

ENGINEERS PLANNERS CONSULTANTS

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July 3, 2019

Mr. Mike Hamm, PE State of North Carolina Department of Insurance Manufactured Building Division 322 Chapanoke Road Suite 200 Raleigh, NC 27603

RE: CMH Manufacturing, Inc. #958

Model: 2484 - NC

Dear Mr. Hamm,

Enclosed, you will find one (1) copy of the above mentioned project for your files.

Should you have any questions or comments, please contact me at your earliest convenience.

Sincerely,

David Richter

David Richter Account Manager

Enclosures







Date: 7/1/2019

TYPE: MODULAR

MODEL PLAN INDEX

Model #	2484	State
Manufacturer	CMH Manufacturing, Inc.	
Brand Name	CLAYTON	NC
Unit Size	29'-8" x76'-0"	
Description	4 BEDROOM / 2.5 BATH	

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Technical Sheet	HVAC System Calc	ATTACHED
Technical Sheet	Electrical Load Calc	TS-5
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Technical Sheet	Trusses	ATTACHED
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	ODULAR MANUAL FOR ;	
1. SECTIONS		2. TYPICAL DETAILS
3. REQUIRED CON	STRUCTION METHODS	4. MATERIALS

CMH

Manufacturing, Inc. engineering department - modular

	REVISIO	NS
DATE:	REVISION BY :	GCK
July 1, 2019	REVISION DATE :	

TECHNICAL SHEET FOR LIGHT / VENT DATA

MODEL NUMBER	2	2484
SIZE OF UNIT	29'-8	" x76'-0"
WINDOW SQ. FTG. STD.		
WINDOW SQ. FTG. W/ OPT.		
FIGURED FOR :	CLAYTO	N WINDOWS
PERCENTAGE OF LIGHT REQ'D.	•	8%
PERCENTAGE OF VENT REQ'D.		4%

		Square	Footage			Percen	tage of		
		Inst	talled	Requ	uired	Insta	alled	Artifical	Artifical
Room	Area	Light	Vent	Light	Vent	Light	Vent	Light	Vent
LIVING ROOM	369.3	48.8	24.8	29.5	14.8	13.2%	6.7%		
MASTER BEDROOM	217.2	36.6	18.6	17.4	8.7	16.9%	8.6%		
BEDROOM 2	143.9	24.4	12.4	11.5	5.8	17.0%	8.6%		
BEDROOM 3	128.1	24.4	12.4	10.2	5.1	19.0%	9.7%		
BEDROOM 4	112.0	12.2	6.2	9.0	4.5	10.9%	5.5%		
DINING ROOM	133.2	33.6	16.8	10.7	5.3	25.2%	12.6%		
DEN	158.0	24.4	12.4	12.6	6.3	15.4%	7.8%		
KITCHEN	198.0	11.5	2.6	15.8	7.9	5.8%	1.3%	YES	YES



APPLICATION ENGINEERING FOR HEATING AND COOLING

CMH Mfg., Inc. 2225 South Holden Road Richfield, NC 27417-0386

BASE MODEL 2484 Manufacturer's Model #:

HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Prepared By LaSalle Air Systems 7/1/2019 {Method & Output 2019}

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Calculations on this page are based on design standards set forth in ASHRAE and ACCA Manuals J Rev 8.2 and D Rev 1.1. System registers are located for best distribution based on Manual T. Design calculations are based on worst case orientation. Room loads may vary based on actual conditions.

ENTIRE HOUSE VALUES - DESIGN ZONE: NC, Region 4 NCECC (2018)/IECC (2015NC) 36N Latitude

COOLING LOAD: 41,179 Btuh for Outside Temp/Humidity of 92 $^{\circ}$ F (33 C)/ 48% and Inside reduced to 75 $^{\circ}$ F (23 C)/ 50%

HEATING LOAD: 16 $^{\circ}$ F (-9 C) with inside temp raised to 72 ° F (22 C) 43,991 Btuh based on outside temp of

Crawlspace is not heated by the primary air handler. Actual UA = 388.3 Max UA (Table R402.1.2) = 414.5 Use net wall area, not gross wall

CONSTRUCTION DETAILS & U / SHGC VALUES: (22+Non-ins Rim - 15 - 38)

Total Cond. Floor Area:	2231.66 s.f.	TRUE Outside Perimeter:	221.33 ft	
Level 1 Ceiling: 108	to 108 in. Level	2 Ceiling: 0 to 0 in.	Level 3 Ceiling: 0 to 0 in.	Net Roof Area (less ducts): 2125.4 s.f.
Primary Wall Area:	1649.44 s.f. (Net)	Dark Roof(U): 0.027	FLOOR DUCTS (U):	0.0444 Duct TEL
Secondary Wall Area:	0.00 s.f. (Net)	Prim Wall (U): 0.070	ATTIC DUCTS (U):	0.125 527.3 ft
TOTAL Low-E window	282.56 s.f.	Sec Wall (U): 0.035	EXT. DUCTS (U):	0.125
TOTAL S.G.D.	40.00 s.f.	Exp Floor(U): 0.044	INFLOOR DUCT AREA:	430.67 S.F. @ 51.2 TD/ 26.6 TD
TOTAL Glass Block	0.00 s.f.	Low-E wi 0.350 / 0.28	ATTIC DUCT AREA:	144.73 S.F.(return) @ 96 TD/ 88.2 TD
TOTAL Skylite	0.00 s.f.	S.G.D. 0.320 / 0.29	EXT. DUCT AREA:	207.34 S.F. @ 96 TD/ 45 TD
TOTAL Door1 Area:	20.00 s.f.	Glass Blc 0.510 / 0.48	PEOPLE: 5	4121.4 Btuh Total Appliances
TOTAL Door2 Area:	0.00 s.f.	Skylite 0.460 / 0.75	FIREPLACES:	1
All Glass % of Floor:	14.45 %	Door 1: 0.140	DUCT GAIN: @ Semi-Tight	4512 Btuh
All Glass % of Wall:	16.19 %	Door 2: 0.670	DUCT LOSS:	10240 Btuh
LATENT GAIN:	8677 Btuh		Summer Infiltr (7.5 mph):	39.0 cfm
Mech. Ventilation:	118.13 cl (55.7 L/s)	Altitude: 1000 ft	Winter Infiltration (15 mph):	86.7 cfm @ Semi-Tight

ROOM BY ROOM VALUES:

99 deg

A/C Exiting:

Heat Exiting Furnace:

1037.7 FPM, max velocity in trunk #:

2

0.34 Max pressure at A/H

Actual	heating	g and cooling re	equired in each r	oom and	Cooling Air		Heating Air			
flow set to	o maxir	num of either he	eating or cooling		Values for		Values for	60	15.0 KW	Maximum A/C capacity
		HEATING	COOLING	CFM	4 to	on unit	90	% Gas/Oil	Elec	Calibrated Blower Test
ROOM NAME		LOSS (Btu)	GAIN (Btu)	DIST	CFM	Btuh	CFM	Btuh E	Btuh	Btuh (alt adj)
Bedroom #4	С	3,518	2,944	107	136	3,887	126	4,469	4,236	3,897
Kitchen	С	3,429	2,983	108	132	3,770	123	4,335	4,108	3,824
Dining Room	С	3,181	3,554	128	126	3,592	117	4,130	3,914	3,718
Living Room	С	5,725	5,879	206	191	5,440	177	6,255	5,928	5,603
M.B.W.I.C.	h	1,300	829	39	48	1,363	44	1,567	1,485	1,376
M. Bath	С	4,840	4,800	175	156	4,459	145	5,128	4,860	4,461
M. Bedroom	С	5,427	5,218	184	232	6,611	215	7,602	7,205	6,810
Foyer	h	1,952	1,400	58	55	1,555	51	1,788	1,694	1,613
Bedroom #2	С	4,156	3,908	141	144	4,115	134	4,731	4,484	4,324
Bath #2	h	1,147	715	34	63	1,796	58	2,065	1,957	1,853
Bedroom #3	С	3,105	3,143	114	104	2,976	97	3,421	3,243	3,049
Den	С	2,990	3,077	112	115	3,280	107	3,772	3,575	3,259
Utility	С	2,013	1,964	77	42	1,193	39	1,372	1,300	1,204
1/2 Bath	h	1,206	764	36	43	1,236	40	1,421	1,347	1,226
TOTALS	-	43,991	41,179	1,519	1,589	45,270	1,473	52,056	49,338	46,216







CMH Mfg., Inc.

APPLICATION ENGINEERING DUCT AIR FLOW AND SIZING WORKSHEET (MANUAL D)

Manufacturer: CMH Mfg., Inc. Model #: BASE MODEL 2484

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC)

Prepared by LaSalle Air Systems 7/1/2019 All rights reserved. This information proprietary to LaSalle Bristol Co. and

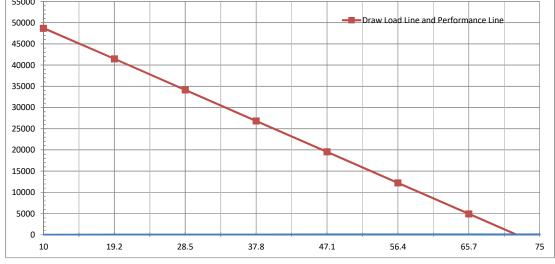
Blower CFM	1754	@	0.8	E.S.P.		TEL=	596.3975		FR=	0.0822	(A/C C	Coil inclu	ded)			
					Al	titude =	1,000 f	ft				i		User Inpu		
Branch Duct Listing		Motol	E C	Floy	Panda/	Total Ea	Hoot	Cool	Elec	(Altitud	• •	Pound	Postor	nalo Cizo	Final	Final Velocity
BR #	Trunk #	Metal (ft)	F. G. (ft)	Flex (ft)	Bends/ Fittings(ft)		Heat Btuh	Cool Btuh	Heat cfm	Cool cfm	Design cfm	Round Size		ngle Size x (i.d.)	Round Size	fpm
1 Bedroom #2	4	34	0	36	449.1	519.1	1,750	1,646	70	59	70	6.35	(/	()	6.0	355.
2 Bath #2	4	34	0	31	437.6	502.6	1,147	715	46	26	46	5.34			6.0	232.
3 Bedroom #3	4	34	0	36	429.1	499.1	1,890	1,913	75	69	75	6.47			6.0	383
4 Bedroom #3	4	34	0	36	419.1	489.1	1,216	1,230	48	44	48	5.41			5.0	355.
5 Utility	4	34	0	36	409.1	479.1	2,013	1,964	80	71	80	6.55			5.0	588
6 1/2 Bath	4	34	0	36	399.1	469.1	1,206	764	48	27	48	5.33			5.0	352.
7 Den	4	34	0	30	387.3	451.3	1,852	1,906	74	68	74	6.21			6.0	375.
8 Den	4	34	0	33	396.5	463.5	1,138	1,171	45	42	45	5.20			5.0	332
9 Bedroom #2	5	34	0	36	419.1	489.1	2,406	2,262	96	81	96	7.09			6.0	488
10 Foyer	5	34	0	32	407.9	473.9	1,952	1,400	78	50	78	6.45			5.0	570.
11 M. Bedroom	5	34	0	36	399.1	469.1	2,074	1,994	83	72	83	6.58			6.0	420
12 M. Bedroom	5	34	0	38	398.8	470.8	2,049	1,971	82	71	82	6.56			6.0	415.
13 M. Bedroom	5	34	0	37	397.7	468.7	1,303	1,253	52	45	52	5.48			5.0	380
14 Dining Room	6	34	0	45	448.3	527.3	1,939	2,167	77	78	78	6.65			6.0	396.
15 Dining Room	6	34	0	45	438.3	517.3	1,242	1,388	49	50	50	5.56			5.0	365
16 Kitchen	6	34	0	43	427.7	504.7	2,087	1,815	83	65	83	6.76			6.0	423
17 Kitchen	6	34	0	43	417.7	494.7	1,342	1,167	53	42	53	5.65			5.0	392
18 Bedroom #4	6	34	0 0	45 47	408.3	487.3	1,359	1,137	54	41	54	5.65			5.0	397
19 Bedroom #4	6	34		47 45	407.5	488.5	2,159	1,806	86	65	86 05	6.79			6.0	438
20 Living Room	7 7	34 34	0 0	45 45	448.3 438.3	527.3	2,135 2,177	2,192	85 87	79 80	85 87	6.90 6.93			6.0 6.0	433 441
21 Living Room	7	34	0	45 44	430.3	517.3 502.1		2,236	56	52	56	5.79				413
22 Living Room 23 M.B.W.I.C.	7	34	0	41 44	418	496.0	1,413 1,300	1,451 829	52	30	52	5.79			5.0 5.0	379
24 M. Bath	7	34	0	42	407.4	483.4	2,427	2,407	97	86	97	7.09			6.0	492
25 M. Bath	7	34	0	45	406.9	485.9	2,427	2,393	96	86	96	7.09			6.0	489
N/A Other Rooms	,	34	U	40	400.3	400.0	2,410	2,000	30	00	30	7.03			0.0	403
							43,991	41,179	1,753	1,480	1,754					
RUNK DUCT LISTING		50			100	150.0	43,991	41,179			1754	15.43	12	14	14.2	1503
TRUNK#				28	252.598	280.6	21,997	20,190			876	13.62		•	12.0	1115
TRUNK#					272.725	310.7	21,993	20,989			877	14.09			12.0	1116
TRUNK# 4	4	34			280.598	314.6	12,212	11,309			487	11.21	5	14	8.9	1001
TRUNK#	5	34			280.598	314.6	9,785	8,881			390	10.33	5	14	8.9	802
TRUNK# 6	6	34			310.725	344.7	10,128	9,481			404	10.78	5	14	8.9	832
TRUNK #	7	34			310.725	344.7	11,865	11,508			473	11.45	5	14	8.9	972
TRUNK# 8							-	-			0		0	C	1	
TRUNK # 9							-	-			0		0	C		
TRUNK # 1							-	-			0		0	C)	
TRUNK# 1							-	-			0		0	C		
TRUNK # 1							-	-			0		0	C		
TRUNK # 1							-	-			0		0	C)	
TRUNK # 1				23			-	-			0					
TRUNK # 1	5			49			-	-			0					
LONGEST				40	20	60					1754	14.04	40	0.4	22.7	E0.4
RETURN DUCT				49	20	69					1754	14.24	18	24	22.7	584

