



6/20/2024

Tarheel Basement Systems
3333 Air Park Rd.,
Fuquay Varina, NC, 27526

RE: Foundation Repair for 124 Harborview Dr., Sanford, NC 27332

Existing structure is a wood framed building with a concrete foundation. Based on an assessment provided by Tarheel Basement Systems (TBS), the structure is experiencing settling toward the back and right of the house. TBS has recommended helical piers to stabilize the foundation. TBS also recommends Intellijacks to assist in supporting the main support beam. Scapular Engineering has reviewed the data provided by TBS and has verified that the proposed products are adequate per the manufacturers’ requirements for the loads applied.

Attachment contents

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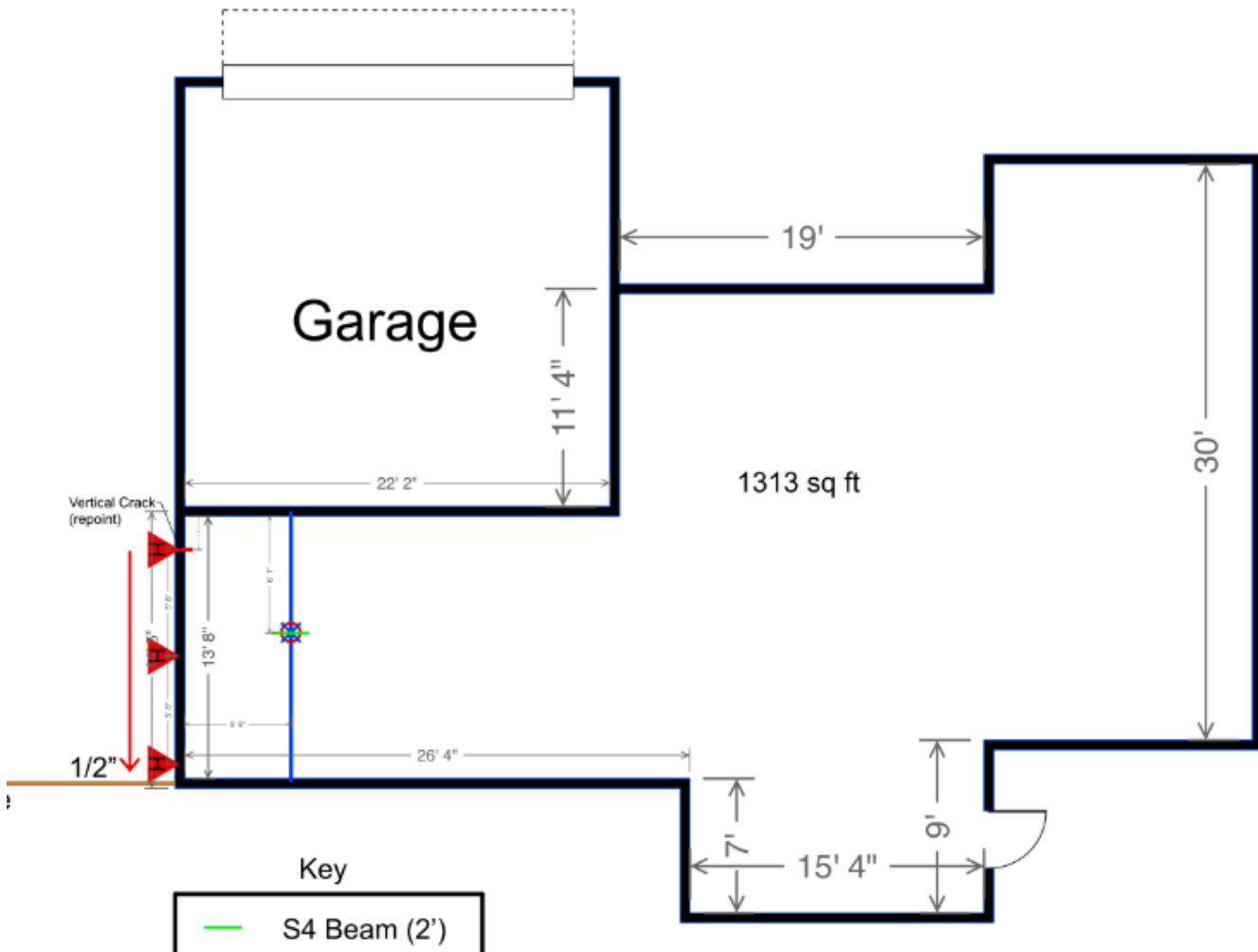
Jacob John Bullinger
PE: 056953
Scapular Engineering Firm
Licensee Number: P-2995
23576 State Route J
St. Mary, MO 63673
(573) 275-7647



6/20/2024

Foundation Plan

Front of Home



3 Helical Piers will be used.

1 Intellijack will be used.

Design of Structural Repair provided by TBS.

Helical Pier notes:

- 1) Helical piers shall be installed per manufacturers specifications, reference attached details.
- 2) Spacing of helical piers shall be as indicated on plan U.N.O.
- 3) Helical piers shall be installed to torque specifications.
- 4) The structure is assumed to be laterally supported as currently built and piers are designed for vertical loads only.
- 5) If piers are used to lift foundations, voids under slabs shall be filled by mudjacking or polyfoam.
- 6) For piers being installed on a party wall, foundation shall be stabilized only (no lifting).
- 7) For multi-family dwellings, contractor is required to obtain written approval by each affected property owner.
- 8) Stacked stone foundations to be stabilized only. Do not attempt to lift.

Intellijack Notes:

1. Intellijack shall be installed per manufacturer's specifications, reference attached details.
2. Spacing of Intellijack shall be as indicated on plan U.N.O.
3. Height of Intellijack shall not exceed 10 feet.
4. Intellijack and Intellibeam shall be used for supplemental supports only and only used for additional support to the current structure.
5. Existing post and/or beam shall not be removed or modified in any way. If Intellijack is used to lift, the existing posts shall be shimmed to ensure the beam bears fully on all existing posts/supports.
6. A. Option A: Intellijacks to be placed between existing posts that are unchanged to bear on a 2'x2'x2' thick concrete footing over a 12" thick minimum compacted aggregate base, or an 18"x18"x18" minimum compacted aggregate base with the 'IJ-IC' Precast Concrete Footing placed underneath the Intellijack, or the IJ-AB Aluminum Base.
B. Option B: Existing posts to be replaced with Intellijacks in the same location as existing posts to bear on existing footings or on a new concrete footing that matches existing concrete footing size and meets a minimum footing size of 2'x2'x2' thick concrete footing over a 12" thick minimum compacted aggregate base, or an 18"x18"x18" minimum compacted aggregate base with the 'IJ-IC' Precast Concrete Footing placed underneath the Intellijack, or the IJ-AB Aluminum Base.
7. Concrete shall have a minimum 28 day compressive strength of 3,000 PSI and rebar shall be #4 bars at 6" O.C., EW and grade 60.
8. If Intellijacks are supporting existing beams, contractor shall verify that the existing beam is of acceptable condition. If existing beam is damaged or shows signs of deterioration, beam shall be replaced with like-kind materials.

Scapular Engineering has provided a design based on a plan developed by others as shown grayscale. Scapular Engineering does not take responsibility for the accuracy of items grayscale on this drawing or if installation does not meet product specifications.

Product Details

GTRDS2875-0203 SERIES

ROUND SHAFT HELICAL PILES

50 kips Ultimate Compressive Capacity From Torque
 New Construction and Retrofit Construction

STEEL SPECIFICATIONS

Shaft (ASTM A500)	HSS 2.875" OD x 0.203" wall (73mm OD x 5.1mm Wall) $F_y = 50$ ksi min., $F_u = 58$ ksi min.
Cross section area, A	1.70 in ² (110 mm ²)
Moment of inertia, I	1.17 in ⁴ (49 cm ⁴)
Sectional modulus, S	0.82 in ³ (13.4 cm ³)
Circumference, c	9.0 in (228.6 mm)
Radius of gyration, r	0.95 in (24.1 mm)
Coupling (ASTM A500)	3.5" OD x 7" x 0.300" wall $F_y = 50$ ksi min., $F_u = 58$ ksi min.
Bolts (Zinc plated ASTM B633) Grade 8	3 - 3/4" diam. ASTM A354 or A490 $F_y = 130$ ksi min., $F_u = 150$ ksi min.
Helices (ASTM A36)	Thickness - 0.375"; 8", 10", 12", 14" Diam. $F_y = 36$ ksi min., $F_u = 58$ ksi min.
Coating	Hot-Dip Galvanization to ASTM A123 Optional

COMPRESSION AND TENSION CAPACITIES

MIN 50 YEAR DESIGN LIFE - Not Galvanized

Ultimate Capacity-to-Torque Ratio, K_t	Compression 9 ft ⁻¹ , Tension 7 ft ⁻¹
Recommended Torsional Strength, T	4,800ft-lbs (7,500 N-m)
Ultimate Mechanical Compression Capacity	90 kips ⁽¹⁾ (400 kN)
Allowable Mechanical Compression Capacity	35 kips ⁽¹⁾⁽²⁾ (155 kN)
Ultimate Capacity - By Torque Compression and Tension	Compression 50 kips, Tension 38 kips ⁽¹⁾⁽²⁾ (220 kN) (170 kN)
Allowable Capacity - By Torque Compression and Tension	Compression 25 kips, Tension 19 kips ⁽¹⁾⁽²⁾ (110 kN) (85 kN)

HELIX DIAMETER (in)	NET HELIX AREA (ft ²)
8	0.30
10	0.50
12	0.75
14	1.0

(1) Load test may be required to verify actual geotechnical capacities. A factor of safety greater than 2 may be necessary to meet project settlement tolerances. Settlements are estimated to be less than 1/2 inch for helical piles designed using the above allowable capacities.

(2) Minimum factor of safety of 2 is recommended.



