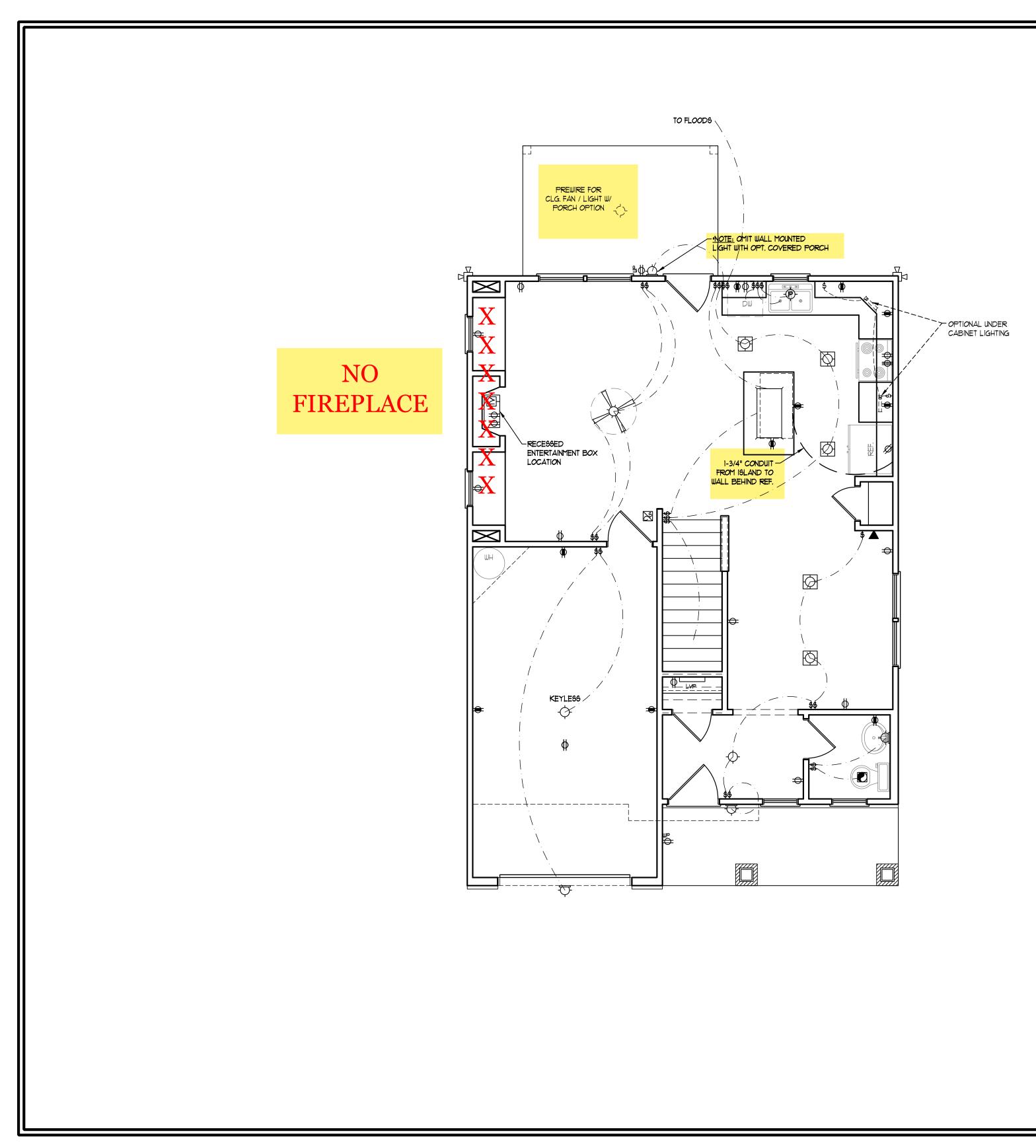




SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE











DATE: AUGUST 25, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
FIRST FLOOR
ELECTRICAL
PLAN
-
E-1

ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90/" A.F.F. (TY.P.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- = 110 V GFI OUTLET
- BB IIO V BASEBOARD OUTLET
- + 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI
- € 220 V OUTLET
- Ø 10 V DEDICATED CIRCUIT
- 120 V DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- -P- PENDANT LIGHT

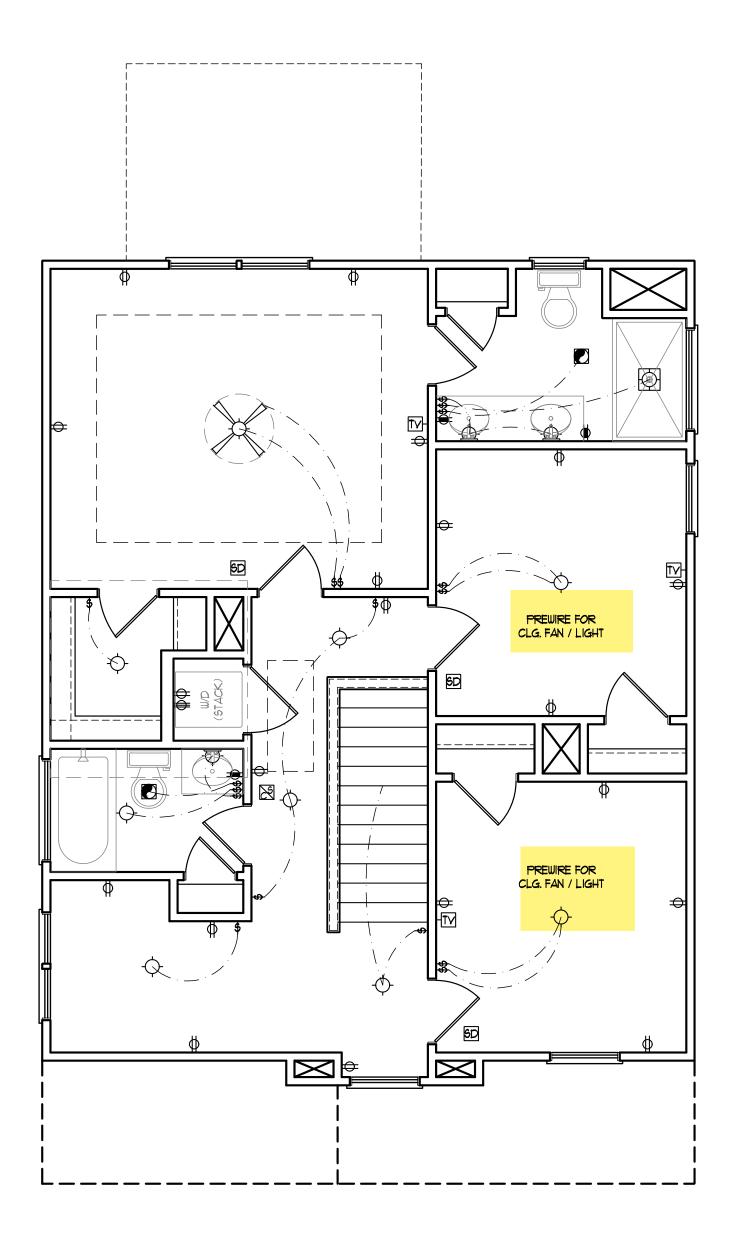
- MINI CAN LIGHT

- UNDERCABINET LIGHT
- SWITCH \$
- \$_D DIMMER SWITCH
- TELEPHONE
- TELEPHONE AND DATA
- TY- TY CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- 6P SPEAKER
- K 110 V SMOKE/ CM DETECTOR
- 6D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM ALARM PANEL

CEILING FAN

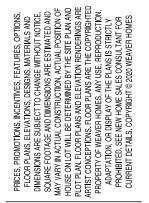
CEILING: FAN W/ LIGHT

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**





WEAVER —HOMES 350 Wagoner Drive - Fayetteville, NC 283 910.630.2100 - 800.892.2190 www.weaver-homes.com



WEAVER HOMES CAROLINA COLLECTION HICKORY DRIVE LEFT

DATE: AUGUST 25, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
SECOND FLOOR
ELCTRICAL
PLAN

E-2

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

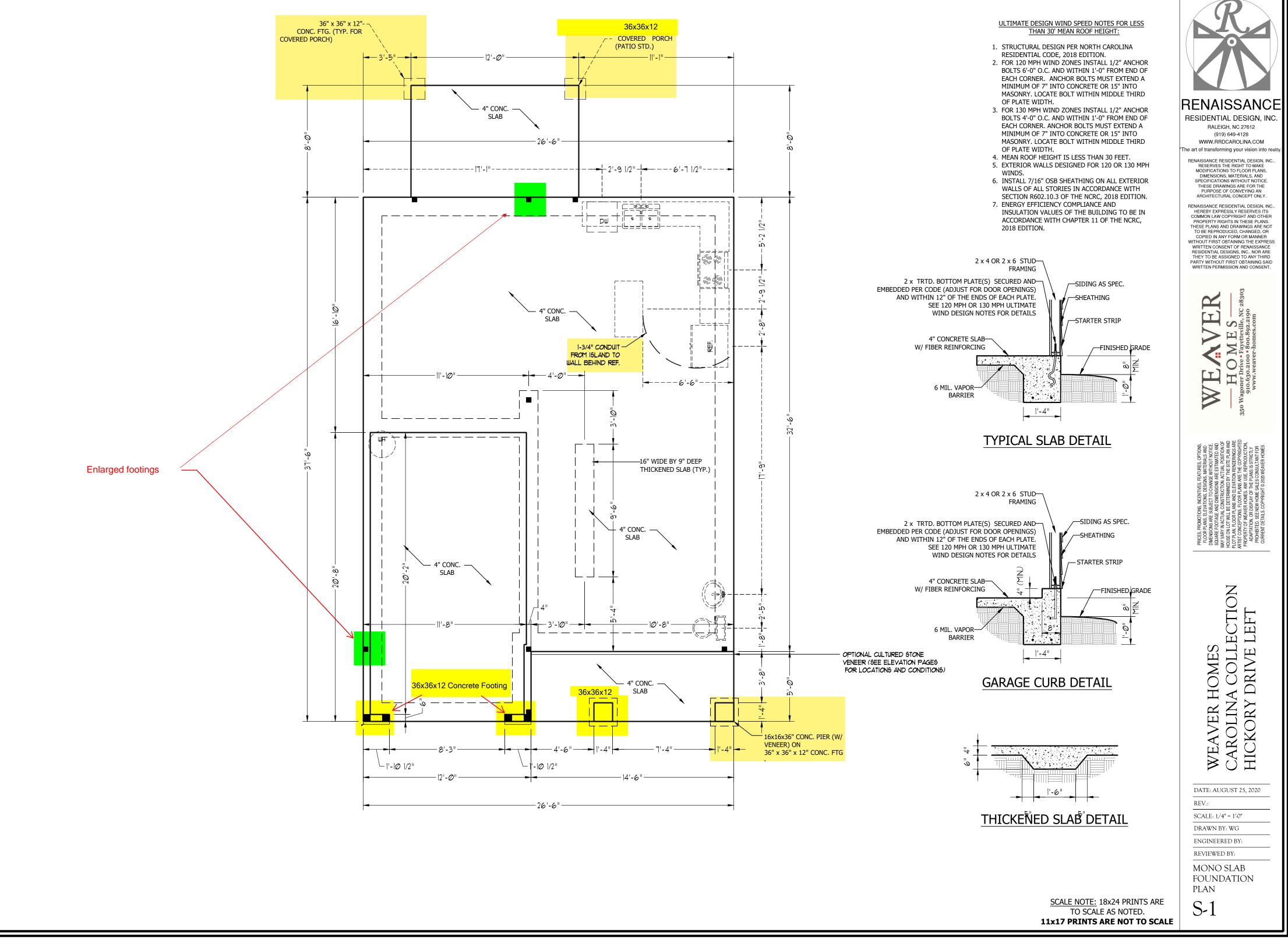
- ELECTRICAL LAYOUT NOTES:
- 1.) BLOCK AND WIRE FOR ALL CELING FANG PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 30" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

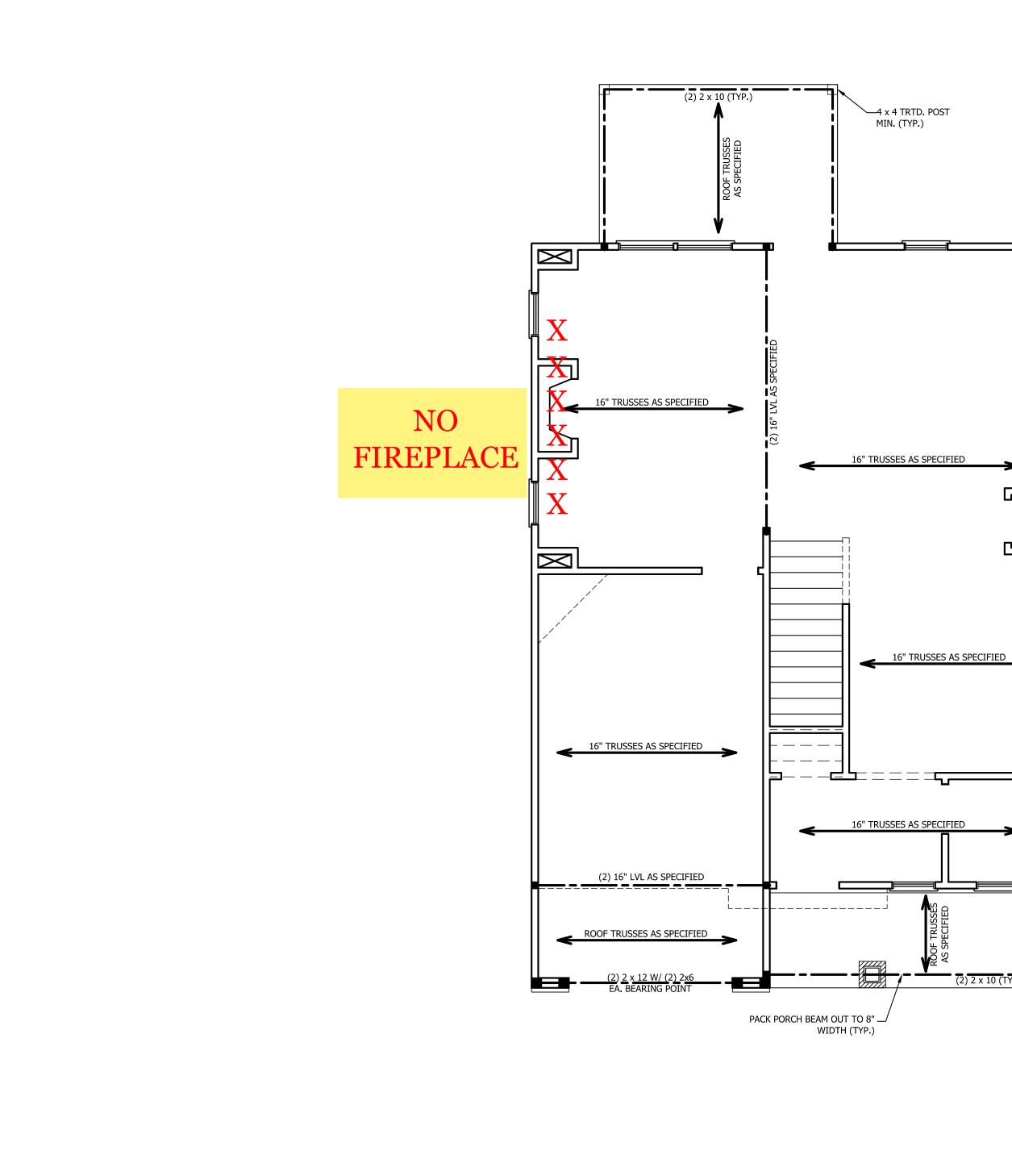
ELECTRICAL LEGEND

- BB 🕂 110 Y BASEBOARD OUTLET
- -∰H 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI
- € 220 ∨ OUTLET
- Ø 10 V DEDICATED CIRCUIT
- 120 V DEDICATED CIRCUIT
- H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT
- \$ SWITCH
- \$_D DIMMER SWITCH
- TELEPHONE
- TELEPHONE AND DATA
- TY- TY CONNECTION
- ED- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 110 V SMOKE/ CO DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM ALARM PANEL

l

CEILING FAN W/ LIGHT





STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO). 3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- 4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- 7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

BRACE WALL PANEL NOTES:

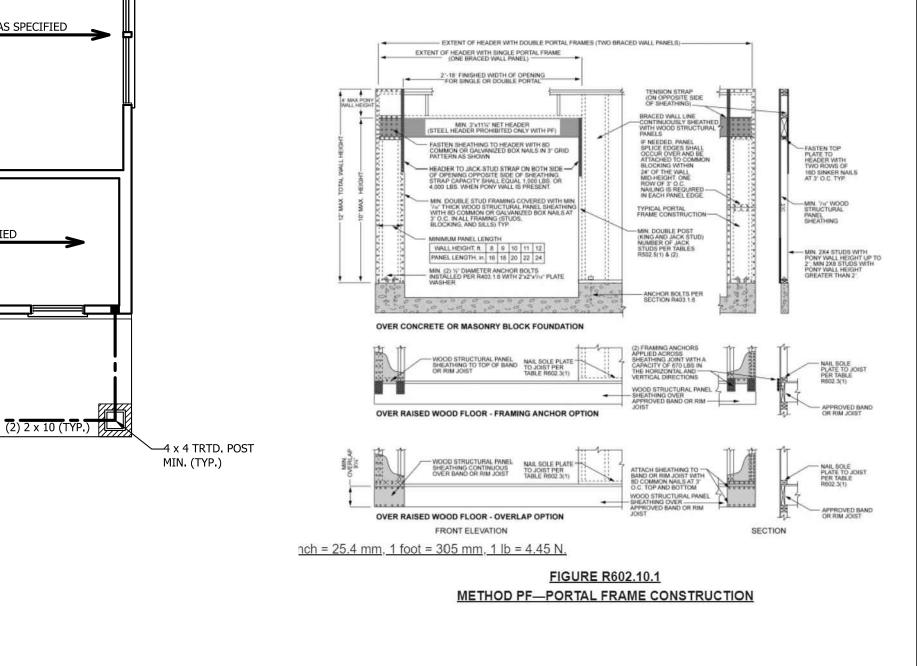
EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