APPLICATION ENGINEERING EQUIPMENT SELECTION AND SIZING WORKSHEET (MANUAL S)

Manufacturer: CMH Mfg., Inc. Model #: BASE MODEL 2484

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC) Prepared by LaSalle Air Systems 7/1/2019 All rights reserved. This information proprietary to LaSalle Bristol Co. and CMH Mfg., Inc. RESULTS FROM MANUAL-J CALCULATIONS: Worst Case Orientation 16 ° REQ'D BLOWER CFM: 1,589 cfm at altitude of HEATING LOAD: 43.991 Blub at 1 000 ft SENSIBLE CLG LOAD: 32,502 Btuh at 92 ° Entering Air DRY Bulb: 76.1 ° Mech. Ventilation: 118 61.2 ° **8,677** Btuh at 92 ° LATENT CLG LOAD: Entering Air WET Bulb: Entering Air RH: 53 % 72.0 ° GRAINS DIFFERENCE: Outside wet bulb: outside RH: 48.2 % FILL IN BLANKS IN EACH SECTION FROM THE H.V.A.C. EQUIPMENT DATA CHARTS: (Do not use ARI Ratings!) Air handler model #: Condenser model #:_____ Blower Data Select blower speed in COOLING mode: _ Blower CFM is between 1491 > < 2017 for Total (External) Static Pressure between 0.7> <0.9 Electric, Gas or Oil Furnace Select blower speed in HEATING mode: ______ Output Btuh is between 46190> <61586 Blower CFM is between 797 >____< 942 for Temp. rise of 55-65 Blower CFM is between 942 >____< 1152 for Temp. rise of 45-55 **APPROVED BY** Blower CFM is between 1152 > < 1481 for Temp. rise of 35-45 Cooling Equipment S/T Ratio = 0.78 Leaving Temp = 50.8° TD = 24.2 $^{\circ}$ At 92F outside, Total A/C output from 42002 btuh _____ 47355 btuh is GOOD. to 47355 btuh _____ to At 92F outside, Total A/C output from 49414 btuh is MARGINAL. approve any deviation or deviations from the requirements of applicable State Laws. Sensible Capacity is from 28163 btuh _____ to 36840 btuh David Richter Latent Capacity is from 8503 btuh _____ to 13015 btuh Mechanical Ventilation is 6.7 % of blower cfm. Dry bulb increases by: 1.1 ° Wet bulb increases by: 0.7 ° Heat Pump with Supplemental Heating Coils **Data from performace charts Data from load calculation** ____ btuh at _____ F outside 0 btuh at 72 F outside ____ btuh at ____ F outside 43,991 btuh at 16 Foutside 55000 Draw Load Line and Performance Line 50000 45000 40000 35000



At winter design temperature of 16 F outside, the distance between the lines is _____ btuh

which is the Supplemental Heat divided by 3400 = _____ KW.

APPLICATION ENGINEERING INTERNATIONAL MECHANICAL CODE - Chapter 4 Ventilation Worsheel

Manufacturer: CMH Mfg., Inc. Model #: BASE MODEL 2484

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC)

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RESULTS FROM MANUAL-J CALCULATIONS: Worst Case Orientation

HEATING LOAD: 43,991 Btuh at 16 ° REQ'D BLOWER CFM: 1,589 cfm at altitude of 1000 ft

92 ° 76.1 ° SENSIBLE CLG LOAD: 32,502 Btuh at Entering Air DRY Bulb: Mech. Ventilation: 61.2 ° 92 ° LATENT CLG LOAD: 8,677 Btuh at Entering Air WET Bulb: Entering Air RH: **GRAINS DIFFERENCE:** Outside wet bulb: 72.0 ° outside RH: 48 %

Natural or Mechanical: Test the infiltration at 50 Pa should result in 647.2 CFM infiltration being 1.933 ACH (to be confirmed by testing)

(5 ACH = 1673 CFM) (3 ACH = 1004 CFM) **Mechanical ventilation is required**

To Meet Natural Ventilation: Increase Openable Area by 158 %

		Openal	ole Area			Opena	able Area
ROOM NAME	Room Area	Required	Built	ROOM NAME	Room Area	Require	Built
Bedroom #4	150.8	6.0	7.50	Bath #2	80.3	3.2	0.00
Kitchen	210.1	8.4	7.78	Bedroom #3	153.3	6.1	15.00
Dining Room	139.7	5.5	20.00	Den	159.0	6.3	15.00
Living Room	317.7	12.7	30.00	Utility	76.0	3.0	7.50
M.B.W.I.C.	97.7	3.9	0.00	1/2 Bath	34.5	1.3	0.00
M. Bath	219.0	8.7	21.00		0.0	0.0	0.00
M. Bedroom	244.8	9.7	22.50		0.0	0.0	0.00
Foyer	83.1	3.3	0.00		0.0	0.0	0.00
Bedroom #2	265.8	10.6	15.00		0.0	0.0	0.00
				TOTAL	2231.7	88.7	161.28

Mechanical Ventilation Is Required In These Areas To Meet IMC 2012/2015 Per Table 403.3.1.1:

			Outdoor	Exhaust		Air
SPACE CLASSIFICATIONS	Occupancy	Area	Air	Air	ZONE AIR DISTRIBUTION	Flow
Private Living Area	5.1	1687.7	118.1	0.0	Floor Supply of Warm Air/Floor Return	1193
Private Kitchen	0.0	210.1	0.0	25.0	Floor Supply of Warm Air/Floor Return	132.3
Private Baths	0.0	333.9	0.0	105.0	Floor Supply of Warm Air/Floor Return	262.9
	0.0	0.0	0.0	0.0		0
	0.0	0.0	0.0	0.0		0
	0.0	0.0	0.0	0.0		0
Total	5.1	2,231.7	118.1	130.0		1,588
					System Ventitlation Efficiency	y: 1

APPROVED BY

7/3/2019

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

David Richter

ELECTRICAL FEEDER CALCULATION

CMH Manufacturing, Inc. engineering department - modular PAGE: 1 of 1 DATE: 1-Jul-19 BY: GCK

MODEL NO. 2484	2404		
MODEL NO.	2404	Per NEC 220-30	

1. LIGHTING LOAD:										
1st floor			2nd floor							
length =	76.00	FT.	length =	0.00	FT.					
width =	29.67	FT.	width =	0.00	FT.					
		-		-	-					
Total area =	2231	SQ. FT.	Minimum number	4						
X	3	VA	of 15 Amp circuits =	4						
TOTAL	6693	VA								

2. SMALL AP	PLIANCE	LOAD:	3. LAUNDRY LOAD:				
Number of 3			Number of	1			
circuits			circuits				
X	1500	VA	X	1500	VA		
TOTAL	4500	VA	TOTAL	1500	VA		

4. APPLIANCE LOAD:		
Electric Range =	12100	VA
Electric Water Heater =	5000	VA
Electric Clothes Dryer =	5600	VA
Cooktop =	0	VA
Wall Oven =	0	VA
Freezer =	1200	VA
Dishwasher & Disposal =	2376	VA
Gas furnace motor =	0	VA
Micro-wave oven	1200	VA

5. TOTAL OF OTHER LOADS (1, 2 & 3)									
	LEG A								
Lighting load =	6693								
Small appliance load =	4500								
Laundry =	1500								
Appliance load =	27476								
Sub-Total =	40169								
10000 VA @ 100% =	10000								
Remainder @ 40% =	12068								
Total =	22068	VA							
	91.95	AMPS							



6. HVAC LOAD:									
Lineal feet of bas	seboard hea	aters =		0					
Number of basel	oard heate	r circuits =		0					
Total baseboard	heater load	=		0.0	Amps				
Use 65% w/ less than 4 or 40% w/ 4 or more circuits (*)									
Electric furnace (@ 65% (*)								
Circuit 1 =	40	Amps		26.00	Amps				
Circuit 2 =	30	Amps		19.50	Amps				
Air conditioner (*)				Amps				
Total HVAC load	(*- Use lar	45.50	Amps						
			•						

7. TOTAL OF ALL LOADS =

FURN SIZE 12KW

137.45 Amps

NOTE: FLOOR PLAN WINDOW SIZES WITH AN "SG" DESIGNATION REPRESENTS SAFETY GLAZING REQUIRED PER IRC SECTION R308.4 **SIZES** ROUGH OPENING LIGHT (@8%) VENT (@4%) 14 X 40 WDW. 14 1/4" X 40 1/4" 2.50 1.30 24 X35 WDW. 24 1/4" X 35 1/4" 4.10 2.10 24 X54 WDW. 24 1/4" X 54 1/4" 6.80 3.50 30 X 60 WDW. 30 1/4" X 60 1/4" 9.90 5.20 36 1/4" X 35 1/4" 6.60 3.40 36 X 35 WDW. 36 X 54 WDW. 36 1/4" X 54 1/4" 10.80 5.60 36 X 60 WDW. 36 1/4" X 60 1/4" 12.20 6.20 7.70 36 X 72 WDW. 36 1/4" X 72 1/4" 14.90 36 X 08 WDW. 36 1/4" X 08 1/4" 0.50 0.00 36 x 12 WDW. 36 1/4" X 12 1/4" 1.10 0.00 64 1/4" X 35 1/4" 11.50 2.60 64 x 35 WDW. 58 x 35 WDW. 58 1/4" X 35 1/4" 10.10 2.20 **DOORS** 2-8 X 6-8 DOOR 35 1/2" X 80" 3-0 X 6-8 DOOR 38" X 80" PATIO DOOR 72" X 80" 16.8 33.6 ATRIUM DOOR 75 3/8" X 82 1/2" 21.15 17.3

DOOR AND WINDOW SCHEDULE

FASTENING REQUIREMENTS: FOR DOORS AND WINDOWS, USE EITHER #8 X 1" SCREWS, 7/16" X 1 1/2" X 16 GA. STAPLES, OR .092 X 2 1/4" PD NAILS, AT 12" ON CENTER MAXIMUM.

DESIGN CRITERIA

- FLOOR LIVE LOAD = 40 PSF
- GROUND SNOW LOAD = 30 PSF
- ATTIC LIVE LOAD = 10 PSF

CLASSIFICATION: USE GROUP = R

R3 RESIDENTIAL (NON-TRANSIENT) CONSTRUCTION TYPE IS V-B (UNPROTECTED)

- SOIL PROFILE CATEGORY "C"

SEISMIC DESIGN CATEGORY "C" WIND EXPOSURE - 'C' DESIGN WIND SPEED = 90 MPH

-ROOF MEAN HT 22'-0" 100 MPH 120 MPH

ULITMATE WIND SPEED = 117 MPH 130 MPH 152 MPH

ATTENTION LOCAL INSPECTION DEPARTMENT

SET-UP INSTRUCTIONS FOR THIS MODULAR UNIT ARE INCLUDED BY ATTACHMENT TO THESE PLANS. ANY PLAN SET WHICH DOES NOT INCLUDE AN ATTACHMENT ENTITLED "SET UP MANUAL" IS INCOMPLETE SET- UP INSTRUCTIONS

SEE SETUP MANUAL SENT WITH HOME

REQUIREMENTS FOR FIRESTOPPING

INSTALLATION OF NON- COMBUSTIBLE MATERIALS AROUND ALL OPENINGS THAT ARE VERTICAL PENETRATIONS IN THE FLR. AND CLG. ATTENTION LOCAL INSPECTION DEPARTMENT

THE FOLLOWING ITEMS LISTED HAVE NOT BEEN COMPLETED BY CMH MFG, Inc., HAVE NOT BEEN INSPECTED BY NTA, INC AND ARE NOT CERTIFIED BY THE STATE OF NORTH CAROLINA MODULAR LABEL. CODE COMPLIANCES MUST BE DETERMINED BY THE LOCAL JURIS-**DICTION FOR THE FOLLOWING:**

- HVAC SYSTEM (SITE INSTALLATION AND CONNECTIONS) THIS UNIT MUST BE CONNECTED TO A PUBLIC WATER SUPPLY AND SEWER SYSTEM, IF THESE ARE AVAILABLE.