Helical Pier Calculations

Input Information			
Stories Above Grade	2	stories	As reported
Tributary Width (T_w)	10	feet	From design
Wall Height (H_w)	8	feet	As reported
Foundation Height (H_FDN)	4	feet	As reported
Foundation Wall Thickness (t_FDN,WALL)	8	inches	As reported
Unbraced Wall Length (L_unbraced)	6	feet	From design
Soil Capacity	1500	lb/sqft	Worst case from design
Floor Load			
DL_floor	10	psf	
LL_floor	40	psf	
Floor Load	1000	plf	(# stories)*T_w*(DL+LL)
Roof & Ceiling Joist Load			
DL_joist	5	psf	
LL_joist	10	psf	
DL_roof	10	psf	
LL_roof/SN_roof	30	psf	
Roof & Ceiling Load	550	plf	T_w*(DL_j+LL_j+DL_r+LL_r)
Wall Load			
WT_wall	15	psf	
Wall Load	240		(# stories)*WT_wall*H_w
Foundation Wall Load			
WT_FDN	168	pcf	Assuming stone FDN (worst case)
Foundation Wall Load	448	plf	WT_FDN*H_FDN*t_FDN,WALL/12
Total Load			
Total Load on Foundation (W_total)	2238	plf	Sum all loads)
Total Load per Pier, (P_per-pier)	13428	lbs	L_unbraced*W_total
Bracket Pier Capacity (P_allow,bracket)	20000	lbs	Bracket Load capacity per Data Sheet
Installation Pressure (P_install)	3500	psi	Assumed 4,000 psi is target value
Starter End Cap Diameter (D_end)	4	inches	
Factor of safety (FS)	1.5		Factor of safety for end bearing
End Bearing Pier Capacity (P_allow-bearing)	29306.67	lbs	$\pi/4 * (D_{end}^2) * P_{install} / FS$
Pier stress Ratio	0.6714		$P_{per-pier} / \min(P_{allow,bracket}; P_{allow, bearing})$
Pier Check	OK		If Pier Stress Ration is <1, all is good

Product Specifications

SPECIFICATIONS

▼ HELICAL PIER/ANCHOR TUBE

- 2.875-inch Outside Diameter (O.D.) x 0.262-inch wall tube steel, ultimate tensile strength = 58,000 psi (min.), yield tensile strength = 50,000 psi (min.)
- Torque Strength Rating = 9,000 ft.-lb.
- Torque Correlation Factor (Kt) = 9 ft-1
- **Connecting Bolts**
0.75-inch diameter Heavy Hex Machine Bolt (SAE J429 ' Grade 8)
- **Weld**
E70 ' minimum tensile strength = 70,000 psi
- Ultimate axial capacity based upon installation torque = 100 kips (tension/compression)
- Allowable axial capacity based upon installation torque = 50 kips (FOS = 2)

▼ RETROFIT FOUNDATION BRACKET

- **Bracket Tube**
3.875-inch O.D. x 3.125-inch Inside Diameter (I.D.) x 10-inch long DOM mechanical steel tubing conforming to ASTM 513 ' Type 5 / Grade 1026. Ultimate tensile strength = 100,000 psi (min.), yield tensile strength = 90,000 psi (min.)
- **Angle Bracket**
0.5-inch thick flat plate conforming to ASTM A-36 hot rolled steel bent to form a 90 degree angle with equal sides of approximately 8 inches. Ultimate tensile strength = 58,000 psi (min.), yield tensile strength = 36,000 psi (min.)
- **Bracket Support Strap**
0.375-inch thick x 3 inches wide x 21.25 inches flat bar conforming to ASTM A-36 hot rolled steel which is bent into a "horseshoe" shape around the bracket tube. Ultimate tensile strength = 58,000 psi (min.), yield tensile strength = 36,000 psi (min.)
- **Top Plate**
0.75-inch thick x 5.5 inches wide x 9.5 inches long conforming to ASTM A-36 hot rolled steel. Ultimate tensile strength = 58,000 psi (min.), yield tensile strength = 36,000 psi (min.)
- **Cap Plate**
1.0-inch thick x 4.0 inches wide x 8.5 inches long conforming to ASTM A-36 hot rolled steel. Ultimate tensile strength = 58,000 psi (min.), yield tensile strength = 36,000 psi (min.)
- **Weld**
E71T1, minimum tensile strength = 71,000 psi ' preformed by AWS certified welder
- **Threaded Rod**
0.75-inch diameter x 12 inches long ASTM A311 ' Class B / Grade 8 Zinc Plated. Ultimate tensile strength = 150,000 psi, yield tensile strength = 130,000 psi



● Compliance with International Codes
● Compliance with State Codes

ICC-ES Evaluation Report

Reissued May 2022

ESR-3533

This report is subject to renewal May 2023.

DIVISION: 31 00 00—EARTHWORK
Section: 31 63 00—Bored Piles

REPORT HOLDER:

GRIP-TITE MANUFACTURING CO., LLC

EVALUATION SUBJECT:

GRIP-TITE® HELICAL FOUNDATION SYSTEMS

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2021, 2018, 2015 and 2012 *International Building Code*® (IBC)

Properties evaluated:

- Structural
- Geotechnical

2.0 USES

Grip-Tite Helical Pile Systems include a helical steel pile (screw) system used to transfer compressive, tension, and lateral loads from a new or existing structure to a soil bearing strata. Brackets are used to transfer the loads from the structure to the helical pile systems.

3.0 DESCRIPTION

3.1 General:

The Grip-Tite Helical Pile Foundation System consists of helical piles connected to brackets that are in contact with the structure's load-bearing foundation or wall. Each helical pile, consisting of a lead section and one or more extension sections, are screwed into the ground by application of torsion to a depth that conforms to project requirements for avoidance of unsatisfactory subsurface conditions and ensures a suitable soil or bedrock bearing stratum has been reached. The bracket is then installed to connect the pile to the concrete foundation or wall of the supported structure.

3.2 System Components:

3.2.1 RDS2875 Lead Section and Extensions: The lead shafts and extensions consist of 2 1/4-inch (73 mm) outside diameter (OD) structural steel tubing having a nominal shaft

thickness of 0.262-inch (6.6 mm). Helical-shaped discs, welded to the shaft, advance the helical piles into the soil when the piles are rotated. The extensions and lead sections are made from the same shafts and the extensions can be constructed with or without helical plates. The helical pile lead sections and extensions are connected to each other using an external coupler factory-welded to the leading end of each extension. Each extension consists of a factory-welded coupler and three (3) 3/4-inch-diameter (19.0 mm) structural bolts. The lead shafts, extensions and bolts are available bare steel or offered with an optional galvanization coating complying with ASTM A123 as described in the approved quality documentation. See Figure 1 and Table 1 provide details for RDS2875 lead sections and Figure 2 and Table 2 provide details for RDS2875 extension sections.

3.2.2 RDS2875 Helical Plates: The helical plates (helices or discs) are 8, 10, 12 or 14-inches (203, 254, 305 or 356 mm) in diameter, and are cut from minimum 3/8-inch (9.5 mm) thick steel plate conforming to ASTM A36, having a minimum yield strength of 36 ksi (248 MPa) and a minimum ultimate tensile strength of 58 ksi (400MPa). The helical plates are deformed using a hydraulic press and die, to achieve a 3-inch +/- 1/4-inch (76 +/- 6.4mm) helical pitch, and are then factory-welded to the helical lead shaft or extension in accordance with the manufacturers approved quality documentation. The helical plates are available bare steel or galvanized in accordance with ASTM A123.

3.2.3 RDS2875 Couplers: The external coupler consist of 3 1/2-inch (89 mm) OD ASTM A500, Grade B structural steel tubing having a nominal shaft thickness of 0.300 inch (7.6 mm) having a minimum yield strength of 50 ksi (345MPa) and a minimum tensile strength of 58 ksi (400MPa). The coupler is factory welded to the extension in accordance with the approved quality control documentation.

3.2.4 Brackets: Brackets are constructed from steel plate and round structural steel tubing components which are factory-welded together. The different brackets are described in the following subsections. Bracket assemblies are available in bare steel and with an optional galvanization coating complying with ASTM A123 as described in the approved quality documentation.

3.2.4.1 Standard Duty Bracket GTFP3BA (FP3BA): This retrofit or repair bracket is used on existing structures and existing foundations. The bracket is constructed from two (2) ½-inch (12.7 mm) thick steel plates factory-welded to achieve a 90-degree angle. The L-shaped bracket measures 10-inches (254 mm) wide by 8-inches (203 mm) long. The L-shaped seat is factory-welded to a bracket sleeve consisting of a 12-inch (305 mm) long structural tube with an OD of 3¼-inch (98 mm) and a 3⅝-inch (79 mm) inside diameter (ID). A bracket plate consisting of a 5-inch (127 mm) by 9⅞-inch (241 mm) by ¾-inch (19.1 mm) thick flat plate is factory-welded to the top of the bracket sleeve and onto the back side of the vertical face of the L-shaped seating angle. Two gusset wedges made from ¾-inch (9.5 mm) thick plate steel are factory-welded to the bottom of the L-shaped seat and the side of the bracket sleeve. An 8-inch (203 mm) by 4-inch (102 mm) by 1-inch (25.4 mm) thick cap plate is centered on top of the pile and is attached to the bracket sleeve with two 12-inch (305 mm) long ¾-inch (19 mm) all-thread bolts and matching nuts (longer rods are available). Figure 3 along with Tables 4 and 5 provides additional FP3BA details.

3.2.4.2 New Construction Brackets GT2875-NCB: The new construction brackets are used in the new construction of concrete foundations where the steel bearing plate of the bracket is cast into the new concrete foundations (grade beam, footing or pile cap). The brackets are available to transfer compression, tension and lateral loads between the pile and the concrete foundation. The new construction brackets come with a 1-inch thick-by-4-inch-wide-by-8-inch-long (25.4, 102 and 203 mm) bearing steel plate. Each steel bearing steel plate comes with three (3) 13/16-inch (20.6 mm) diameter predrilled holes and is factory welded to a 3¼-inch (89 mm) OD, 0.3-inch-thick (7.6 mm) and 7-inch (178 mm) long steel tube (sleeve) with three 13/16-inch (20.6 mm) pre-drilled holes used to fasten bracket to helical pile shaft. The steel plate is made from steel components as indicated in approved quality documentation. The bracket is attached to the pile shaft with either one (1), two (2) or three (3) ¾-inch (19 mm) structural bolts with matching nuts, as shown in Figure 4 of this report along with Tables 4 and 6.

3.2.5 Threaded Rods, Bolts and Nuts:

3.2.5.1 Helical Lead Shaft and Extensions Coupler Bolts and Nuts: The 3/4-in (19 mm) bolts used to connect helical lead shafts and extensions must conform to SAE J429, Grade 8 specification, having a minimum yield tensile stress of 130 ksi (897 MPa) and a minimum ultimate tensile stress of 150 ksi (1034 MPa) with matching nuts conforming to SAE J995.