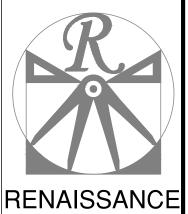
REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH, METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1











VER HOMES	OLINA COLLECTION	KORY DRIVE LEFT
WEAVE	CAROLI	HICKOR

7

DATE: AUGUST 25, 2020	
REV.:	
SCALE: 1/4" = 1'-0"	
DRAWN BY: WG	
ENGINEERED BY:	
REVIEWED BY:	
SECOND FLOOR	
FRAMING PLAN	

SCALE NOTE: 18x24 PRINTS ARE S-2 TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**

l r

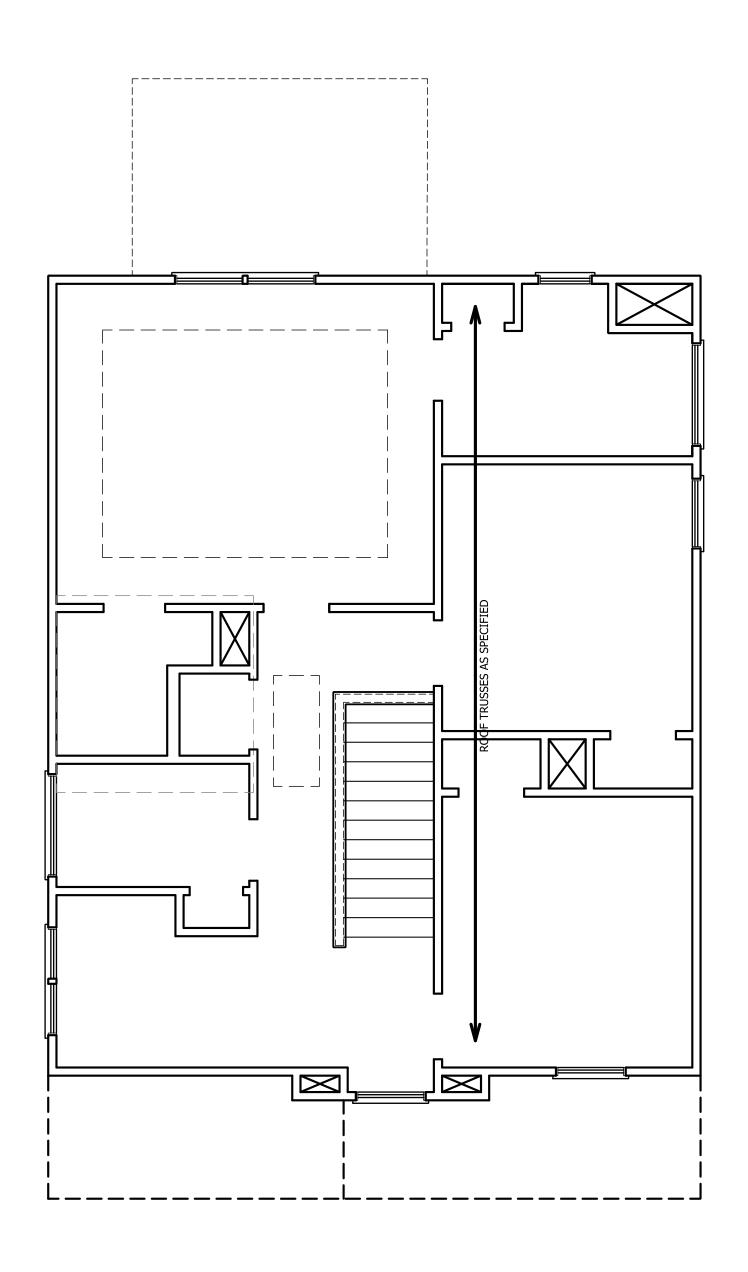


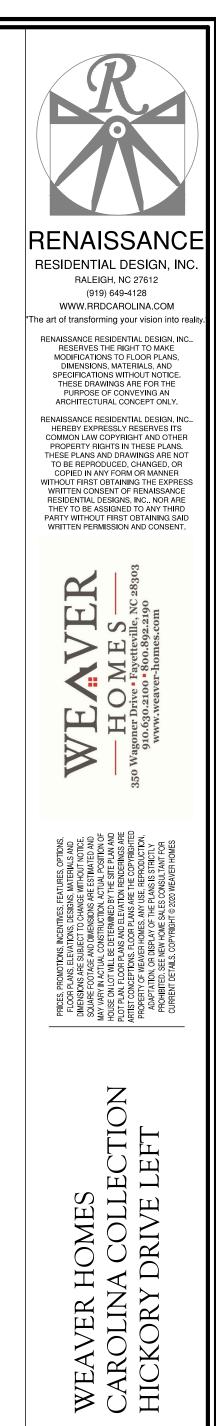
TABLE R602.7.5
MINIMUM NUMBER OF FULL HEIGHT STUDS
T EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5)				
(. == .)	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			

STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



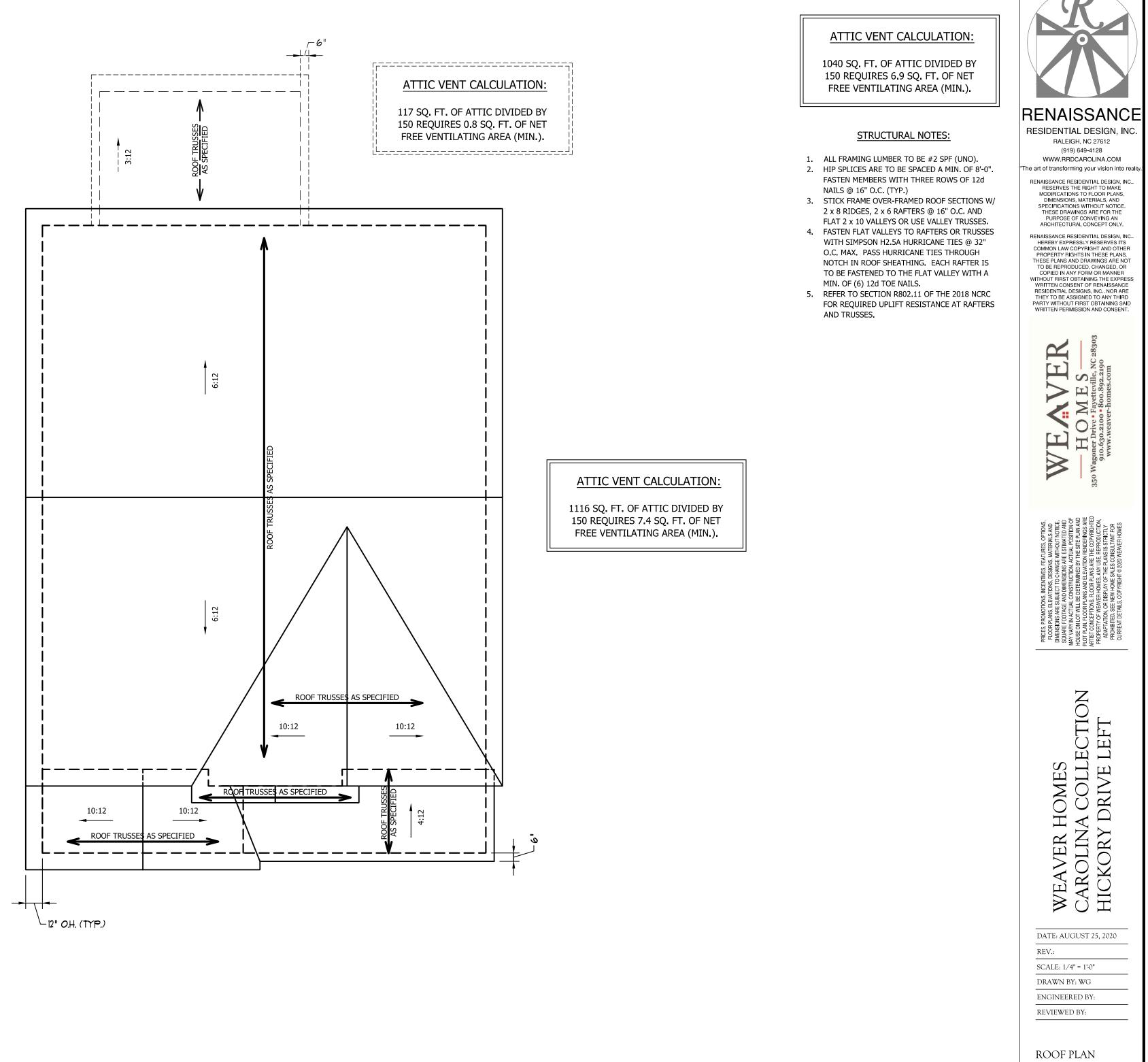
DATE: AUGUST 25, 2020

REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



ROOF PLAN

S-4

RALEIGH, NC 27612

(919) 649-4128

WWW.RRDCAROLINA.COM

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TION

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DRIVE

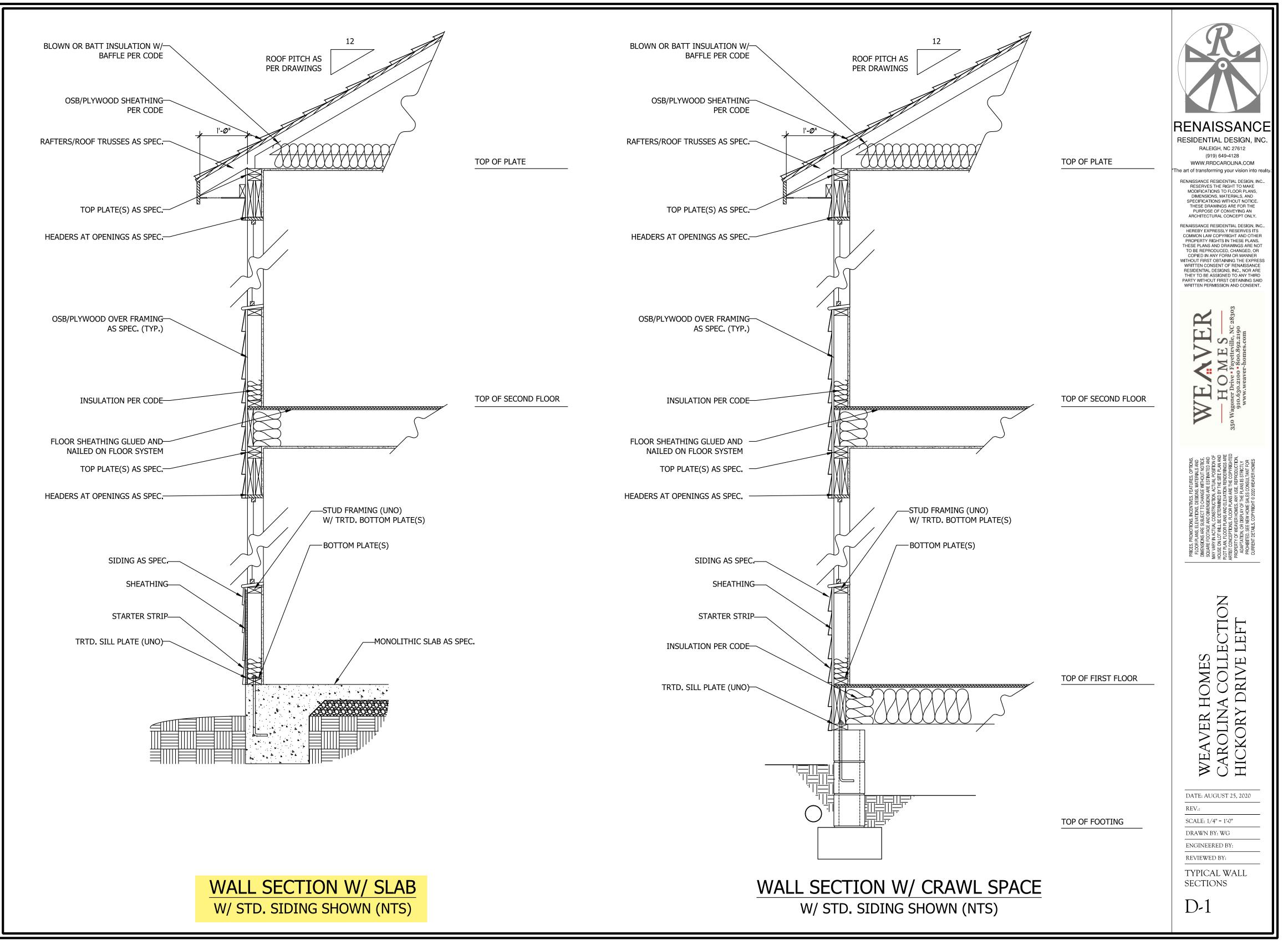
CAROLINA HICKORY

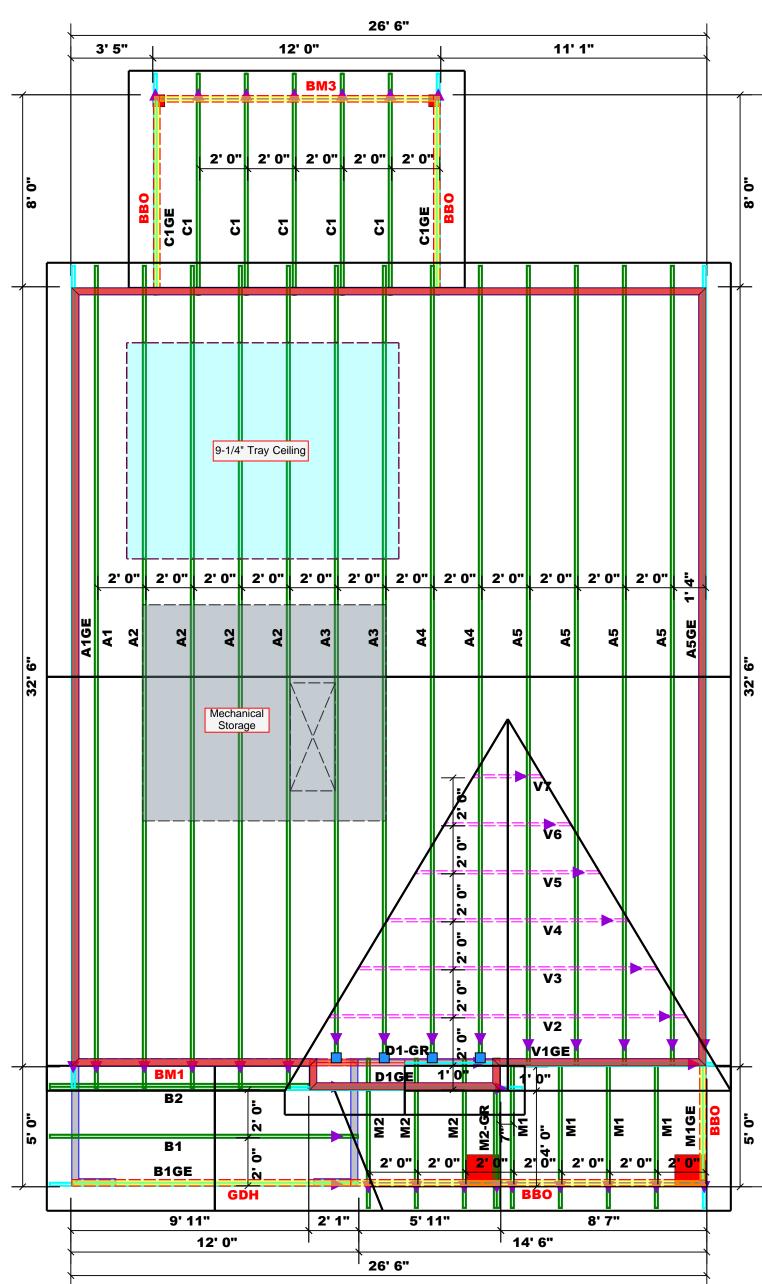
DATE: AUGUST 25, 2020

SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY: **REVIEWED BY:**

REV.:

WEAVER HOMES





	R Bearing deemed requirem attached reaction Tables. / retained reaction Signatur	ROC RUS Feilly R Fayet Phon Fax: reactions to complete to design to complete to design to design that exce A register to design s that exce re	DF & SES coad Ir teville e: (910) : (9	k FL k B dustr , N.C.) 864 864-4 b or equa e prescriptor shall r for shall r	I to 3000# tive Code efer to th prescriptiv imum fou uired to s ti greater t ional sha em for any di in the a ional shal am for all MCK STU Di (b)) © E A END CK STU Di (b)) © E A END SU SU SU SU SU SU SU SU SU SU SU SU SU	k are e code ndation upport han I be //ttached I be
atch Legend Padded HVAC 2nd Floor Walls Tray Ceiling Drop Beam	. Lillington / Harnett	4182 Darroch Rd.	Roof	. 12/05/24	/ David Landry	. Lenny Norris
I Information er Truss 1/2" 16d/3-1/2"	СІТУ / СО.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
s Net Qty 2 2 2 2	Weaver Homes, Inc.	Lot 4 Maple Hill	Hickory "A" / GLF, CP	N/A		J1224-6494
	BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards	These to comport design See ind identified designed perman for the support and col designed consult	russes an nents to b at the sp ividual de ed on the er is resp ent braci overall st t structur umns is er. For ge BCSI-B1	re designe be incorpo ecification esign she placemen onsible fo ng of the ructure. 1 e includin the respo neral guid and BCS	IENT DIA ed as indi prated int n of the b ets for ea nt drawin r tempor roof and he desig g header nsibility of dance reg I-B3 prov	GRAM ON ividual bu o the buil uilding de the truss g. The bu arry and floor syst n of the tu s, beams, of the buil arding br ided with sbcindus	ilding ding esigner. design ilding rem and russ walls, ding acing, the

