

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. - Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Roof Area = 1402.88 sq.ft. Ridge Line = 45.04 ft. Hip Line = 0 ft. Horiz. OH = 99.08 ft.Raked OH = 120.39 ft. Decking = 48 sheets

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise

3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



Truss Placement Plan

COMTECH **ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787

Fax: (910) 864-4444

Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS										
(BASED ON TABLES R502.5(1) & (b))										
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER										
(UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER			
700	1		2550	1		3400	1			
400	2		5100	2		6800	2			
100	3		7650	3		10200	3			
300	4		10200	4		13600	4			
500	5		12750	5		17000	5			
200	6		15300	6						
900	7									
600	8									

Weaver Homes	CITY / CO.	CITY / CO. Lillington / Harnett	13600 15300
Lot 1 Maple Hill	ADDRESS	4238 Darroch Road	9
Magnolia "A" / GRF, CP	MODEL	Roof	
N/A	DATE REV.	12/06/24	
	DRAWN BY	DRAWN BY Jonathan Landry	
J1224-6496	SALES REP.	SALES REP. Lenny Norris	

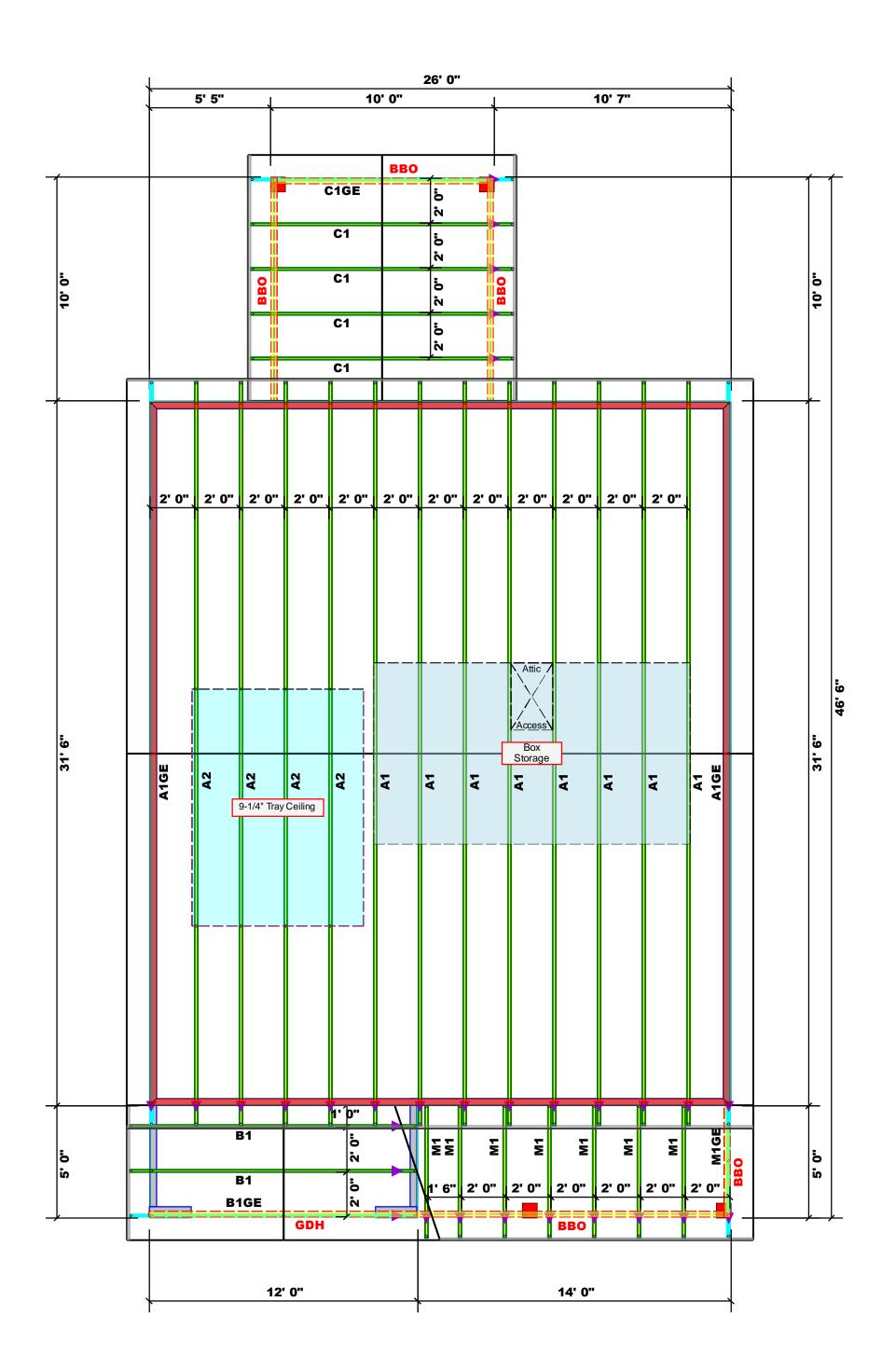
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

QUOTE;

JOB NAME

BUILDER

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. - Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Roof Area = 1402.88 sq.ft. Ridge Line = 45.04 ft. Hip Line = 0 ft. Horiz. OH = 99.08 ft. Raked OH = 120.39 ft. Decking = 48 sheets

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



Truss Placement Plan

соттесн **ROOF & FLOOR TRUSSES & BEAMS**

> Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Jonathan Landry

Jonathan Landry

OAD CHART FOR JACK STUDS											
(BASED ON TABLES R502.5(1) & (b))											
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER											
(UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER				
00	1		2550	1		3400	1				
00	2		5100	2		6800	2				
00	3		7650	3		10200	3				
00	4		10200	4		13600	4				
00	5		12750	5		17000	5				
00	6		15300	6							
00	7										
00	8										
00	9										

CITY / CO. Lillington / Harnett	4238 Darroch Road	Roof	12/06/24	Jonathan Landry	SALES REP. Lenny Norris
CITY / CO.	ADDRESS	WODEL ,	DATE REV.	DRAWN BY	SALES REP.

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Z/A

SEAL DATE

QUOTE;

Lot 1 Maple Hill

JOB NAME

Weaver Homes

BUILDER



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J1224-6496 Lot 1 Maple Hill

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I70033751 thru I70033759

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



December 6,2024

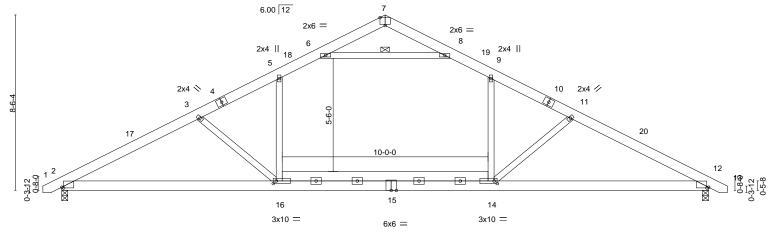
Johnson, Andrew

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 24-8-8 31-5-0 32-4-0 0-11-0 -0-11-0 0-11-0 20-8-8 6-8-8 4-0-0 5-0-0 5-0-0 4-0-0 6-8-8

Scale = 1:56.0



	10-8-8	1	20-8-8	L	31-5-0		
	10-8-8		10-0-0	ı	10-8-8		
Plate Offsets (X,Y) [2:0-1-0,0-0-7], [7:0-3-0,Edge], [12:0-1-0,0-0-7], [14:0-1-12,0-1-8]							
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)		PLATES GRIP		
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.60 BC 0.54	Vert(LL) -0.24 12-14 Vert(CT) -0.35 12-14		MT20 244/190		
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.40 Matrix-S	Horz(CT) 0.06 12 Wind(LL) 0.18 2-16		Weight: 223 lb FT = 20%	,	

LUMBER-

2x6 SP No.1 TOP CHORD **BOT CHORD** 2x6 SP No.1 WEBS 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD **WEBS**

Structural wood sheathing directly applied or 4-6-8 oc purlins. Rigid ceiling directly applied or 9-10-6 oc bracing.