3.2.5.2 Repair Brackets Threaded Rod: The standard 3/4-in (19 mm) and 7/8-in (22 mm) all-thread rods and matching nuts must conform to ASTM A193 having a minimum yield tensile stress of 105 ksi (725 MPa) and a minimum ultimate tensile stress of 125 ksi (863 MPa) with matching nuts complying with ASTM A194, Grade 2H. The bolts and all-thread rods and matching nuts must be either zinc coated in accordance to ASTM B633 or hot-dip galvanized in accordance to ASTM A153, as required by the registered design professional.

3.2.5.3 New Construction Bracket Bolts and Nuts: The 3/4-in (19 mm) bolts used to connect helical pile shaft to GT2875-NCB sleeve must conform to SAE J429, Grade 8 specification having a minimum yield tensile stress of 130 ksi (897 MPa) and a minimum ultimate tensile stress of 150 ksi (1034 MPa) with matching nuts conforming to SAE J995.

3.2.5.4 Repair Brackets Anchor Bolts: Each repair bracket must be installed by connecting the brackets to the

concrete foundations. The vertical face of the repair bracket includes four (4) 9/16-inch (14.3 mm) holes in order to utilize up to four (4) ½-inch or 5/8-inch (12.7 mm or 15.9 mm) diameter with 5-inch (114 mm) effective embedment, Simpson Strong-Tie Company, Inc. Titen HD Screw Anchors (ESR-2713) or equivalent as determined by a registered structural engineer design professional. The concrete anchors may be factory zinc coated or hot-dip galvanized complying with ASTM B633 or ASTM A153.

4.0 DESIGN AND INSTALLATION

4.1 Design:

4.1.1 General: Structural calculations and drawings prepared by a registered design professional must be submitted to the code official for each project based on accepted engineering principles, as described in IBC Section 1604.4 and must conform to IBC Section 1810. The design method for steel components is Allowable Strength Design (ASD), described in IBC Section 1602 and AISC 360 Section B3.4. The structural analysis must consider all applicable internal forces (shear, bending moments and torsional moments, if applicable) due to applied loads, structural eccentricity and maximum span(s) between helical pile foundations. The result of the analysis and the structural capacities must be used to select a helical pile foundation system. The minimum embedment depth for various loading conditions must be included based on the most stringent requirements of the following: engineering analysis, tested conditions described in this report, site-specific geotechnical investigation report, and site-specific load tests, if applicable. For helical pile foundation systems subject to combined lateral and axial (compression or tension) loads, the allowable strength under combined loads must be determined using the interaction equation prescribed in Chapter H of AISC 360.

A soils investigation report must be submitted to the code official as part of the required submittal documents, prescribed in IBC Section 107, at the time of permit application. The geotechnical report must include, but not be limited to, all of the following (as applicable):

1. A plot showing the location of the soil investigation.
2. A complete record of the soil boring and penetration test logs and soil samples.
3. A record of soil profile.
4. Information on groundwater table, frost depth and corrosion-related parameters, as described in Section 5.5 of this report.
5. Soil properties, including those affecting the design such as support conditions for the piles.
6. Recommendations for design criteria, including but not limited to mitigations of effects of differential settlement and varying soil strength, and effects of adjacent loads.
7. Field inspection and reporting procedures (to include procedures for verification of the installed bearing capacity when required).
8. Any questionable soil characteristics and special design provisions, as necessary.

4.1.2 Foundation System: The Grip-Tite helical pile system described in this report complies with IBC Section 1810.3.1.5. The allowable axial design load of helical piles shall comply with IBC Section 1810.3.3.1.9. The overall capacity of the Grip-Tite helical pile foundation system (in tension and compression) depends upon the analysis of interaction of brackets, shafts, helical plates and soils, and

must be the lowest value of the bracket capacity (P1), the shaft capacity (P2), the helical bearing plate capacity (P3) and the allowable soil capacity (P4), and the allowable capacity must be limited to no more than 60 kips (267 kN).

4.1.3 Bracket Capacity (P1): The concrete foundation must be designed and justified to the satisfaction of the code official with due consideration to the eccentricity of applied loads, including reactions provided by the brackets, acting on the concrete foundation. Only localized limit states of supporting concrete including punching (two-way) shear and concrete breakout have been evaluated in this evaluation report. All other limit states described in Chapter 17 of ACI 318-19 under the 2021 IBC, Chapter 17 of ACI 318-14 under the 2018 and 2015 IBC and Appendix D of ACI 318-11 under the 2012 IBC for anchors in shear (such as concrete breakout and pry out), beam (one-way) shear, and flexural (bending) related limit states, and all limit states related to bending moment transfer among pile shaft/new construction pile cap/and concrete footing, have not been evaluated in this evaluation report and must be determined by registered design professional, as applicable. The concrete foundation and interaction of pile shaft, new construction pile cap and concrete footing for moment transfer, as applicable, must be designed and justified to the satisfaction of the code official, with due consideration to all applicable limit states and the direction and eccentricity of applied loads, including reactions provided by the brackets, acting on the concrete foundation. Refer to Tables 5 and 6 for selected allowable bracket capacity ratings.

4.1.4 Pile Shaft Capacity (P2): The top of shaft must be braced as described in IBC Section 1810.2.2. In accordance with IBC Section 1810.2.1, any soil other than fluid soil must be deemed to afford sufficient lateral support to prevent buckling of the systems that are braced, and the unbraced length is defined as the length of piles standing in air, water, or in fluid soils plus an additional five (5) feet (1524 mm) when embedded into firm soil or an additional ten (10) feet (3054 mm) when embedded into soft soil. Firm soils must be defined as any soil with a Standard Penetration Test blow count (SPT N value) of five (5) or greater. Soft soils must be defined as any soil with SPT N value greater than zero (0) and less than five (5). Fluid soils must be defined as any soil with a SPT N value of zero (0) [weight of hammer (WOH) or weight of rods (WOR)]. The SPT N value must be conducted under the supervision of a registered design professional in accordance with ASTM D1586. The allowable stress design (ASD) shaft capacities are provided in Table 3.

4.1.5 Helix Plate Capacity(P3): The allowable helix compression (+) and tension (-) load capacities (P3) are listed below. For helical piles with more than one helix, the allowable helix capacity (P3) for the helical foundation system may be taken as the sum of the least allowable capacity of each individual helix:

- 8-inches by 3/8-inch thick: ±84.97 kips (378 kN)
- 10-inches by 3/8-inch thick: ±68.37 kips (304 kN)
- 12-inches by 3/8-inch thick ±66.00 kips (293 kN)
- 14-inches by 1/2-inch thick: ±74.44 kips (331 kN)

4.1.6 Soil Capacity (P4): The design axial compressive and tensile load capacities of helical piles based on soil resistance (P4) must be conducted under the supervision of a registered design professional in accordance with a site-specific geotechnical report, as described in Section 4.1.1 of this report, combined with the individual helix bearing method (Method 1), or from field loading tests conducted under the supervision of a registered design professional (Method 2). For either Method 1 or Method 2, the predicted axial load capacities must be confirmed during the site-

specific production installation, such that the axial load capacities predicted by the torque correlation method must be equal to or greater than that predicted by Method 1 or Method 2, described above. With the individual helix bearing method, the total nominal axial load capacity of the helical pile is determined as the sum of the individual areas of the helical bearing plates times the ultimate bearing capacities of the soil or rock comprising the respective bearing strata for the plates, as follows:

$$Q_{tot} = \sum(A_n q_u) \text{ (Equation 1)}$$

Where:

Q_{tot} = Predicted nominal axial tensile or compressive capacity of the helical pile, lbf (N).

A_n = Net area of an individual helix bearing plate, in², (mm²).

q_u = Ultimate unit bearing capacity of the soil or rock comprising the bearing stratum for the individual helix bearing plate, psf (MPa).

The unit bearing capacity of the bearing stratum for each helix plate can be estimated using the general bearing capacity equation for deep foundations as follows:

$$q_u = cN_c + q'N_q \text{ (Equation 2)}$$

where:

c = Undrained shear strength parameter, considering the effect of soil disturbance due to the helix pile installations, psf (MPa).

N_c, N_q = Bearing capacity factors.

q' = Effective overburden pressure at helix plate foundation depth, psf (MPa).

Either the bearing capacity factors (N_c and N_q) and the soil parameters needed to estimate the ultimate/allowable soil bearing capacities of the major soil types or the recommended ultimate/allowable soil bearing capacities for the major soil types must be obtained from the site-specific geotechnical report.

The design allowable axial load must be determined by dividing the total ultimate axial load capacity predicted by either Method 1 or Method 2, above, by a safety factor of at least two (2).

The Foundation Design Documentation must include documentation of the derivation of the design allowable capacity and the minimum effective torsional resistance pile termination criterion, derived using the torque correlation method. With the torque correlation method, the total ultimate axial load capacity of the helical pile is predicted as follows:

$$Q_{ult} = Kt(T) \text{ (Equation 3)}$$

$$Q_{all} = 0.5(Q_{ult}) \text{ (Equation 4)}$$

Q_{ult} = Ultimate axial tensile or compressive capacity, lbf (N), of the helical pile.

Q_{all} = Allowable axial tensile or compressive capacity, lbf (N), of the helical pile.

Kt = Maximum torque correlation factors of 9 ft-1 and 7 ft-1 should be used for compression and tension piles, respectively.

T = Effective torsional resistance, which is defined as follows: For single-helix piles supporting axial compression loads, it is the installation torque measured when the pile reaches its final tip embedment; for all piles supporting axial tension loads and for multi-helix piles supporting axial compression

loads, it is the average of the last three (3) installation torque measurements. Such torque measurements, in-lbf (N-m), must be made at one (1) foot (305 mm) increments of tip embedment as the lead helix moves from a position, which is two (2) feet (710 mm) prior to the final tip embedment, to the final tip embedment.

The minimum effective torsional resistance pile termination criterion is calculated as follows:

$$T_{req} = (F.S.)(Q_{all})/(Kt) \text{ (Equation 5)}$$

Where:

T_{req} = Minimum effective torsional resistance pile termination criterion, in lbf-ft (N-m).

F.S. = An appropriate factor of safety for the project, but not less than two (2).

The allowable lateral capacity of the pile is 318 lbf and is based on field testing of the helical pile with a single 8-inch diameter helical plate installed in firm clay soil, having an average standard penetration test blow count of 20, at a minimum embedment of 15 feet. Installation is limited for use with the new construction brackets having minimum concrete edge distance of 4 inches. For soil conditions other than firm clay, the lateral capacity of the pile must be determined by a registered design professional.