Dimension Notes
1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

	Hatch Legend				
Roof Area = 1468.1 sq.ft. Ridge Line = 52.07 ft.	Padded HVAC				
Hip Line = 0 ft. Horiz. OH = 98.57 ft. Raked OH = 159.04 ft.	2nd Floor Walls				
Decking = 50 sheets	Tray Ceiling Drop Beam				
Connector Information Nail Information					
Sym Product Manuf Oty Supported					

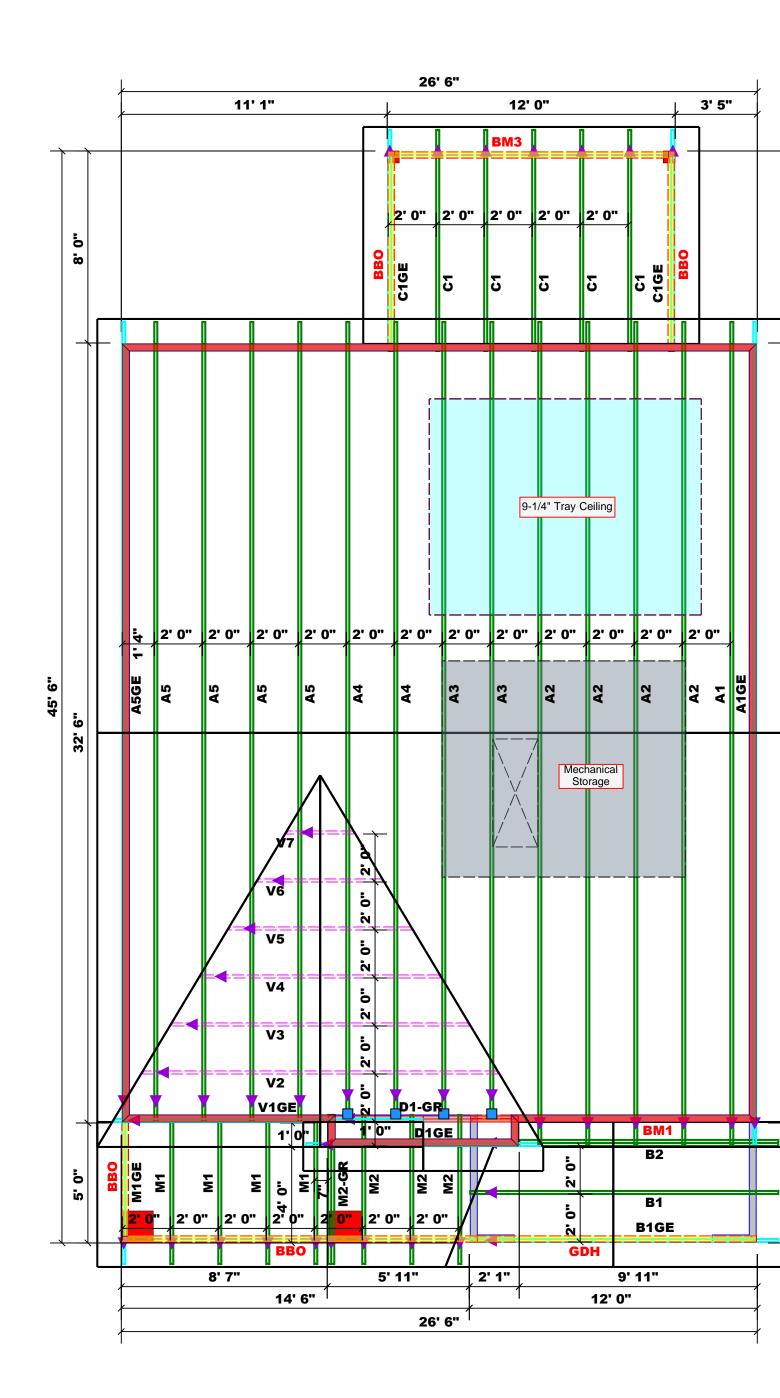
	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	4	NA	16d/3-1/2"	16d/3-1/2"

Г	Products							
L			11000013					
	PlotID	Length	Product	Plies	Net Qty			
Γ	BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
	BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
	BM3	12' 0"	2x10 SPF No.2	2	2			
L	GDH	12' 0"	2x12 SPF No.2	2	2			

Truss Placement Plan Scale: 1/4"=1'

9

5.



	<u> </u>		Bearing deemed requirer attached requirer size and reaction 15000#.	Fax reactions to compl nents. The Tables (I Tables) to I number s greater A registe	OF & SES coad Ir teville e: (910) : (910) : less that y with the e contract determin of wood s than 3000 read design	& FL & B ndustr , N.C.)) 864 864-4 n or equa prescription a shall r from the pe e the min studs req b# but no n profession	OOF EAN ial Par 28309 -8787 444	t are k e re Code indation upport han ii be
8' 0"			reaction Tables. retained	to design that exce A register to design s that exc re	eds thos ed design the supp	e specifie n profess oort syste 0#.	ed in the a ional shal em for all	ttached I be
	~	Dimension Notes	NUM	ABER OF J/ ABER OF J/ LOS 2 21022 HO HG V 21022	ON TABLE	S R502.5(1) REQUIREC (GIRDER WOJ SCIPALA NJA (E) 1 2 3) 4) 5	0) & (b)) 0) & (b)) 0) & (c) 0) &	1 REQ'D STUDS FOR (4) PLY HEADER
32' 6"	45' 6"	1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise 3. Hierdror wall to trus dimensions are to face of frame wall unless noted otherwise March Legend Padded HVAC Padded HVAC Padded HVAC Horiz OH = 98.57 ft. Horiz OH = 98.57 ft. Raked OH = 159.04 ft. Decking = 50 sheets Drop Beam Image: true dimension of the face of	CITY / CO. Lillington / Harnett	ADDRESS 4182 Darroch Rd.	MODEL Roof	DATE REV . 12/05/24	DRAWN BY David Landry	SALES REP. Lenny Norris
5. 0.	~	$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	omes, Inc.	e Hill	" / GLF, CP			44
			BUILDER Weaver Homes,	JB NAME Lot 4 Maple	PLAN Hickory "A"	SEAL DATE N/A	QUOTE #	# J1224-649
		= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards	THIS IS These compo design See indi identifi design permar for the suppor and co design consult	A TRUSS trusses and trusses and ent so the at the sp tividual ded on the er is resp nent braci overall st t structur lumns is er. For ge t BCSI-B1 elivery pa	s PLACEN re designed be incorpore esign she placement onsible for ng of the ructure. T e includim the respon neral guid and BCS	IENT DIA ed as indi prated int n of the b ets for ea nt drawin r tempor roof and he desig g header nsibility of lance reg I-B3 prov	GRAM ON ividual bui o the buil uilding de ach truss g. The bu ary and floor syst n of the t s, beams, of the buil arding br ided with	ilding ding ssigner. design ilding em and russ walls, ding acing, the



RE: J1224-6494 Lot 4 Maple Hill Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer:Project Name:Lot/Block:MoAddress:SulCity:Sta

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings.

17 I70024565 M2 12/6/2024 18 I70024566 M2-GR 12/6/2024 19 I70024567 V1GE 12/6/2024	18	170024566	M2-GR	12/6/2024	No. 21 22 23 24 25	Seal# 170024569 170024570 170024571 170024572 170024573	Truss Name V3 V4 V5 V6 V7	Date 12/6/202 12/6/202 12/6/202 12/6/202
19 I70024567 V1GE 12/6/2024 20 I70024568 V2 12/6/2024	-		-					

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

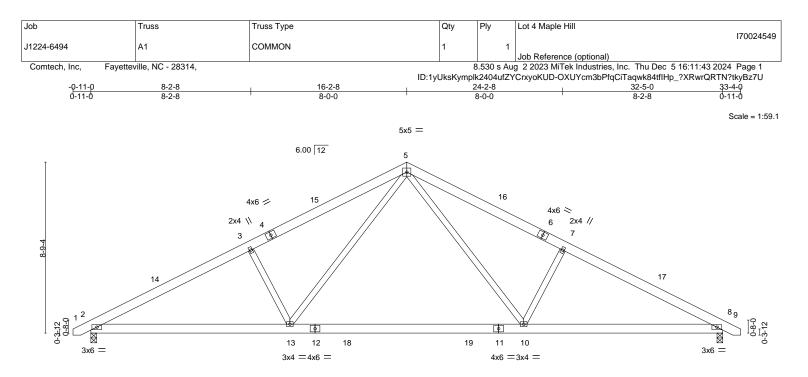
My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



	10-2-8 10-2-8	<u>22-2-8</u> 12-0-0	<u>32-5-0</u> 10-2-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. DEFL. in (k TC 0.28 Vert(LL) -0.34 10- BC 0.64 Vert(CT) -0.47 10- WB 0.27 Horz(CT) 0.05	13 >999 360 MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Wind(LL) 0.05 2-	13 >999 240 Weight: 208 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

```
LUMBER-
```

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

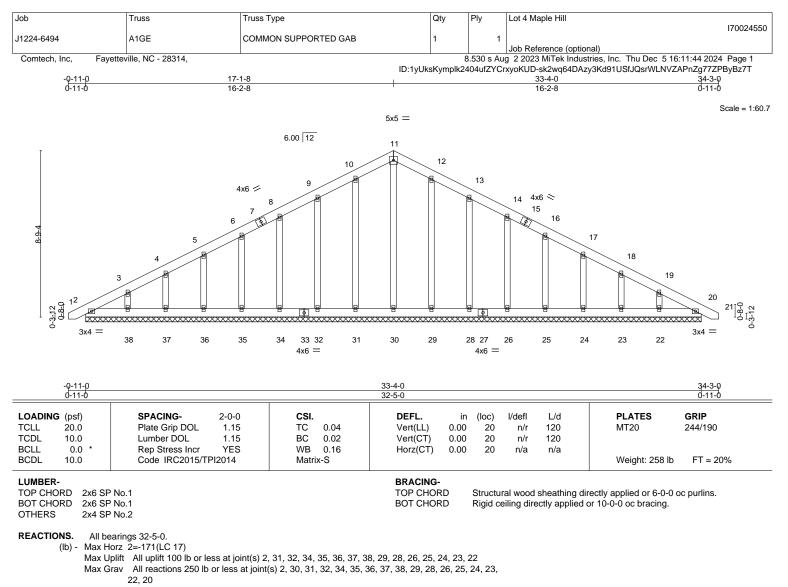
6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-114/284, 11-12=-114/284

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.6) Gable studs spaced at 2-0-0 oc.

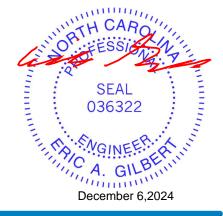
o) Gable studs spaced at 2-0-0 0C.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

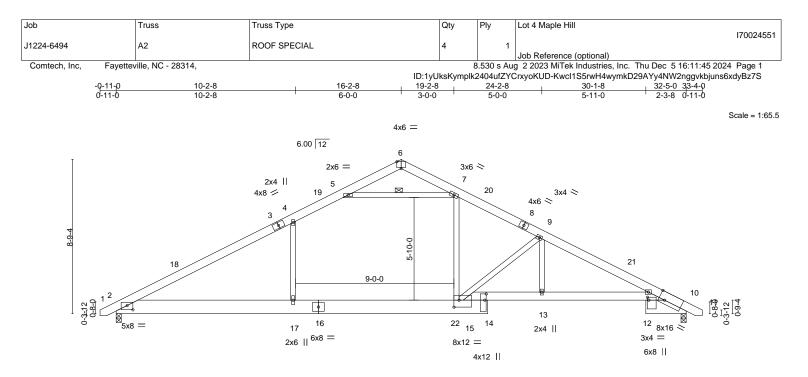
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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	10-2-8	16-2-8	19-2-8	21-1-8	1 24	-2-8	30-1-8	32-5-0	I
	10-2-8	6-0-0	3-0-0	1-11-0	3-	·1-0	5-11-0	2-3-8	1
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0	4-0,Edge], [12:0-2-0,0-1-0]], [14:0-4-4,0-1-0], [15:0-3-	8,0-4-1	2]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.75	Vert(LL)	-0.21	17	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.67	Vert(CT)	-0.38	17	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.70	Horz(CT)	0.09	10	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.17	2-17	>999	240	Weight: 247 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x10 SP No.1 *Except* 10-15: 2x6 SP 2400F 2.0E WEBS 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 4-0-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 5-7 1 Row at midpt

REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-90(LC 12), 10=-90(LC 13)

Max Grav 2=1393(LC 2), 10=1353(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551

- BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525
- WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

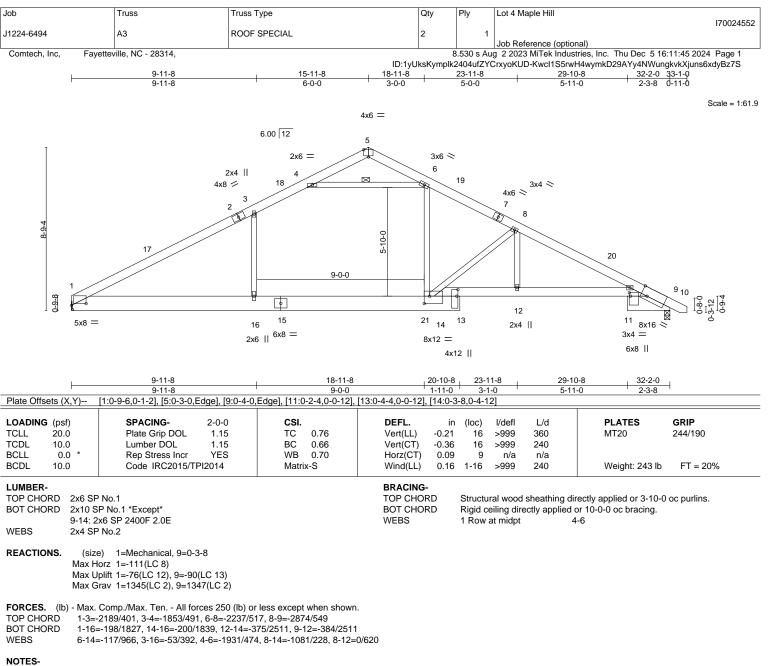
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

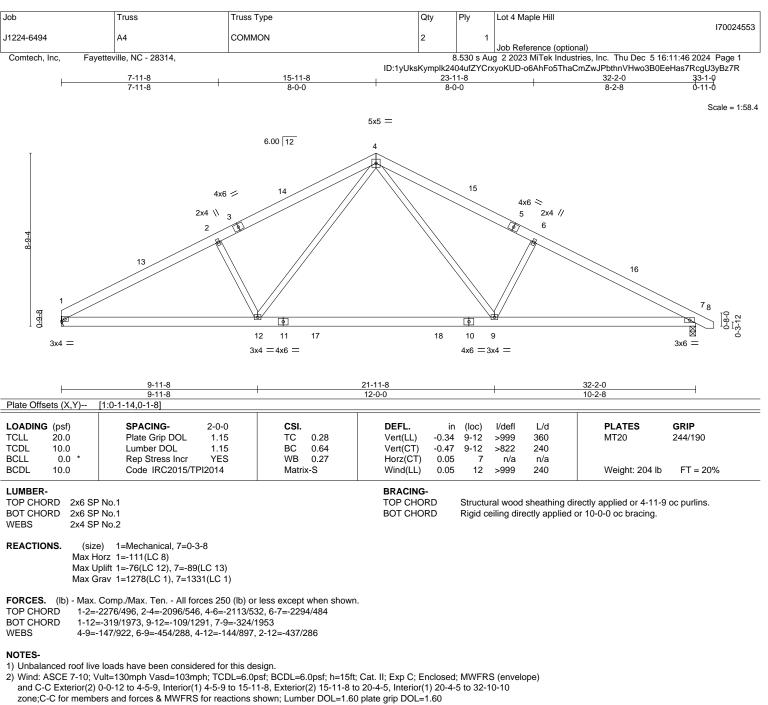
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



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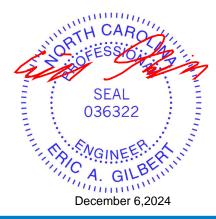
818 Soundside Road



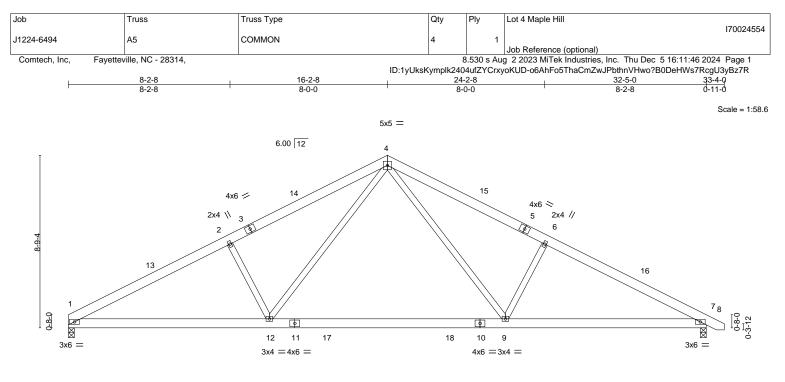
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

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6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



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	10-2-8 10-2-8	22-2-8 12-0-0		+	32-5-0 10-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.29 Vert(LL) BC 0.65 Vert(CT) WB 0.27 Horz(CT) Matrix-S Wind(LL)	in (loc) -0.34 9-12 -0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240		RIP 4/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with PCDL = 10.0pcf

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

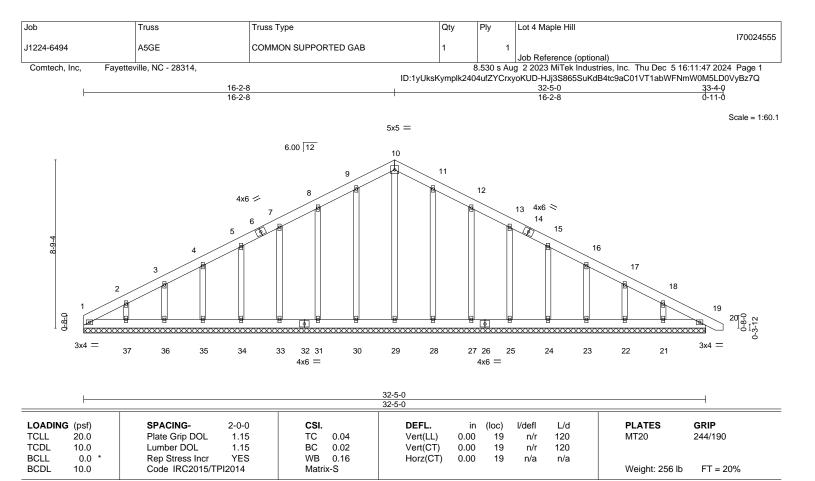


Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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BRACING-TOP CHORD

BOT CHORD

LUMBER-	
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TOP CHORD 2x6 SP No 1 2x6 SP No.1 BOT CHORD

OTHERS 2x4 SP No.2

REACTIONS. All bearings 32-5-0.

