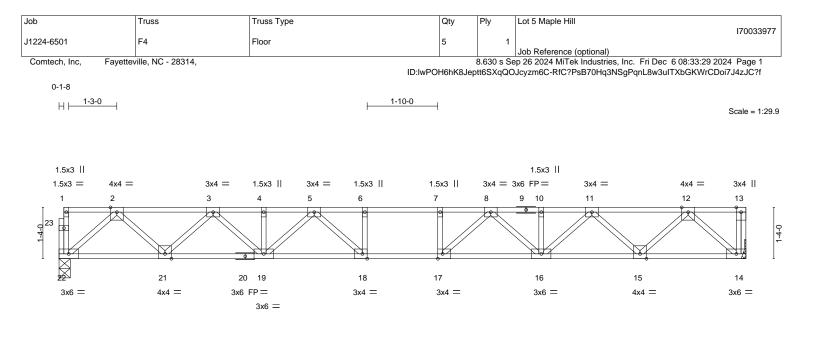
 WEBS
 2-35=-1206/0, 2-34=0/821, 3-34=-789/0, 3-33=0/483, 5-33=-293/0, 5-32=-299/249,
 - 12-27=-1559/0, 12-29=0/1172, 10-29=-1132/0, 10-30=0/846, 8-30=-672/0, 8-31=0/772, 7-31=-372/0, 14-27=-1269/0, 14-26=0/871, 15-26=-926/0, 15-25=0/901, 16-25=-369/0, 19-21=-892/0, 19-22=0/562, 18-22=-533/29, 17-23=0/404, 17-24=-284/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



			<u>17-10-0</u> 17-10-0					
Plate Offsets (X,Y	[17:0-1-8,Edge], [18:0-1-8,Edge]		17-10-0					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.49 BC 0.72 WB 0.47	Vert(LL) -0.2	in (loc) 21 17-18 29 17-18 06 14	l/defl >996 >725 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			n/a	n/a	Weight: 95 lb	FT = 20%F, 11%E
LUMBER-TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)			BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.					
REACTIONS.	(size) 22=0-3-8, 14=Mechanical ax Grav 22=961(LC 1), 14=967(LC 1)							
TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) oi 2-3≕1757/0, 3-4≕2926/0, 4-5≕2926/0, 5-6≕ 3-10≕-2926/0, 10-11≕-2926/0, 11-12≕1757//	-3487/0, 6-7=-3487/0, 7-8						
	21-22=0/1042, 19-21=0/2442, 18-19=0/3275 14-15=0/1043	, ,	, , ,					

	14-13=0/1043
WEBS	2-22=-1385/0, 2-21=0/994, 3-21=-953/0, 3-19=0/657, 12-14=-1388/0, 12-15=0/994,
	11-15=-953/0, 11-16=0/658, 8-16=-474/0, 8-17=-71/583, 5-19=-474/0, 5-18=-71/583,
	6-18=-290/0, 7-17=-290/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

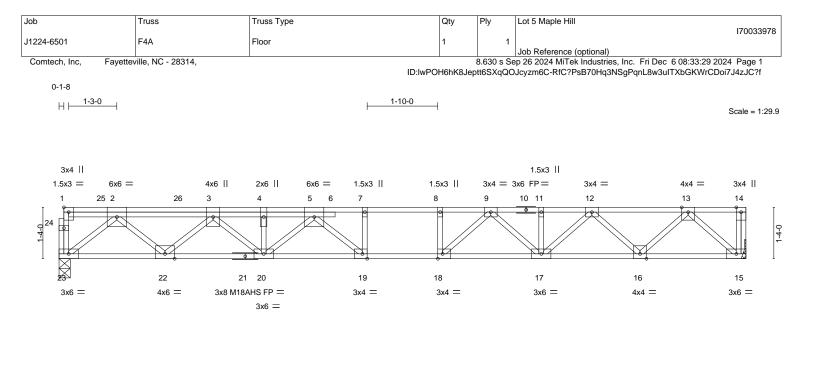
3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