4.1.7 Settlement Analysis: The pile head vertical movement at allowable load of a Grip-Tite® helical pile may be estimated as the sum of the following: the movement at helix plates due to soil deformation and helix plate deflection, and the shaft elastic shortening or lengthening. The corresponding equation is described below:

$$\Delta_{total} = \Delta_{helix} + \Delta_{shaft} \text{ (Equation 6)}$$

Where:

Δ_{total} = Total pile head vertical movement, in (mm).

Δ_{helix} = Movement of helix plates within the soil, in (mm).

Δ_{shaft} = Shaft elastic shortening/lengthening, in (mm).

The reliability of the foundation system capacity and settlement predictions may be improved by performing full-scale field tests at the construction site using piles of same configuration as the intended production piles.

4.1.8 Shaft Elastic Shortening and Lengthening: Elastic shortening or lengthening of a Grip-Tite® RDS2875 helical pile shaft may be a significant contributor to overall pile head movement under load for long piles. For loads up to and including the allowable load limits found in the tables of this report, the length change can be estimated as:

$$\Delta_{shaft} = PL/AE$$

shaft = Length change of shaft resulting from elastic shortening or lengthening, in (mm)

P = Applied axial load, lbf (N).

L = Effective length of the shaft, in (mm).

A = Cross sectional area of the shaft, in² (mm²), see Tables 1, and 2.

E = Young's modulus of the shaft, may be taken as 29,000 ksi (200,000 MPa).

The effective length of the shaft, L, may be approximated as the average of the distances from the point of load application to each helical plate.

The elastic shortening/lengthening of the pile shaft will be controlled by strength and section properties of the helical shaft and the following:

- Bare Steel
 - Potential elastic shortening due to compression load= 0.012 in/ft of shaft and 0.003 in/coupler.
 - Potential elastic lengthening due to tensile load= 0.007 in/ft of shaft and 0.003 in/coupler.
 - Slip in coupler due to compression of tensile load= 0.197 in/coupler.
- Galvanized Steel
 - Potential elastic shortening due to compression load= 0.012 in/ft of shaft and 0.003 in/coupler.
 - Potential elastic lengthening due to tensile load= 0.008 in/ft of shaft and 0.002 in/coupler.
 - Slip in coupler due to compression of tensile load= 0.151 in/coupler.

4.1.9 Helix Movement: The evaluation of helix movement due to helix deformation, soil deformation, and helix-soil interaction, is beyond the scope of this evaluation report. It is recommended that the user of this report consult with the helical pile manufacture (Grip-Tite® Manufacturing Co., LLC).

4.2 Installation:

The Grip-Tite® Helical Pile Foundation Systems must be installed by Grip-Tite® Manufacturing Co., LLC certified and trained installers. The Grip-Tite® Helical Pile Foundation Systems must be installed in accordance with this section (Section 4.2) and the manufacturer's installation instructions, or the site-specific approved construction documents, whichever is most stringent governs. If galvanized product is used, all field-cut or drilled pilings must be protected from corrosion as recommended by the registered design professional.

4.3 Special Inspections:

Continuous special inspection in accordance with IBC Section 1705.9 must be provided for the installation of foundation piles and foundation brackets. Where on-site welding is required, special inspection in accordance with IBC Section 1705.2 is also required. Items to be confirmed by the special inspector include, but are not limited to, the manufacturer's certification of installers, verification of the product manufacturer, helical pile and bracket configuration and identification, inclination and position of the helical pies, the installation torque and depth of the foundation piles, compliance of the installation with the approved construction documents and this evaluation report.

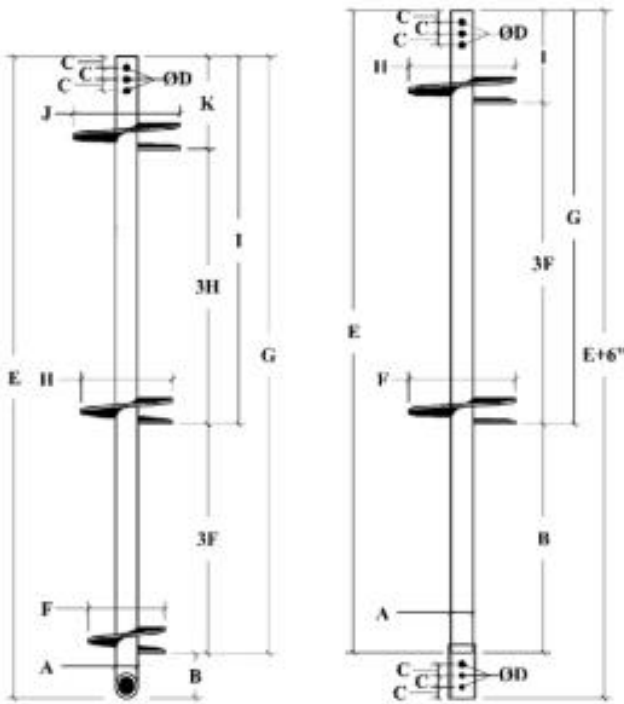
5.0 CONDITIONS OF USE

The Grip-Tite® Manufacturing Co., LLC Helical Foundation Systems described in this report complies with, or is a suitable alternative to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 The Grip-Tite® Manufacturing Co., LLC helical foundation systems are manufactured, identified and installed in accordance with this report, the approved construction documents (engineering drawings and

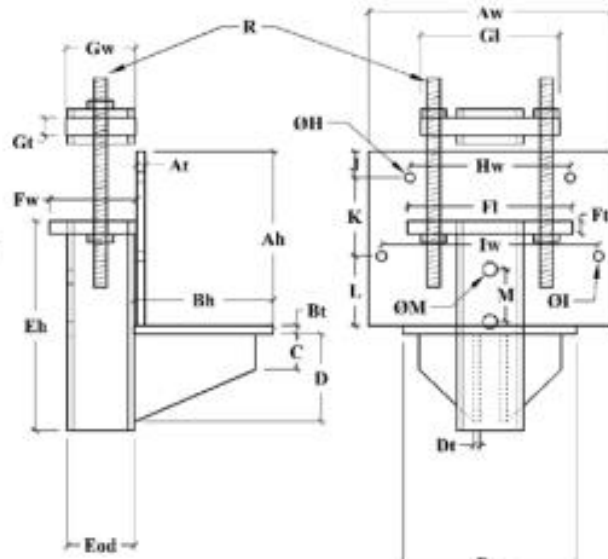
- specifications), and the manufacturer's written installation instructions, which must be available at the jobsite at all times during installation. In case of conflict, the most stringent requirement governs.
- 5.2** The Grip-Tite® Manufacturing Co., LLC helical foundation systems have been evaluated for support of structures assigned to Seismic Design Categories A, B and C in accordance with IBC Section 1613. Helical foundation systems that support structures assigned to Seismic Design Category D, E or F, or that are located in Site Class E or F, are outside the scope of this report, and are subject to the approval of the building official based upon submission of a design in accordance with the code by a registered design professional.
- 5.3** Installations of the helical foundation systems are limited to regions of concrete members where analysis indicates no cracking will occur at service load levels.
- 5.4** Remedial Repair Brackets and New Construction Brackets must be used only to support structures that are laterally braced as defined in IBC Section 1810.2.2. Shaft couplings must be located within firm or soft soil as defined in Section 4.1.3. Table 5 provides the allowable capacities at various unbraced lengths and coupling configurations.
- 5.5** The helical foundation systems must not be used in conditions that are indicative of potential pile deterioration or corrosion situations as defined by the following: (1) soil resistivity less than 1,000 ohm-cm; (2) soil pH less than 5.5; (3) soils with high organic content; (4) soil sulfate concentrations greater than 1,000 ppm; (5) soils located in a landfill, or (6) soil containing mine waste.
- 5.6** Hot-dip galvanized steel and bare steel components must not be combined in the same system, unless bare steel capacity is used as the design capacity. All helical foundation components must be isolated from concrete reinforcing steel, building structural steel, or any other metal building components.
- 5.7** The new construction helical piles (piles with new construction brackets) must be installed vertically into the ground with a maximum allowable angle of inclination of $\pm 1^\circ$ from the design installation angle. The tops of pile caps must be embedded into the concrete footing with a minimum 4-inch (101.6 mm) vertical embedment and a minimum 4-inch (101.6 mm) side embedment beyond the perimeter of the steel cap plates, except as specifically noted in Table 6 and Figure 5.
- 5.8** The retrofit helical piles must be installed such that the angle of inclination does not exceed $0^\circ \pm 1^\circ$.
- 5.9** Special inspection is provided in accordance with Section 4.3 of this report.
- 5.10** Engineering calculations and drawings, in accordance with recognized engineering principles as described in IBC Section 1604.4, are prepared by a registered design professional and approved by the code official.
- 5.11** The adequacy of the concrete structures that are connected to the Grip-Tite® Manufacturing Co., LLC brackets must be verified by a registered design professional in accordance with applicable code provisions such as Chapter 13 of ACI 318-19 under the 2021 IBC, Chapter 13 of ACI 318-14 under the 2018 and 2015 IBC (Chapter 15 of ACI 318-11 under the 2012) and Chapter 18 of the IBC. Verification is subject to the approval of the code official.
- 5.12** A geotechnical investigation report for each project site must be provided to the code official for approval in accordance with Section 4.1.1 of this report.
- 5.13** When using the alternative basic load combinations prescribed in IBC Section 1605.3.2, the allowable stress increases permitted by material chapters of the IBC or the referenced standards are prohibited.
- 5.14** The minimum helical pile center-to-center spacing is three (3) times and eight (8) times the diameter of the largest helical bearing plate for axially and laterally loaded piles, respectively. For piles with closer spacing, the recommended allowable load reductions due to pile group effects must be included in the geotechnical report described in Section 4.1.1 of this report and must be considered in the helical pile design by a registered design professional. Load reductions are subject to the approval of the code official.
- 5.15** Evaluation of compliance with IBC Section 1810.3.6.1 for buildings assigned to Seismic Design Category (SDC) C, is outside of the scope of this evaluation report. Such compliance must be addressed by a registered design professional for each site, and the work of the design professional is subject to approval by the code official.
- 5.16** The Grip-Tite® Manufacturing Co., LLC helical foundation systems are manufactured by Grip-Tite® Manufacturing Co., LLC, 115 W. Jefferson St., Winterset, IA 50273; under a quality control program with inspections by ICC-ES.
- 6.0 EVIDENCE SUBMITTED**
- Data in accordance with the ICC-ES Acceptance Criteria for Helical Foundation Systems and Devices (AC358), dated June 2020, Revised March 2021.
- 7.0 IDENTIFICATION**
- 7.1** Product labeling shall include, the name of the report holder, and the ICC-ES mark of conformity. The listing or evaluation report number (ICC-ES ESR-3533) may be used in lieu of the mark of conformity.
- 7.2** The report holder's contact information is the following:
- GRIP-TITE® MANUFACTURING CO., LLC**
505 EAST MADISON STREET
WINTERSET, IOWA 50273
(515) 462-1313
www.griptite.com
steve@griptite.com