CODE COMPLIANCE

ALL PLANS MEET OR EXCEED THE FOLLOWING:

North Carolina State Building Code Compliance:

- NC Residential Code - 2018 Edition

- NC Electrical Code - 2017



APPROVED BY requirements of applicable State Laws. **David Richter**

RIDGE BEAMS-SIZES AND MAX. SPAN CHART RIDGE BM. CHART-SEE MATING WALL PG. RC-60.0 FOR MAX. **CALCULATIONS-SEE MATING WALL PGS. CRC SECTION**

Soffitt materials for this unit assume that the building face will be 10 feet or greater from the property line when installed on site. Where the building face is less than 10 feet from the property line, underlayment materials and ventilation in accordance with Section R302.1.1,NC Residential Code, must be provided and installed at the site and inspected by the local jurisdiction

THERMAL ZONE REQUIREMENT

THIS BUILDING DESIGN COMPLIES WITH OR EXCEEDS MINIMUM **REQUIREMENTS FOR NORTH CAROLINA THERMAL ZONE 5** -MODEL IS DESIGNED TO MEET THERMAL ZONE 5 AND BELOW PER TABLE N1101.2 REFERENCED IN THE NORTH CAROLINA RESI-DENTIAL CODE. 2018 EDITION FOR ONE & TWO FAMILY DWELL-INGS. REScheck ANALYSIS AND COMPLIANCE REPORT FOR THERMAL ZONE CALCULATION IS PROVIDED FOR EACH SPE-CIFIC MODEL AND IS ATTACHED IN THE SUBMITTED MODEL APPROVAL PACKAGE.

BTUS PER HVAC CALCS

FURNANCE SIZE PER HVAC CALCS

INSULATION PACKAGES

PRESCRIPTIVE

MODULAR MANUAL REFERENCES

ITEMS BELOW ARE REFERENCED FOR NON PRESCRIPTIVE USE

FLOOR: ON FRAME CONSTRUCTION

DETAILS - SECTIONS ON FLOORS FOR ON FRAME: FL-500

CALCULATIONS - SEE CFL SECTION

FLOOR: OFF FRAME CONSTRUCTION

DETAILS - SECTIONS ON FLOORS FOR OFF FRAME: FL - 100

MARRIAGE WALLS - 2x CONSTRUCTION

DETAILS - MW-20.0, MW-30.0, MW-40.0 CALCULATIONS - SEE CMW SECTION

PLUMBING FIXTURES

SEE PAGE PLN - 1.8

ELECTRICAL APPLIANCES AND LOADS

ELECTRICAL - SEE PAGES PLN-1.0 for WH & PLN-1.5 for FURN CALCULATION - SEE TECHNICAL SHEET ATTACHED FOR MODEL SPECIFIC ELECTRICAL PANEL LOAD CALC FOR 200 AMP SERVICE

ANCHORAGE REQUIREMENTS

FOUNDATION SECTIONS FOR PERIMETER ON FRAME:

PER SETUP MANUAL

ANCHORAGE REQUIREMENTS

FOUNDATION SECTIONS FOR PERIMETER OFF FRAME:

PER SETUP MANUAL

TRUSSES - DETAILS / CALCULATIONS

PER TRUSS PRINTS

ALL MODELS ARE AVAILABLE WITH FLOOR PLAN REVERSED FROM LEFT TO RIGHT AND / OR FRONT TO BACK.

MARRIAGE WALL COLUMNS SPAN CHART

DETAIL - SEE MATING WALL COLUMNS (PAGE MW-20.0)

CALCULATIONS - SEE CMW SECTION

INSTRUCTIONS ON FILLING OUT PLAN SET BEFORE CONSTRUCTION

YOU MUST CHECK THE APPROPREATE BOX OF WHAT THE STRUCTURE IS TO BE BUILT TO BEFORE PRODUCTION BEGINS. THE MARK SET MUST ACCOMPANY THE UNIT THROUGH THE PRODUCTION PROCESS.

EXTERIOR SIDEWALL HEADERS - SIZES AND MAXIMUM SPAN CHART

HEADER CHART - SEE EXTERIOR WALL PAGE EW - 20.0 CALCULATIONS - CEW SECTION

ATTENTION LOCAL INSPECTION DEPARTMENT:

IF THIS STRUCTURE IS IN A THERMAL ZONE MORE STRINGENT THAN THAT LISTED ON THESE PLANS, IS SET ON PILINGS, OR IS INSTALLED AT A MOUNTAIN REGION OR COASTAL HIGH HAZARD SITE SUCH THAT WIND OR OTHER DESIGN PARAMETERS ARE INCREASED. THE DESIGN MUST BE DETERMINED TO BE ADEQUATE FOR ACTUAL SITE CONDITIONS. ALTERATIONS MAY BE REQUIRED TO BRING THE HOME INTO COMPLIANCE WITH THE MORE STRINGENT CONDITIONS.

'Service entrance conductors routed from their point of entrance into the structure, to their point of attachment to the service enclosure a distance horiontally not more than twice the nominal width of the service enclosure and vertically not more than the greater of 5 feet or twice the nominal height of the service enclosure shall be considered to be in compliance with the requirements of 230-70(a) of the current National Electrical Code. Service entrance conductors may be routed in the most direct route or at right angles. Service entrance conductors in excess of these specified limits will not be allowed unless specifically authorized by special permission from the electrical inspector having jurisdiction to accommodate adverse site conditions which would not reasonably allow installation within this criteria."

This home is NOT designed for placement in Coastal High Hazard Areas or Ocean Hazard Areas.

СМН	REVISIONS	BY	DATE	ALL MODULAR MODELS
Manufacturing, Inc.				COVER SHEET 1-0

TYPICAL FASTENING SCHEDULE

FLOOR FASTENING REFERENCE 'CFL' - FLOOR CONSTRUCTION CALCULATIONS OF THE MANUAL

RIM JOIST TO JOIST PER FL-110 OR FL-510.0 IN APPROVED MANUAL

FLOOR BLOCKING TO JOIST PER FL-100.0 IN APPROVED MANUAL

MULTIPLE JOIST .131 x 3" NAILS @ 10" O.C., W/ GLUE 80%

DECKING TO FLOOR FRAMING PER FL-10 IN APPROVED MANUAL

EXTERIOR WALL FASTENING

FASTENING REFERENCE 'CEW' — EXTERIOR WALL CONSTRUCTION CALCULATIONS OF THE MANUAL

LOWER TOP PLATE &
BOTTOM PLATE TO STUD PER EW-25 IN APPROVED MANUAL DOUBLE STUDS 7/16" x 2-1/2" x 15 GA. STAPLES @ 6" O.C.

DOUBLE TOP PLATES PER EW-1 IN APPROVED MANUAL

HEADER TO STUDS PER EW-20 CHARTS IN APPROVED MANUAL HEADER COMPONENTS PER EW-20 IN APPROVED MANUAL

STUDS TO SILLS PER EW-20 IN APPROVED MANUAL
EXTERIOR SIDING PER THE MANUFACTURER'S SPECIFICATIONS

BOTTOM PLATE TO FLOOR PER EW-31 IN APPROVED MANUAL

SIDEWALL TO ENDWALL

PER EW-30 FOR NON-SHEARWALL OR PER SW-40 FOR SHEARWALL OR PER EW-0.0 IN APPROVED MANUAL

WALL WALL TO WALL TO WALL TOP PLATES

3" x 6" x .036" (20 GA.) GALVANIZED STEEL PLATE W/ (6) .131 x 3" NAILS AT EACH SIDE AT EACH

EXTERIOR WALL SHEATHING FOR APA RATED SHEATHING; 7/16" X 1-3/4" x 15 GA. STAPLES AT 6" O.C. AT ALL EDGES & 12" O.C.

FOR APA RATED SHEATHING; 7/16" X 1-3/4" x 15 GA. STAPLES AT 6" O.C. AT ALL EDGES & 12" O.C. FIELD. FOR COMPOSITE WALLS, FASTEN PER EW-40. FOR SHEARWALL FASTEN PER SW-40 OR ATTACHED PAGES (IF ATTACHED). ALL

OTHER SHEATHING FASTENED PER

MANUFACTURER'S INSTALLATION INSTRUCTIONS.

MATING WALL FASTENING REFERENCE 'CMW' - MARRIAGE WALL CALCULATIONS OF THE MANUAL

LOWER TOP PLATE TO STUD PER MW-40 IN APPROVED MANUAL BOTTOM PLATE TO STUD PER MW-40 IN APPROVED MANUAL

MULTIPLE STUDS 7/16" x 2-1/2" x 15 GA. STAPLES OR .131 x 3" NAILS @ 16" O.C. TO EACH MEMBER

STANDARD COLUMN PER MW-20 IN APPROVED MANUAL DOUBLE TOP PLATES PER MW-40 IN APPROVED MANUAL BOTTOM PLATE TO FLOOR PER MW-31 IN APPROVED MANUAL MATING WALL TO ENDWALL PER EW-30 IN APPROVED MANUAL

WALL TO WALL TOP PLATES 3" x 6" x .036" (20 GA.) GALVANIZED STEEL PLATE W/ (6) .131 x 3" NAILS AT EACH SIDE AT EACH WALL OR OVERLAPPED

PLATE PER EW-0.

INTERIOR WALL FASTENING

BOTTOM PLATE TO STUDS PER PT-40 IN APPROVED MANUAL TOP PLATE TO STUD PER PT-40 IN APPROVED MANUAL

DOUBLE STUDS 7/16" x 2-1/2" x 16 GA. STAPLES @ 16" O.C.

FLAT HEADER TO STUDS

PER PT-20 IN APPROVED MANUAL
WALL TO FLOOR

PER PT-40 IN APPROVED MANUAL
WALL TO WALL

PER PT-30 IN APPROVED MANUAL
TOP PLATE TO ROOF SYSTEM

PER PT-40 IN APPROVED MANUAL

GYPSUM TO WALL FRAMING PER THE RESIDENTIAL BUILDING CODE TABLES

REFERENCE 'CRC' - ROOF CONSTRUCTION CALCULATIONS OF THE MANUAL
CEILING BOARD TO TRUSS FOAM-SEAL 2100 SPRAY ADHESIVE PER THE MANUFACTURER'S SPECIFICATIONS

BLOCKING TO TRUSS (2) $7/16" \times 2-1/2" \times 15$ GA. STAPLES DIRECT

TRUSS TO SIDEWALL TOP PLATE PER RC-30 IN APPROVED MANUAL
TRUSS TO RIDGE BEAM PER RC-65 IN APPROVED MANUAL
TRUSS TO EDGE RAIL PER MW-31 CHARTS IN APPROVED MANUAL

EDGE RAIL TO MATING WALL

PER MW—31 CHARTS IN APPROVED MANUAL

TRUSS TO ENDWALL TOP PLATE PER SW-40 IN APPROVED MANUAL FOR SHEARWALLS AND RC-33.0 FOR NON-SHEARWALLS

ROOF DECKING TO TRUSS PER SW20.0 THRU SW-389E.2 (IF NOT ATTACHED) IN APPROVED MANUAL

SHINGLE TO ROOF DECKING PER THE MANUFACTURER'S OR ARMA SPECIFICATIONS

OUTLOOKER TO TRUSS PER RC-70 IN APPROVED MANUAL

INSTALLATION FASTENING REFERENCE INSTALLATION PAGES PROVIDED IN EACH APPROVAL.