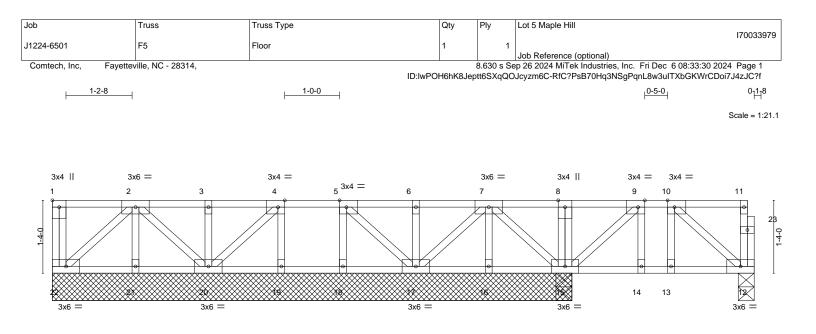


I			17-10-0				
Plate Offsets (X,Y)	[1:Edge,0-1-8], [18:0-1-8,Edge], [19:0-1	-8,Edge]	17-10-0				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.83 BC 0.95 WB 0.54 Matrix-S	DEFL. in Vert(LL) -0.22 Vert(CT) -0.31 Horz(CT) 0.07	19 >94 19 >67	7 480 7 360	PLATES MT20 M18AHS Weight: 104 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
REACTIONS. (size			BRACING- TOP CHORD BOT CHORD	except end v	erticals.	ectly applied or 6-0-0 or 10-0-0 or 10-0-0 oc bracing.	oc purlins,
TOP CHORD 2-3=- 9-11= BOT CHORD 22-23= 15-1 WEBS 2-23= 13-16	Comp./Max. Ten All forces 250 (lb) or :2158/0, 3-4=-3538/0, 4-5=-3538/0, 5-7= =-3102/0, 11-12=-3102/0, 12-13=-1847/(3=0/1315, 20-22=0/2972, 19-20=0/3746, 6=0/1090 =-1708/0, 2-22=0/1143, 3-22=-1104/0, 3 5=0/1053, 12-16=-1010/0, 12-17=0/718, =-320/291, 8-18=-341/0	3783/0, 7 ⁻ 8=-3783/0, 8-) , 18-19=0/3783, 17-18=0/ -20=0/752, 4-20=-279/0,	9=-3783/0, /3496, 16-17=0/2573, 13-15=-1451/0,				
 All plates are MT20 Plates checked for a Refer to girder(s) for Recommend 2x6 str Strongbacks to be a CAUTION, Do not e Hanger(s) or other c Ib down at 3-1-12, a of others. In the LOAD CASE(LOAD CASE(S) Standing Dead + Floor Live (b Uniform Loads (plf) Vert: 15-23: Concentrated Loads 	connection device(s) shall be provided su and 159 lb down at 5-1-12 on top chord. S) section, loads applied to the face of the dard palanced): Lumber Increase=1.00, Plate =-10, 1-14=-100	ts center. ts center. strained by other means. ufficient to support concer . The design/selection of he truss are noted as fror	ntrated load(s) 159 lb dow such connection device(s	vn at 1-1-12, a		SE 036	AL

Vert: 4=-79(F) 25=-81(F) 26=-79(F)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication component of component development properties. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



		9-4-8		9 ₁ 6 ₁ 0	12-10-0		
		9-4-8		0-1-8	3-4-0	1	
Plate Offsets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,	Edge], [9:0-1-8,Edge], [10	:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.08 BC 0.05 WB 0.04	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	(loc) l/defl L/d 13 >999 480 13 >999 360 12 n/a n/a	PLATES MT20	GRIP 244/190	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 80 lb	FT = 20%F, 11%E	
LUMBER-			BRACING-				
	P No.1(flat) P No.1(flat)		TOP CHORD	Structural wood sheathing dir except end verticals.	ectly applied or 6-0-0	oc purlins,	
WEBS 2x4 SF	P No.3(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 16-17,15-16.				

REACTIONS. All bearings 9-6-0 except (jt=length) 12=0-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 16, 17, 18, 21, 20, 19 except 15=301(LC 9), 15=290(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

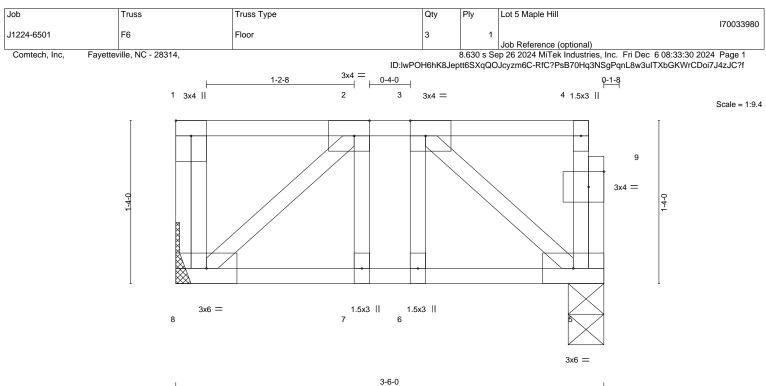
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



					000						
	I				3-6-0					1	
Plate Offsets (X,Y)	[1:Edge,0-1-8], [2:0-1-	8,Edge], [3:0-1-8	3,Edge], [9:0-	1-8,0-1-8]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inci	1.00	CSI. TC BC WB	0.08 0.05 0.04	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 7 7 5	l/defl >999 >999 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015		Matri				-			Weight: 24 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)					BRACING- TOP CHOF BOT CHOF	RD	Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.) oc purlins,

REACTIONS. (size) 8=Mechanical, 5=0-3-8 Max Grav 8=179(LC 1), 5=173(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