**GTRDS2875
Ø2.875" ROUND SHAFT HELICAL PILES**

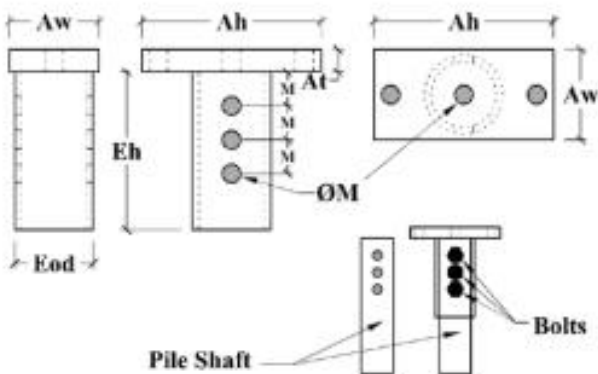


**Figure No. 1 -
RDS2875 Lead Sections**

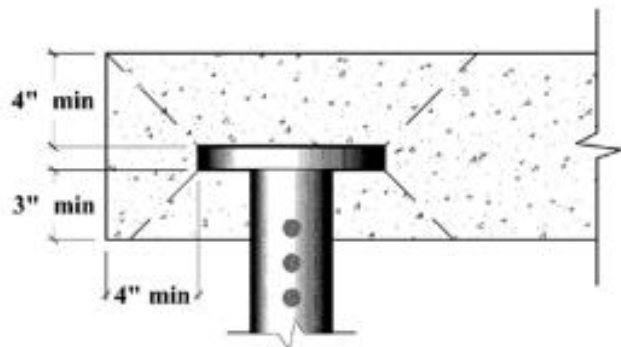
**Figure No. 2 -
RDS2875 Extensions**



**Figure No. 3 - FP3BA
Standard Duty Remedial Repair Bracket**



**Figure No. 4 - RDS2875-NCB
New Construction Brackets**



**Figure No. 5 - Grade Beam with
RDS2875-NCB New Construction Bracket**

TABLE—1 DIMENSIONS OF RDS2875 HELICAL PILE LEAD SECTIONS BY THE PRODUCT CATALOG NUMBER¹

Helical Pile Lead Steel Material (See Figure 1)	Catalog Number ²	Shaft Cross-Sectional Area, in ²	Nominal Dimensions, (in) ²																
			A	B	C	ØD	E	F	3F	G	H	3H	I	J	K				
ASTM A500 Grade B Fy=50 ksi Fu=58 ksi	2LS8	1.72 (1.96)	2.839 (2.869)	5.75	1.5	0.8125 (13/16)	24	8		18.125									
	24						10		18.125										
	24						12		18.125										
	24						14		18.125										
	60						8		54.125										
	60						10		54.125										
	60						12		54.125										
	60						8	24	54.125	10		30.125							
	60						14		54.125										
	60						10	30	54.125	12		24.125							
	60						12	36	54.125	14		18.125							
	84						8		78.125										
	84						10		78.125										
	84						12		78.125										
	84						8	24	78.125	10		54.125							
	84						14		78.125										
	84						10	30	78.125	12		48.125							
	84						8	24	78.125	10	30	54.125	12	24.125					
	84						12	36	78.125	14		42.125							
	84						10	30	78.125	12	36	42.125	14	12.125					

For SI: 1 inch = 25.4 mm; 1 in² = 645.2 mm²; 1 psi = 6.89 kPa

¹Bare steel dimensions (galvanized dimensions): Includes reduced dimensions for corrosion and a 50-year design life in accordance with Section 3.9 of AC308. Helical pile components consist of steel materials as described in Section 3.0 of this report. The helical lead sections and extensions are either bare or hot-dip galvanized steel per ASTM A123.

²See Figure No. 1 for dimension designations.

³Galvanized product will have a "G" suffix added to the end of the products' catalog number.

i.e. 2LS8 is bare steel and 2LS8G is galvanized steel.

TABLE—2 DIMENSIONS OF RDS2875 HELICAL PILE EXTENSIONS BY THE PRODUCT CATALOG NUMBER^{1,4}

Helical Pile Extensions Steel Material (See Figure 2)	Catalog Number ²	Shaft Cross-Sectional Area, in ²	Nominal Dimensions, (in) ²										
			A	B	C	ØD	E	F	3F	G	H	I	
ASTM500 Grade B Fy= 50 ksi Fu= 58 ksi	3E	1.72 (1.96)	2.839 (2.869)	1.5	0.8125 (13/16)	36	Plain Extensions						
	60					Plain Extensions							
	84					Plain Extensions							
	3E14					11.875	36	14		24.125			
	5E14					11.875	60	14		48.125			
	5E14x					29.875	60	14		30.125			
	7E14					11.875	84	14		72.125			
	7E14x					29.875	84	14		54.125			
	7E1414					11.875	84	14	42	72.125	14	30.125	
	7E1414x					29.875	84	14	42	54.125	14	12.125	

For SI: 1 inch = 25.4 mm; 1 in² = 645.2 mm²; 1 psi = 6.89 kPa

¹Bare steel dimensions (galvanized dimensions): Includes reduced dimensions for corrosion and a 50-year design life in accordance with Section 3.9 of AC308. Helical pile components consist of steel materials as described in Section 3.0 of this report. The helical lead sections and extensions are either bare or hot-dip galvanized steel per ASTM A123.

²See Figure No. 2 for dimension designations.

³Galvanized product will have a "G" suffix added to the end of the products' catalog number.

i.e. 2LS8 is bare steel and 2LS8G is galvanized steel.

⁴Coupler bolts must have a protection, zinc coated per ASTM B633 or ASTM B695.

TABLE—3 ALLOWABLE CAPACITIES* AND RECOMMENDED MAXIMUM INSTALLATION TORQUE FOR RDS2875 HELICAL PILES 1,2

Helical Pile Steel Material	Allowable Axial Capacity Limits ⁴ Bare Steel, (Hot-Dip Galvanized)								Recommended Capacity-Torque Ratio ⁶ , K _t (ft ⁻¹)	Recommended Max Installation Torque ⁷ , T (ft-lbs)	Ultimate Load at Max Torque Capacity, Q _u =TK _t (kips)	Allowable Load at Max Torque Capacity, Q _a /2=Q _a ⁴ (kips)
	Compression, (kips)				Tension, (kips)	Shear, (kips)	Bending, (ft-k)					
	Unbraced Length, L _u (ft) ¹	kL _u =0	kL _u =5	kL _u =10				kL _u =15				
RDS2875A STM A500 Grade B F _y =50 ksi F _u =58 Ksi	0 Couplings- no eccentricity	55.5 (60.0)	18.1 (19.9)	8.2 (9.0)	4.4 (4.8)	32.1 (37.7)	16.7 (18.4)	2.52 (2.76)	Comp K _t =9 Ten K _t =7	9,000 9,000	67 51	33.5 25.5
	1 Coupling	35.0 (40.8)	15.0 (19.0)	8.1 (8.9)	4.4 (4.8)							
	2 Couplings	22.8 (26.0)	13.4 (15.0)	7.2 (7.9)	4.1 (4.5)							

For SI: 1 inch = 25.4mm; 1 lbf = 1.356 N-m; 1 kip (1000 lbf) = 4.48 kN; 1 in² = 645.2mm²; 1 psi = 6.89 kPa

¹Refer to Section 4.1.4 Pile Shaft Capacity for the description of unbraced length, soft and firm soil conditions.

²Capacity ratings include an allowance for corrosion over a minimum 50-year service design life in accordance with Section 3.9 of AC308 and presume the support structure is braced in accordance with IBC Section 1810.2.1 and the lead section will provide sufficient helical capacity to develop the full shaft capacity. See section 4.1.3 entitled Bracket Capacity (P1) for applicable limit states that must be evaluated by a registered design professional.

³Maximum useable torque for calculating the pile's ultimate capacity using the equation Q=TK. Maximum torque applied to the helical pile anytime during installation should not exceed 9495 ft-lbs for the RDS2875 helical piles.

⁴The listed capacity-to-torque ratios can be adjusted to lower values on a case-by-case, if necessary. The listed allowable capacity includes a minimum Factor of Safety (FS) of 2. A higher Factor of Safety may be used to estimate the pile's allowable capacity on a case-by-case basis, if necessary.

TABLE—4 REMEDIAL REPAIR AND NEW CONSTRUCTION BRACKET DIMENSIONS BY THE PRODUCT CATALOG NUMBER

Product ⁴	Helical Pile	Aw Ah At ¹	Bw Bh Bt ¹	C D Dt	Eh Eod	Fl Fw Ft	Gl Gw Gt	Hw ØH lw Øl	J K L	M ØM	All-Threaded Rods/Bolts dia-pitch x Length, in (R _i) ²	Concrete Anchors ³
Remedial Repair Bracket FP3BA FP3BAG (Figure 3)	RDS2875	14 10 ½	10, 8, ½	2 5 3/8	12 3-7/8	9.5 5 ¼	8 4 1	9.25 9/16 12.15 9/16	1.5 4.5 4	3 13/16	¼ - 10 x 12 ASTM A193 Gr B7 and ASTM A194 2H Heavy Hex Nuts F _y = 105 Ksi F _u = 125Ksi	Titen HD Screw Anchors 1/2" x 5.5"
New Construction Bracket RDS2875-NCB RDS2875-NCBG (Figure 4)	RDS2875	8 4 1			7 3- ½					1.5 13/16	¼ - 10 x 4 ¼ SAE J429 Grade 8	

For SI: 1 inch = 25.4 mm; 1 psi = 6.89 kPa

¹Main plate dimensions in direct contact with foundation.

²12-inch long all-thread rods and bolts are provided with the remedial brackets. Longer or additional rods can be used if additional lift is needed, as necessary.

³Concrete anchors shall be provided by the installer. Either 1/2" or 5/8" concrete anchors can be used to support the remedial brackets. The brackets were placed under a 2500 psi concrete foundation with 4 post-installed Titen HD screw anchors fastened through the face of the bracket into the side of the foundation. The screw anchors should be installed in accordance to the manufacturer's recommendations (ICC-ESR-2713).