- (CS1) 7/16" APA RATED ROOF DECKING 24/16 SPAN RATING.
- CS2 15# MIN. ROOF UNDERLAYMENT; SINGLE LAYER w/ GREATER THAN 4:12 ROOF PITCH; DOUBLE LAYER w/ 4:12 OR LESS
- CS3 MIN. 20 YEAR SHINGLES.
- CS4) 1 1/2" WIDE ENGINEERED WOOD BEAM, EACH HALF IN OPEN SPAN AREAS GREATER THAN 48".
- CS5 ENGINEERED WOOD TRUSSES: COMPONENTS & SPACING PER TRUSS PRINT
 - * FOR CONNECTION AND SET-UP OF ROOF: SEE MODULAR SET-UP PAGES ATTACHED TO APPROVAL
- CS6) CEILING INSULATION, BLOWN OR BATT.
- CS7) CONTINUOUS VENTED SOFFIT.
- CS8 DOUBLE 2x4 TOP PLATE (MIN.).
- CS9 2x4 STUDS @ 16" O.C. STUD GRADE SPF (MIN.).
- (CS10) WALL INSULATION (BATT)
- (CS1) 3/8" OSB SHEATHING WITH WATER RESISTIVE BARRIER BELOW ALL EXT. FINISH MATERIAL.

 CORROSION-RESISTANT FLASHING REQUIRED AT ALL LOCATIONS AS SHOWN ON APPROVED MANUAL DETAILS
- (CS12) SINGLE 2x4 BOTTOM PLATE SPF #3 (MIN.).
- CS13) 3/8" (MIN.) GYPSUM WALL BOARD.
- (CS14) FLOOR INSULATION (BATT.)
- (CS15) MIN. 19/32" RATED DECKING 16" O.C. OR 32/16 SPAN RATING.

CS16a Duct I

<u>Duct Insulation:</u> 1 — Min R-8

2 - A VAPOR RETARDER HAVING A MAXIMUM 0.05 PERM IN ACCPRDANCE WITH ASTM E96, OR ALUMINUM FOILI WITH A MINIMUM THICHNESS OF 2 MILLS, SHALL BE INSTRALLED ON THE EXTERIOR OF THE INSULATION ON THE COOLING SUPPLY DUCT THAT PASS THROUGH UNCONDITIONED SPACE CONDUCIVE TO CONDENSATION EXCEPT WHERE THE INSULATION IS SPRAY POLYURETHANCE FOAM WITH A MAXIMUM WATER VAPOR PERMEANCE OF 3 PERM PER INCH AT THE INSTALLED THICKNESS. CS4 (CS25) FLAT (CS22 ALTERNATE ₩ W/ FLAT IØ CÉILING CS19 (CS8) (CS11) (CS13) (CS9) (CS29) (CS12) CS23 (CS14)

CS16 MAIN HEAT DUCT. (MAY BE SITE INSTALLED BY OTHERS)

(CS25) WEDGE SUPPORT AT CATHEDRAL CEILING, EACH END OF TRUSS.

CS29 LAP BOARD, WOOD OR VINYL SIDING, HARDI SIDING, OR EXPOSED

SHEATHING FOR ON SITE EXTERIOR FINISH INSTALLATION.

CS19 2x4 (MIN.) MARRIAGE WALL STUDS @ 16" O.C.

(S20) LISTED BOTTOM BOARD, WHERE OCCURS.

(CS21) 1/2" SHIM FOR COMPRESSION STRIP.

(CS24) 1/2" (MIN.) GYPSUM BOARD CEILING.

CS27 CONTINUOUS 2x3 SPF #3 MINIMUM FOR

(CS28) 2x FULL DEPTH BLOCKING 24" O.C. (2)

TRUSS TOP RAIL FOR RIDGE CONNECTION

JOIST BAY MIN. ENDWALL LOCATION ONLY

CS22 DOUBLE 2x4 (MIN.) TOP PLATE.

CS23 2x4 (MIN.) BOTTOM PLATE.

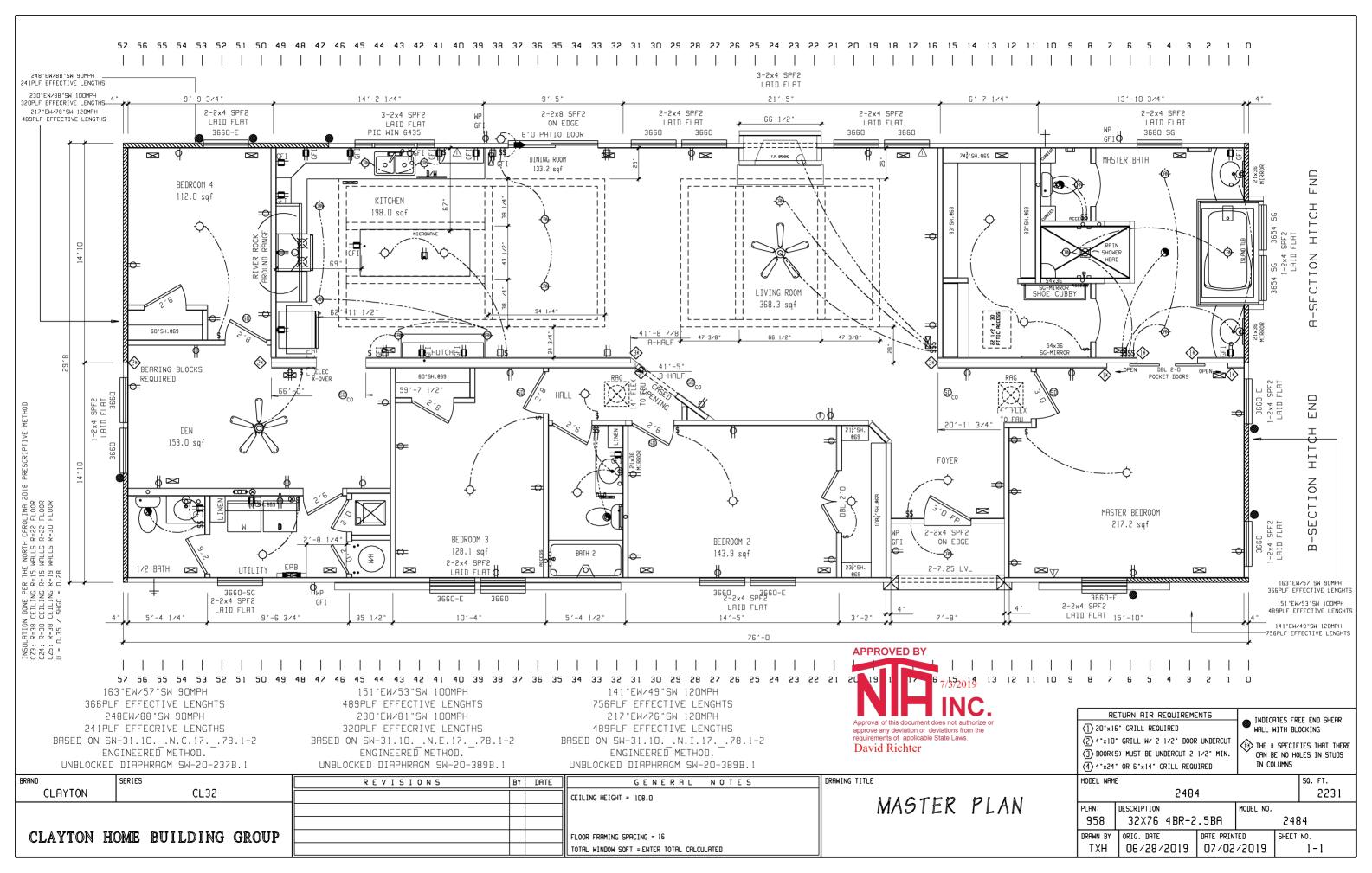
(CS17) OFF FRAME PER FL-110.0

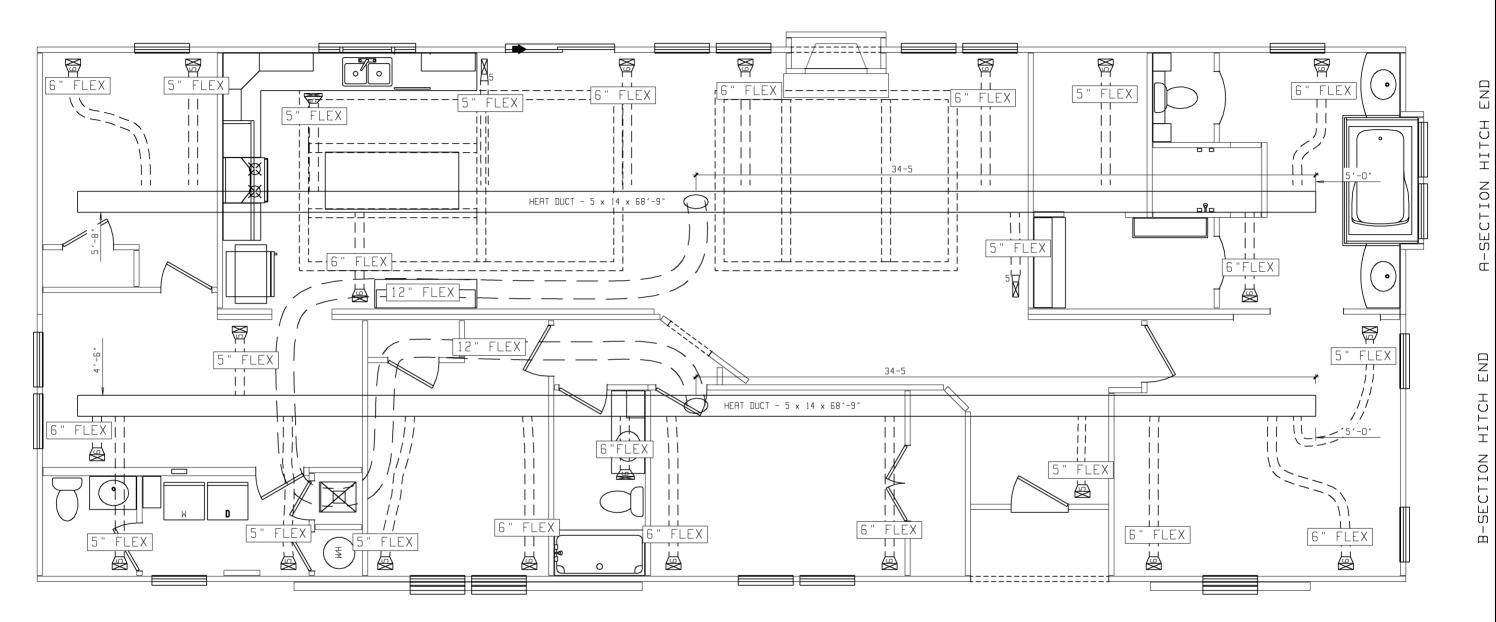
(CS18) OFF FRAME PER FL-110.0

MANUFACTURING,

(CS18)