6-8 1 Row at midpt

REACTIONS. (size) 2=0-3-8, 12=0-3-8

Max Horz 2=-142(LC 10)

Max Uplift 2=-243(LC 12), 12=-243(LC 13) Max Grav 2=1364(LC 2), 12=1364(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2405/866, 3-5=-2109/763, 5-6=-1688/732, 6-7=0/269, 7-8=0/269, 8-9=-1688/732,

9-11=-2109/763, 11-12=-2406/866

BOT CHORD 2-16=-624/2126, 14-16=-380/1759, 12-14=-617/2079

WEBS 3-16=-532/324, 5-16=-97/742, 9-14=-97/742, 11-14=-532/324, 6-8=-2012/704

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 32-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 4x6 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2 and 243 lb uplift at
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



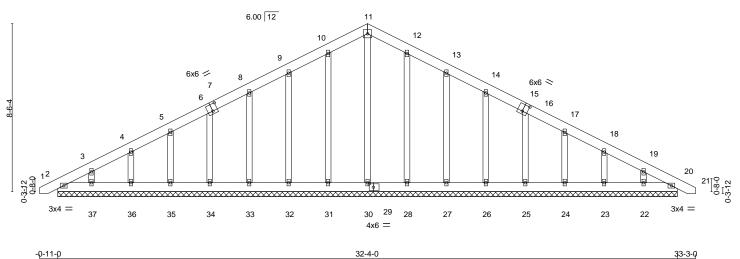


Job Truss Truss Type Qty Lot 1 Maple Hill 170033752 J1224-6496 A1GE **GABLE** 2 Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:36 2024 Page 1 Comtech, Inc, Fayetteville, NC - 28314,

ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f -0-11-0 0-11-0 15-8-8 15-8-8

5x5 =

Scale = 1:58.4



[7:0-3-0,0-4-4], [15:0-3-0,0-4-4], [29:0-2-8,0-2-0]

Plate Off	Plate Offsets (X,Y) [7:0-3-0,0-4-4], [15:0-3-0,0-4-4], [29:0-2-8,0-2-0]											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	0.00	20	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	0.00	20	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	20	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 248 lb	FT = 20%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. 2x6 SP No.1 **BOT CHORD BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.2

REACTIONS. All bearings 31-5-0.

Max Horz 2=221(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 31, 28 except 32=-118(LC 12), 33=-108(LC 12),

34=-108(LC 12), 35=-108(LC 12), 36=-109(LC 12), 37=-131(LC 12), 27=-121(LC 13), 26=-108(LC 13), 25=-107(LC 13), 24=-108(LC 13), 23=-109(LC 13), 22=-123(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 20, 30, 31, 32, 33, 34, 35, 36, 37, 28, 27, 26, 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD $2\text{-}3\text{--}299/97, \, 9\text{-}10\text{--}121/306, \, 10\text{-}11\text{--}142/366, \, 11\text{-}12\text{--}142/366, \, 12\text{-}13\text{--}121/306}$

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 31, 28 except (it=lb) 32=118, 33=108, 34=108, 35=108, 36=109, 37=131, 27=121, 26=108, 25=107, 24=108, 23=109, 22=123.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033753 J1224-6496 A2 **ROOF SPECIAL** Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:37 2024 Page 1 Comtech, Inc, Fayetteville, NC - 28314,

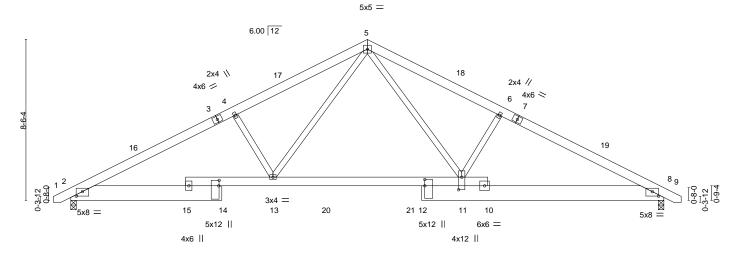
7-0-0

ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f 22-8-8 7-0-0 8-8-8

Structural wood sheathing directly applied or 4-9-13 oc purlins.

Rigid ceiling directly applied or 9-7-8 oc bracing.

Scale = 1:61.0



	8-0-0	2-8-8 7-	3-7-0 10-8	20-8-8 22-8-8 2-1-8 2-0-0		31-5-0 8-8-8	
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [8:0-4-0,0-2-14], [11:0	<u>0-8-0,0-2-0], [12:0-4-0,0-0-</u>	<u>0], [14:0-3-8,0-0-4]</u>]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.24 BC 0.63 WB 0.48 Matrix-S	Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) I/defl -0.17 11-13 >999 -0.29 11-13 >999 0.07 8 n/a 0.09 11-13 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 243 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1

BOT CHORD 2x10 SP No.1 *Except*

10-15: 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=142(LC 11)

Max Uplift 2=-243(LC 12), 8=-243(LC 13) Max Grav 2=1297(LC 1), 8=1297(LC 1)

8-8-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-2430/922, 4-5=-2182/940, 5-6=-2124/882, 6-8=-2325/855 TOP CHORD **BOT CHORD**

2-13=-650/2096, 11-13=-277/1352, 8-11=-608/1980

WEBS 4-13=-429/393, 5-13=-356/993, 5-11=-271/919, 6-11=-430/396

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 32-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



December 6,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033754 J1224-6496 **B1** COMMON 2 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

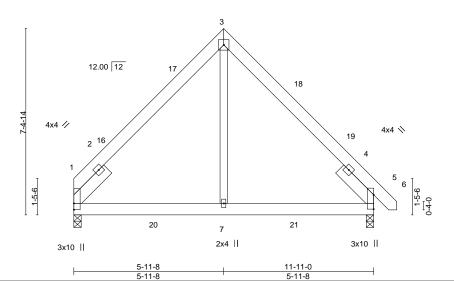
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:37 2024 Page 1

ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 11-11-0 5-11-8 5-11-8

> 5x5 = Scale = 1:45.8

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.02	7-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.14	Vert(CT)	-0.02	7-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.02	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI	2014	Matri	x-MS						Weight: 90 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WEBS

SLIDER Left 2x6 SP No.1 1-11-0, Right 2x6 SP No.1 1-11-0

REACTIONS. (size) 1=0-3-8, 5=0-3-8

Max Horz 1=-206(LC 8) Max Uplift 1=-76(LC 13), 5=-80(LC 12)

Max Grav 1=568(LC 20), 5=596(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-570/273, 3-5=-570/272 **BOT CHORD** 1-7=-23/381, 5-7=-23/381

WFBS 3-7=-66/387

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.





Job Truss Truss Type Qty Lot 1 Maple Hill 170033755 J1224-6496 B1GE **GABLE** Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:38 2024 Page 1

ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 5-11-8 5-11-8

> 5x5 = Scale = 1:43.7

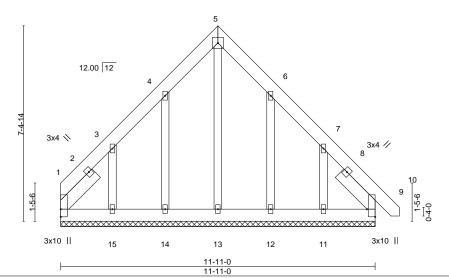


Plate Off	sets (X,Y)	[9:Edge,0-3-11]			
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.00 9 n/r 120	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) -0.00 9 n/r 120	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.14	Horz(CT) 0.00 9 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 111 lb FT = 20%

LUMBER-**BRACING-**

2x6 SP No.1 TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x6 SP No.1 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

OTHERS 2x4 SP No.2

SLIDER Left 2x6 SP No.1 1-11-0, Right 2x6 SP No.1 1-11-0

REACTIONS. All bearings 11-11-0.

Max Horz 1=276(LC 9) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 9 except 1=-129(LC 10), 14=-162(LC 12), 15=-361(LC 12),

12=-159(LC 13), 11=-343(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 12 except 15=274(LC 19), 11=251(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-299/208, 7-9=-256/153 **WEBS** 3-15=-319/352, 7-11=-319/335

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 1=129, 14=162, 15=361, 12=159, 11=343.