Max Horz 1=-175(LC 17) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except 37=-101(LC 12) Max Grav

All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

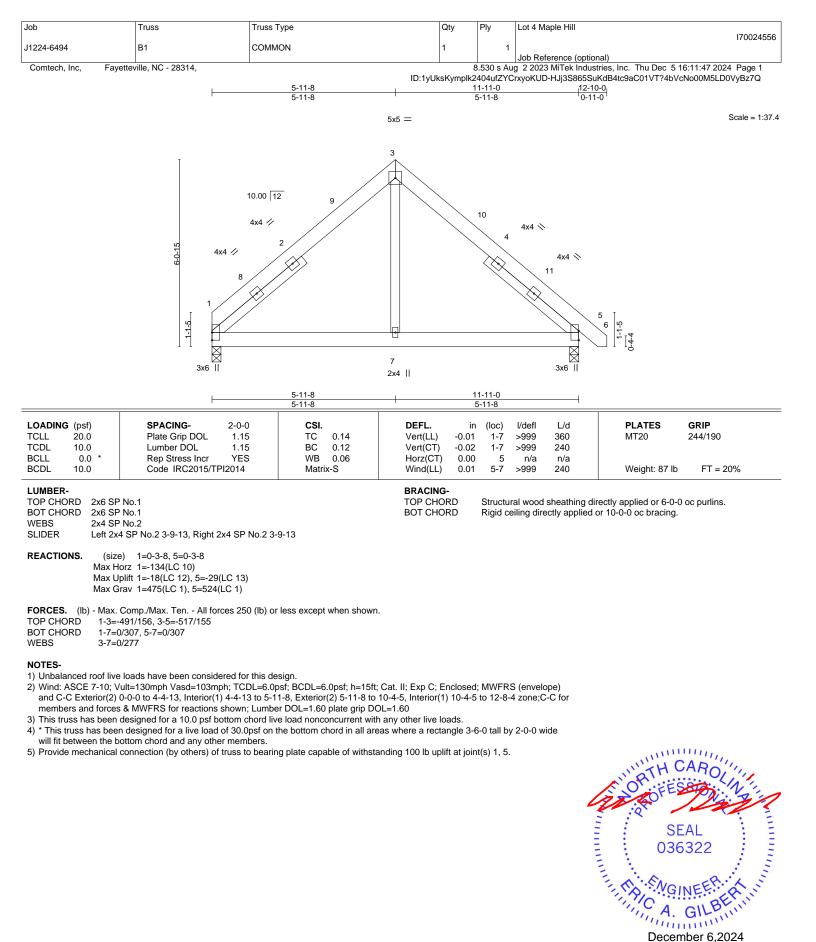
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



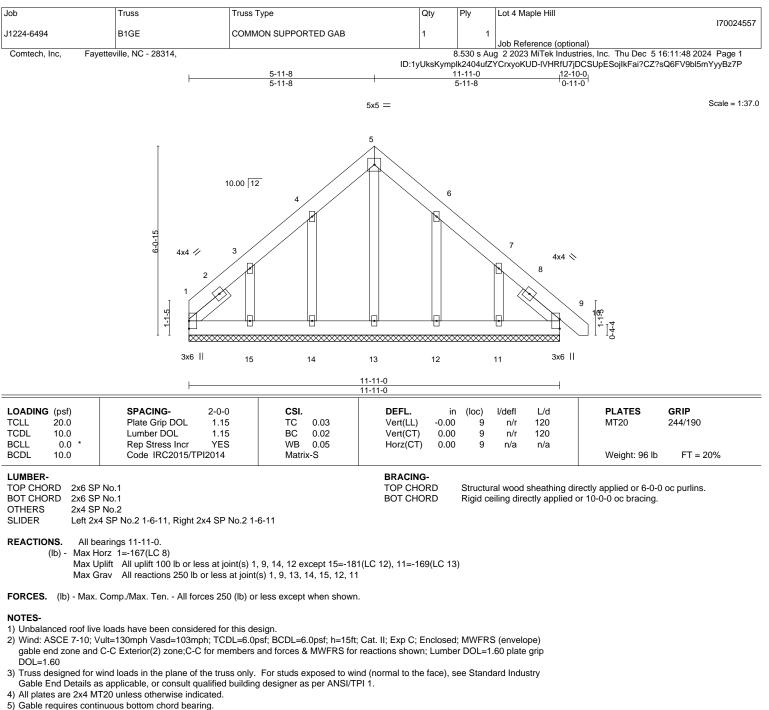
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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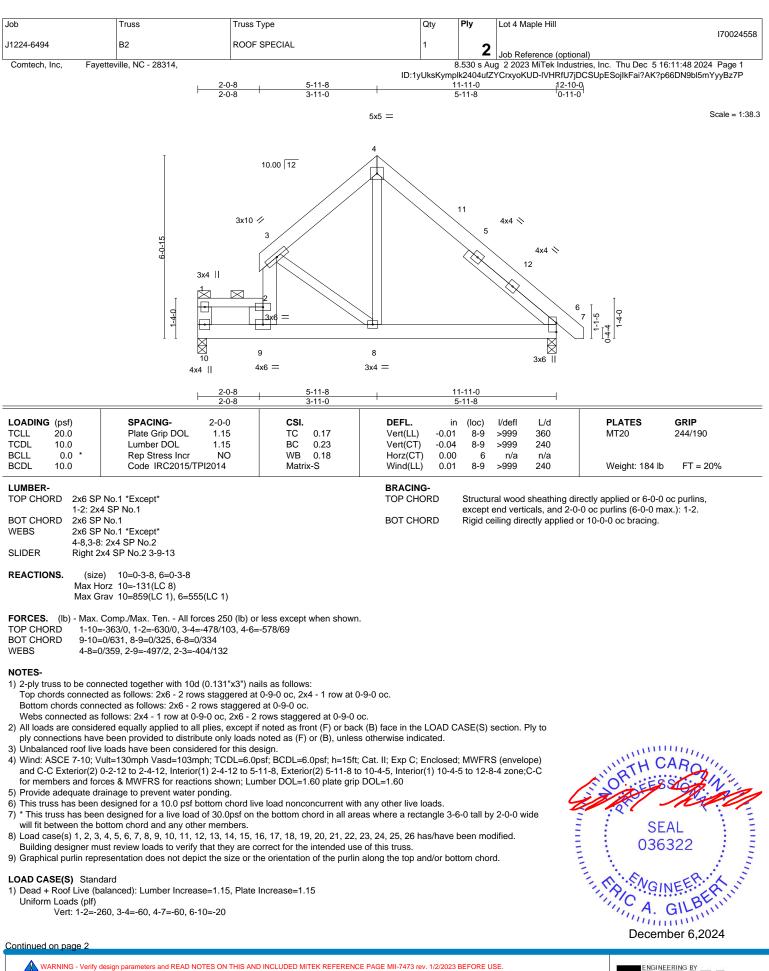


- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (jt=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



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Job	Truss	Truss Type	Qty	Ply	Lot 4 Maple Hill	
J1224-6494	B2	ROOF SPECIAL	1	2		170024
Comtoch Inc. E	ayetteville, NC - 28314,			2		nc. Thu Dec 5 16:11:48 2024 Page 2
Comtech, Inc, F	ayelleville, NC - 26314,		ID:1yUksKy			ESojlkFai?AK?p66DN9bl5mYyyBz7f
LOAD CASE(S) Sta	undard					
• • •		ncrease=1.15, Plate Increase=1.15				
Uniform Loads (pl	f)					
	-250, 3-4=-50, 4-7=-50, 6	10=-20 Lumber Increase=1.25, Plate Increase=1	25			
Uniform Loads (pl	•	Lumber increase=1.23, Flate increase=1	.25			
Vert: 1-2=	-220, 3-4=-20, 4-7=-20, 6					
 Dead + 0.6 C-C W Uniform Loads (pli 		: Lumber Increase=1.60, Plate Increase=1	.60			
N N	,, ⊶156, 3-4=27, 4-12=35, 6-	12=27, 6-7=20, 6-10=-12				
Horz: 3-4	=-39, 4-12=47, 6-12=39, 6	-7=32				
5) Dead + 0.6 C-C W Uniform Loads (pl		: Lumber Increase=1.60, Plate Increase=1	.60			
u u	 -170, 3-4=35, 4-11=27, 6-	11=35, 6-7=58, 6-10=-12				
	=-47, 4-11=39, 6-11=47, 6					
6) Dead + 0.6 C-C W Uniform Loads (pl		: Lumber Increase=1.60, Plate Increase=7	1.60			
u u		7=-51, 6-10=-20				
	=38, 4-6=-38, 6-7=-31					
Uniform Loads (pl		: Lumber Increase=1.60, Plate Increase=7	1.60			
u u	235, 3-4=-58, 4-6=-58, 6	7=11, 6-10=-20				
	=38, 4-6=-38, 6-7=31		4.00			
Uniform Loads (pl		t: Lumber Increase=1.60, Plate Increase=	1.60			
	, -179, 3-4=-13, 4-6=11, 6-	7=4, 6-10=-12				
	=1, 4-6=23, 6-7=16	the lumber increase 1.60. Plate increase	1.60			
Uniform Loads (pl		ht: Lumber Increase=1.60, Plate Increase	=1.00			
Vert: 1-2=	-191, 3-4=11, 4-6=-13, 6-	7=2, 6-10=-12				
	=-23, 4-6=-1, 6-7=14	eft: Lumber Increase=1.60, Plate Increase	-1.60			
Uniform Loads (p		en. Lumber morease=1.00, Flate morease	=1.00			
	2=-201, 3-4=-35, 4-6=-11,	6-7=-4, 6-10=-20				
	4=15, 4-6=9, 6-7=16 RS Wind (Neg. Internal) F	ight: Lumber Increase=1.60, Plate Increa	se-1 60			
Uniform Loads (p		ight. Lumber increase - 1.00, 1 late increa	56-1.00			
	2=-213, 3-4=-11, 4-6=-35,	6-7=-28, 6-10=-20				
	4=-9, 4-6=-15, 6-7=-8 RS Wind (Pos_Internal) 1	st Parallel: Lumber Increase=1.60, Plate I	ocrease=1.60			
Uniform Loads (p			1010000-1.00			
	2=-179, 3-4=21, 4-6=9, 6-7	=2, 6-10=-12				
	4=-33, 4-6=21, 6-7=14 RS Wind (Pos_Internal) 2	nd Parallel: Lumber Increase=1.60, Plate	Increase=1.60			
Uniform Loads (p						
	2=-191, 3-4=9, 4-6=21, 6-7	=14, 6-10=-12				
	4=-21, 4-6=33, 6-7=26 RS Wind (Pos. Internal) 3	rd Parallel: Lumber Increase=1.60, Plate I	ncrease=1.60			
Uniform Loads (p	olf)					
	?=-179, 3-4=21, 4-6=9, 6-7 4=-33, 4-6=21, 6-7=14	=2, 6-10=-12				
	, ,	th Parallel: Lumber Increase=1.60, Plate I	ncrease=1.60			
Uniform Loads (p	olf)					
	?=-191, 3-4=9, 4-6=21, 6-7 4=-21, 4-6=33, 6-7=26	=14, 6-10=-12				
		st Parallel: Lumber Increase=1.60, Plate I	ncrease=1.60			
Uniform Loads (p	olf)					
	?=-201, 3-4=-1, 4-6=-13, 6 4=-19, 4-6=7, 6-7=14	7=-6, 6-10=-20				
		nd Parallel: Lumber Increase=1.60, Plate	Increase=1.60			
Uniform Loads (p						
	?=-213, 3-4=-13, 4-6=-1, 6 4=-7, 4-6=19, 6-7=26	-7=6, 6-10=-20				
	crease=0.90, Plate Increa	se=0.90 Plt. metal=0.90				
Uniform Loads (p						
	2=-220, 3-4=-20, 4-7=-20, of Live (bal.) + 0 75(0.6 MV	5-10=-20 VFRS Wind (Neg. Int) Left): Lumber Increa	se=1.60 Plate Incre	ase=1.60		
Uniform Loads (p	. , .			400-1.00		
	2=-236, 3-4=-61, 4-6=-43,	6-7=-38, 6-10=-20				
	4=11, 4-6=7, 6-7=12 of Live (bal.) + 0.75(0.6 MV	VFRS Wind (Neg. Int) Right): Lumber Incre	ease=1.60 Plate Inc	rease=1 60		
Uniform Loads (p	. , .					
	2=-245, 3-4=-43, 4-6=-61,	6-7=-56, 6-10=-20				
Horz 2	4=-7, 4-6=-11, 6-7=-6					

Continued on page 3

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Job	Truss	Truss Type	Qty	Ply	Lot 4 Maple Hill
J1224-6494	B2	ROOF SPECIAL	1	_	170024558
0.22.0.00			•	2	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.530 s Aug 2 2023 MiTek Industries, Inc. Thu Dec 5 16:11:48 2024 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-IVHRfU7jDCSUpESojlkFai?AK?p66DN9bl5mYyyBz7P

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20

Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20

Horz: 3-4=-5, 4-6=14, 6-7=19

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

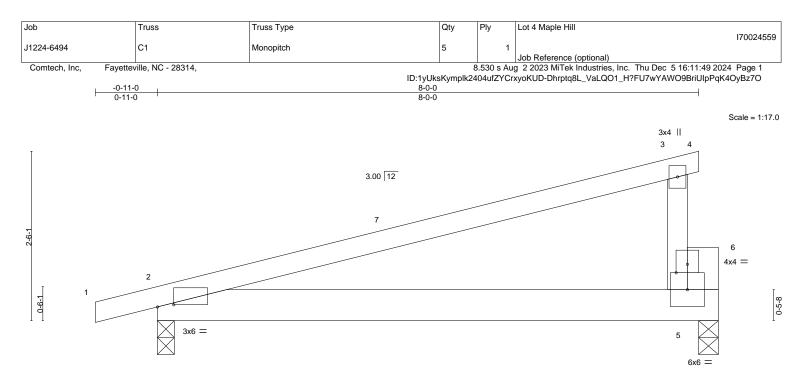
Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20

26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			<u>8-3-8</u> 8-3-8						
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]							-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.24 WB 0.00 Matrix-P	Vert(CT) - Horz(CT)	in -0.05 -0.10 0.00 0.10	(loc) 2-5 2-5 5 2-5	l/defl >999 >969 n/a >886	L/d 360 240 n/a 240	PLATES MT20 Weight: 37 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOD CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 OTHERS 2x6 SP No.1)	Structu	ral wood end verti	sheathing di cals.	rectly applied or 5-3-4 or 10-0-0 oc bracing.	

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



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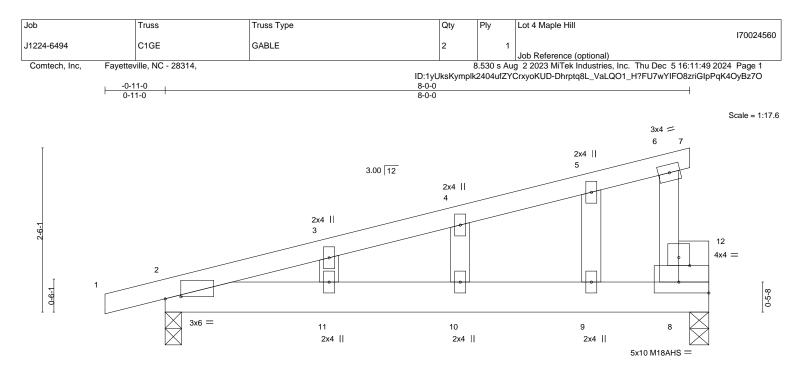


Plate Offsets (X,Y)	[2:0-2-14.0-0-6], [8:Edge.0-2-0], [12:0-2	-0.0-1-8]	8-3-8 8-3-8		I
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	Vert(LL) 0.0	n (loc) l/defl L/d 9 10-11 >999 240 8 10-11 >999 240 0 8 n/a n/a	PLATES GRIP MT20 244/190 M18AHS 186/179 Weight: 41 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	' No.1	1	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied of	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

OTHERS 2x4 SP No.2 *Except* 8-12: 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8)

Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are MT20 plates unless otherwise indicated.