⁴Galvanized product includes the suffix "G".

TABLE—5 REMEDIAL REPAIR ALLOWABLE CAPACITIES BY THE PRODUCT CATALOG NUMBER

Product ²	Main Plate Dim., inches ¹		RDS2875 ASTM A500 Fy = 50 ksi Fu = 58 ksi			All-Thread Rods ⁵	Concrete Anchors ^{5,7}	
	Aw, Ah, At ¹	Bw, Bh, Bt ¹	Allowable Capacities, kips ^{2,3,4,5}					
			Comp.	Tension	Lateral	Ø – pitch x Length, inches (R ₁)		
Remedial Repair Brackets FP3BA FP3BAG (see Figure 3)	14, 10, ½	10, 8, ½	27.2	To be determined by registered design professional		¾ - 10 x 12 AST A193 Gr B7 and ASTM A194 2H Heavy Hex Nuts Fy = 105 ksi Fu = 125 Ksi	Titen HD Screw Anchors, or equivalent	1/2" x 5.5"

For SI: 1 inch = 25.4 mm; 1 lbf = 1.356 N-m; 1 kip (1000 lbf) = 4.48 kN; 1 in² = 645.2 mm²; 1 psi = 6.89 kPa

¹Main plate dimensions in direct contact with foundation.

²Based on full scale load tests conducted using the Grip-Tite Standard foundation Pier Bracket (GTFP3BA). The allowable capacities include a minimum Factor of Safety of 2. A higher Factor of Safety value may be used on a case-by-case basis, if necessary.

³Systems that require uplift capacities must include two (2) Ø3/4"x2" bolts thru the bracket tube sleeve into the existing shaft. One (1) bolt or two (2) Ø3/4"x2" long bolts can be installed utilized. As an alternative, the pile shaft could be welded in the field to the bracket using the two holes in the bracket sleeve, below the bracket tube or by other means as directed by the registered engineer.

⁴Fully braced foundation and shaft conditions.

⁵Lateral capacities are provided by four Titen HD concrete anchors with a minimum embedment of 5 inches.

⁶Corrosion protection, zinc coated per ASTM B633 or ASTM B695.

⁷The brackets were placed under a 2500 psi concrete foundation with four (4) Titen HD screw anchors fastened through the face of the bracket into the side of the foundation. The screw anchors must be installed in accordance with ESR-2713.

TABLE—6 GRIP-TITE NEW CONSTRUCTION BRACKET MINIMUM EMBEDMENT DEPTHS AND ALLOWABLE CAPACITIES BY THE PRODUCT CATALOG NUMBER

Product	Main Plate Dimensions, inches ¹	RDS2875 ASTM A500 Fy = 50 ksi Fu = 58 ksi			
		Aw, Ah, At	Minimum Concrete Cover Below (Tension) and Above (Compression) the Bracket Plate, inches	Allowable Compression and Tension Capacities, kips	Allowable Lateral Capacities, kips
				2500 psi PCC	2500 Psi PCC
New Construction Bracket RDS2875-NCB (see Figure 4)	8	3	Tension	1.4	2.5
		4	Comp	8.2	
	4	6	Tension	8.2	
		8	Comp	14.8	
		8	Tension	14.8	
		10	Comp	23.0	
1	10	Tension	32.9		
	12	Comp	40.0		

For SI: 1 inch=25.4mm; 1 lbf=1.356 N-m; 1 kip (1000 lbf) = 4.48 kN; 1 in² = 645.2 mm²; 1 psi=6.89 kPa

¹Main plate dimensions in direct contact with foundation.

²The listed allowable capacities are limited by either the punching shear capacity of the grade beam, bearing or bending capacity of the bracket plate the weld strength between the bearing plate and the bracket sleeve or the shear capacities of the connecting bolts. The allowable capacities include a minimum Factor of Safety of 2. A high Factor of Safety value may be used on a case-by-case basis, if necessary.

³Systems that require uplift capacities must include one (1), two (2) or three (3) 3/4"x4 1/2" bolts through the bracket tube sleeve into the existing shaft. As an alternative, the existing pile shaft can be plug-welded to the bracket using the three bolt holes in the bracket sleeve or the New Construction Bracket could be welded to the shaft at the base of the bracket.

⁴Fully braced foundation and shaft conditions.

⁵Corrosion protection, zinc coated per ASTM B633-07 or ASTM B695-04.

⁶Galvanized product includes the suffix "G".

DIVISION: 31 00 00—EARTHWORK
Section: 31 63 00—Bored Piles

REPORT HOLDER:

GRIP-TITE MANUFACTURING CO., LLC

EVALUATION SUBJECT:

GRIP-TITE® HELICAL FOUNDATION SYSTEMS

1.0 REPORT PURPOSE AND SCOPE**Purpose:**

The purpose of this evaluation report supplement is to indicate that Grip-Tite Helical Pile Foundation Systems, described in ICC-ES evaluation report ESR-3533, have also been evaluated for compliance with the code noted below.

Applicable code edition:

- 2019 *California Building Code* (CBC)

For evaluation of applicable chapters adopted by the California Office of Statewide Health Planning and Development (OSHPD) and Division of State Architect (DSA), see Sections 2.1.1 and 2.1.2 below.

2.0 CONCLUSIONS**2.1 CBC:**

The Grip-Tite Helical Pile Foundation Systems, described in Sections 2.0 through 7.0 of the evaluation report ESR-3533, comply with CBC Chapter 18, provided the design and installation are in accordance with the 2018 *International Building Code*® (IBC) provisions noted in the evaluation report and the additional requirements of CBC Chapters 16, 17 and 18, as applicable.

2.1.1 OSHPD:

The applicable OSHPD Sections and Chapters of the CBC are beyond the scope of this supplement.

2.1.2 DSA:

The applicable DSA Sections and Chapters of the CBC are beyond the scope of this supplement.

This supplement expires concurrently with the evaluation report, reissued May 2022.

Grip-Tite Standard, Heavy Duty and New Construction Brackets

RETROFIT (STANDARD DUTY) BRACKET CAPACITIES

Ultimate Capacity- Compression	60 kips
Allowable Capacity - Compression	30 kips
Ultimate Capacity - Tension	20 kips
Allowable Capacity - Tension	10 kips
Ultimate Capacity - Lateral	20 kips
Allowable Capacity - Lateral	10 kips

RETROFIT (HEAVY DUTY) BRACKET CAPACITIES

Ultimate Capacity- Compression	90 kips
Allowable Capacity - Compression	45 kips
Ultimate Capacity - Tension	20 kips
Allowable Capacity - Tension	10 kips
Ultimate Capacity - Lateral	20 kips
Allowable Capacity - Lateral	10 kips

NEW CONSTRUCTION BRACKET

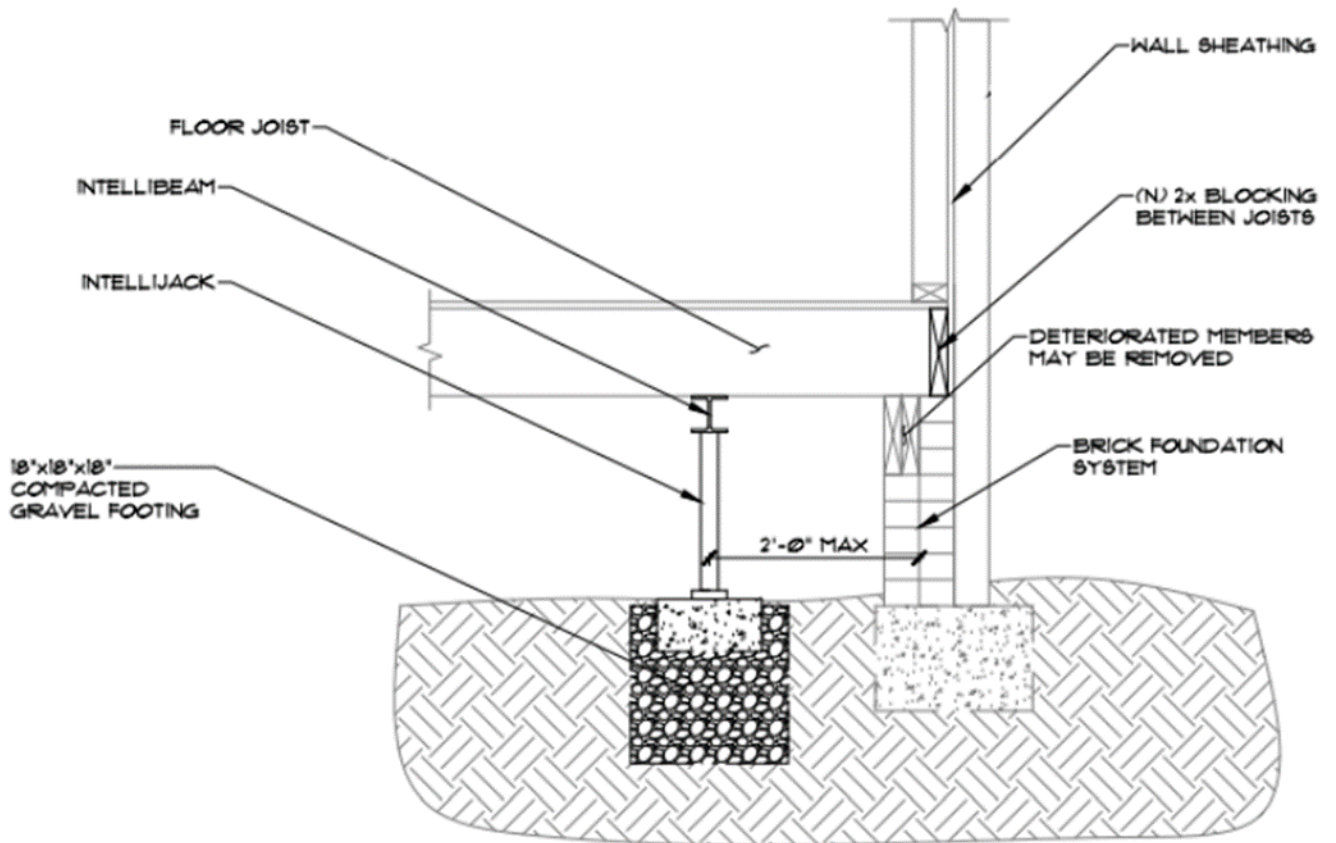
Plate (ASTM A36)	4" x 8" x 1" thick
Coupler (ASTM A500)	3.5" OD x 7.0" x 0.300" wall
Bolt (Zinc Plated ASTM 354)	1 - 3/4" diam. SAE J429 Grade 8