December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033756 J1224-6496 COMMON C₁ Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:38 2024 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f

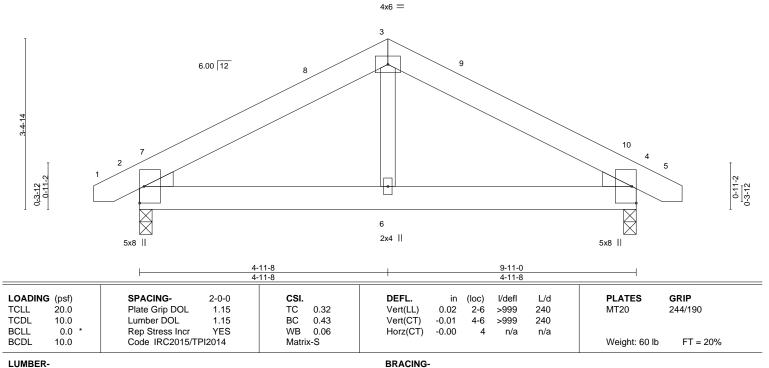
4-11-8

4-11-8

Scale = 1:23.0

10-10-0

0-11-0



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WEBS

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=-52(LC 8)

-0-11-0 0-11-0

Max Uplift 2=-158(LC 9), 4=-158(LC 8)

Max Grav 2=437(LC 1), 4=437(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-492/756, 3-4=-492/756 **BOT CHORD** 2-6=-534/359, 4-6=-534/359

WEBS 3-6=-433/229

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 4-11-8, Exterior(2) 4-11-8 to 9-4-5, Interior(1) 9-4-5 to 10-7-10 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=158, 4=158.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

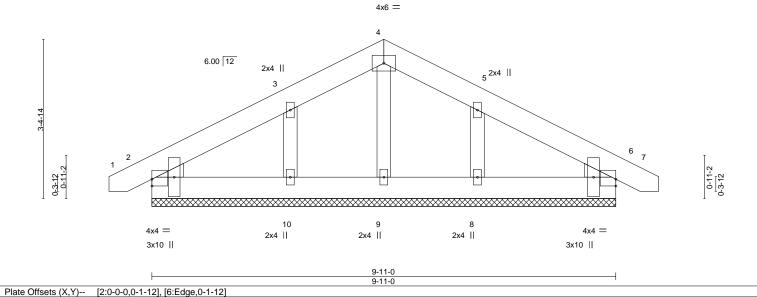
December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033757 J1224-6496 C1GE COMMON SUPPORTED GAB Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:39 2024 Page 1 Comtech, Inc.

ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f 10-10-0 9-11-0 4-11-8 4-11-8 0-11-0

Scale = 1:24.6



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.04 BC 0.02 WB 0.04	DEFL. in (loc) l/defl L/d Vert(LL) 0.00 6 n/r 120 Vert(CT) 0.00 6 n/r 120	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	WB 0.04 Matrix-S	Horz(CT) 0.00 6 n/a n/a	Weight: 64 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 **OTHERS** 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. All bearings 9-11-0.

Max Horz 2=-76(LC 13) (lb) -

0-11-0

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-177(LC 12), 8=-172(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WFBS 3-10=-169/255, 5-8=-169/255

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb)
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job Truss Truss Type Qty Lot 1 Maple Hill 170033758 J1224-6496 M1 MONOPITCH Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:39 2024 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 5-0-0 5-0-0 0-11-0 Scale = 1:14.6 3x4 | 4.00 12 3x4 | 3x4

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) 0.02 2-4 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.01 2-4 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 29 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD BOT CHORD

2x6 SP No.1 2x6 SP No.1

WEBS 2x6 SP No.1

REACTIONS. 2=0-3-0, 4=0-1-8 (size) Max Horz 2=79(LC 12)

Max Uplift 2=-140(LC 8), 4=-127(LC 8) Max Grav 2=235(LC 1), 4=182(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psg bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=140, 4=127,



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



Job Truss Truss Type Qty Lot 1 Maple Hill 170033759 J1224-6496 M1GE **GABLE** Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:28:39 2024 Page 1

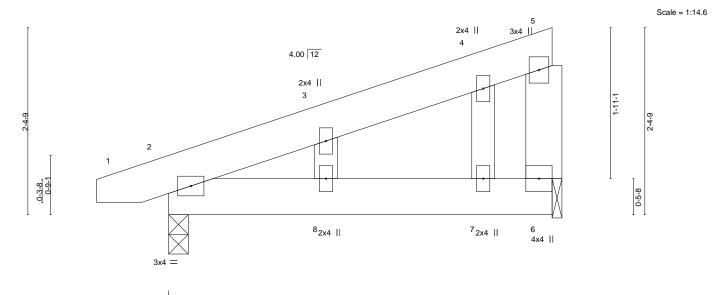
Comtech, Inc, Fayetteville, NC - 28314, ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

5-0-0 5-0-0 0-11-0



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) 0.01 8 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.01 8 >999 240	
BCLL 0.0 ³	Rep Stress Incr YES	WB 0.01	Horz(CT) -0.00 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 32 lb FT = 20%

BOT CHORD

LUMBER-BRACING-TOP CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x6 SP No.1 WEBS

OTHERS 2x4 SP No.2

> (size) 2=0-3-0, 6=0-1-8 Max Horz 2=113(LC 12)

Max Uplift 2=-192(LC 8), 6=-177(LC 8) Max Grav 2=235(LC 1), 6=182(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

REACTIONS.

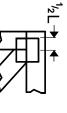
- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=192, 6=177.



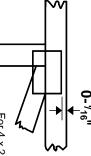


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

₹

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 × 4

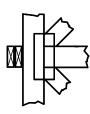
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur Min size shown is for crushing only.

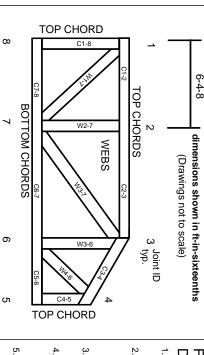
Industry Standards: ANSI/TPI1: National I

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

DSB-22:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

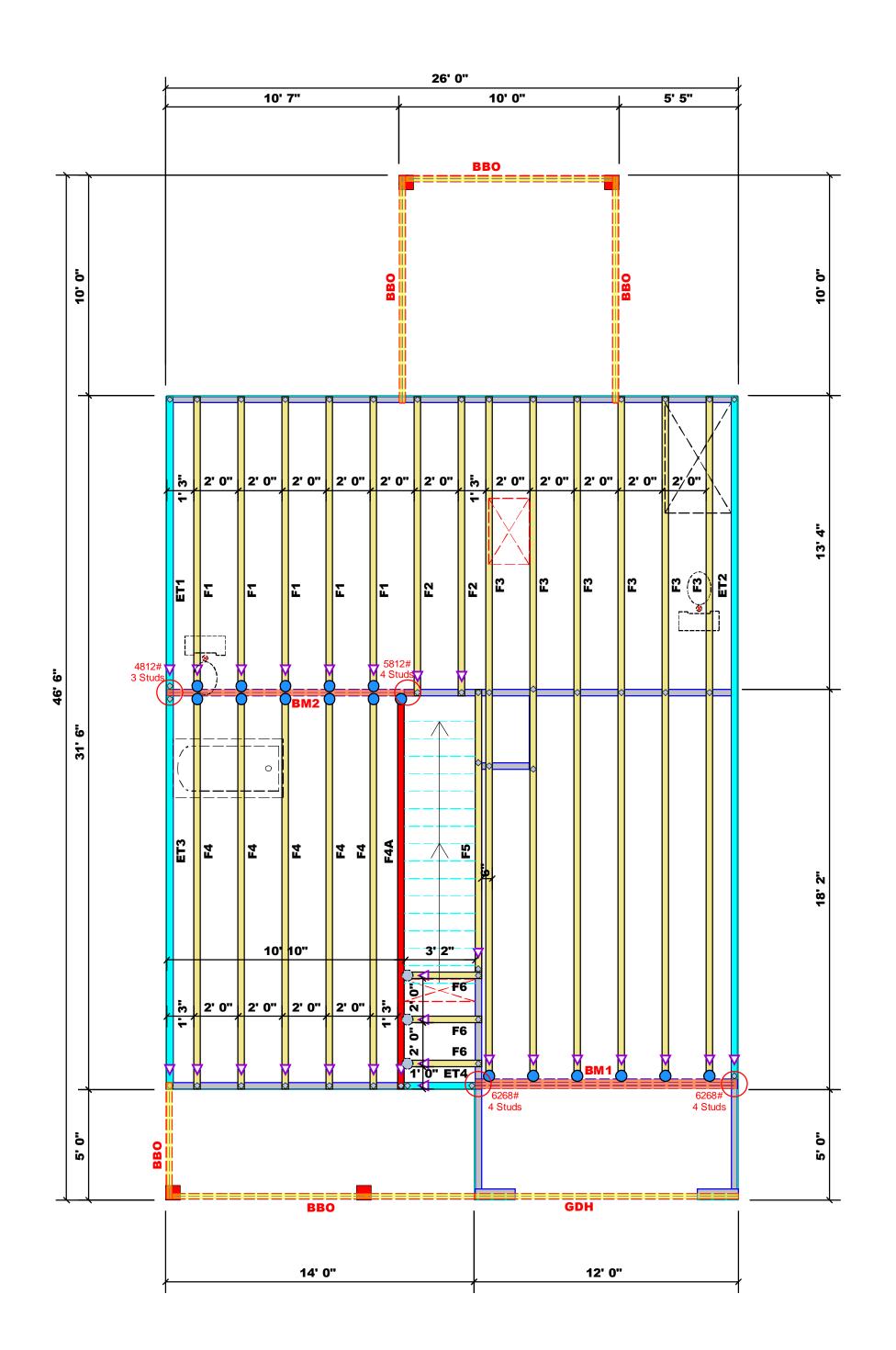
▲ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.