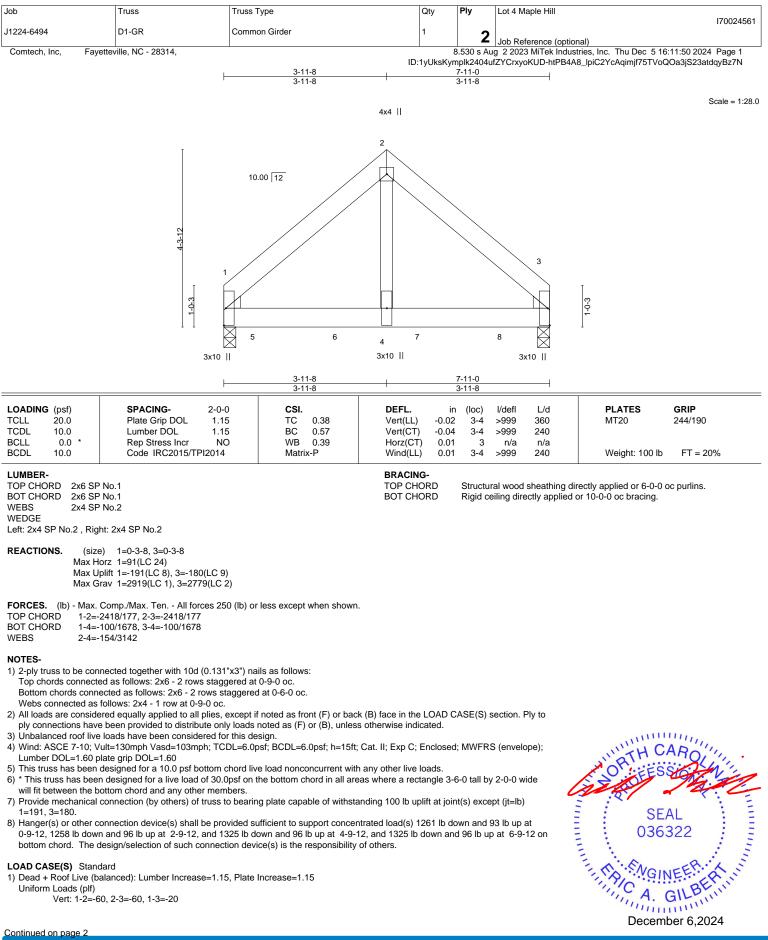
4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.



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ſ	Job	Truss	Truss Type	Qty	Ply	Lot 4 Maple Hill
						170024561
	J1224-6494	D1-GR	Common Girder	1	ົ	
					2	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	3.530 s Au	g 2 2023 MiTek Industries, Inc. Thu Dec 5 16:11:50 2024 Page 2

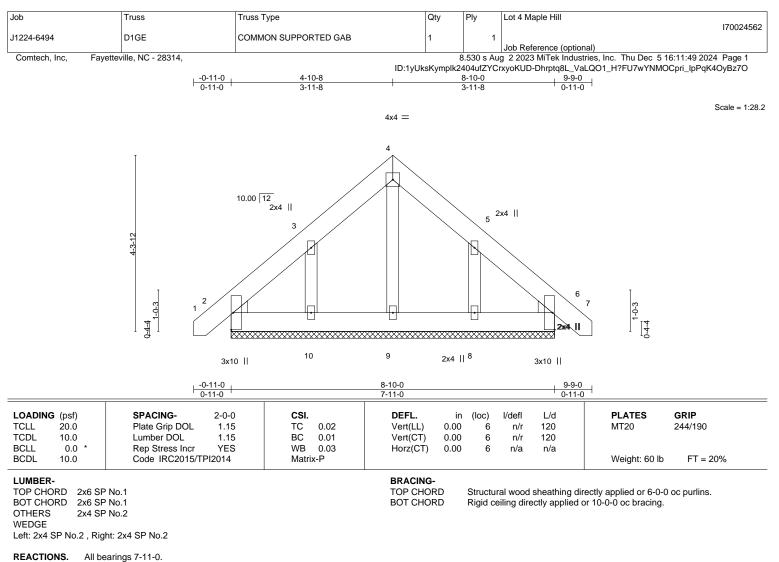
ID:1yUksKymplk2404ufZYCrxyoKUD-htPB4A8_lpiC2YcAqimjf75TVoQOa3jS23atdqyBz7N

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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REACTIONS.

Max Horz 2=-118(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2. 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

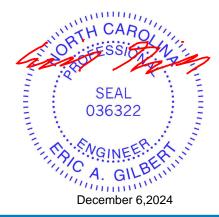
5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

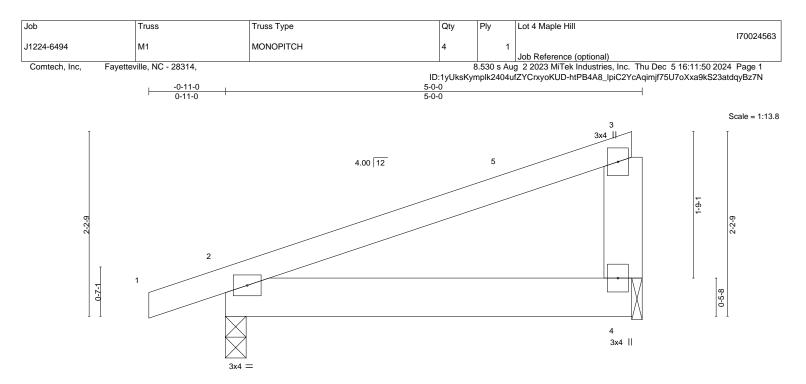
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152, 8=148

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.0)1 2-4	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.0	01 2-4	>999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.0	00	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.0)1 2-4	>999 240	Weight: 24 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

BRACING-TOP CHORD Stru

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

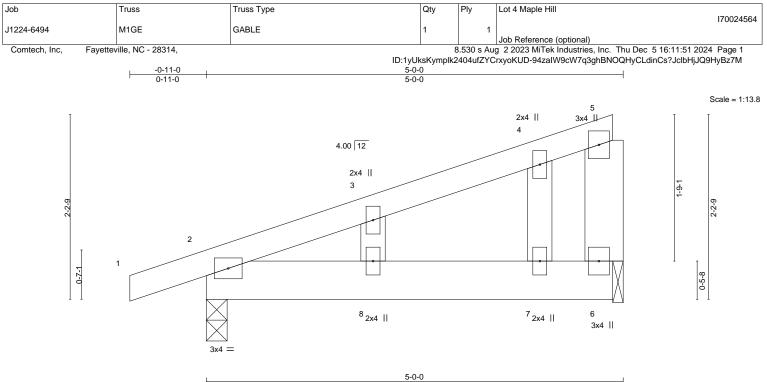
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



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			5-0-0			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/de	fl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) 0.01	8 >99	9 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) -0.01	8 >999	9 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT) -0.00	6 n/	a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 27 lb FT = 20%
LUMBER-			BRACING-			

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-1-8

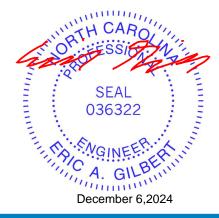
Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

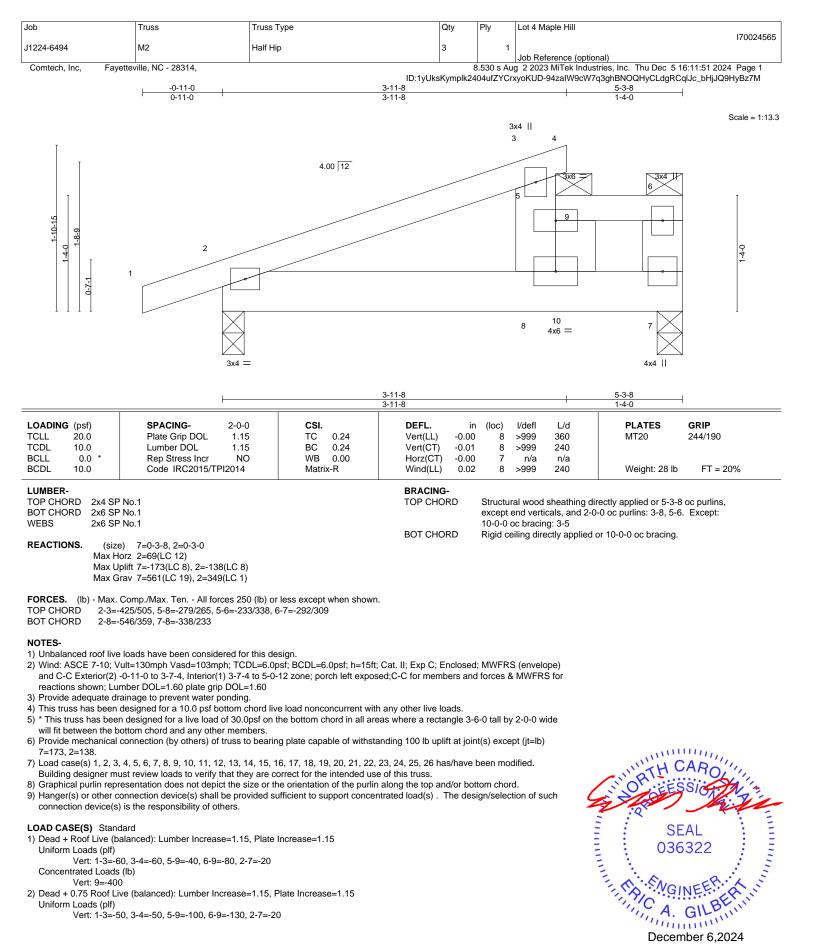
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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Continued on page 2

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A MiTek Affi 818 Soundside Road

I	Truss	Truss Type	Qty	,	Ply	Lot 4 Maple Hill	17002456
1224-6494	M2	Half Hip	3		1	Job Reference (optional)	17 002430.
Comtech, Inc, Fayettev	ille, NC - 28314,					g 2 2023 MiTek Industries, Inc. Thu E	
			ID:1yUksKyr	прік24	04utZYCr	xyoKUD-94zalW9cW7q3ghBNOQHyC	LagkCdijc_pHjjdaHABz1M
LOAD CASE(S) Standard Concentrated Loads (Ib)							
Vert: 9=-350	tic Without Storage: Lumb	er Increase=1.25, Plate Increase=1.	25				
Uniform Loads (plf)			25				
Vert: 1-3=-20, 3 Concentrated Loads (lb)	-4=-20, 5-6=-40, 2-7=-40						
Vert: 9=-300 4) Dead + 0.6 C-C Wind (P	os Internal) Case 1: Lumh	per Increase=1.60, Plate Increase=1	60				
Uniform Loads (plf)	,						
Horz: 1-2=-82, 2	3=58, 3-4=153, 5-6=12, 2- 2-3=-70, 3-4=-165, 3-5=-55	8=52, 8-10=115, 7-10=52 5					
Concentrated Loads (lb) Vert: 9=548							
5) Dead + 0.6 C-C Wind (P	os. Internal) Case 2: Lumb	per Increase=1.60, Plate Increase=1	.60				
Uniform Loads (plf) Vert: 1-2=51, 2-	3=58, 3-4=51, 5-6=42, 2-8	=52, 8-10=115, 7-10=52					
Horz: 1-2=-63, 2 Concentrated Loads (lb)	2-3=-70, 3-4=-63, 3-5=-55						
Vert: 9=566	ea. Internal) Case 1: Lum	per Increase=1.60, Plate Increase=1	60				
Uniform Loads (plf)	·		.00				
	8=-45, 3-4=17, 5-6=-58, 2- 8-3=25, 3-4=-37, 3-5=51	8=-9, 8-10=2, 7-10=-9					
Concentrated Loads (lb) Vert: 9=-420							
7) Dead + 0.6 C-C Wind (N	eg. Internal) Case 2: Luml	per Increase=1.60, Plate Increase=1	.60				
Uniform Loads (plf) Vert: 1-2=-39, 2	-3=-45, 3-4=-39, 5-6=-58,	2-8=-9, 8-10=2, 7-10=-9					
Horz: 1-2=19, 2 Concentrated Loads (lb)	3=25, 3-4=19, 3-5=51						
Vert: 9=-420							
3) Dead + 0.6 MWFRS Wir Uniform Loads (plf)	d (Pos. Internal) Left: Lur	ber Increase=1.60, Plate Increase=	1.60				
	3=21, 3-4=14, 5-6=-11, 2- 2-3=-33, 3-4=-26, 3-5=7	8=10, 8-10=33, 7-10=10					
Concentrated Loads (lb)							
	d (Pos. Internal) Right: Lu	mber Increase=1.60, Plate Increase	=1.60				
Uniform Loads (plf) Vert: 1-2=6, 2-3	=12, 3-4=28, 5-6=1, 2-7=-	12					
Horz: 1-2=-18, 2 Concentrated Loads (Ib)	-3=-24, 3-4=-40, 3-5=-27						
Vert: 9=43							
 Dead + 0.6 MWFRS W Uniform Loads (plf) 	ind (Neg. Internal) Left: Lu	mber Increase=1.60, Plate Increase	=1.60				
	3=-1, 3-4=6, 5-6=-33, 2-8= 2-3=-19, 3-4=-26, 3-5=34						
Concentrated Loads (Ib							
Vert: 9=-339 11) Dead + 0.6 MWFRS W	nd (Neg. Internal) Right: L	umber Increase=1.60, Plate Increas	e=1.60				
Uniform Loads (plf) Vert: 1-2=-2_2	-3=-9, 3-4=-2, 5-6=-21, 2-3	7=-20					
Horz: 1-2=-18,	2-3=-11, 3-4=-18, 3-5=-0						
Concentrated Loads (lb Vert: 9=-234	,						
 Dead + 0.6 MWFRS W Uniform Loads (plf) 	nd (Pos. Internal) 1st Para	allel: Lumber Increase=1.60, Plate Ir	crease=1.60				
Vert: 1-2=14, 2	-3=21, 3-4=14, 5-6=-11, 2 2-3=-33, 3-4=-26, 3-5=-39						
Concentrated Loads (Ib							
Vert: 9=43 3) Dead + 0.6 MWFRS W	ind (Pos. Internal) 2nd Par	allel: Lumber Increase=1.60, Plate I	ncrease=1.60				
Uniform Loads (plf)	3=9, 3-4=2, 5-6=1, 2-7=-1	2					
Horz: 1-2=-14,	2-3=-21, 3-4=-14, 3-5=-27						
Concentrated Loads (lb Vert: 9=43	,						
14) Dead + 0.6 MWFRS W Uniform Loads (plf)	nd (Pos. Internal) 3rd Par	allel: Lumber Increase=1.60, Plate Ir	ncrease=1.60				
Vert: 1-2=14, 2	-3=21, 3-4=14, 5-6=-11, 2						
Concentrated Loads (Ib	2-3=-33, 3-4=-26, 3-5=-39)	9					
Vert: 9=43							

Continued on page 3

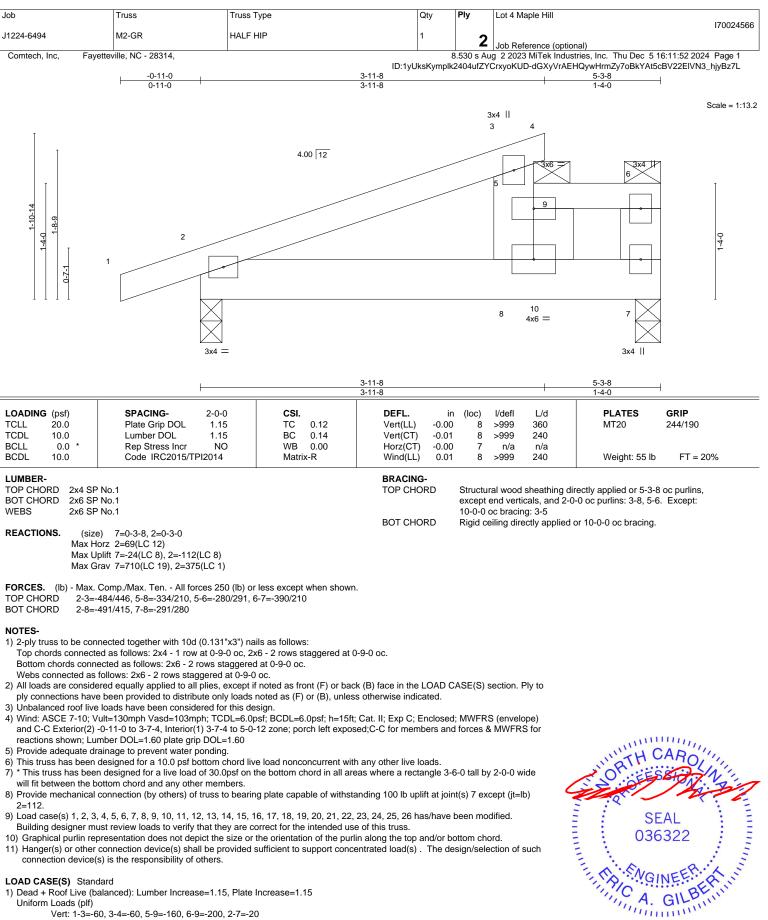
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qt	ty	Ply	Lot 4 Maple Hill		170024565
J1224-6494	M2	Half Hip	3		1			170024905
Comtech, Inc, Faye	teville, NC - 28314,				8.530 s Au	Job Reference (optional) Jg 2 2023 MiTek Industries,	Inc. Thu Dec 5 16:11:51 202	24 Page 3
			ID:1yUksKy	/mplk24	404ufZYCi	rxyoKUD-94zalW9cW7q3ghB	3NOQHyCLdgRCqlJc_bHjJQ	9HyBz7M
LOAD CASE(S) Standa	ard							
Uniform Loads (plf) Vert [.] 1-2=2	2-3=9, 3-4=2, 5-6=1, 2-7	/=-12						
Horz: 1-2=-	14, 2-3=-21, 3-4=-14, 3-5							
Concentrated Loads Vert: 9=43	(lb)							
	Wind (Neg. Internal) 1st	Parallel: Lumber Increase=1.60, Plate	Increase=1.60					
Uniform Loads (plf)	(0)							
	2-3=-1, 3-4=6, 5-6=-33, 26, 2-3=-19, 3-4=-26, 3-5							
Concentrated Loads		=-12						
Vert: 9=-23	1							
 Dead + 0.6 MWFRS Uniform Loads (plf) 	Wind (Neg. Internal) 2nd	Parallel: Lumber Increase=1.60, Plate	Increase=1.60					
	, 2-3=-13, 3-4=-6, 5-6=-2	1, 2-7=-20						
Horz: 1-2=-	14, 2-3=-7, 3-4=-14, 3-5=							
Concentrated Loads Vert: 9=-23								
	+ ase=0.90, Plate Increase	=0.90 Plt. metal=0.90						
Uniform Loads (plf)								
Vert: 1-3=-2 Concentrated Loads	0, 3-4=-20, 5-6=-120, 2-7	/=-20						
Vert: 9=-20	. ,							
	ve (bal.) + 0.75 Attic Floo	r + 0.75(0.6 MWFRS Wind (Neg. Int) L	eft): Lumber Incre	ease=1	.60, Plate	e Increase=1.60		
Uniform Loads (plf)	1 2-336 3-431 5-0-	=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=	-3					
	19, 2-3=-14, 3-4=-19, 3-5		-0					
Concentrated Loads								
Vert: 9=-45 20) Dead + 0 75 Roof Li		r + 0.75(0.6 MWFRS Wind (Neg. Int) F	ight): Lumber Incr	rease=	1 60 Pla	te Increase=1.60		
Uniform Loads (plf)			light). Europer mer	10030-	- 1.00, 1 10			
	7, 2-3=-42, 3-4=-37, 5-9=							
Horz: 1-2=- Concentrated Loads	13, 2-3=-8, 3-4=-13, 3-5= (lb)	-0						
Vert: 9=-37	()							
	ve (bal.) + 0.75 Attic Floo	r + 0.75(0.6 MWFRS Wind (Neg. Int) 1	st Parallel): Lumb	er Incr	ease=1.6	0, Plate Increase=1.60		
Uniform Loads (plf) Vert: 1-2=-3	1, 2-3=-36, 3-4=-31, 5-9=	95, 6-9=-125, 2-7=-20						
	19, 2-3=-14, 3-4=-19, 3-5							
Concentrated Loads								
Vert: 9=-37 22) Dead + 0.75 Roof Li		r + 0.75(0.6 MWFRS Wind (Neg. Int) 2	nd Parallel): Lumb	ber Inc	rease=1.6	60. Plate Increase=1.60		
Uniform Loads (plf)								
	0, 2-3=-45, 3-4=-40, 5-9=							
Concentrated Loads	10, 2-3=-5, 3-4=-10, 3-5= (lb)	-0						
Vert: 9=-37	5							
23) 1st Dead + Roof Liv Uniform Loads (plf)	e (unbalanced): Lumber I	ncrease=1.15, Plate Increase=1.15						
	0, 3-4=-60, 5-6=-40, 2-7=	=-20						
Concentrated Loads								
24) 2nd Dead + Roof Liv		Increase=1.15, Plate Increase=1.15						
Uniform Loads (plf)	e (unbalanceu). Lumber	increase=1.15, Flate increase=1.15						
	0, 3-4=-20, 5-9=-40, 6-9=							
Concentrated Loads Vert: 9=-40								
		nber Increase=1.15, Plate Increase=1.7	5					
Uniform Loads (plf)								
Vert: 1-3=-5 Concentrated Loads	i0, 3-4=-50, 5-6=-100, 2-7 (lb)	/=-20						
Vert: 9=-35)ີ້							
/	of Live (unbalanced): Lun	ber Increase=1.15, Plate Increase=1.1	5					
Uniform Loads (plf) Vert: 1-3=-2	0, 3-4=-20, 5-9=-100, 6-9	9=-130. 2-7=-20						
Concentrated Loads	(lb)	,						
)							