TYPICAL CROSS SECTION & FASTENING SCHEDULE



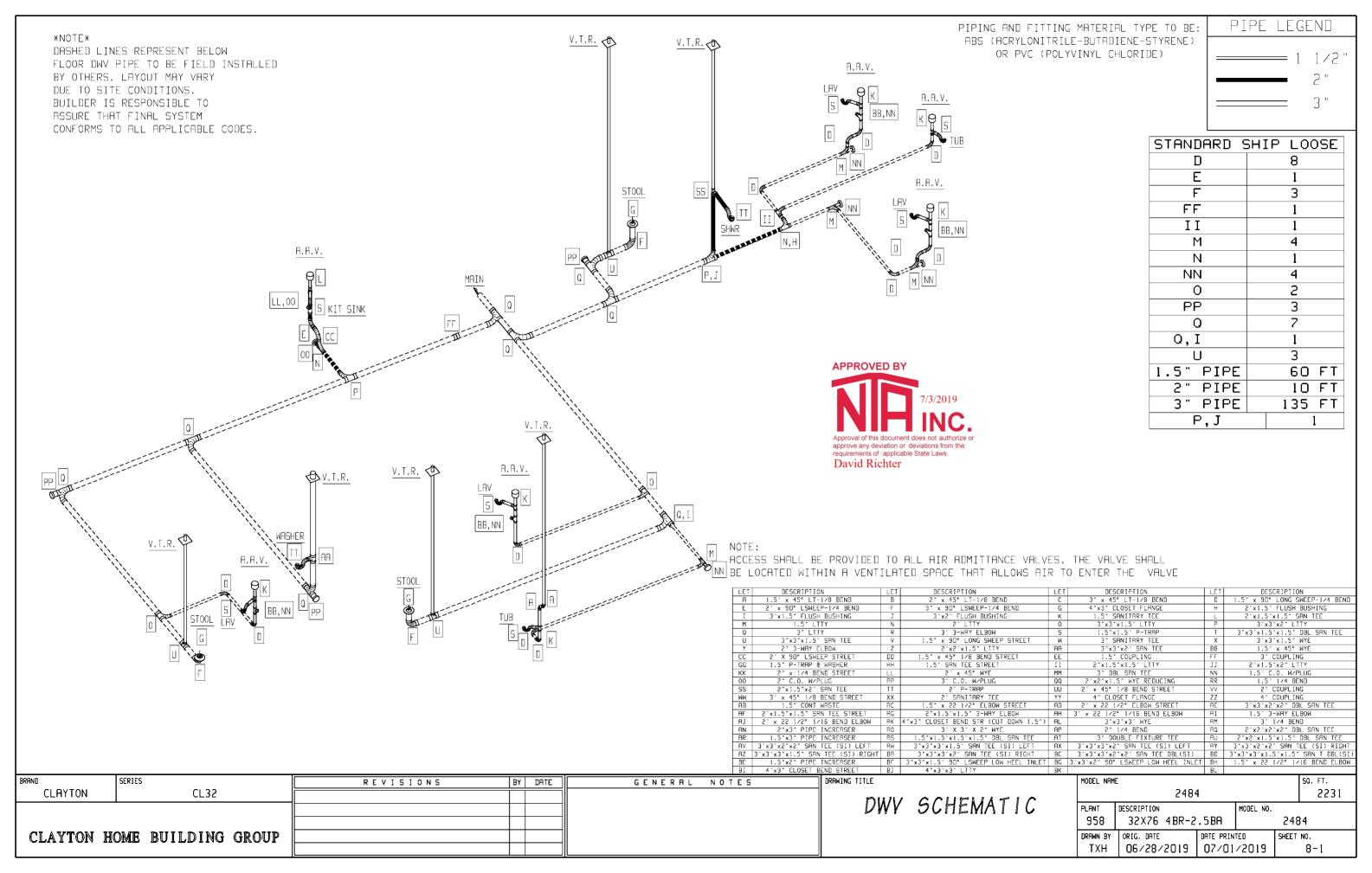


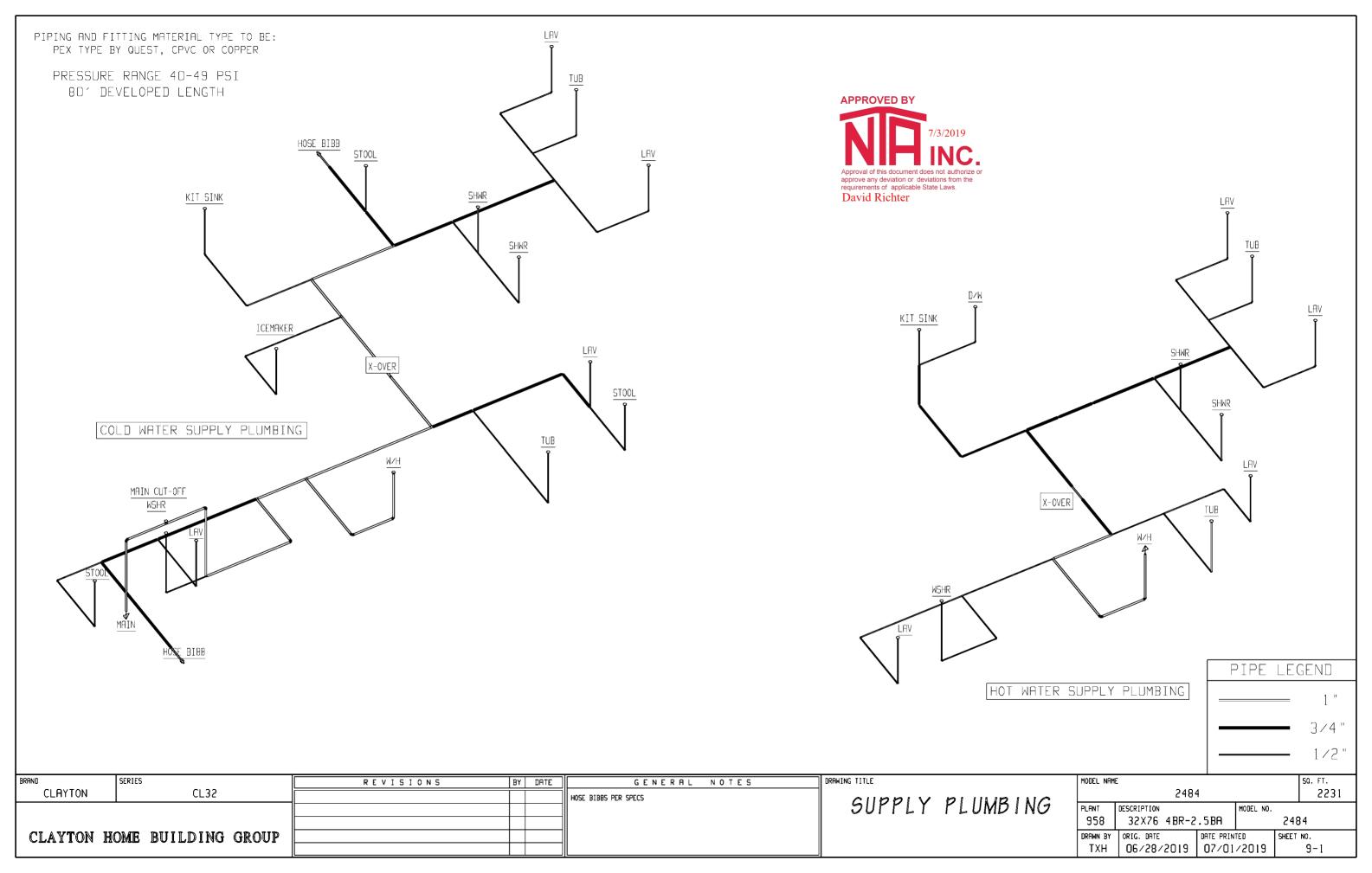
A-SECTION HITCH END

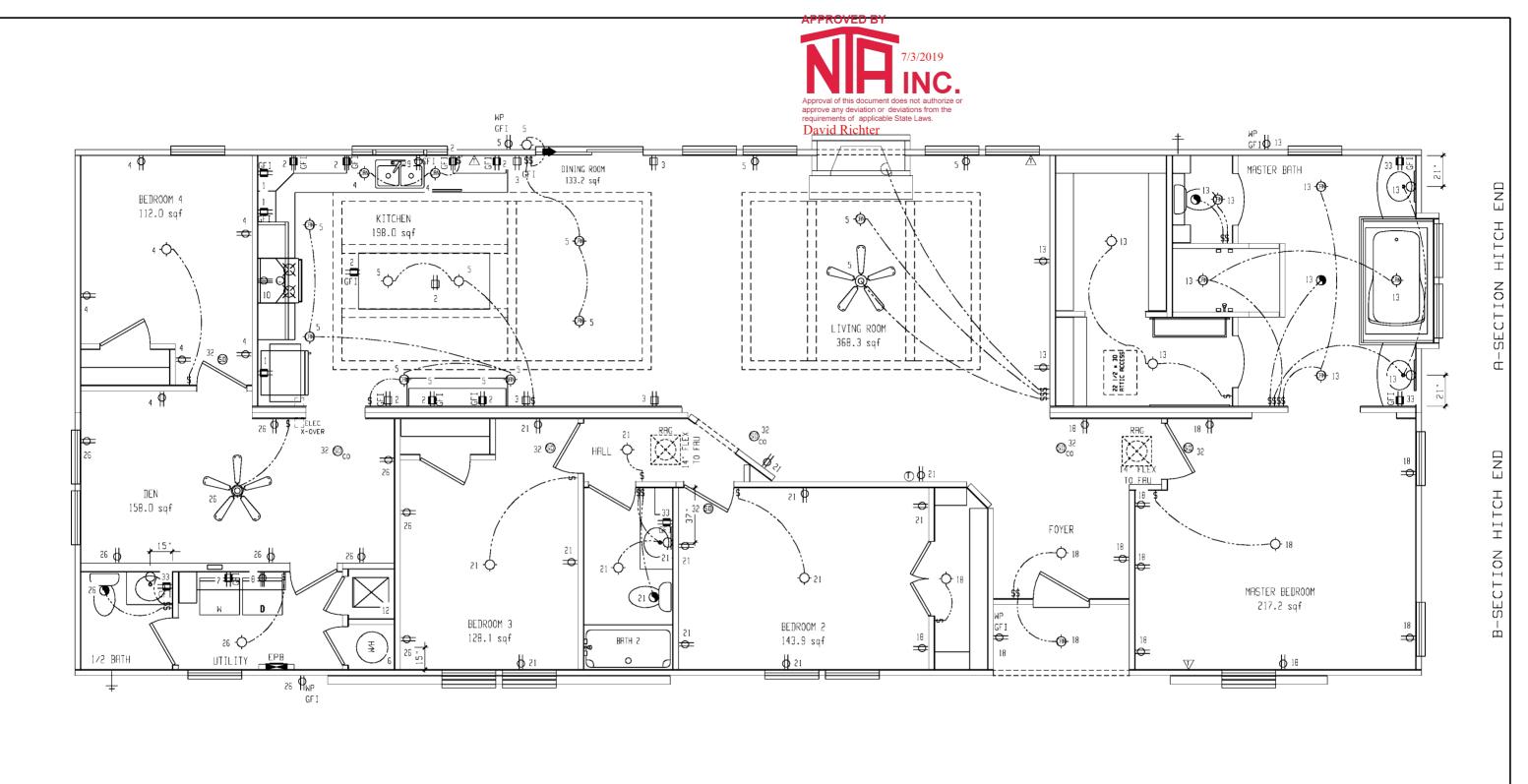
END



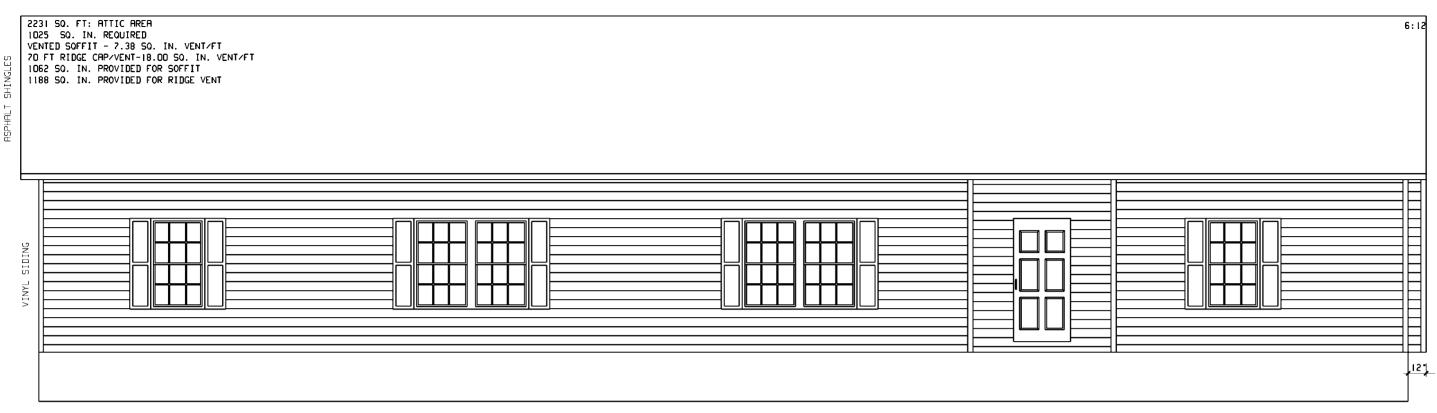
BRAND	SERIES	REVISIONS	BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAM	Ε		SO. FT.
CLAYTON	CL32				PEDIMETED HVAC_A		2484		2231
					PERIMETER HVAC-A		DESCRIPTION	MODEL NO.	
						958	32X76 4BR-2	.5BA	2484
CLAYTON H	IOME BUILDING GROUP					DRAWN BY	1 1		SHEET NO.
						TXH	06/28/2019	07/02/2019	4-1A