All Truss Reactions are Less
than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes
 Plumbing drop locations shown are NOT exact. Contractor to verify ALL plumbing drop
locations prior to setting Floor Trusses.

3. Adjust spacing as needed not to exceed 24"oc.

Dimension Notes	
1. All exterior wall to wall dimensions are to	
face of sheathing unless noted otherwise	
2. All interior wall dimensions are to face of	
stud unless noted otherwise	
3. All exterior wall to truss dimensions are to	
face of stud unless noted otherwise	



	Conne	ion	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header Truss		
	HUS410	USP	17	NA			
\bigcirc	MSH422	USP	3	Varies	10d/3"	10d/3"	

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH	12' 0"	2x12 SPF No.1	2	2	FF

Truss Placement Plan

Scale: 1/4"=1'

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

searing reactions less than or equal to 3000# are leemed to comply with the prescriptive Code equirements. The contractor shall refer to the ttached Tables (derived from the prescriptive Code equirements) to determine the minimum foundatic ize and number of wood studs required to suppore actions greater than 3000# but not greater than 5000#. A registered design professional shall be etained to design the support system for any eaction that exceeds those specified in the attach Tables. A registered design professional shall be etained to design the support system for all eactions that exceed 15000#.

Jonathan Landry

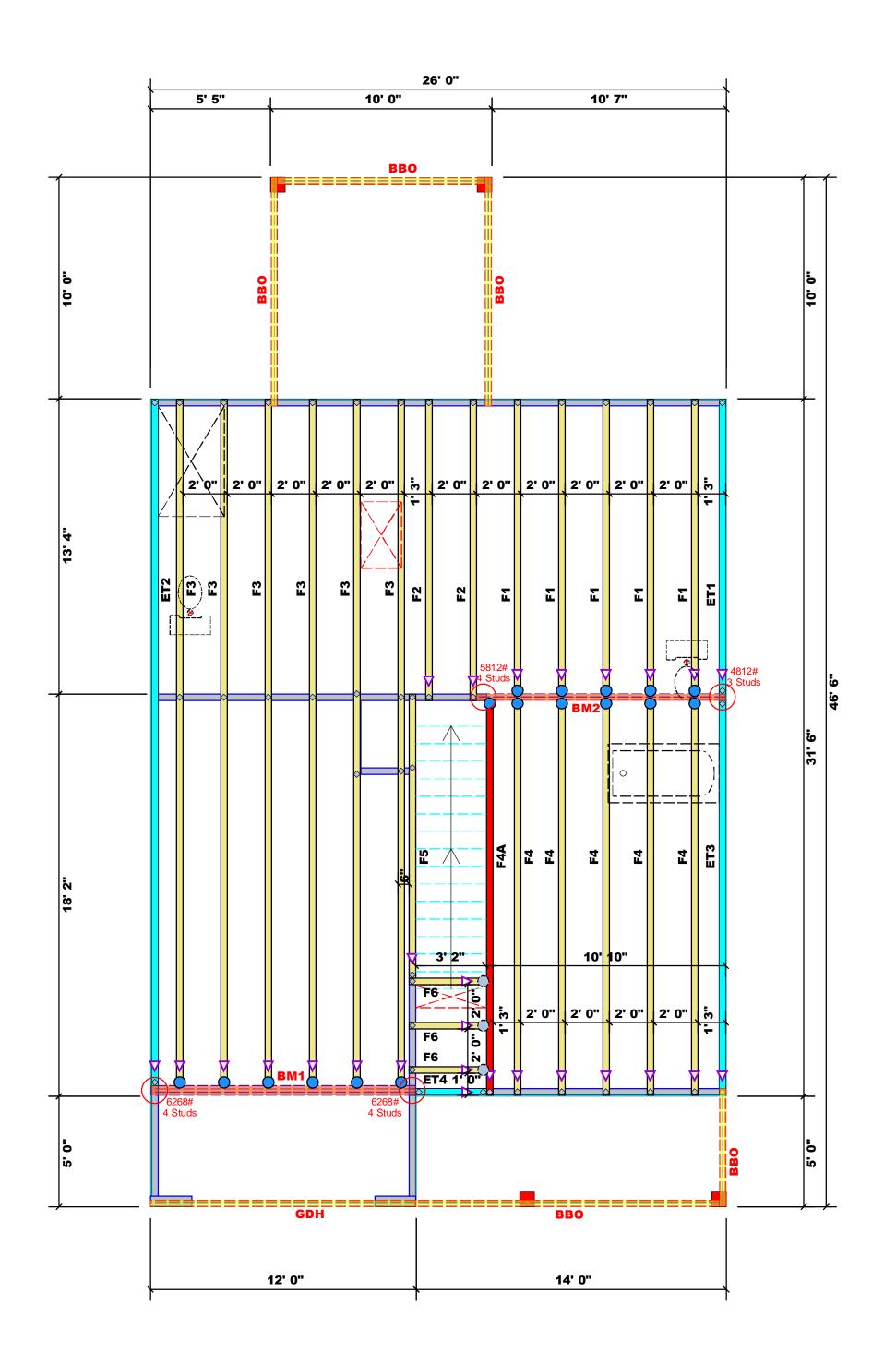
Jonathan Landry

LOAD CHART FOR JACK STUDS												
(BASED ON TABLES R502.5(1) & (b))												
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER												
END REACTION (UP TO) REQ'D STUDS FOR (2) PLY HEADER (3) PLY HEADER (3) PLY HEADER (4) PLY HEADER (4) PLY HEADER												
1700												
3400	2		5100	2		6800	2					
5100	3		7650	3		10200	3					
5800	4		10200	4		13600	4					
3500	5		12750	5		17000	5					
0200	6		15300	6								
1900	7											
3600	8											
5300	9											
					Т							

DER	Weaver Homes	CITY / CO.	CITY / CO. Lillington / Harnett	13600 15300
NAME	NAME Lot 1 Maple Hill	ADDRESS	4238 Darroch Road	8
_	Magnolia "A" / GRF, CP	MODEL	Floor	
DATE N/A	N/A	DATE REV.	12/06/24	
TE #		DRAWN BY	DRAWN BY Jonathan Landry	
#	J1224-6497	SALES REP.	SALES REP. Lenny Norris	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.cor



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

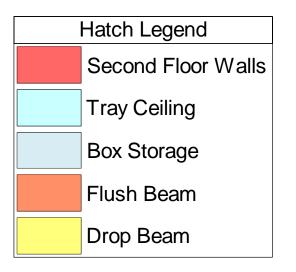
Plumbing Drop Notes

- Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop
- locations prior to setting Floor Trusses.