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Vert: 1-3=-60, 3-4=-60, 5-9=-160, 6-9=-200, 2-7=-20

Continued on page 2

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818 Soundside Road Edenton, NC 27932

December 6,2024

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Fayetteville, NC - 28314, Comtech, Inc,

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400
 Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb)
Vert: 9=-350 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40
Concentrated Loads (lb) Vert: 9=-300
 Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
Concentrated Loads (lb) Vert: 9=548
 Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (lb) Vert: 9=566
 bead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51 Concentrated Loads (lb)
Vert: 9=-420 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51 Concentrated Loads (lb)
Vert: 9=-420 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7 Concentrated Loads (lb)
Vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (lb) Vert: 9=43
 Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Concentrated Loads (lb) Vert: 9=-339
 Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Concentrated Loads (lb) Vert: 9=-234
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb)
Vert: 9=43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb)
Vert: 9=43 Yest (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Continued on page 3

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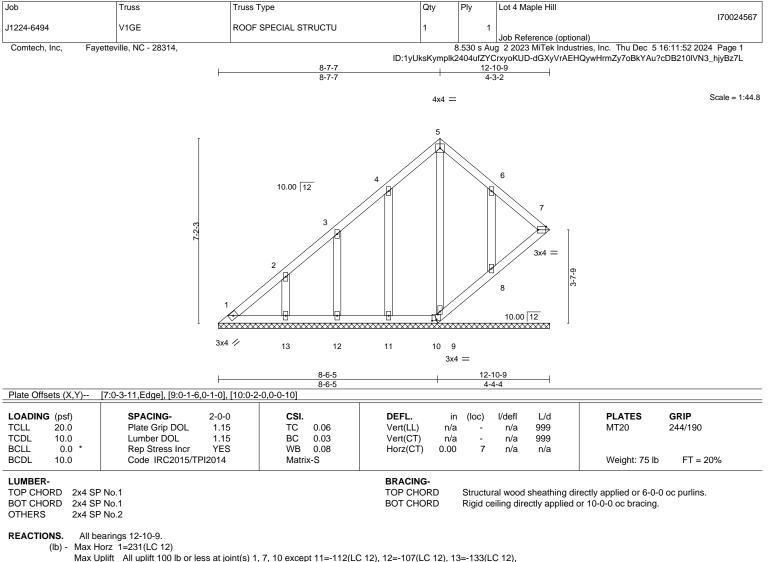


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ID:1yUksKymplk2404ufZYCrxyoKUD-dGXyVrAEHQywHrmZy7oBkYAt5cBV22EIVN3_hjyBz7L LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20 Concentrated Loads (lb) Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26 Concentrated Loads (lb) Vert: 9=-454 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (lb) Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350

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Max Uplift All uplift 100 8 8=-126(LC 13)

1-2=-295/189

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

TOP CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.

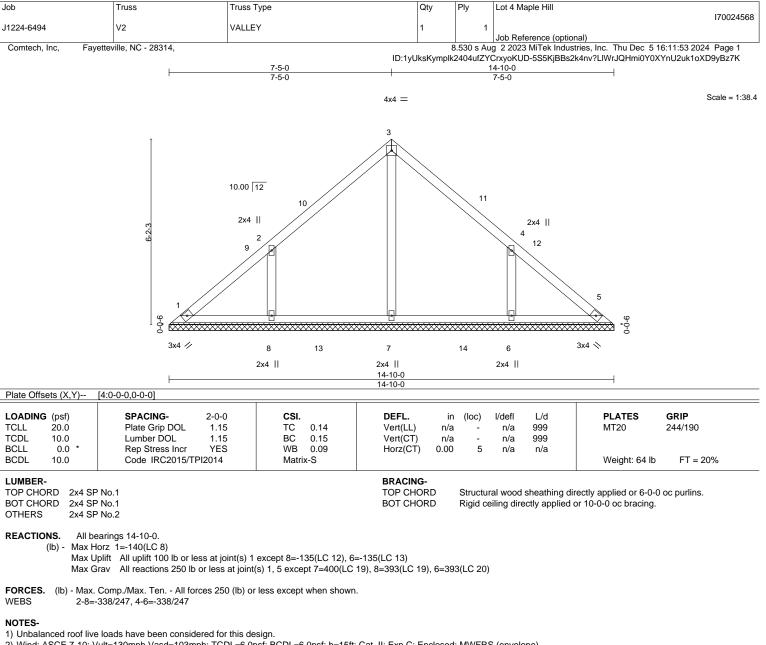
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.

SEAL 036322 December 6,2024

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TRENCO A Mitek Affiliate

818 Soundside Road



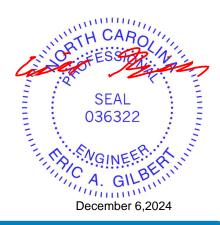
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

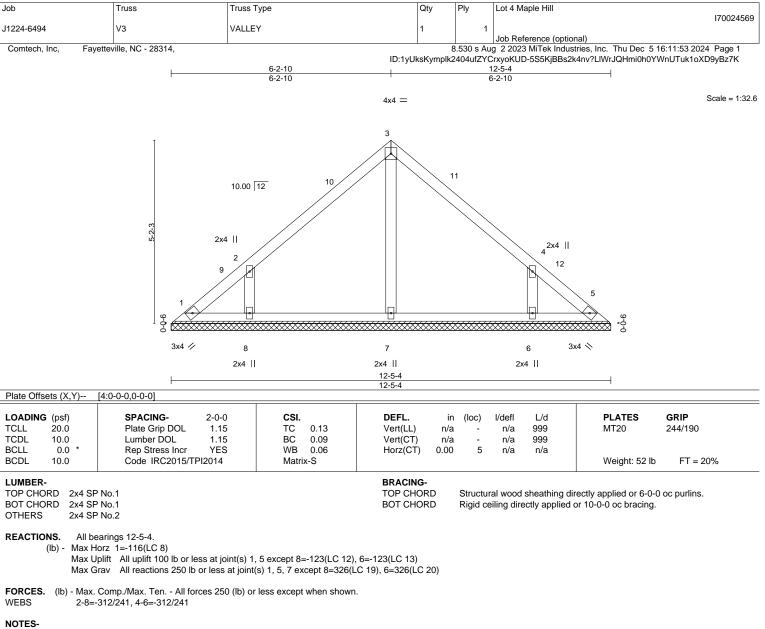
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135.



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1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

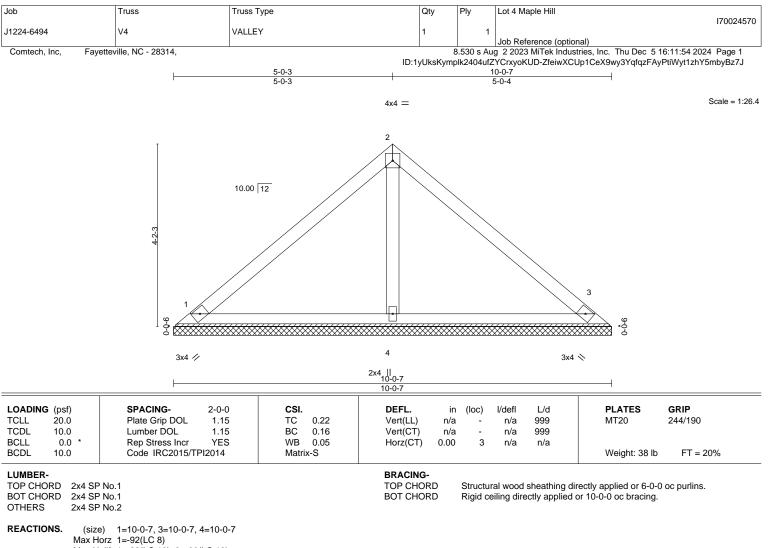
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123. 6=123.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Max Uplift 1=-22(LC 13), 3=-30(LC 13) Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

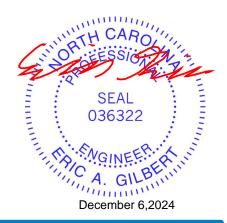
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

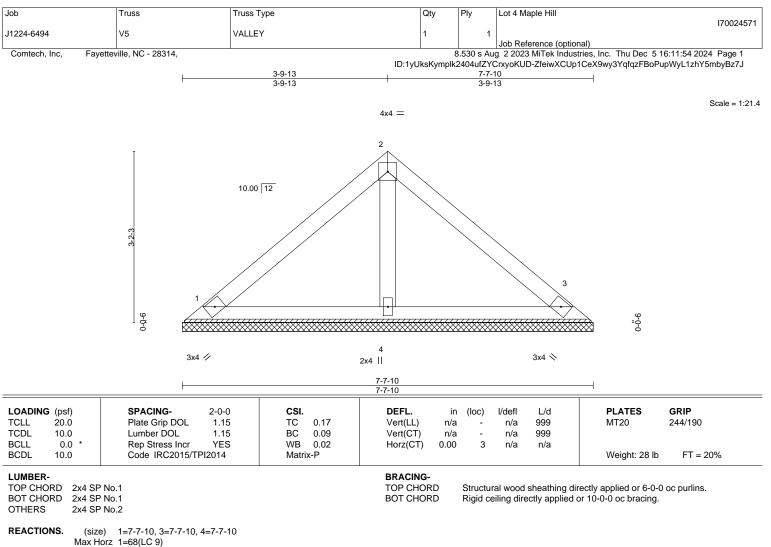
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Max Holz 1=68(LC 9) Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

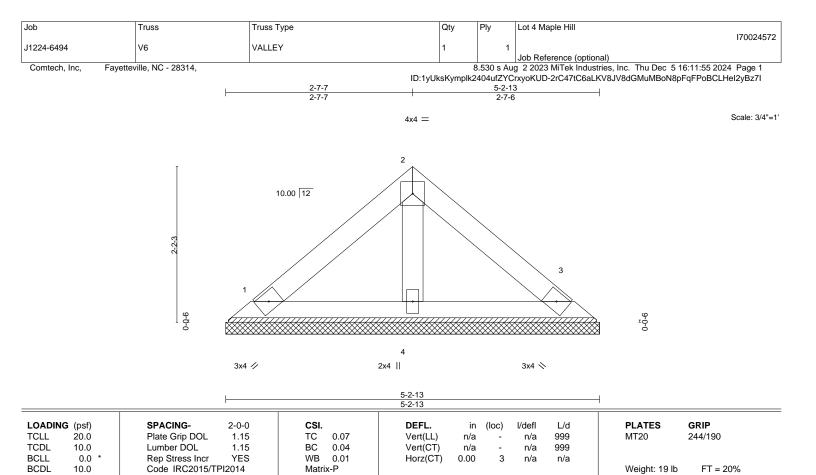
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

A MITEK Affilia 818 Soundside Road



LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-2-13 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13 Max Horz 1=-44(LC 8) Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

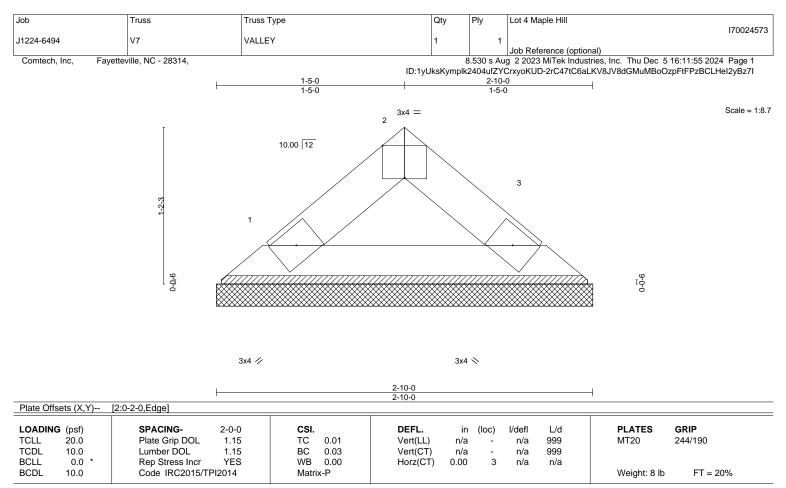
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0 Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



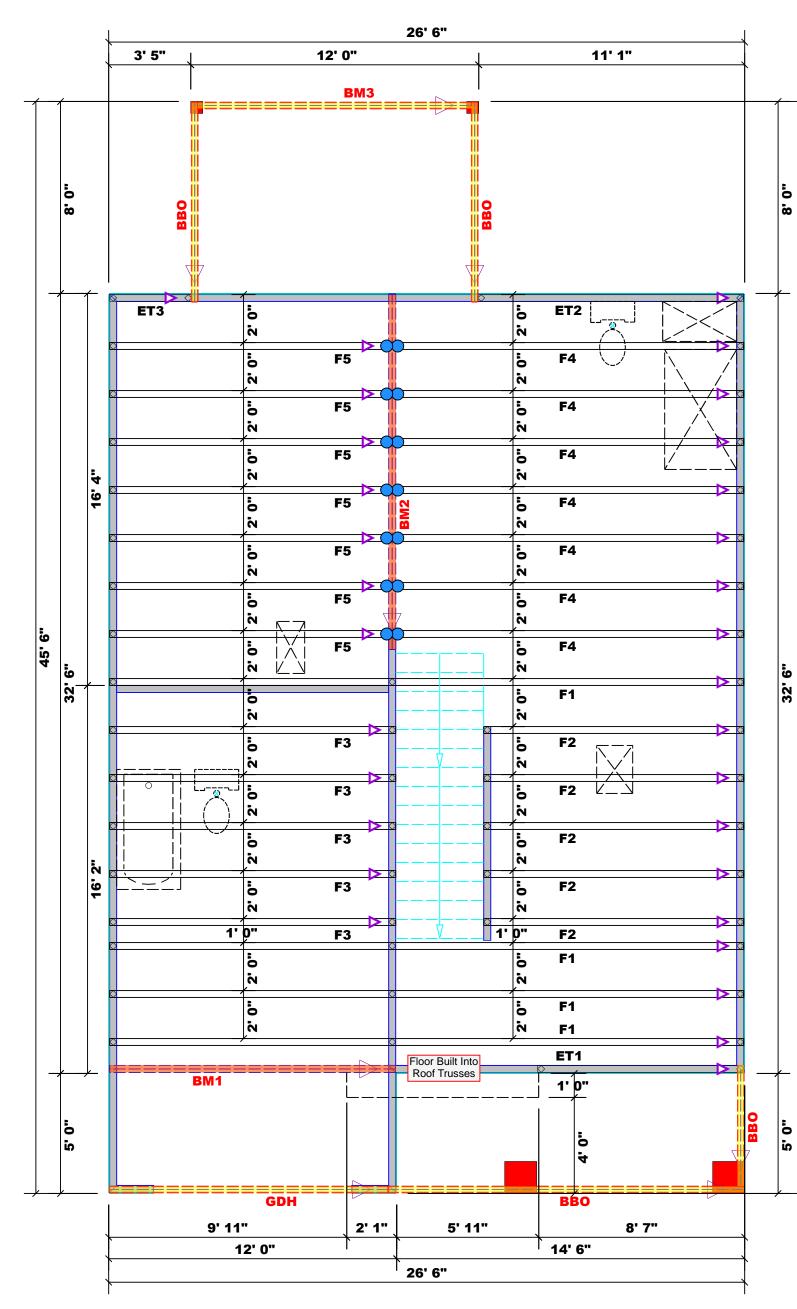
Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek Affil 818 Soundside Road Edenton, NC 27932