		ELECTRICAL SCHEDULE			
CIR. DESCRIPTION BRK. VOLTS COPR. NO. DESCRIPTION BRK. VOLTS HIRE	COPR. CIR. DESCRIPTION BRK. VOLTS COPR. NO. DESCRIPTION F	S. VOLTS COPR. NO. DESCRIPTION BRK. VOLTS COPR. CIR. DESCRIPTION BRK. VOLTS WIRE NO. DESCRIP	PTION BRK. VOLTS COPR. CIR. DESCRIPTION BRK. VOLTS COPR. (IR. DESCRIPTION BRK: VOLTS COPR. NO. DESCRIPTION	BRK: VOLTS COPR.
1 PORTABLE APPLIANCES 20 120 12 5 GEN. LIGHTING/RECEPT. 15 120	14 B DRYER RECEPT. 30 240 10 12 ELECTRIC FURNACE (RCUITS VARY, SEE 15 OPT. GARBAGE DISPOSAL 15 120 14 19 GEN. LIGHTI	NG/RECEPT. 15 120 14 24 ELECT. BUILT-IN OVEN 20 240 12/3	28 GEN. LIGHTING/RECEPT. 15 120 14 32 SHOKE PLRRAS	15 120 14
2 PORTABLE APPLIANCES 20 120 12 6 ELEC. WATER HEATER CIRCUITS VAN	RY, SEE 9 OPT. DISHWASHER 15 120 14 DAPIA PAGE PLN-3.5 FOR HUI	PLN-1.5 FOR MOD 16 FREEZER 20 120 12 20 GEN. LIGHTI	NG/RECEPT. 15 120 14 25 GEN. LIGHTING/RECEPT. 15 120 14	29 GEN. LIGHTING/RECEPT. 15 120 14 33 BATH GFI (MOD ONL)	() 20 120 12
3 PORTABLE APPLIANCES 20 120 12 DAPIA PAGE PLN-3.1 FOR HUD, PLN-1.1 F	FOR MOD 10 ELECT. RANGE/CKTOP 40 240 8 13 GEN. LIGHTING/RECEPT.	5 120 14 17 OPT. WHIRLPOOL 20 120 12 21 GEN. LIGHTI	NG/RECEPT. 15 120 14 26 GEN. LIGHTING/RECEPT. 15 120 14	30 GEN. LIGHTING/RECEPT. 15 120 14	
4 GEN. LIGHTING/RECEPT. 15 120 14 7 WASHER RECEPT. 20 120	12 11 GRS FURNACE 15 120 14 14 OPT. COOLER BOX	5 120 14 18 GEN. LIGHTING/RECEPT. 15 120 14 22 OPT. MIC	CROWRVE 20 120 12/2 27 GEN. LIGHTING/RECEPT. 15 120 14	31 SITE INSTALLED HEAT PUMP 40 240 8/3	
BRAND SERIES	REVISIONS BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAME	SQ. FT.
CLAYTON CL32		LOCK-OUT BREAKER ON CIRCUIT #6		2484	2231
·		-	ELECTRICAL PLAN	PLANT DESCRIPTION MODEL NO.	•
				958 32X76 4BR-2.5BA 3	2484
CLAYTON HOME BUILDING GROUP		- 1∥		DRAWN BY ORIG. DATE DATE PRINTED SH	EET NO.
		□		TXH	11-1



FRONT ELEVATION

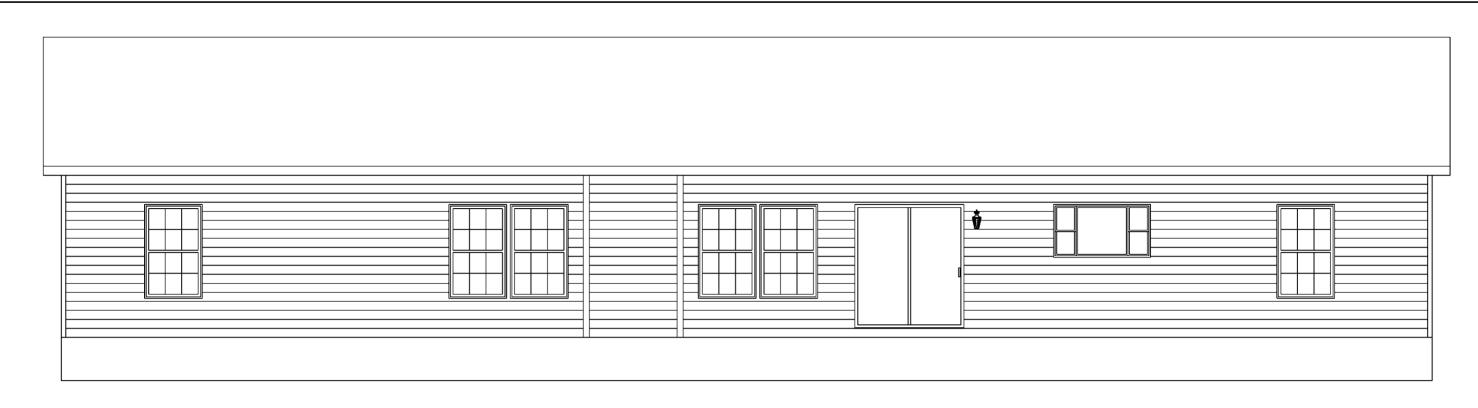


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David Richter

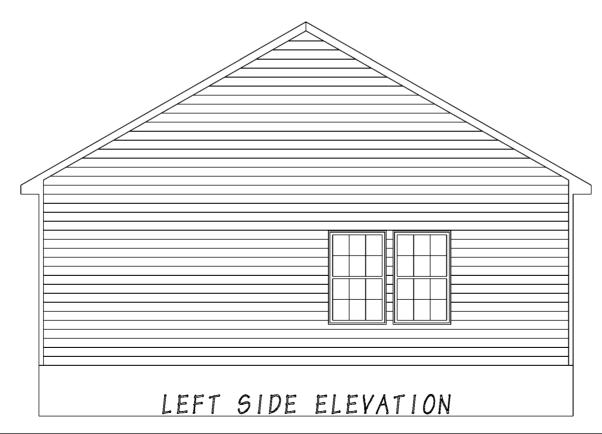
Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

L												
	BRAND	SERIES	R E V I S I O N S BY	DATE	GENERAL NOTES	DRAWING TITLE			MODEL NAME	E		SQ. FT.
	CLAYTON	CL32				PVTFBIA	18 11 11/1	71011		248	4	so. FT. 2231
, [EXTERIC	OR ELEVA	TION		DESCRIPTION	MODEL NO.	
						FRONT &	DICUT	GIDE	958	32X76 4BR-2	2.5BA	2484
	CLAYTON HO	OME BUILDING GROUP				PRUNI a	RIGHT	SIDE		I		SHEET NO.
									TXH	06/28/2019	07/01/2019	20-1

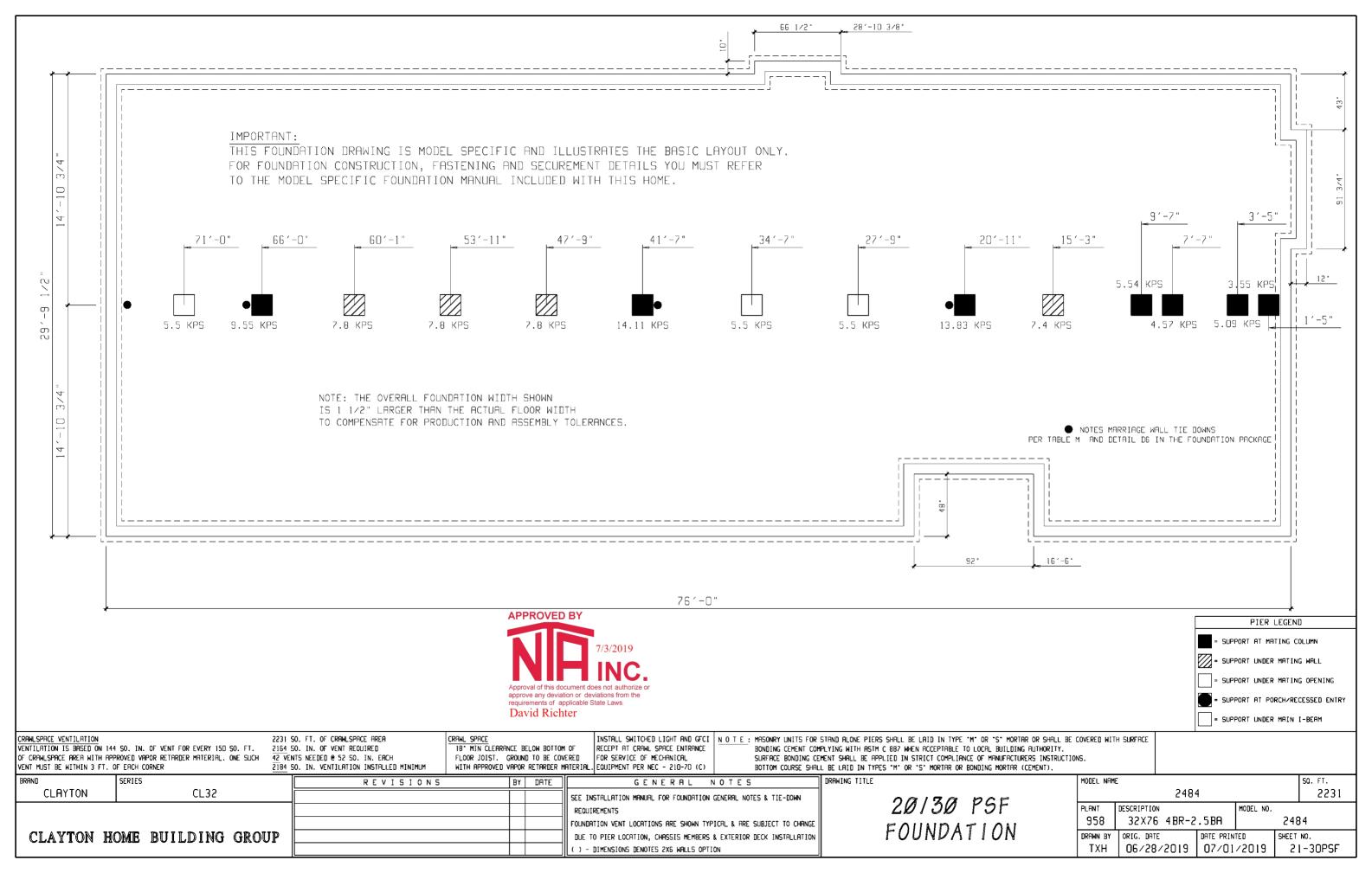


BACK ELEVATION





BRAND	SERIES	REVISIONS	BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAME		SO. FT.
CLAYTON	CL32		Ti		PUTEBIAB FIFULTIAL		2484	sq. FT. 2231
					EXTERIOR ELEVATION	PLANT DESCRIPTI		
					RACK & IEET GIDE	958 32X7	6 4BR-2.5BA	2484
CLAYTON H	NOME BUILDING GROUP				BACK & LEFT SIVE	DRAWN BY ORIG. DA		SHEET NO.
						TXH 06/2	8/2019 07/01/2019	20-2





OFF FRAME BASEMENT & CRAWL FOUNDATION DESIGN FOR:

29' - 8 " 2-SECTION MODULAR 1 STORY- W.O ATTIC

Attic without storage where the maximum clear height between joist and rafter is less than 42 inches or req'd insulation depth exceeds the depth of the bottom chord.

PERIMETER ANCHORED SYSTEM- BUILDING IS SECURED TO FOUNDATION WALLS TO SUPPORT WIND AND SEISMIC FORCES.

SIDEWALLS ARE SUPPORTED (PERIMETER BLOCKED)

BUILDING CODE INFORMATION:

IRC (2015) ASCE 7-10 2018 NORTH CAROLINA RESIDENTIAL CODE

BUILDING SITE INFORMATION:

*MAXIMUM ULTIMATE/DESIGN WIND SPEED & EXPOSURE: 130/ 100 MPH EXPOSURE C-enclosed

MINIMUM SOIL BEARING CAPACITY: 1500 PSF MAXIMUM GROUND SNOW(S): 20 PSF, 30 PSF

Flat roof snow load (Pg)=20.0 PSF ,23.1 PSF

SEISMIC DESIGN CATEGORY: C DESIGN SPECTRAL RESPONSE (S_{DS}): 0.49

SEISMIC SOIL SITE CLASS: D

HOME INFORMATION:

UNIT WIDTH: 29' - 8 "

MAX. UNIT LENGTH: 76 ft.

ROOF PITCH: 6/12 to 6/12

DESIGN LOADS: 40 PSF FL. LL., 7PSF T.C.D.L., 8PSF B.C.

D.L., 13PSF FL. DL. &, 10PSF B.C.L.L

David Richter

APPROVED BY

Approval of this document does not authorize or approve any deviation or deviations from the

requirements of applicable State Laws

MAX. SIDEWALL HEIGHT: 108 INCHES

TOTAL MATING WALL RIM JOIST BEAMS: (4) 2X10 #2 SPF

RIM JOIST SPLICES: 6" X 8" MiTek MT20 metal plates each side



OFF FRAME FLOOR PLANT NUMBER: 958

* Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd. All wind speeds are indicated as (Vasd) design speeds unless otherwise indicated.

This design is the property of CMH Manufacturing and cannot be used without authorization. This design is exclusively for use with new homes built by CMH Manufacturing. Use with homes built by other companies is strictly prohibited.