 3. Adjust spacing as needed not to exceed 24"oc.

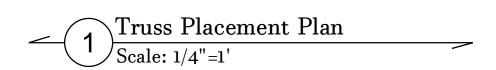
Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



	Conne	Nail Info	rmation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	17	NA	16d/3-1/2"	16d/3-1/2"
0	MSH422	USP	3	Varies	10d/3"	10d/3"

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH	12' 0"	2x12 SPF No.1	2	2	FF



▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

соттесн **ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS												
(BASED ON TABLES R502.5(1) & (b))												
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER												
END REACTION (UP TO)	EQU STUDS FOR (2) PLY HEADER (2) PLY HEADER (4) PLO) (4) PLO) REQ'D STUDS FOR (3) PLY HEADER (1) PLO) END REACTION (1) PLO) (1) PLO) END REACTION (1) PLO) END REACTION (1) PLO) END REACTION (1) PLO) (1) PLO) (1) PLO) (2) PLO) (3) PLO) (4) PLO) (4) PLO) (5) PLO) (6) PLO) (7) PLO) (7) PLO) (8) PLO) (9) PLO) (1) PLO) (1) PLO) (1) PLO) (1) PLO) (2) PLO) (3) PLO) (4) PLO) (4) PLO) (5) PLO) (6) PLO) (7) PLO) (7) PLO) (8) PLO) (9) PLO) (9) PLO) (1) PLO) (2) PLO) (3) PLO) (4) PLO) (4) PLO) (4) PLO) (5) PLO) (6) PLO) (6) PLO) (7) PLO) (7) PLO) (8) PLO) (9) PLO) (1) PLO)											
1700	1		2550	1		3400	1					
3400	2		5100	2		6800	2					
5100	3		7650	3		10200	3					
5800	4		10200	4		13600	4					
3500	5		12750	5		17000	5					
0200	6		15300	6								
1900	7											

Homes	CITY / CO.	CITY / CO. Lillington / Harnett	13600 15300	11900	10200	8500
ple Hill	ADDRESS	4238 Darroch Road	9	7	5 6 7 8	5
"A" / GRF, CP	MODEL	Floor			1530	1275
	DATE REV.	12/06/24				0 5
	DRAWN BY	Jonathan Landry				170
497	SALES REP.	SALES REP. Lenny Norris				00 5

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Weaver H

BUILDER

JOB NAME



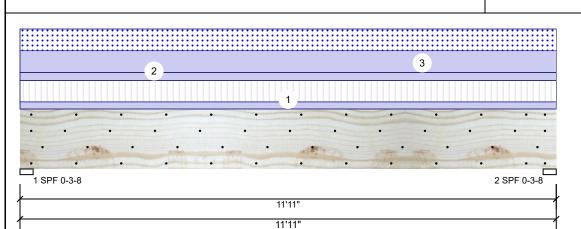
Weaver Homes Magnolia

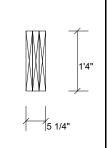
Date: 12/6/2024 Input by: Jonathan Landry Job Name: Lot 1 Maple Hill

Level: Level

4238 Darroch Road Lillington, NC 27546 Project #: J1224-6497

Kerto-S LVL 1.750" X 16.000" 3-Ply - PASSED BM₁





Page 1 of 6

Member Information

Type: Plies: 3 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Yes Not Checked Deck: Ceiling: Gypsum 1/2"

Reactions UNPATTERNED Ib (Uplift) Direction Snow Wind Brg Live Dead Const 1895 3394 1936 0 Vertical 0 2 Vertical 1895 3394 1936 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+0.75(L+S) Vert 3394 / 2873 6268 L 3.500" D+0.75(L+S) 2 - SPF Vert 80% 3394 / 2873 6268 L

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17327 ft-lb	5'11 1/2"	62010 ft-lb	0.279 (28%)	D+0.75(L+S)	L
Unbraced	17327 ft-lb	5'11 1/2"	17347 ft-lb	0.999 (100%)	D+0.75(L+S)	L
Shear	4437 lb	1'7 1/2"	17920 lb	0.248 (25%)	D+L	L
LL Defl inch	0.063 (L/2171)	5'11 1/2"	0.287 (L/480)	0.221 (22%)	0.75(L+S)	L
TL Defl inch	0.138 (L/995)	5'11 1/2"	0.383 (L/360)	0.362 (36%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 10'7 13/16" o.c.

7 Lateral slenderness ratio based on single bly width

/ Lateral Sieri	iderriess ratio based on singi	e piy widiii.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	106 PLF	318 PLF	0 PLF	0 PLF	0 PLF	F3	
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
3	Uniform			Тор	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	A2	
	Self Weight				19 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S Reilly Road Fayetteville Cumberland 28314

Version 23.40.705 Powered by iStruct™ Dataset: 24070801.3993



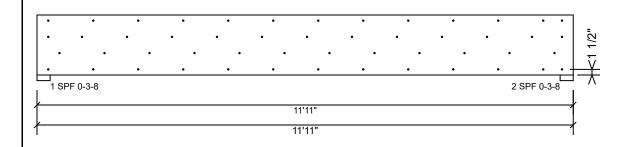
Weaver Homes Magnolia

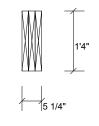
4238 Darroch Road Lillington, NC 27546 Date: 12/6/2024 Input by:

Jonathan Landry Job Name: Lot 1 Maple Hill Project #: J1224-6497

3-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" BM₁

Level: Level





Page 2 of 6

Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	86.3 %	
Load	282.7 PLF	
Yield Limit per Foot	327.4 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination	D+L	
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Informing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

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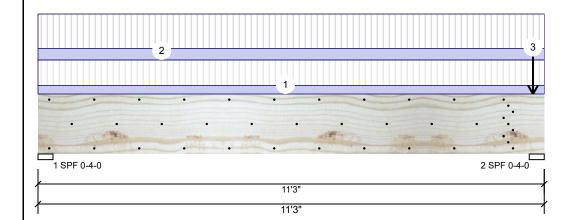
Weaver Homes Magnolia

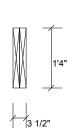
4238 Darroch Road Lillington, NC 27546 Date: 12/6/2024 Input by: Jonathan Landry Job Name: Lot 1 Maple Hill

Project #: J1224-6497

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED **BM2**

Level: Level





Page 3 of 6

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Not Checked Deck: Ceiling: Gypsum 1/2"

Reactions UNPATTERNED Ib (Uplift) Snow Wind Brg Direction Live Dead Const 3555 1257 Vertical n 0 0 2 Vertical 4305 1507 0 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 4.000" D+L Vert 1257 / 3555 4812 L 2 - SPF 4.000" Vert 98% 1507 / 4305 5812 L D+I

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	12309 ft-lb	5'7 1/2"	34565 ft-lb	0.356 (36%)	D+L	L
Unbraced	12309 ft-lb	5'7 1/2"	12310 ft-lb	1.000 (100%)	D+L	L
Shear	4527 lb	1'8"	11947 lb	0.379 (38%)	D+L	L
LL Defl inch	0.098 (L/1320)	5'7 1/2"	0.268 (L/480)	0.364 (36%)	L	L
TL Defl inch	0.132 (L/975)	5'7 1/2"	0.358 (L/360)	0.369 (37%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top must be laterally braced at a maximum of 9'9 3/16" o.c.

7 Lateral slende	erness ratio based on single	ply width.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	90 PLF	269 PLF	0 PLF	0 PLF	0 PLF	F1
2	Uniform			Near Face	121 PLF	363 PLF	0 PLF	0 PLF	0 PLF	F4
3	Point	11-0-0		Near Face	250 lb	750 lb	0 lb	0 lb	0 lb	F4A
	Self Weight				12 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S Reilly Road Fayetteville Cumberland 28314

Version 23.40.705 Powered by iStruct™ Dataset: 24070801.3993



Weaver Homes Magnolia

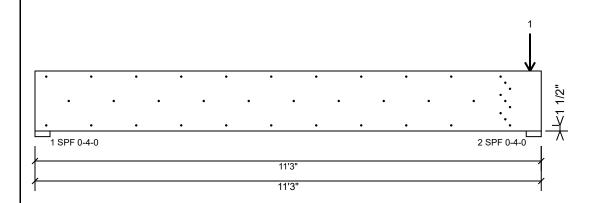
4238 Darroch Road Lillington, NC 27546 12/6/2024

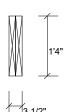
Input by: Jonathan Landry Job Name: Lot 1 Maple Hill Project #: J1224-6497

Kerto-S LVL 1.750" X 16.000" BM₂

2-Ply - PASSED

Level: Level





Page 4 of 6

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6".