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Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmat	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

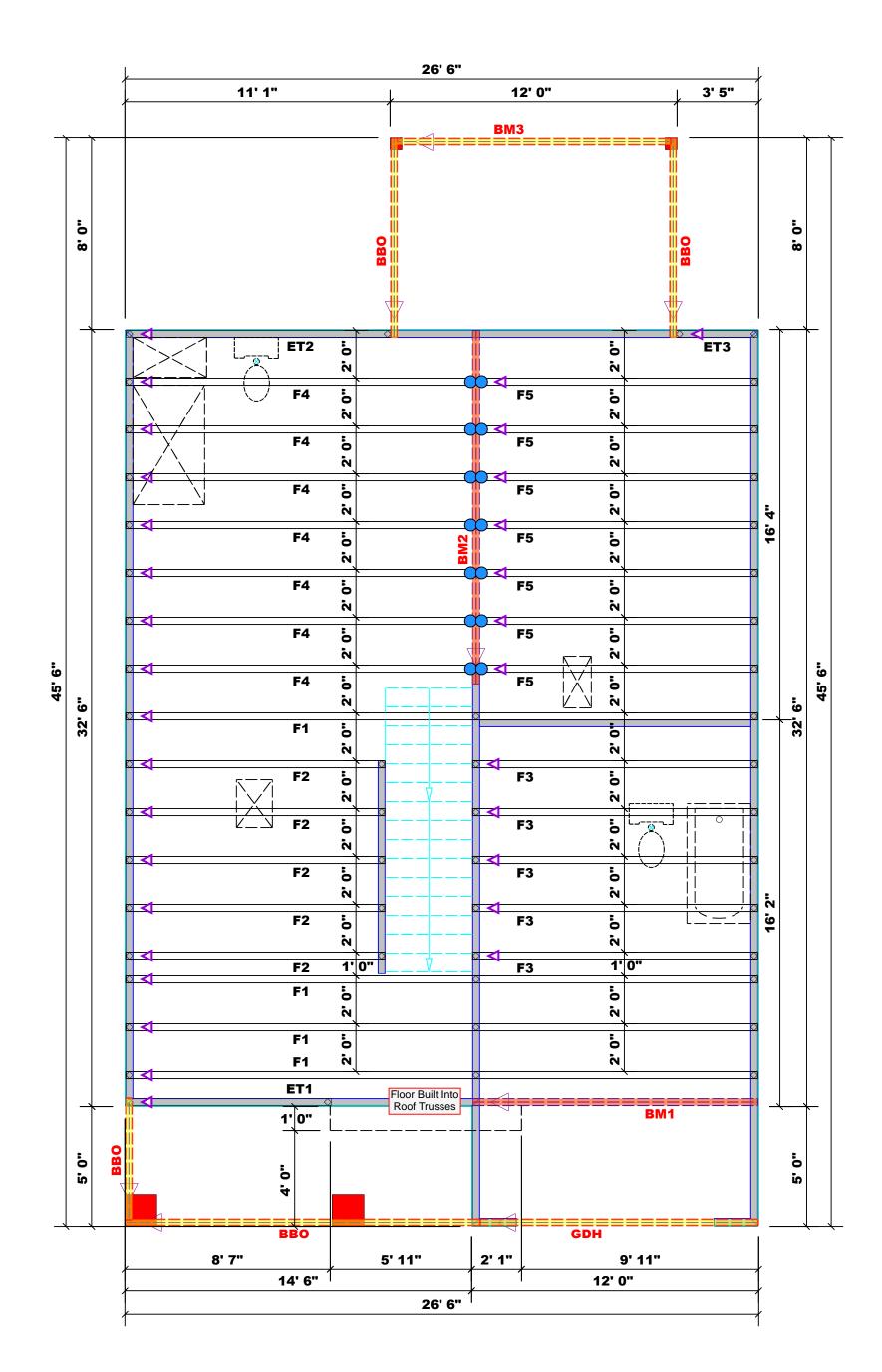
		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	12' 0"	2x12 SPF No.2	2	2

Truss Placement Plan Scale: 1/4"=1'



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Bearing	Fax:	e: (910 : (910)	864-4		are
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CITY / CO. Lillington / Harnett	ADDRESS 4182 Darroch Rd.	MODEL Floor	DATE REV . 12/05/24	DRAWN BY David Landry	SALES REP. Lenny Norris
Weaver Homes, Inc.	Lot 4 Maple Hill	Hickory "A" / GLF, CP	N/A		J1224-6495
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
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Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

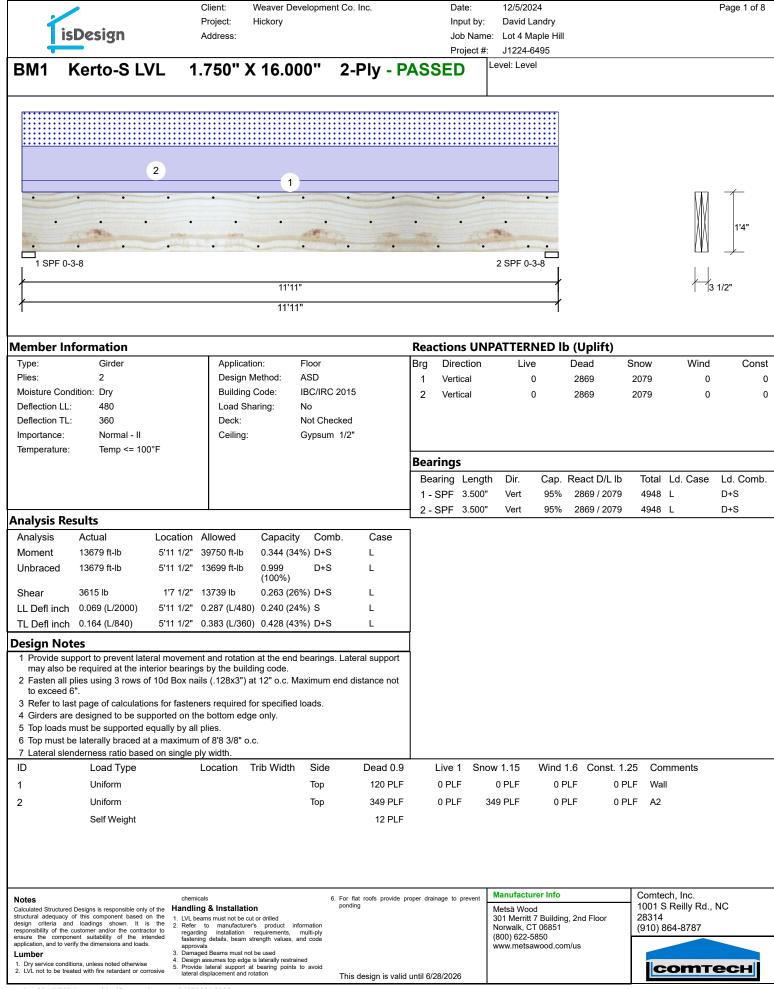
Plumbing Drop Notes
1. Plumbing drop locations shown are NOT exact.
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	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	If Qty Supported Member		Header	Truss
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

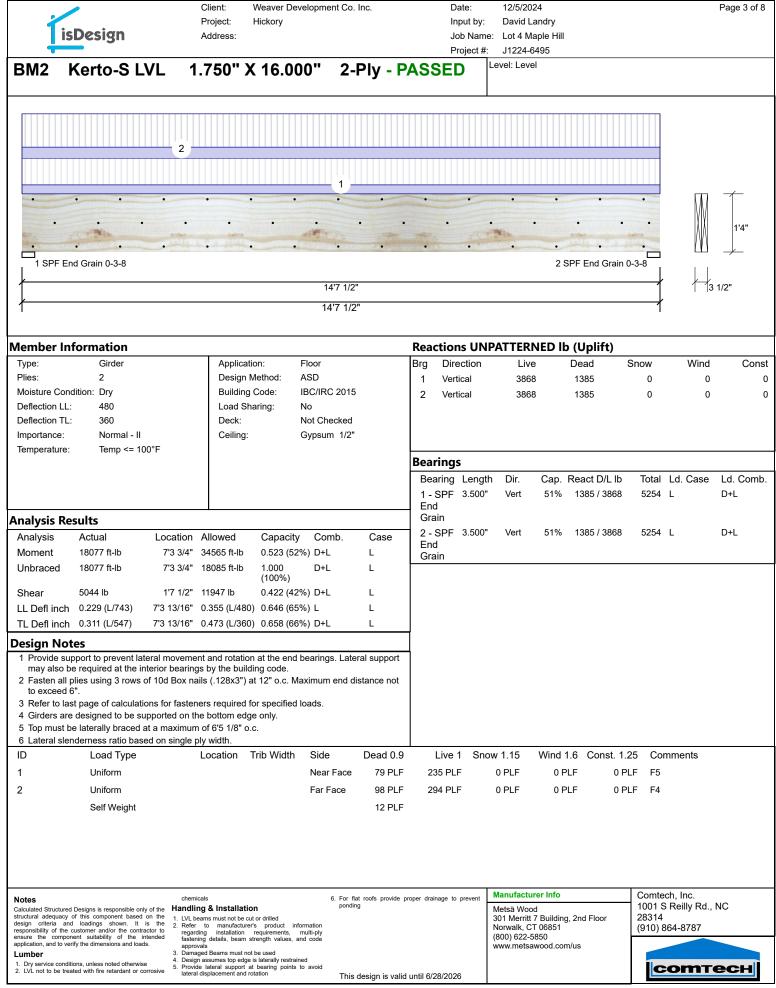
		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	12' 0"	2x12 SPF No.2	2	2

Truss Placement Plan Scale: 1/4"=1'



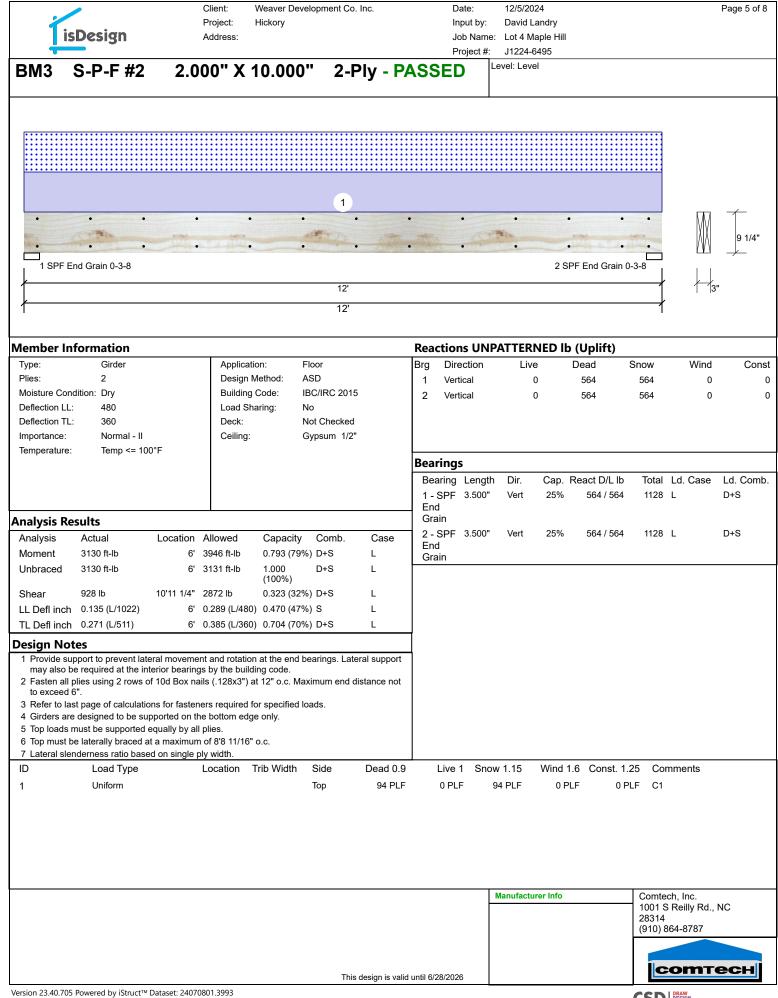


	Client: Project:	Weaver Developm Hickory	ent Co. Inc.	Date: Input b			Page 2 of 8
isDesign	Address:			Job Na Projec	t #: J1224-6495	11	
BM1 Kerto-S L	VL 1.750"2	X 16.000"	2-Ply	- PASSED	Level: Level		
	· · ·	• •	•	· · · · · · · · · · · · · · · · · · ·	• • • •	 <1 1/2"	1'4"
1 SPF 0-3-8		11'11"			2 SPF 0-3-8	,	3 1/2"
<u>/</u>		11'11"				,	3 1/2
Fasten all plies using 3 rov Capacity Load Yield Limit per Foot Yield Limit per Fastener CM Yield Mode Edge Distance Load Combination Duration Factor	0.0 % 0.0 PLF 245.6 PLF 81.9 lb. 1 IV 1 1/2" 3" 1.00						
Notes	chemicals		6. For flat roofs pro	ovide proper drainage to preven	Manufacturer Info	·	Comtech, Inc.
Calculated Structured Designs is responsible only structural adequacy of this component based design criteria and loadings shown. It responsibility of the customer and/or the contra ensure the component suitability of the in	on the 1. LVL beams must not be is the 2. Refer to manufactur regarding installation	cut or drilled er's product information requirements, multi-ply	ponding		Metsä Wood 301 Merritt 7 Buildi Norwalk, CT 06851 (800) 622-5850		1001 S Reilly Rd., NC 28314 (910) 864-8787
ensure the component suitability of the in application, and to verify the dimensions and load Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or cc	s. approvals 3. Damaged Beams must r 4. Design assumes top edg	e is laterally restrained at bearing points to avoid			www.metsawood.c	om/us	соттесн
	ateral displacement and	rotation	This design is	valid until 6/28/2026			



Version 23.40.705 Powered by iStruct[™] Dataset: 24070801.3993

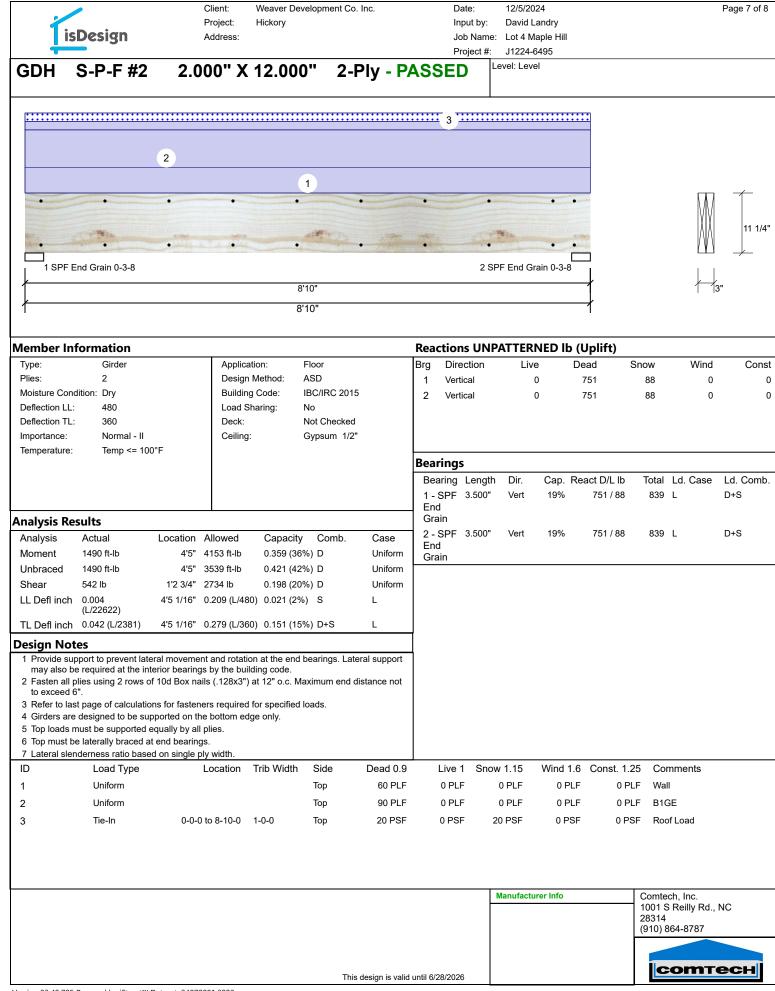
isDesign	Client: Wea Project: Hick Address:	ver Development Co. Inc. ory	Date: Input by: Job Name Project #:	12/5/2024 David Landry b: Lot 4 Maple Hill J1224-6495	Page 4 of 8
BM2 Kerto-S LVL	. 1.750" X 1	6.000" 2-Ply		Level: Level	
1 SPF End Grain 0-3-8	· · ·	· · · ·	• • •	2 SPF End Grain 0	
		14'7 1/2"			3 1/2"
 		14'7 1/2"			
Load 1	9.8 % 96.0 PLF	x3") at 12" o.c Maxim	um end distance nc	ot to exceed 6".	
Yield Limit per Fastener 8 Cm 1 Yield Mode 1					
	")+L .00				
Notes Calculated Structured Designs is responsible only of th structural adequacy of this component based on th design criteria and loadings shown. It is th responsibility of the customer and/or the contractor the ensure the component suitability of the intender	 1. LVL beams must not be cut or drill 2. Refer to manufacturer's properties of the second secon	ponding ed vduct information ments, multi-ply	vide proper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	Comtech, Inc. 1001 S Reilly Rd., NC 28314 (910) 864-8787
application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	approvals 3. Damaged Beams must not be use 4. Design assumes top edge is later: 5. Brouide lateral support at hearing	d Illy restrained Ig points to avoid	valid until 6/28/2026	www.metsawood.com/us	соттесн