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Preface

This foundation design manual is dedicated to the ever-growing trend to place homes over basements and permanent foundations. CMH Manufacturing, Inc. has attempted to address the more common installation configurations. These may or may not be the only acceptable designs for basements or permanent foundations. If deviations are made from these details, it is the homeowner's and/or installation contractor's responsibility to obtain proper documentation and engineer's details of construction acceptable to the local authority having jurisdictions. CMH Manufacturing, Inc. will not supply any details other than what is contained in the following design manual. If an alternate design is requested it must be provided by an independent engineer subject to local approval. The owner/contractor is responsible for any additional construction details, permits, inspections and fees associated with these items.

Setting a home over a basement or permanent foundation requires special knowledge, experience and equipment to accomplish a safe and proper set. Contractors performing this type of installation must be licensed, bonded and insured to protect all aspects of this type of work.



Instructions

- 1. Determine site soil classification, (see table R405.1).
- 2. The provided foundation and anchorage designs are not applicable for the following conditions. In all these cases a complete geotechnical evaluation must be performed and foundation must be designed by a professional engineer in accordance with section 1805.8 (IBC) for site specific conditions.
- Site contains OL, OH or Pt class soils.
- Site contains compressible or shifting soils.
- Site contains expansive soils per IRC (R403.1.8.1) or per local authority and adopted code.
- Site contains soils which do not provide the minimum allowable soil bearing strength as specified per the provided designs.
- Foundation walls support unbalanced loads on opposite sides of building, such as a daylight basement or walk out basement where the building aspect ratio, L/W, exceeds the values specified in Table L.
- Site with soils subject to liquifaction or soil containing high concerntration of sulfate.
- 3. Determine foundation wall height for each wall of foundation. Reference **Detail D1 or D2** for wall height.
- 4. Determine height of backfill for each wall of foundation. Reference *Table L* when backfill heights along the foundation wall are unbalanced. Reference *Detail D1 or D2* for perimeter foundation wall construction.
- 5. Determine what type of mateline supports will be used. Reference **Detail D3, D4, D5 or D7** for mateline columns and **Detail D14** for cross beams.
- 6. Determine if type H connector plates will be used around the perimeter of the building. Fastening and anchoring tables have been provided with and without the use of the H connectors.
- 7. Find the Floor to Sill Plate & Sill Plate to Foundation table for site soil classification.
- 8. Find site wall height and backfill height line and follow this line across. Heights are listed as maximums, therefore any line beneath (greater height) may be utilized for items 10,11 & 12 below.
- 9. If type H connectors will be installed the table labeled *With Type H Plate Connectors* can be utilized. Note (6) will specify spacing for H plates along sidewalls and Note (7) will specify spacing for H plates along each endwall.
- 10. Select desired rim to sill connection from line in table (E, F or G for sidewalls and E or G for endwalls).
- 11. Select desired anchor type (4 or 5) for sill to foundation wall connection and determine anchor spacing for sidewall and endwall under corresponding column.
- 12. Determine if shearwall foundation holddowns are required by checking far right column within selected row. See *Shearwall Foundation Holddown Detail (Detail D18)* for connection requirements

The above process may be repeated as desired for different foundation wall and backfill combinations.



General Notes

- 1. Foundation plans and details developed by CMH Manufacturing, Inc. are provided to our company owned sales centers and wholesale distribution partners. Alternate foundation systems may be used in lieu of these plans provided they are designed by a local professional Engineer or Architect familiar with the local soil and climate conditions, and are approved by the local authority having jurisdiction.
- 2. All notes stating "in field" or "by owner" are obligations pertaining to owner/contractor.
- 3. Owner /Contractor shall provide complete foundation, including footing drains, vapor barrier, sill plate, anchor bolts, stair area, slab and footing reinforcement along with damp proofing, waterproofing, backfill, and all finish work per Chapter 4 of IRC or per adopted local building code.
- 4. Owner/Contractor shall be responsible for performing all work in accordance with previously approved construction details and obtaining all necessary inspections as required by local or state authorities.
- 5. Not designed for areas likely to have collapsible, expansive, compressible, shifting, liquifaction, soil containing high concentration of sulfate or other unknown soil characteristics. In these conditions a local engineer must provide foundation design and the building official shall determine whether to require a soil test to determine the soil characteristics. This soil test shall be made by an approved testing agency using an approved method.
- 6. Pier spacing is dimensioned to centerline unless otherwise noted.
- 7. The foundation dimensions shown are nominal. An increase in module width should be expected due to module expansion, setting tolerances, etc. The foundation contractor should consult with the manufacturer of the modules prior to construction of the foundation to determine the actual width of the home and placement of anchors.
- 8. All steel support columns shall have protective coating and a load capacity equal to or greater than specified on foundation plan (k=1000 pounds).
- 9. All foundation construction materials and installation shall be in accordance with all state and local codes.
- 10. Backfill shall not be placed against the wall until the wall has sufficient strength and has been anchored to the floor above or has been sufficiently braced to prevent damage by the backfill. Heavy-equipment must be restricted to a minimum distance to the foundation at least equal to the depth of the foundation.
- 11. Solid cap block or cement fill required at top courses of all masonry piers or pilasters.
- 12. The foundation design has been designed to be placed in the seismic zone indicated on the cover of this document. Please note that all CMH structures have been designed for seismic (zone/category) A, B, or C only, unless otherwise noted on floor plan and cover page of these instructions.
- 13. All piers shall be constructed of 8"x8"x16" concrete masonry units conforming to ASTM C90 with a minimum compressive strength of 700 psi. Masonry foundation walls must be laid in type m or s mortar. When required per tables or details, piers of masonry units shall be laid in type m or s mortar. All dry stack masonry should be surfaced bonded with an approved adhesive product.



- 14. All reinforcing steel shall be Grade 60 minimum. All splices shall be lapped 24" minimum and splices shall be offset 30" minimum within same footer.
- 15. All concrete grout shall be 3000 psi at 28 days.
- 16. Reference the model plan drawing for specific foundation layout.
- 17. Concrete footings shall have a minimum compressive strength of 3000 psi at 28 days. Concrete foundation walls and other concrete exposed to weather shall have a minimum compressive strength of 3000 psi at 28 days and in moderate and severe weather areas the concrete shall be air entrained no less than 5 percent and not more than 7 percent. See table R301.2(1) and R402.2 of IRC
- 18. All exterior footings shall be placed at least 12" below the undisturbed ground surface. All exterior footings shall extend below the frost line or otherwise frost protected in accordance with Sections R403.1.4.1 through R403.1.4.2 of IRC or per adopted local building code.
- 19. Top of foundation walls shall extend a minimum of 6-1/2" above finished adjacent grade. Wood framing members, including wood sheathing, that rest on exterior foundation walls and are less than 8" from exposed earth shall be of naturally durable or preservative-treated wood. Wood floor joist shall not be closer than 18" from exposed ground in under floor space.
- 20. Contractor shall verify all site conditions and dimensions prior to starting foundation. Notify home manufacturer of any discrepancies immediately.
- 21. The foundation must be designed and built to local codes and ordinances and must be approved and inspected by local building officials.
- 22. Access shall be to all under floor spaces. Access shall be a minimum of 18" by 24". If mechanical equipment is installed is this area, please refer to the Mechanical Code for minimum access opening. Through wall access openings shall not be located under an exterior door.
- 23. Under floor space shall be ventilated with a net area ratio not less than 1 square foot for each 150 square feet of under floor space area placed in accordance with local codes. Ratio may be reduced to 1/1,500 where ground is covered with a 6-mil polyethylene or approved vapor retarderl.
- 24. Field installed wiring in basement is subject to local inspection. Basement smoke alarms must be installed at foot of stairs and interconnected with home smoke alarms and tested on site. Smoke alarms must be located, installed, and tested in conformance with local building requirements.
- 25. Large clear spans along mating wall require a column or pier at each end. See model specific foundation plan for required capacity and additional column requirements.
- 26. Basement stairs (widths, handrails, clearances, headroom, landings, fire protection, etc.) are the responsibility of the owner/contractor and must be constructed to comply with local building codes.
- 27. Owner/contractor shall not alter basement stair opening without written approval from CMH Manufacturing, Inc.



- 28. Lighting and receptacles in basement are the responsibility of owner/contractor.
- 29. Termite protection shall be provided per the building code and local requirements and are responsibility of owner/contractor.
- 30. Ground snow load is indicated on foundation plans. Snow load must be verified per locality. Building has not been designed to be located within a Tsunami design zone.
- 31. This structure has not been designed to be located within flood hazard locations or in Coastal A Zones. When site is located in a flood hazard area or in Coastal A Zones as determined by the local authority having jurisdiction or flood hazard maps. The unit shall have lowest floor elevated above the design floor elevation. Foundation and anchorage designs shall be provided by a local engineer in conformance with locally adopted building code and ASCE-24-14.
- 32. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be minimum of ASTM A653 Type G185 zinc coated galvanized or stainless when in contact with pressure treated sill plates or other pressure treated lumber.
- 33. Radon control, when required by a local jurisdiction, shall be provided and installed by others in accordance with appendix F of the IRC.
- 34. Topographic wind effects have not been considered. Home has not been designed to be located in areas designated as having local historical data documenting structural damage to buildings caused by wind speed-up at isolated hills, ridges and escarpments.
- 35. Surface drainage shall be devirted to a storm sewer or other approved collection point. Lots shall be graded to drain surface water away from foundation walls. The grade shall fall a minimum of 6 inches within the first 10 feet.
- 36 A 6-mil-thick polyethylene moisture barrier shall be applied over the porous to be a polyethylene.
- 37. Concrete and Masonry Foundation walls that retain earth and enclose the jicr spaces and floors below grade shall be damp proofed from the top of the footing to the finished grade. It is nry walls that have not less than 3/8" Portland cement parging applied to the exterior of the wall. The parging shall be damp proofed in accordance with one of the following.

 The parging shall be state Laws.

 David Richter
 - a. Bituminous coating, b. 3 pound per sq. yard of arcylic modified cement, c. 1/8" coat of surface-bonding cement complying with ASTM C887, d. Material permitted for waterproofing per Section R406.2, e. Other approved methods or materials.
- 38. Concrete and masonry foundation walls that retain earth and enclose interior spaces and floors below grade in areas of high water table or other severe soil-water conditions shall be waterproofed from the top of the footing to the finished grade in accordance with one of the following:
 - a. 2-ply hot-mopped felts, b. 55 pound rolled roofing, c. 6-mil polyvinyl chloride, 6-mil polyethylene,
 - d. 40-mil polymer-modified asphalt., e, 60-mil flexible polymer cement, f. 1/8" cement-based, fiber-reinforced, waterproof coating, g. 60-mil solvent-free liquid-applied synthetic rubber.
- 39. If building is located within a wind borne debris region glazed openings shall be protected from wind borne debris. Wind Borne debris protection is the responsibility of others.
- 40. When Geotechnical report is required or available, all recommendations shall be followed and geotechnical engineer shall review all foundation plans to verify applicability with recommendations and engineer shall be present on regular basis during site preparation, fill placement and foundation excavation.
- 41. Self-closing rated doors shall be installed between garage and house (on-site by other).(R302.5.1) 42.Reserved.
- 43. A 6-mil polyethylene or approved vapor retarder with joints lapped not less than 12 inches shall be placed between the concrete floor slab and the base course or the prepared subgrade.

SOIL CLASSIFICATION

		TABLE R405.1 W/N	NC admendments				
LATERAL SOIL LOAD	UNIFIED SOIL CLASSIFICATION SYSTEM SYMBOL	SOIL DESCRIPTION	DRAINAGE CHARACTERISTICS ^a	FROST HEAVE POTENTIAL	VOL. CHANGE POTENTIAL EXPANSION ^b	ALLOWABLE SOIL PRESSURE	
	GW	Well-graded gravels, gravel sand mixtures, little or no fines	Good	Low	Low	5000	
30 psf	GP	Poorly graded gravel or gravels sand mixtures, little or no fines	Good	Low	Low	5000	
LATERAL SOIL LOAD	SW	Well-graded gravels, gravelly sands, little or no fines	Good	Low	Low	3000	
	SP	Poorly graded sand, or gravelly sands, little or no fines	Good	Low	Low	3000	
45 maf	GM	Silty gravels, gravel-sand-silt mixtures	Good	Medium	Low	3000	
45 psf LATERAL	SM	Silty sand, sand-silt mixtures	Good	Medium	Low	3000	
SOIL LOAD	GC	Clayey gravels, gravel-sand-clay mixtures	Medium	Medium	Low	3000	
	SC	Clayey sands, sand-clay mixture	Medium	Medium	Low	3000	
	ML	Inorganic silts and very find sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity	Medium	High	Low	2000*	
60 psf LATERAL SOIL LOAD	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	Medium	Medium	Medium to Low	2000*	
	СН	Inorganic clays of high plasticity, fat clays	Poor	Medium	High	2000*	
	МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	Poor	High	High	2000*	
SPECIAL	OL	Organic silts and organic silty clays of low plasticity	Poor	Medium	Medium	SPECIAL	
INSPECTION REQUIRED	OL	Organic clays of medium to high plasticity, organic silts	Unsatisfactory Medium		High	INSPECTION REQUIRED	
	Pt	Peat and other highly organic soils	Unsatisfactory	Medium	High		

a. The percolation rate for good drainage is over 4 inches per hour, medium drainage is 2 inches to 4 inches per hour, and poor is less than 2 inches per hour.