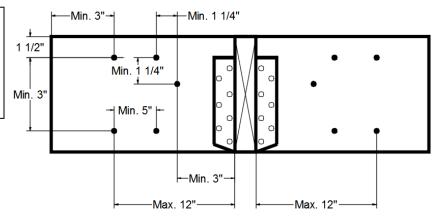
aa ona aistance not to exceed o				
Capacity	98.6 %			
Load	242.0 PLF			
Yield Limit per Foot	245.6 PLF			
Yield Limit per Fastener	81.9 lb.			
См	1			
Yield Mode	IV			
Edge Distance	1 1/2"			
Min. End Distance	3"			
Load Combination	D+L			
Duration Factor	1.00			

Concentrated Load

Fasten at concentrated side load at 11-0-0 with a minimum of (9) - 10d Box nails (.128x3") in the

pattern snown.		
Capacity	67.9 %	
Load	500.0lb.	
Total Yield Limit	736.5 lb.	
Cg	0.9998	
CM	1	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Load Combination	D+L	
Duration Factor	1.00	

Min/Max fastener distances for Concentrated Side Loads



Notes NOtes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S Reilly Road Fayetteville Cumberland 28314

CSD DESIGN



Weaver Homes Magnolia

4238 Darroch Road Lillington, NC 27546 Date: 12/6/2024 Input by: Jonathan Landry

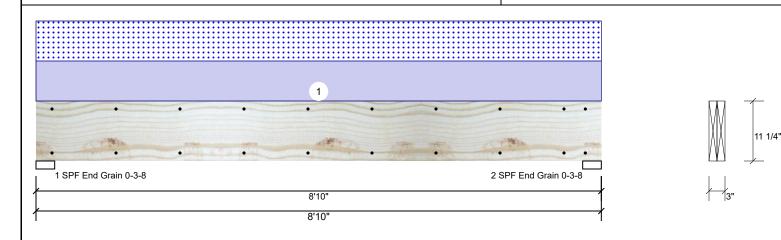
Job Name: Lot 1 Maple Hill Project #: J1224-6497

GDH S-P-F #1

2.000" X 12.000"

2-Ply - PASSED

Level: Level



Member Information Type: Girder

Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 360
Importance: Normal

Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015

Load Sharing: No

Deck: Not Checked

Ceiling: Gypsum 1/2"

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	530	530	0	0
2	Vertical	0	530	530	0	0

Page 5 of 6

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2104 ft-lb	4'5"	5306 ft-lb	0.397 (40%)	D+S	L
Unbraced	2104 ft-lb	4'5"	3969 ft-lb	0.530 (53%)	D+S	L
Shear	765 lb	1'2 3/4"	3493 lb	0.219 (22%)	D+S	L
LL Defl inch	0.027 (L/3770)	4'5 1/16"	0.209 (L/480)	0.127 (13%)	S	L
TL Defl inch	0.053 (L/1885)	4'5 1/16"	0.279 (L/360)	0.191 (19%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

Bearings	S						
Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	24%	530 / 530	1060	L	D+S
2 - SPF	3.500"	Vert	24%	530 / 530	1060	L	D+S

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	120 PLF	0 PLF	0 PLF	B1GE

End Grain

Manufacturer Info

Comtech, Inc.
1001 S Reilly Road
Fayetteville
Cumberland
28314

This design is valid until 6/28/2026



Weaver Homes Magnolia

Magnolia 4238 Darroch Road Lillington, NC 27546 Date: 12/6/2024 Input by: Jonathan I

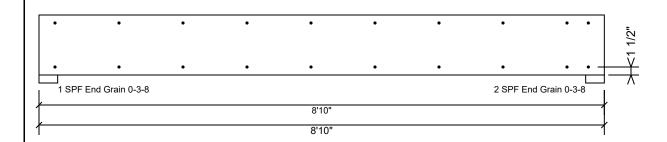
Input by: Jonathan Landry
Job Name: Lot 1 Maple Hill
Project #: J1224-6497

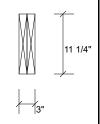
GDH S-P-F #1

2.000" X 12.000"

2-Ply - PASSED

Level: Level





Page 6 of 6

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	157.4 PLF
Yield Limit per Fastener	78.7 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Manufacturer Info	Comtech, Inc. 1001 S Reilly Road Fayetteville Cumberland 28314



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J1224-6497 Lot 1 Maple Hill

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I70033970 thru I70033980

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



December 6,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

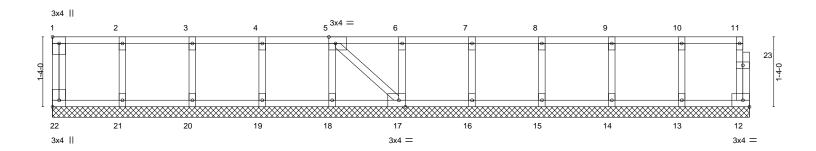
Job	Truss	Truss Type	Qty	Ply	Lot 1 Maple Hill
J1224-6497	ET4	GABLE	1	1	170033970
31224-0497		GABLE	'		Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:25 2024 Page 1 ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0₁1₁8

Scale = 1:22.0



L	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0		9-4-	0	10-8-0	12-0-0	13-3-8
	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	- 1	1-4-	0	1-4-0	1-4-0	1-3-8
Plate O	Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8	,Edge], [17:0	-1-8,Edge], [22	:Edge,0-1-8]							
LOADII	NG (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	12	n/a	n/a		
BCDL	5.0	Code IRC2015/7	ΓPI2014	Matri	x-S						Weight: 62 lb	FT = 20%F, 11%E

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat)

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

TOP CHORD **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



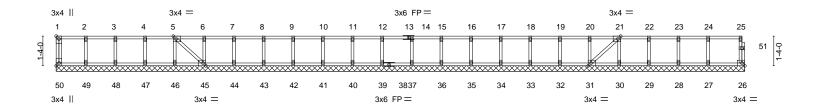
Job	Truss	Truss Type	Qty	Ply	Lot 1 Maple Hill
					170033971
J1224-6497	ET2	GABLE	1	1	
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:26 2024 Page 1 ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-1₁-8

Scale = 1:51.8



 $\frac{1.4-0}{1.4-0} \quad \frac{2.8-0}{1.4-0} \quad \frac{4.0-0}{1.4-0} \quad \frac{5.4-0}{1.4-0} \quad \frac{6.8-0}{1.4-0} \quad \frac{8-0-0}{1.4-0} \quad \frac{9.4-0}{1.4-0} \quad \frac{10.8-0}{1.4-0} \quad \frac{12-0-0}{1.4-0} \quad \frac{13-4-0}{1.4-0} \quad \frac{14-8-0}{1.4-0} \quad \frac{16-0-0}{1.4-0} \quad \frac{17-4-0}{1.4-0} \quad \frac{18-8-0}{1.4-0} \quad \frac{20-0-0}{1.4-0} \quad \frac{21-4-0}{1.4-0} \quad \frac{22-8-0}{1.4-0} \quad \frac{25-4-0}{1.4-0} \quad \frac{26-8-0}{1.4-0} \quad \frac{28-0-0}{1.4-0} \quad \frac{29-4-0}{1.4-0} \quad \frac{30-11-12}{1.4-0} \quad \frac{11-12}{1.4-0} \quad \frac{11-12}{1.$

Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [21:0-1-	8,Edge], [31:0-1-8,Edge],	[45:0-1-8,Edge], [50:Edge,0-1-8]	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.08	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.01	Vert(CT) n/a - n/a 999	
BCLL 0.0	Rep Stress Incr YES	WB 0.04	Horz(CT) -0.00 31 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 138 lb FT = 20%F, 11%E

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals. **WEBS** 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 30-11-12.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 50, 26, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.





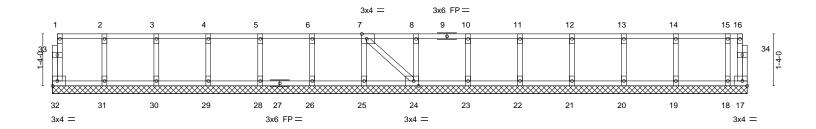
Jo	ob	Truss	Truss Type	Qty	Ply	Lot 1 Maple Hill	7
						170033972	2
J1	1224-6497	ET3	GABLE	1	1		
						Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:26 2024 Page 1 ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

0-11-8

0-<u>11</u>-8 Scale = 1:29.6



	1-4-0 1-4-0	2-8-0 4-0-0 1-4-0 1-4-0	5-4-0 1-4-0	6-8-0 1-4-0	8-0-0 1-4-0	-4-0 10-8 -4-0 1-4	-	12-0-0 1-4-0	13-4-0	14-8-0	16-0-0 1-4-0	17-4-0 17-10-0 1-4-0 0-6-0
Plate Offse	ets (X,Y)	[7:0-1-8,Edge], [24:0-1-8	,Edge]									
LOADING	(psf)	SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl L	/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a 99	99	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a 99	99		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	24	n/a n	/a		
BCDL	5.0	Code IRC2015/TI	PI2014	Mat	rix-S						Weight: 82 lb	FT = 20%F, 11%E
LUMBER-						BRACING	j -					

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat)

BOT CHORD **WEBS**

2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat)

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 17-10-0.