NEW CONSTRUCTION BRACKET CAPACITIES

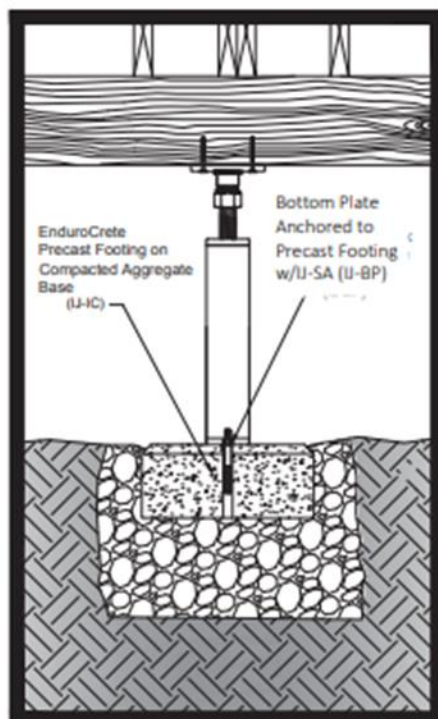
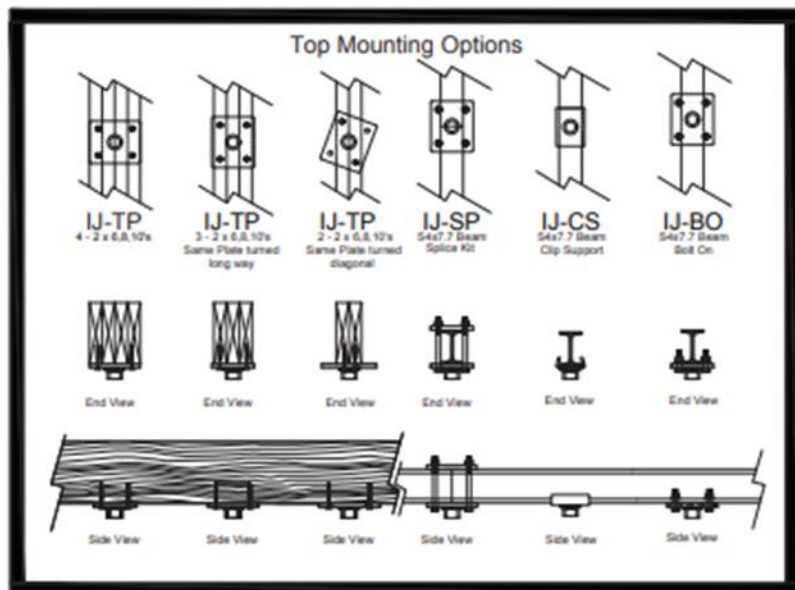
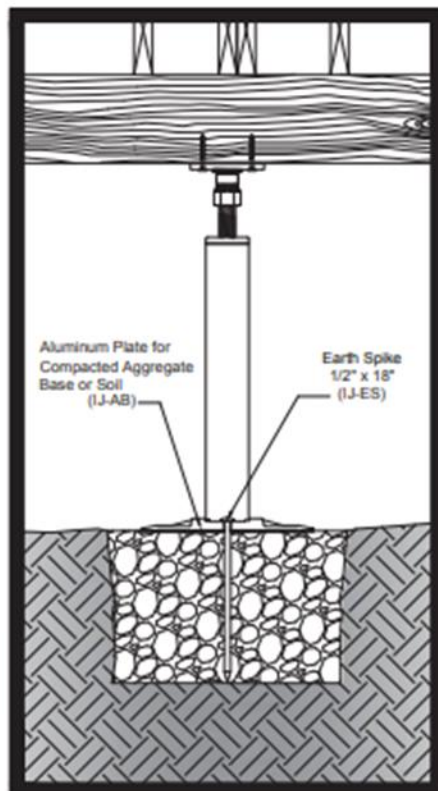
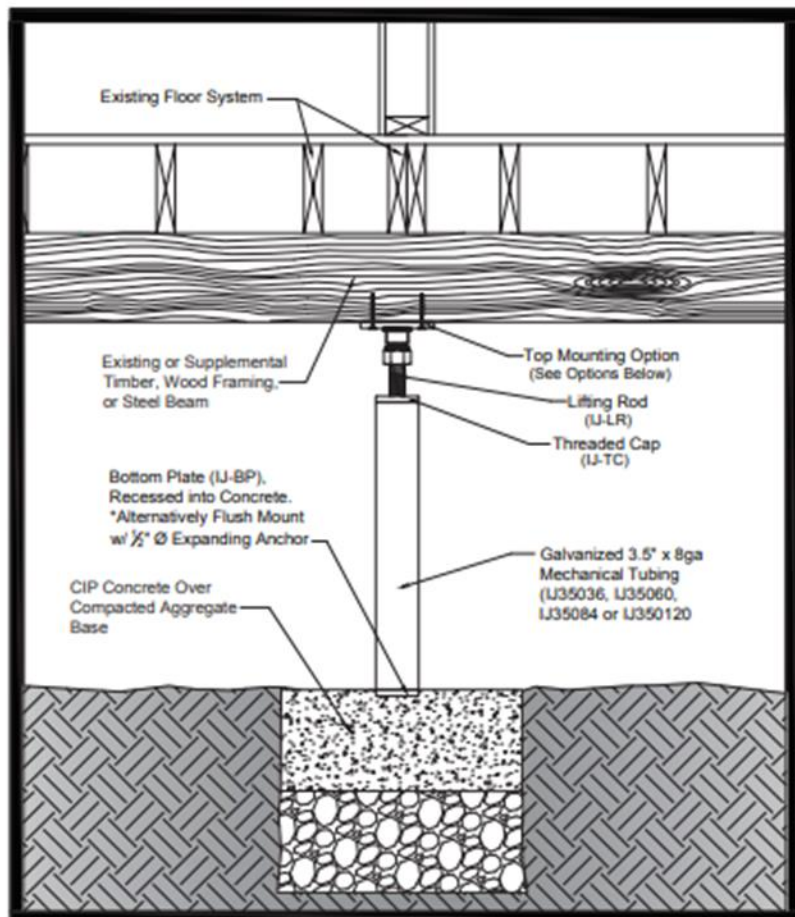
Ultimate Capacity - Compression	85 kips
Allowable Capacity - Compression	42.5
Ultimate Capacity - Tension	66 kips
Allowable Capacity - Tension	33 kips



Product Detail - Intellijacks



Crawl Space and Basement Detail



IntelliJack™

Product Specifications

IMG INDEPENDENCE
MATERIALS
GROUP

IntelliJack Load Capacity

The allowable load carrying capacity for the IntelliJack is shown the PEI Code Evaluation Report, PER-19250. Allowable downward compressive load varies with column height but is established at 20.7 kips at an assembled height of 10 feet. Refer to the Evaluation Report for higher load values allowed at shorter column lengths. Allowable capacity for the IntelliJack system is likely to be governed by the footing design and available allowable soil pressure.

Installation for Permanent Applications

For applications where the construction is to be permanent, the threads of the lifting rod (IJ-LR) shall be damaged with a cold chisel after adjustment to the desired length. One screw thread immediately above the threaded cap shall be damaged for at least half its depth, and for a length of 1.5 inches to prevent loosening of the rod. Upward adjustment will still be feasible.

IJ-TP (Top Plate)

1/2" THK x 5" x 6" long, ASTM A36 hrp&o plate with four 0.313" holes at corners and one 1.75" OD x 0.25" thick wall x 1.25" long retaining ring centered on plate with 0.25" external fillet weld, all around.

Clear Zinc Electroplated to ASTM B633 Type III

IJ-35036, IJ-35060, IJ-35084, IJ-350120 (Mechanical Tubing)

3.50" OD x 0.165" thick wall ASTM A500, Grade C mechanical tubing, available 36", 60", 84" and 120" long and may be field cut to desired length.

Yield strength = 46 ksi (min)

Tensile strength = 62 ksi (min)

Galvanized to ASTM A1057.

IJ-SP (Splice Plate)

Upper and lower 1/2" THK x 5" x 6" long ASTM A36 hrp&o plates with four 0.563" holes at corners with welded retaining ring on lower plate with same detail as IJ-TP. Plates are bolted with 4- 1/2" x 5.5" SAE Grade 5 bolts and nuts, sandwiching the S4x7.7 steel beam. Center of splices must align with center of support, below.

Clear Zinc Electroplated to ASTM B633 Type III

IJ-BO (Bolt On Beam Mount)

1- 1/2" THK x 5" x 6" long ASTM A36 hrp&o plate with four 0.563" holes at corners with welded retaining ring on lplate with same detail as IJ-TP. Plates bolted with 4- 1/2" x 5.5" SAE Grade 5 bolts and nuts, through the bottom flange of the S4x7.7 steel beam

Clear Zinc Electroplated to ASTM B633 Type III

IJ-BP (Bottom Plate)

1/2" THK x 3.5" x 3.5" long ASTM A36 hrp&o Plate with one 0.563" hole centered on plate, and one 3.13" OD x 0.188" thick wall x 1" long ASTM A513 ring, internally welded to plate.

Clear Zinc Electroplated to ASTM B633 Type III

IJ-PB (Precast Base)

Commercially available, precast concrete post base with 3,000 psi (min) compressive strength, and 12" x 12" base footprint.

IJ-ES (Earth Spike)

5/8" x 18" long ASTM A615 G40 pointed reinforcing rod stake with welded nut.

Yield strength = 40,000 psi(min)

Tensile strength = 60,000 PEI(min)

IJ-LR (Lifting Rod)

1.25" diameter x 10" long ASTM F1554-GR55 S1 threaded rod with tack welded heavy hex nut

Yield strength = 55 ksi (min)

Tensile strength = 75 ksi (min)

Clear Zinc Electroplated to ASTM B633 Type III

IJ-TC (Threaded Cap)

3.50" OD x 1.00" thick solid ASTM A108-13 (1018) disk CNC machined with 1.25"-7 threads and recessed shoulder to fit the inside diameter of the mechanical tubing.

Yield strength = 50 ksi (min)

Tensile strength = 68 ksi (min).

Clear Zinc Electroplated to ASTM B633 Type III

IJ-CS (Clip Support)

1/4" THK x 3.30" x 1.25" x 4.50" long, ASTM A36 hro&p plate with ends shaped to receive insertion of S4x7.7 beam flange, and 1.75" OD x 0.25" thick wall x 1.25" long retaining ring centered on plate with 0.25" external fillet weld, all around. Clear Zinc Electroplated to ASTM B633 Type III

S4x7.7 Supplemental Steel Beam

4.00" Tall X 2.63" wide, steel S shape (I-beam), ASTM A992, available 120" and 144" long, and may be field cut to desired length.