ĺ	isDesign		Client: Projec Addre	t: ⊢	Veaver Develop lickory	ment Co. Inc.		Date: Input by: Job Name: Project #:	12/5/2024 David Land Lot 4 Mapl J1224-649	e Hill			Page 6 of
M3	S-P-F #	‡2	2.000"	X 1	0.000"	2-Ply	- PASS	ED	evel: Level				
•	•	•	•	•	•	•	•	•	•	•	• •	∏ }	1
•	•	•	•	•	•	•	•	•	•	•	• • -	ĮΫ₩	9 1/4'
1 SPF	End Grain 0-3-8									2 SPF En	d Grain 0-3-8		
						12'					,	1 13	"
						12'						ł	

Capacity 0.0 % Load 0.0 PLF Yield Limit per Foot 157.4 PLF Yield Limit per Fastener 78.7 lb. См 1 Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination 1.00 Duration Factor

1001 S Reilly Rd., NC 28314 (910) 864-8787
сотесн



	Client: Weaver [Project: Hickory	evelopment Co. Inc.	Date: Input by:	12/5/2024 David Landry	Page 8 of 8
isDesign	Address:		Job Name:	Lot 4 Maple Hill	
GDH S-P-F #2	2.000" X 12.00)0" 2-Plv	Project #:	J1224-6495 evel: Level	
	2.000 / 12.00		TAGGED		
	• •	• •	• •		M
	• •	• •	• •		_ 11 1/4"
1 SPF End Grain 0-3-8			2 SF	PF End Grain 0-3-8	
¢		8'10"			3"
<i>∤</i>		8'10"			
Multi-Ply Analysis Fasten all plies using 2 row	s of 10d Box nails (128v3"	at 12" o.c. Mavim	um end distance no	t to exceed 6"	
Capacity	0.0 %				
Load Yield Limit per Foot	0.0 PLF 157.4 PLF				
Yield Limit per Fastener См	78.7 lb. 1				
Yield Mode Edge Distance	IV 1 1/2"				
Min. End Distance	3"				
Load Combination Duration Factor	1.00				
	1.00				
			Γ	Manufacturer Info	Comtech, Inc.
			F		1001 S Reilly Rd., NC 28314
					(910) 864-8787
		This design i	s valid until 6/28/2026		соттесн



RE: J1224-6495 Lot 4 Maple Hill Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J1224-6495 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7	Seal# I70024575 I70024576 I70024577 I70024578 I70024579 I70024580 I70024581	Truss Name ET1 ET2 ET3 F1 F2 F3 F4	Date 12/6/2024 12/6/2024 12/6/2024 12/6/2024 12/6/2024 12/6/2024
6			
7	170024581	F4	12/6/2024
8	170024582	F5	12/6/2024

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

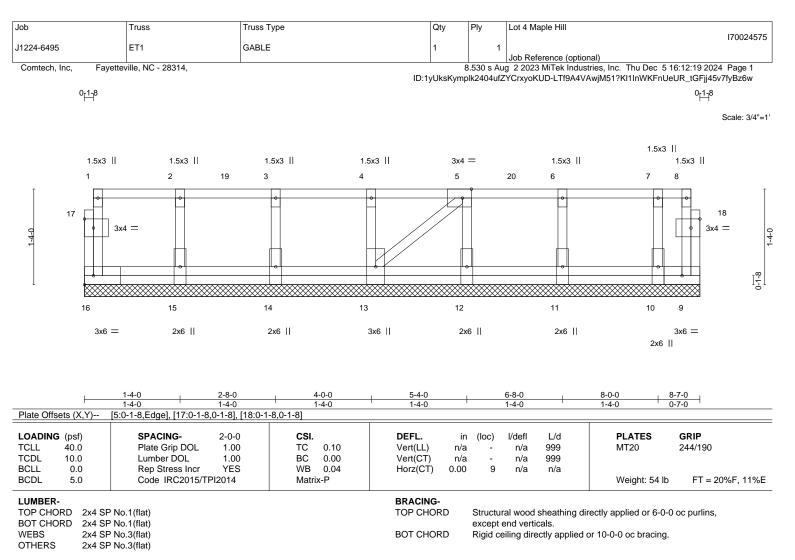
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10. 1-8=-100

Concentrated Loads (lb)

Vert: 4=-71 7=-77 19=-71 20=-71



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Job	Truss	1	Truss Type		Qty	Ply	Lot 4 Maple Hill		170024576
J1224-6495	ET2		GABLE		1	1			170024576
Comtech, Inc,	Fayetteville, NC - 283	314,		ID			Job Reference (optional) g 2 2023 MiTek Industries YCrxyoKUD-LTf9A4VAwjM		
0 _[1] 8									0 ₁ 18
									Scale = 1:18.0
1	2	3	4 3x4 =	5	6		7	8	9 10
•	0	•		0	G	-	0	•	<u> </u>
20									21 ©
0 0 0 0 0 0 0									-4- -4- -6-
	•	•				-	•		
19	18	17	16	15	14		13	12	11
3x4 =				3x4 =					3x6 =

H	1-4-0	2-8-0	4-0-0		5-4-0	6-8-0			8-0-0		9-4-0		0-8-0	11-1-0 0-5-0
Plate Offs	1-4-0 ets (X,Y)		1-4-0 ,Edge]		1-4-0	1-4-0			1-4-0		1-4-0		1-4-0	0-5-0
	i (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d		PLATES	GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a	999		MT20	244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999				
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT	0.00	11	n/a	n/a				
BCDL	5.0	Code IRC2015/T	PI2014	Matri	x-S							Weight: 54 lb	FT =	20%F, 11%E
	-					BRACIN	3-							
TOP CHC BOT CHC	RD 2x4 S	SP No.1(flat) SP No.1(flat)				TOP CHO			ural wood		g directly a	applied or 6-0-	0 oc purlins	,

2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3(flat)

REACTIONS. All bearings 11-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 19, 11, 18, 17, 16, 15, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

WEBS

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

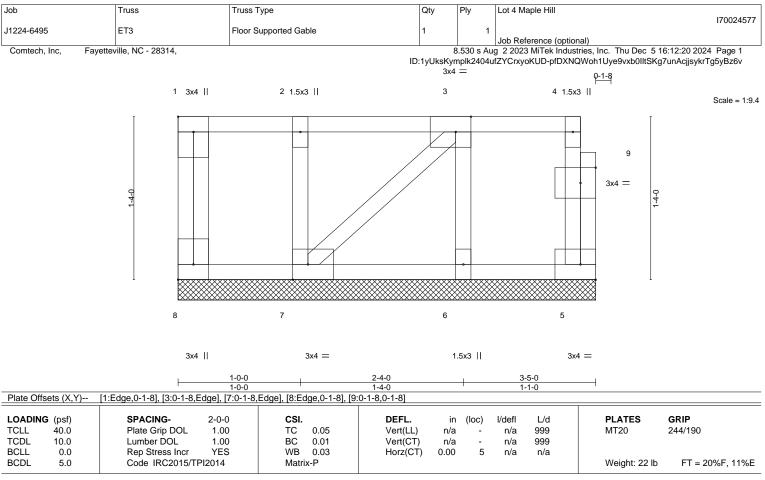
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 3-5-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

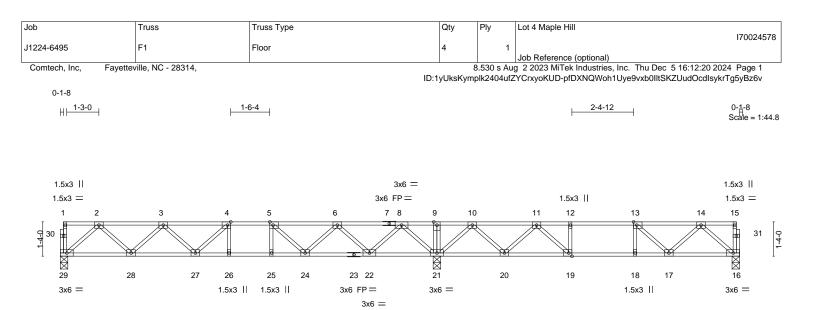
6) CAUTION, Do not erect truss backwards.



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818 Soundside Road



	14-7-12 14-7-12			<u>26-5-0</u> 11-9-4		
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge], [13:0-1-8	3,Edge], [19:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.47 BC 0.70 WB 0.44	Vert(LL) -0.1	in (loc) I/defl L/d 0 26-27 >999 480 3 26-27 >999 360 33 16 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	1012(01) 0.0	5 10 11/a 11/a	Weight: 136 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SF	No.1(flat) No.1(flat)		BRACING- TOP CHORD	Structural wood sheathing dire except end verticals.	, ,,,	oc purlins,
WEBS 2x4 SF	No.3(flat)		BOT CHORD	Rigid ceiling directly applied o	r 6-0-0 oc bracing.	
REACTIONS. (size Max G	e) 29=0-3-8, 21=0-3-8, 16=0-3-8 rav 29=727(LC 10), 21=1671(LC 1), 16	=557(LC 4)				
	Comp./Max. Ten All forces 250 (lb) or		0// /00			

 TOP CHORD
 2-3=-1257/0, 3-4=-1883/0, 4-5=-2002/0, 5-6=-1658/0, 6-8=-756/224, 8-9=0/1400, 9-10=0/1400, 10-11=-468/367, 11-12=-1158/0, 12-13=-1158/0, 13-14=-884/0

 BOT CHORD
 28-29=0/771, 27-28=0/1718, 26-27=0/2002, 25-26=0/2002, 24-25=0/2002, 22-24=-33/1341, 21-22=-443/147, 20-21=-637/20, 19-20=-175/900, 18-19=0/1158, 17-18=0/1158, 16-17=0/585

 WEBS
 2-29=-1023/0, 2-28=0/677, 3-28=-640/0, 8-21=-1289/0, 8-22=0/923, 10-21=-1064/0, 10-20=0/687, 11-20=-690/0, 11-19=0/582, 12-19=-278/0, 6-22=-882/0, 6-24=0/527, 5-24=-633/0, 14-16=-776/0, 14-17=0/416, 13-17=-373/90

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

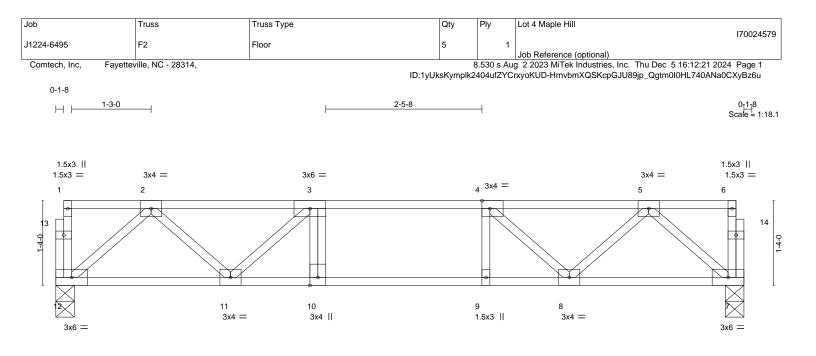
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



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			<u>10-10-0</u> 10-10-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge]		10-10-0			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl L/d	PLATES	GRIP
TCLL 40.0 TCDL 10.0	Plate Grip DOL 1.00 Lumber DOL 1.00	TC 0.36 BC 0.46	Vert(LL) -0.07 Vert(CT) -0.09		MT20	244/190
BCLL 0.0	Rep Stress Incr YES	WB 0.21	Horz(CT) 0.01			
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 57 lb	FT = 20%F, 11%E
LUMBER-			BRACING-			
	' No.1(flat) ' No.1(flat)		TOP CHORD	Structural wood sheathing dire except end verticals.	ectly applied or 6-0-0) oc purlins,
	P No.3(flat)		BOT CHORD	Rigid ceiling directly applied o	r 10-0-0 oc bracing.	
REACTIONS. (size Max G	e) 12=0-3-8, 7=0-3-8 rav 12=576(LC 1), 7=576(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-923/0, 3-4=-1242/0, 4-5=-925/0

BOT CHORD 11-12=0/606, 10-11=0/1242, 9-10=0/1242, 8-9=0/1242, 7-8=0/603

WEBS 2-12=-805/0, 2-11=0/441, 3-11=-469/0, 5-7=-800/0, 5-8=0/449, 4-8=-475/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

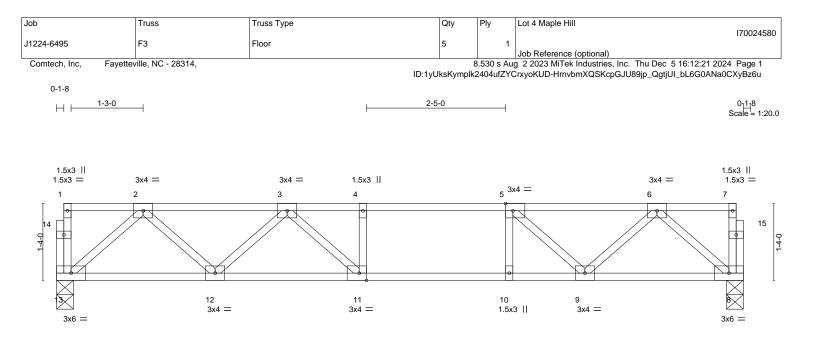
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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			11-11-0 11-11-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.52 BC 0.63 WB 0.27 Matrix-S	Vert(LL) -0.13	n (loc) l/defl L/d 3 11-12 >999 480 3 11-12 >894 360 2 8 n/a n/a	PLATES MT20 Weight: 61 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o) oc purlins,
REACTIONS. (siz Max G	e) 13=0-3-8, 8=0-3-8 Brav 13=635(LC 1), 8=635(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1060/0, 3-4=-1495/0, 4-5=-1495/0, 5-6=-1059/0

BOT CHORD 12-13=0/676, 11-12=0/1395, 10-11=0/1495, 9-10=0/1495, 8-9=0/658

```
WEBS 2-13=-898/0, 2-12=0/534, 3-12=-466/0, 3-11=-19/356, 6-8=-873/0, 6-9=0/557,
```

5-9=-617/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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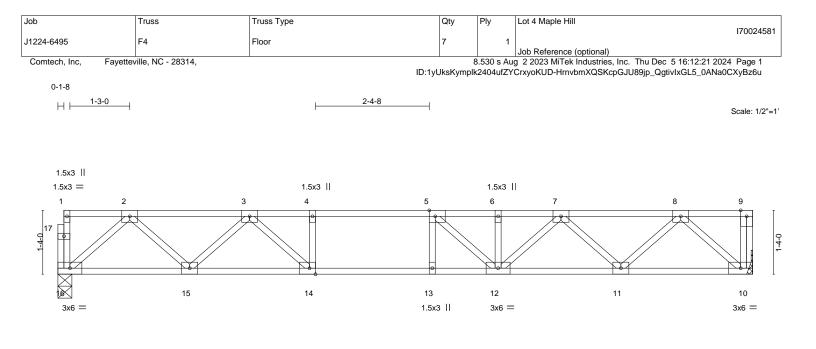


Plate Offsets (X,Y) [5:0-1-8,Edge], [14:0-1-8,Edge]		14-6-0 14-6-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.62 BC 0.78 WB 0.35	Vert(LL) -0.17	n (loc) l/defl L/d 7 12-13 >999 480 2 12-13 >790 360 3 10 n/a n/a	PLATES MT20	GRIP 244/190
BOT CHORD 2x4 SP	Code IRC2015/TPI2014 No.1(flat) No.1(flat) No.3(flat)	Matrix-S	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o		FT = 20%F, 11%E

REACTIONS. (size) 16=0-3-8, 10=Mechanical

Max Grav 16=778(LC 1), 10=784(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1348/0, 3-4=-2244/0, 4-5=-2244/0, 5-6=-2152/0, 6-7=-2152/0, 7-8=-1359/0 BOT CHORD

15-16=0/834, 14-15=0/1856, 13-14=0/2244, 12-13=0/2244, 11-12=0/1857, 10-11=0/835 WEBS

2-16=-1109/0, 2-15=0/714, 3-15=-707/0, 3-14=0/697, 4-14=-339/0, 8-10=-1111/0,

8-11=0/729, 7-11=-693/0, 7-12=0/401, 5-12=-438/123

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

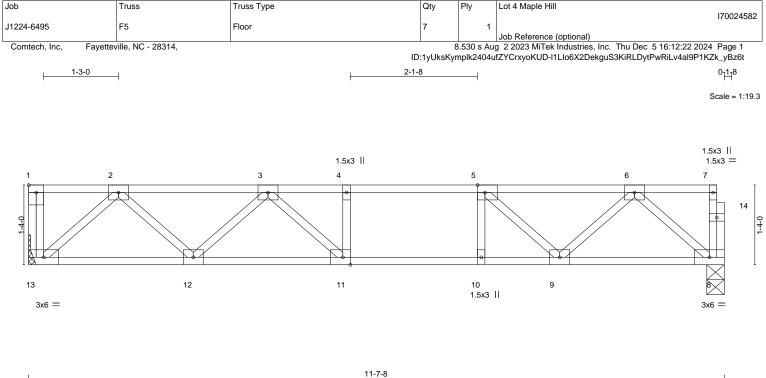
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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			11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [11:0-1-4	8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.56 WB 0.25 Matrix-S	Vert(LL) -0.1	in (loc) l/defl L/d 0 11-12 >999 480 3 11-12 >999 360 2 8 n/a n/a	PLATES MT20 Weight: 61 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	except end verticals.	ng directly applied or 6-0-0 lied or 10-0-0 oc bracing.) oc purlins,		

REACTIONS. (size) 13=Mechanical, 8=0-3-8 Max Grav 13=626(LC 1), 8=619(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1025/0, 3-4=-1427/0, 4-5=-1427/0, 5-6=-1022/0

BOT CHORD 12-13=0/658, 11-12=0/1345, 10-11=0/1427, 9-10=0/1427, 8-9=0/644

WEBS 2-13=-876/0, 2-12=0/510, 3-12=-446/0, 3-11=-41/323, 6-8=-854/0, 6-9=0/526,

5-9=-570/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

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