Max Uplift All uplift 100 lb or less at joint(s) 17

Max Grav All reactions 250 lb or less at joint(s) 32, 31, 30, 29, 28, 26, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



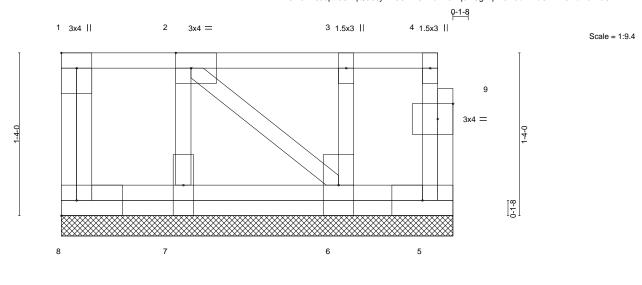
December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033973 J1224-6497 ET4 **GABLE** Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:27 2024 Page 1

Comtech, Inc, Fayetteville, NC - 28314,

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3x6 = 2x6 || 3x6 || 3x6 =

Plate Offsets (A, f)	[1.Euge,0-1-6], [2.0-1-	-o,⊑ugej, [9.0-1-	0,0-1-0]
LOADING (f)	ODAOINO	0 0 0	

LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.05 BC 0.00 WB 0.05	DEFL. in (lower language) Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	- n/a 9 - n/a 9	_/d	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-P			Weight: 25 lb	FT = 20%F, 11%E

LUMBER-

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat) BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-2-8 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-2-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 5-8=-10. 1-4=-100 Concentrated Loads (lb) Vert: 4=-79 2=-72



December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033974 Floor F1 J1224-6497 5

Fayetteville, NC - 28314, Comtech, Inc.

1-3-0

Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:27 2024 Page 1

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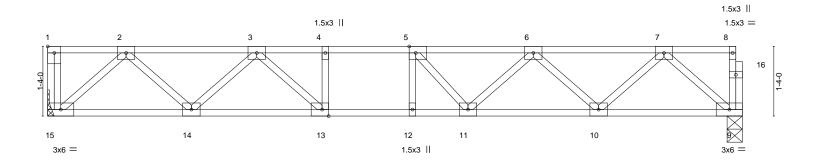
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1-6-8 1-0-0 0₁₁8

Scale = 1:22.0



			13-3-8	
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [13:0-1-8	8,Edge]		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.34	Vert(LL) -0.09 11-12 >999 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.60	Vert(CT) -0.12 11-12 >999 360	
BCLL 0.0	Rep Stress Incr YES	WB 0.31	Horz(CT) 0.03 9 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 71 lb FT = 20%F, 11%E
				= '

BRACING-

TOP CHORD

BOT CHORD

13-3-8

LUMBER-

2x4 SP No.1(flat) TOP CHORD **BOT CHORD** 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 15=Mechanical, 9=0-3-8 Max Grav 15=717(LC 1), 9=711(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1210/0, 3-4=-1904/0, 4-5=-1904/0, 5-6=-1819/0, 6-7=-1220/0

14-15=0/759, 13-14=0/1646, 12-13=0/1904, 11-12=0/1904, 10-11=0/1659, 9-10=0/754 **BOT CHORD** 2-15=-1010/0, 2-14=0/628, 3-14=-606/0, 3-13=0/505, 7-9=-1001/0, 7-10=0/648, WEBS

6-10=-611/0, 6-11=0/305, 5-11=-304/60

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



December 6,2024

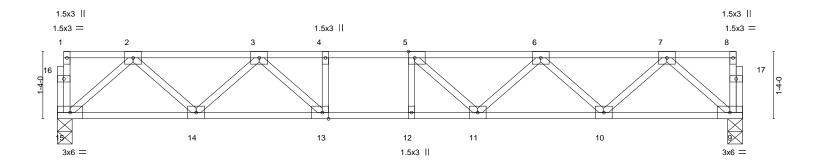


Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033975 Floor J1224-6497 F2 2 Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:28 2024 Page 1

Fayetteville, NC - 28314, Comtech, Inc.

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			13-7-0	
Plate Offsets (X,Y)	[5:0-1-8,Edge], [13:0-1-8,Edge]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.39	Vert(LL) -0.10 11-12 >999 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.65	Vert(CT) -0.14 11-12 >999 360	
BCLL 0.0	Rep Stress Incr YES	WB 0.32	Horz(CT) 0.03 9 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	, ,	Weight: 71 lb FT = 20%F, 11%E

13-7-0

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, **BOT CHORD** 2x4 SP No.1(flat) except end verticals.

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 15=0-3-8, 9=0-3-8 Max Grav 15=727(LC 1), 9=727(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1242/0, 3-4=-1987/0, 4-5=-1987/0, 5-6=-1882/0, 6-7=-1257/0

14-15=0/776, 13-14=0/1698, 12-13=0/1987, 11-12=0/1987, 10-11=0/1718, 9-10=0/770 BOT CHORD WEBS

2-15=-1031/0, 2-14=0/649, 3-14=-634/0, 3-13=0/547, 7-9=-1022/0, 7-10=0/677,

6-10=-642/0, 6-11=0/302, 5-11=-320/48

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033976 Floor J1224-6497 F3 6 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

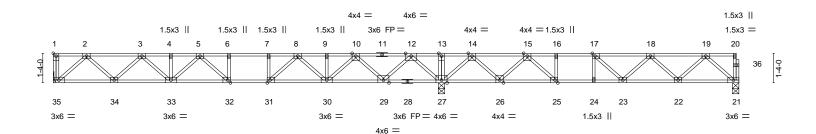
1-3-0

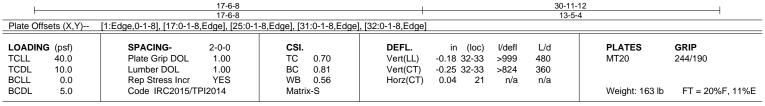
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:28 2024 Page 1 ID:IwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

1-6-12 0-11-8

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Scale = 1:52.1





LUMBER-BRACING-

1-8-0

2x4 SP No.1(flat) TOP CHORD TOP CHORD

BOT CHORD 2x4 SP No.1(flat) except end verticals. WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 35=Mechanical, 27=0-3-8, 21=0-3-8 Max Grav 35=847(LC 3), 27=2005(LC 1), 21=643(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1496/0, 3-4=-2418/0, 4-5=-2418/0, 5-6=-2629/0, 6-7=-2629/0, 7-8=-2629/0,

8-9=-1766/0, 9-10=-1766/0, 10-12=-385/291, 12-13=0/1961, 13-14=0/1961,

14-15=-502/949, 15-16=-1487/329, 16-17=-1487/329, 17-18=-1525/102, 18-19=-1076/0 **BOT CHORD**

34-35=0/906, 33-34=0/2063, 32-33=0/2634, 31-32=0/2629, 30-31=0/2229,

29-30=-53/1172, 27-29=-790/0, 26-27=-1220/0, 25-26=-664/1055, 24-25=-329/1487, 23-24=-329/1487, 22-23=0/1460, 21-22=0/672

2-35=-1206/0, 2-34=0/821, 3-34=-789/0, 3-33=0/483, 5-33=-293/0, 5-32=-299/249, **WEBS**

12-27=-1559/0, 12-29=0/1172, 10-29=-1132/0, 10-30=0/846, 8-30=-672/0, 8-31=0/772, 7-31=-372/0, 14-27=-1269/0, 14-26=0/871, 15-26=-926/0, 15-25=0/901, 16-25=-369/0,

19-21=-892/0, 19-22=0/562, 18-22=-533/29, 17-23=0/404, 17-24=-284/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



December 6,2024



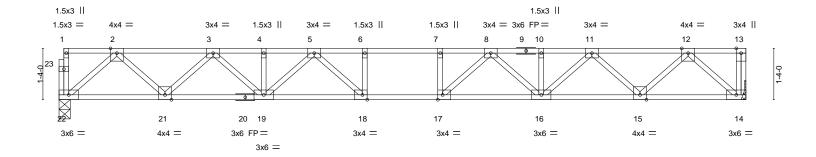
Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033977 Floor J1224-6497 F4 5 Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:29 2024 Page 1

Fayetteville, NC - 28314, Comtech, Inc.