Yield Strength = 50 ksi (min)

Tensile strength = 65 ksi (min)

IJ-AB (Aluminum Base)

1" thick, at the hub, x 14.2" diameter AA 356.0-T5 cast aluminum base plate.

Yield strength = 20 ksi (min)

Tensile strength = 25 ksi (min)

IJ-IC (Precast Concrete Footing)

6" THK x 14" diameter, 5,000 psi (min), fiber reinforced, precast concrete footing with a 0.75" diameter hole at center, and ¼" chamfer at upper circumference.

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Initial Approval
October, 2019

Re-Approved
October, 2020

See all PEI ES reports at: www.p-e-i.com

Report Owner

Independence Materials Group, LLC (IMG)
1741 Corporate Landing Parkway
Virginia Beach, VA 23454

Approved Manufacturing Location

Independence Materials Group (IMG)
634 Rosewood Drive, Door 7
Columbia, SC 29201

Product

IntelliJack Support Columns

Evaluation Report Information

www.independencematerialsgroup.com
IMG contact: Allen Gantt - (803) 807-8629

General Details

The following described support columns are evaluated for vertical compression load only. The support columns are intended for interior use in wood frame or light steel frame construction. These columns have a rust-inhibitive coating.

Product Description

The IntelliJack columns are a steel tube column with a loose bottom plate at one end and an adjustable screw at the other end. The columns are constructed of a Steel Tube, a Collar, Threaded Rod and two nuts. These columns have a 5" adjustability using the threaded rod with a damaged thread to limit travel.

Component Description

Steel Tube - 3-1/2" outside diameter by .165" thick made from ASTM A500 Steel, Grade B-C, $F_y \geq 50\text{ksi}$, $F_u \geq 55\text{ksi}$. and galvanized to ASTM A1057.

Collar (threaded cap) - The collar is placed in one end of the steel tube. A threaded hole is in the center of the collar for the threaded rod. The collar is made from ASTM A1018 steel and zinc electroplated to meet ASTM B633, Type III.

Threaded Rod - The threaded rod is 1.25"-7 x 10" long. The threaded rod is made from ASTM F1554 Grade 55 Steel and is zinc electroplated to meet ASTM B633, Type III.

Nut - 1.25"-7 Heavy Hex nut, ASTM A563 Grade A with a zinc coating. One nut is welded to the threaded rod and one is used as a jam nut against collar.

Bottom Plate - The bottom plate is an assembly made from a 3-1/2" x 3-1/2" x 1/2" thick plate made from ASTM A36 steel and a 3/4" long piece of 3" outside diameter tube welded to the plate and is clear zinc electroplated to meet ASTM B633, Type III.

Top Plate - The top plate is an assembly made from a 6" x 5" x 1/2" thick plate made from ASTM A36 steel, with a 1-1/4" long piece of 1-3/4" outside diameter tube welded to the plate and is clear zinc electroplated to meet ASTM B633, Type III.

Code Compliance

IntelliJack Support Columns	
2012 International Residential Code Section R407.2 and R407.3	2012 International Building Code Section 104.11 and 2205.1
2015 International Residential Code Section R407.2 and R407.3	2015 International Building Code Section 104.11, 2205.1 and 2205.2
2018 International Residential Code Section R407.2 and R407.3	2018 International Building Code Section 104.11, 2205.1 and 2205.2

Compliance with the following Standard

- AISC-360-10** - Specification for Structural Steel Buildings-Allowable Stress Design
- AISC-360-16** - Specification for Structural Steel Buildings-Allowable Stress Design
- ICC-ES AC335** - Acceptance Criteria for Adjustable Steel Columns

General Product Use

1. All columns shall be installed vertically plumb with either end up and bear on a footing capable of carrying the imposed load. The base and top of the column must be restrained to prevent lateral movement except where exempt by the Building Code.
2. These columns are intended for vertical compression load only. They have not been evaluated for any other load direction or type.
3. For applications where the construction is to be permanent, the threads of the lifting rod shall be damaged with a cold chisel after adjustment to the desired length. One screw thread immediately above the threaded cap shall be damaged for at least half its depth and for a length of 1-1/2" to prevent loosening of the rod.
4. The **IMG IntelliJack** nominal size is the support column at its shortest adjustment and must not be adjusted to a longer length by more than 5".
5. The bearing capacity of the top and bottom plates and their attachment to load carrying members are outside of the scope of this evaluation report.
6. If the basement slab is poured around the IntelliJack, a corrosive resistant coating must be applied to the parts of the IntelliJack that will be in contact with concrete.

Items Requiring Verification

The following items are related to the use of **IMG IntelliJack** support columns, but not within the scope of this evaluation specification. However these items are related to the determination of code compliance.

1. Design, calculations, and details for the building system verifying compliance with this report.
2. Connections of the columns to the footing and supported structure.
3. Footing design and calculations for supporting the columns and the imposed load.
4. The bearing capacity of the beam/member supported.

Table 1
Load Rating for IntelliJack Columns

Nominal Size	3-1/2" O.D.	
	Allowable Load for ASD (lbs.)	Design Strength for LRFD (lbs.)
12"	24,860	39,780
96"	24,860	39,780
108"	24,100	36,220
120"	20,700	31,110

Notes:

1. All steel tube to be ASTM A500 $F_y \geq 50\text{ksi}$, $F_u \geq 55\text{ksi}$
2. Nominal size is the column at it's shortest adjustment
3. Designed per AISC-360-16 - ASD
4. LRFD Design Strength must be compared to factored loads

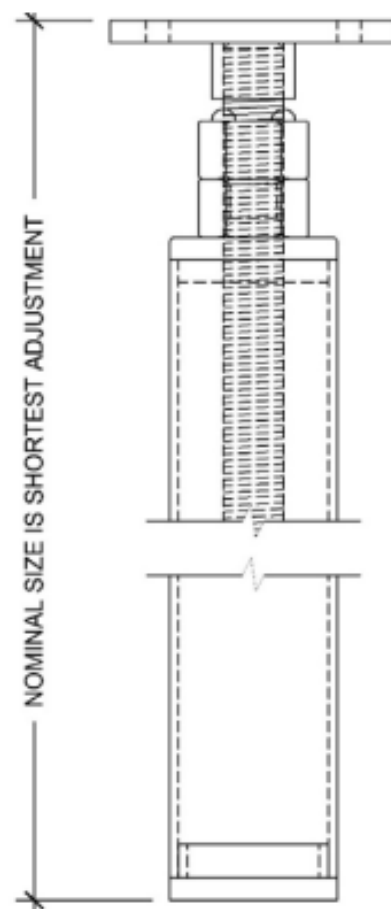


Figure 1 - IntelliJack Column

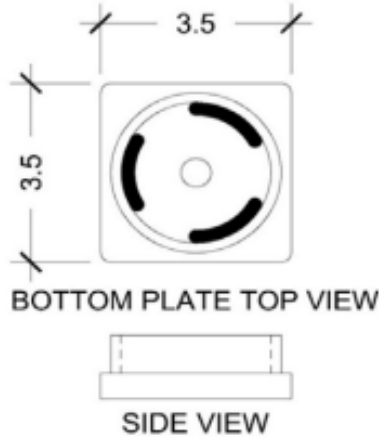


Figure 2 - Bottom Plate

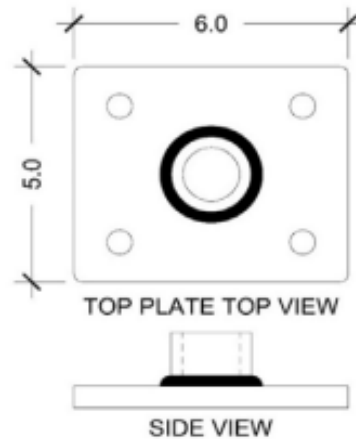


Figure 3 - Top Plate

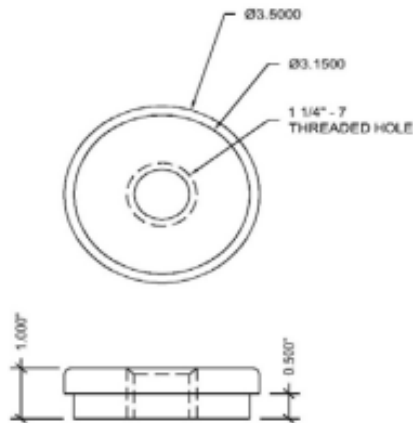


Figure 4 - Collar

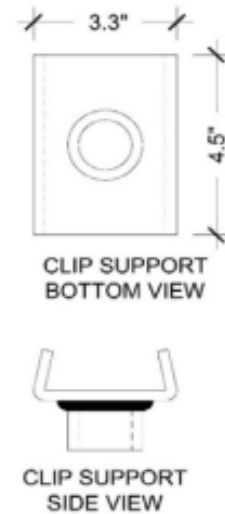


Figure 5 - Clip Support

Product Labeling

All columns manufactured by **Independence Materials Group** that are covered by this **PER** must have a label attached with at least the following information:

1. Manufacturer Name
2. Load Capacity of Column
3. This **PER** Number & **Pei ES** Logo
4. On Adjustable Column Labels, the Minimum and Maximum Column Length

Acceptable Evaluation Marks



Product Documentation

- A Product Evaluation Service Agreement between **Pei Evaluation Service®** and **Independence Materials Group, LLC**
- An Follow-up Inspection Service Agreement between **Progressive Engineering Inc.** and **Independence Materials Group, LLC**
- A **Independence Materials group, LLC** - IntelliJack support columns Quality Control Manual - Dated: 9/23/2020
- A **Independence Materials Group, LLC** - IntelliJack support columns Components Drawings - Dated: September, 2015
- Pei** Calculations No. 2019-3169 - IMG - Column Calculations - Dated: 6/18/2019 - stamped by a Professional Engineer
- Pei** Calculations No. 2019-3169 - IMG - Adjustment Screw Calculation - Dated: 11/1/2019 - stamped by a Professional Engineer
- Pei** Calculations No. 2020-0214 - IMG - IntelliJack Eccentric Loaded Axial Capacity - Dated: 1/23/2020 - stamped by a Professional Engineer
- Pei** Test Report 2019-6392 - Dated: 12/19/2019 - stamped by a Professional Engineer