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Scale = 1:29.9





						17-10-0					<u> </u>
Plate Offsets (2	(,Y) [17:0-1-	8,Edge], [18:0-1-8	,Edge]								
LOADING (ps) S	PACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.) P	late Grip DOL	1.00	TC	0.49	Vert(LL)	-0.21 17-18	>996	480	MT20	244/190
TCDL 10.) L	umber DOL	1.00	BC	0.72	Vert(CT)	-0.29 17-18	>725	360		
BCLL 0.) R	lep Stress Incr	YES	WB	0.47	Horz(CT)	0.06 14	n/a	n/a		
BCDL 5.) C	ode IRC2015/TP	I2014	Matrix	x-S					Weight: 95 lb	FT = 20%F, 11%E

17-10-0

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

BOT CHORD 2x4 SP No.1(flat) except end verticals.

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 22=0-3-8, 14=Mechanical Max Grav 22=961(LC 1), 14=967(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1757/0, 3-4=-2926/0, 4-5=-2926/0, 5-6=-3487/0, 6-7=-3487/0, 7-8=-3487/0, TOP CHORD

8-10=-2926/0, 10-11=-2926/0, 11-12=-1757/0 BOT CHORD $21-22=0/1042,\ 19-21=0/2442,\ 18-19=0/3275,\ 17-18=0/3487,\ 16-17=0/3275,\ 15-16=0/2442,\ 18-19=0/3275,\ 17-18=0/3487,\ 18-19=0/3275,\ 18-1$

14-15=0/1043

2-22=-1385/0, 2-21=0/994, 3-21=-953/0, 3-19=0/657, 12-14=-1388/0, 12-15=0/994, WFBS

11-15=-953/0, 11-16=0/658, 8-16=-474/0, 8-17=-71/583, 5-19=-474/0, 5-18=-71/583,

6-18=-290/0, 7-17=-290/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



December 6,2024



Job Truss Truss Type Qty Lot 1 Maple Hill 170033978 J1224-6497 Floor F4A Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

0-1-8

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:29 2024 Page 1 ID:lwPOH6hK8Jeptt6SXqQOJcyzm6C-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



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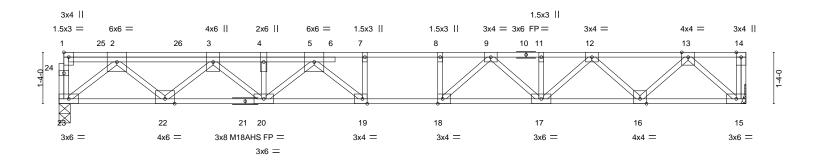


Plate Offsets (X,Y)--[1:Edge,0-1-8], [18:0-1-8,Edge], [19:0-1-8,Edge] LOADING (psf) SPACING-DEFL. (loc) I/def L/d **PLATES GRIP** TCLL 40.0 Plate Grip DOL 1.00 TC 0.83 Vert(LL) -0.22 19 >947 480 MT20 244/190 TCDL 10.0 Lumber DOL 1.00 ВС 0.95 Vert(CT) -0.31 19 >677 360 M18AHS 186/179 **BCLL** 0.0 Rep Stress Incr NO WB 0.54 0.07 Horz(CT) 15 n/a n/a Code IRC2015/TPI2014 **BCDL** 5.0 FT = 20%F, 11%E Matrix-S Weight: 104 lb

17-10-0

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals.

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 23=0-3-8, 15=Mechanical Max Grav 23=1158(LC 1), 15=1008(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2158/0, 3-4=-3538/0, 4-5=-3538/0, 5-7=-3783/0, 7-8=-3783/0, 8-9=-3783/0, TOP CHORD

9-11=-3102/0, 11-12=-3102/0, 12-13=-1847/0

BOT CHORD 22-23=0/1315, 20-22=0/2972, 19-20=0/3746, 18-19=0/3783, 17-18=0/3496, 16-17=0/2573,

15-16=0/1090

WFBS 2-23=-1708/0. 2-22=0/1143. 3-22=-1104/0. 3-20=0/752. 4-20=-279/0. 13-15=-1451/0.

13-16=0/1053, 12-16=-1010/0, 12-17=0/718, 9-17=-535/0, 9-18=0/685, 5-20=-277/0,

5-19=-320/291, 8-18=-341/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 159 lb down at 1-1-12, and 159 Ib down at 3-1-12, and 159 lb down at 5-1-12 on top chord. The design/selection of such connection device(s) is the responsibility
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 15-23=-10, 1-14=-100

Concentrated Loads (lb)

Vert: 4=-79(F) 25=-81(F) 26=-79(F)



December 6,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033979 J1224-6497 F5 Floor Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:30 2024 Page 1

1-0-0

Comtech, Inc, Fayetteville, NC - 28314,

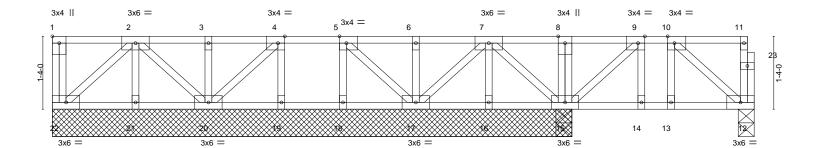
1-2-8

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0-5-0

0118

Scale = 1:21.1



<u> </u>			9-4-8 9-6-0 12-10-0 9-4-8 0-1-8 3-4-0									
Plate Offse	ets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,E	Edge], [5:0-1-8	,Edge], [9:0-	1-8,Edge], [10:0-1-8,Edge]						
LOADING TCLL TCDL	(psf) 40.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.00 1.00	CSI. TC BC	0.08 0.05	DEFL. Vert(LL) Vert(CT)	in -0.00 -0.00	(loc) 13 13	l/defl >999 >999	L/d 480 360	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 5.0	Rep Stress Incr Code IRC2015/TI	YES	WB Matri	0.04	Horz(CT)	0.00	12	n/a	n/a	Weight: 80 lb	FT = 20%F, 11%E

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) TOP CHORD **BOT CHORD** 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 16-17,15-16.

REACTIONS. All bearings 9-6-0 except (jt=length) 12=0-3-8. (lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 16, 17, 18, 21, 20, 19 except 15=301(LC 9), 15=290(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



December 6,2024



Job Truss Truss Type Qty Ply Lot 1 Maple Hill 170033980 Floor J1224-6497 F6 3

Comtech, Inc, Fayetteville, NC - 28314, Job Reference (optional)
8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Dec 6 08:33:30 2024 Page 1

Structural wood sheathing directly applied or 3-6-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Scale = 1:9.4



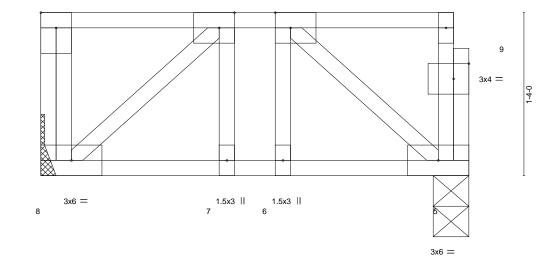


Plate Offsets (X,Y)	[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,Edge], [9:0-1-8,0-1-8]

LOADING (psf) TCLL 40.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00	CSI. TC 0.08 BC 0.05	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 7 >999 480 Vert(CT) -0.00 7 >999 360	PLATES GRIP MT20 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.04 Matrix-S	Horz(CT) 0.00 5 n/a n/a	Weight: 24 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

2x4 SP No.1(flat) TOP CHORD **BOT CHORD** 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

> (size) 8=Mechanical, 5=0-3-8 Max Grav 8=179(LC 1), 5=173(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.

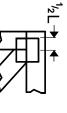


December 6,2024

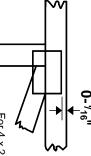


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

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This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 × 4

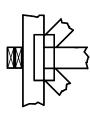
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur Min size shown is for crushing only.

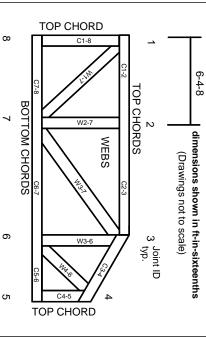
Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction Design Standard for Bracing.

Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-22:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

▲ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.