^{*} Where the building offical determines that in place soils with an allowable bearing capacity of less than 2000 psf are likely to be present at the site, the allowable bearing capacity shall be determined by a soils investigation.



b. Soils with low potential expansion typically have a plasticity index (PI) of 0 to 15, soils with a medium potential expansion have a PI of 10 to 35 and soils with a high potential expansion have PI greater than 20.

TABLE R404.1.1:IRC (2015) PERIMETER FOUNDATION WALL MINIMUM REQUIREMENTS [Seismic Seismic Zone: Design]

		GW, GP, SV	N, & SP Soil Class	(30 PSF)	GM, GC, SM-SC, & ML Soil Class (45 PSF)			SC, MH, ML-CL, & Inorganic CL Soil Class (60 PSF)		
Max.	Maximum	Plain	8" Reinforced	8" Poured	Plain	8" Reinforced	8" Poured	Plain	8" Reinforced	8" Poured
Wall	Unbalanced	Masonry 1	Masonry	Concrete	Masonry 1	Masonry	Concrete	Masonry 1	Masonry	Concrete
Height	Fill*	Walls	Walls ^{5,9}	Walls ^{6, 7}	Walls	Walls ^{5,9}	Walls ^{6, 7}	Walls	Walls ^{5,9}	Walls ^{6, 7}
0 to 5	4	6 in. solid (3) or 8 in.	-	PC	6 in. solid (3) or 8 in.	-	PC	6 in. solid (3) or 8 in.	-	PC
feet	5	6 in. solid (3) or 8 in.	-	PC	8 in.	-	PC	10 in.	-	PC
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC
6 feet	5	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC
to 7 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC	10 in. solid (3)	#5 @ 48 in. o.c.	#5 @ 48 in. o.c.
	7	12 in.	#5 @ 48 in. o.c.	PC	10 in. solid (3)	#6 @ 48 in. o.c.	#5 @ 46 in. o.c.	12 in. solid (3)	'#6 @ 40 in. o.c.	#6 @ 48 in. o.c.
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC
	5	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#4 @ 48 in. o.c.	PC
8 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#5 @ 48 in. o.c.	#6@32in o.c.
	7	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#5 @ 41 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.
	8	10 in. solid (3)	#5 @ 48 in. o.c.	#6@41	12 in. solid (3)	#6 @ 48 in. o.c.	#6 @ 43 in. o.c.	Footnote (4)	'#6 @ 32 in. o.c.	#6@18 in. o.c.
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8 in.	#4 @ 48 in. o.c.	PC
	5	8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC
9 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#4 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#6@35 in. o.c.
9 1661	7	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#6@35 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.
	8	12 in. solid (3)	#6 @ 48 in. o.c.	#6@36 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.	Footnote (4)	#6 @ 24 in. o.c.	#6@28 in. o.c.
	9	Footnote (4)	'#6 @ 40 in. o.c.	#6@35 in. o.c.	Footnote (4)	#6 @ 24 in. o.c.	#6@25 in. o.c.	Footnote (4)	#6 @ 16 in. o.c.	#6@24 in. o.c.
	8	NA	#6 @ 48 in. o.c.	#6 @ 35 in. o.c.	NA	#6 @ 32 in. o.c.	#6 @ 29 in. o.c.	NA	#6 @ 24 in. o.c.	#6 @ 21 in. o.c.
10 feet	9	NA	#6 @ 40 in. o.c.	#6@34 in. o.c.	NA	#6 @ 24in. o.c.	#6@22 in. o.c.	NA	#6 @ 16 in. o.c.	#6@16 in. o.c.
	10	NA	#6 @ 32 in. o.c.	#6 @ 27 in. o.c.	NA	#6 @ 16 in. o.c.	#6 @ 17 in. o.c.	NA	#6 @ 16 in. o.c.	#6 @ 13 in. o.c.

^{*}Unbalanced backfill height is the difference in height between the exterior finish grade level and the top of the basement slab or crawl space grade. Backfill shall be placed only AFTER the home has been anchored to the foundation wall.

- (1) All block must conform to ASTM C90 (700 psi rated) and be laid in a running bond of Type M or S mortar with overlapping pattern . Ungrouted hollow masonry units are permitted except where otherwise indicated.
- (3) Solid grouted hollow units or solid masonry units.
- (4) Wall construction per reinforced units or design required.
- (5) Vertical reinforcement shall be Grade 60 minimum. The distance from the face of the soil side of the wall to the center of vertical reinforcement shall be at least 5".
- (6) PC = Plain Concrete (Concrete with less reinforement than minimum for reinforced concrete)
- (7) All reinforcement shall be Grade 60 minimum. The distance from the face of the soil side of the wall to the vertical reinforcement shall be at least 6 1/16", but not more than 6 11/16".
- 'All information above has been extracted from the 2009 IRC Tables R404.1.1(1), Tables R404.1.1(2) Tables R404.1.2(3)
- (8) Reserved



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Maximum Aspect Ratio, L/W for Unbalanced Foundations

			SOIL CLASS	
Maximum Wall Height	Maximum Unbalanced Fill	GW, GP, SW, & SP (30 PSF)	GM, GC, SM-SC, & ML (45 PSF)	SC, MH, ML-CL, & Inorganic CL (60 PSF)
Height	Olibaianceu Fili		, ,	
7 feet	5 5	4.0 4.0	4.0 3.4	4.0 2.6
7 1001	6	3.0	2.0	1.5
	7	1.9	1.2	0.9
	4	4.0	4.0	4.0
	5	4.0	3.9	2.9
8 feet	6	3.4	2.3	1.7
	7	2.1	1.4	1.1
	8	1.4	1.0	0.7
	4	4.0	4.0	4.0
	5	4.0	4.0	3.3
9 feet	6	3.8	2.6	1.9
	7	2.4	1.6	1.2
	8	1.6	1.1	0.8
	9	1.1	0.8	0.6

Instructions:

Where foundation wall support unbalnced load on opposite sides of building such as daylight basement, the building aspect ratio, L/W, shall not exceed the value specified in Table above.R404.1(3)

- 1 Determine foundation wall height, unbalanced fill depth, and soil class to determine aspect ratio from table above.
- 2 Multiple "W" times aspect ratio.
- 3 Result is equal to the maximum allowable building length on the exposed side.

Example 1 - check sidewall for 26'-8" x 60'-0" home.

Basement Wall Height = 8'-0"

Unbalanced backfill = 7'-0"

Soil Class = SP

Aspect Ratio from Table above = 2.1

26.67 x 2.1 = 56'-0" max. allowable length - **example fails** Try again using 6'-0" max. unbalanced fill with an aspect ratio of 3.4. 26.67 x 3.4 = 90'-8" max. allowable length - **example passes Max. allowable backfill is 6'-0**"

Example 2 - check endwall for 26'-8" x 60'-0" home.

Basement Wall Height = 8'-0"

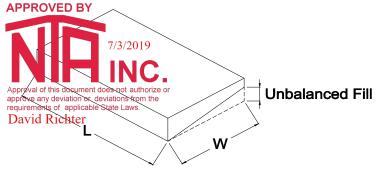
Unbalanced backfill = 7'-0"

Soil Class = SP

Aspect Ratio from Table above = 2.1

60 x 2.1 = 126'-0" max. allowable length - example passes

"L" = total overall dimension of the building on the exposed side "W" = the total overall dimension of the building on the side adjacent to the exposed side



Required Rim Joist to Sill Plate Fastening at wall "L".

Use a 20 Gauge metal angle clip at 24" o.c. with (5) 8d nails per leg or an approved connector supplying 230 pounds per linear foot capacity.

*Page extracted from 2006 IRC section R404.1.5 & Table R404.1(3)

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UNBALANCED FOUNDATIONS (TABLE L)

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TABLE M - MINIMUM CONCRETE BLOCK PIER AND FOOTER SIZE

AT MATING WALL COLUMNS (REF. DETAILS D4 OR D5)								
GRO	UND SNOW	20	30				Ties	
ပ္ပ	4 '	32" Dia. X 12"	(S) 28"x28"X10" OR 32" Dia. X 12"				0	
PORT	6'	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				0	
SUPF	8 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
Ν N	10 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
COLI	12 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
/ALL	14 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
≶ 9 7	16 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
MAXIMUM MATING LINE SPAN BETWEEN MATING WALL COLUMN SUPPORTS	18 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
Z Z	20 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
=TWE	22 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
N BE	24 '	46" Dia. X 19"	(D) 40"x40"X12" OR 46" Dia. X 19"				1	
SP/	26 '	46" Dia. X 19"	(T) 48"x48"X16" OR 56" Dia. X 24" (T) 48"x48"X16" OR				1	
Ë	28 '	46" Dia. X 19"	56" Dia. X 24" (T) 48"x48"X16" OR				1	
TING	30 '	46" Dia. X 19"	56" Dia. X 24" (T) 48"x48"X16" OR				1	
M M	32 '	46" Dia. X 19"	56" Dia. X 24" (T) 48"x48"X16" OR				1	
IMU	34 '	56" Dia. X 24"	56" Dia. X 24" (T) 48"x48"X16" OR				1	
MAX	36 '	56" Dia. X 24"	56" Dia. X 24" (T) 48"x48"X16" OR				1	
	46 '	56" Dia. X 24"	56" Dia. X 24" DER MATING OPE	NING AS CI E	ADSDANS IN E	FET	1	
DIED	CDACING			INING AS CLE	ANOFANO IN F		 	
	SPACING	7.9 ' (S) 28"x28"X10" OR	7.9 ' (S) 28"x28"X10" OR					
PIER	CONFIG.	28" Dia.	28" Dia.		00 4110 111 555		Girder beams constru be (4) 2X10 #2 SPF jo	
PIER	SPACING	SUPPORTS (JNDER MATING W 6.7 '	ALLS- CLEAR	SPANS IN FEE	<u> </u>	Splices 6" X 8" MiTek metal plates each sid	k MT20
	R CONFIG.		(D) 40"x40"X12" OR 33" Dia.				motal places sash sid	-

Chart Key:

(Pier Configuration) Min. footer width (inches) x Min. footer length (inches) x Min. footer depth (inches)

- (S)= Single stack block configuration.
- (D)= Double stack block configuration.
- (T)= Triple stack block configuration.
- (DR)=Double stack reinforced & fully grouted configuration.
- IE. For 20 psf 178" box with 14' opening: Double stack pier on a 40"x 40" sq. footer 12" deep footing.

30' 1 STORY- W.O ATTIC OFF FRAME BASEMENT & CRAWL With Roof Pitch of 6/12 Min. to 6/12 Max. NOTES: 1 DESIGNED FOR 100 MPH MAX. WIND SPEED.

2 DESIGNED FOR 1500 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.

3 DESIGN TO * Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd. All wind speeds are indicated as (Vasd) design speed 4 MAX. MATING WALL OPENINGS LISTED IN CHART ASSUME OPENING IN BOTH HALVES. IF ANCHOR IS TIED TO ONLY ONE COLUMN (ONE HALF) THEN HALF THE OPENING SIZE CAN BE USED WHEN LOOKING UP VALUE IN TABLE ABOVE. PIER SUPPORTS REQUIRED AT EACH SIDE OF DOOR OPENINGS AND ALL EXTERIOR WALL OPENINGS GREATER THAN 4'.

5 WHEN PIER CONFIGURATION IS NOT GIVEN IN CHART THE ACTUAL LOADS EXCEED ALL PREDESIGNED PIERS AND A LOCAL ENGINEER MUST DESIGN THE SUPPORTS FOR THE GIVEN LOADS (- UPLIFT/ + GRAVITY LOADS).

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Approval of this document does not authorize or

approve any deviation or deviations from the requirements of applicable State Laws.

APPROVED BY

David Richter

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6 ALL PIERS SHALL BE EMBEDDED IN TYPE M OR S MORTAR.

7.Round footers or Round Piles with diameter as required above may be used as alternate to square footing or square footing and block piers.

TABLE N - STRUCTURAL STEEL POST AND FOOTER SIZE AT MATING WALL COLUMNS (REF DETAIL D7)

		IV	IATING WALL	COLUMNS (R	EF. DETAIL D	7)	Uplift
GROU	IND SNOW	20	30				force
S	4 '	(9k) 30"x30"X11"	(9k) 30"x30"X11"				0 #
MAXIMUM MATING LINE SPAN BETWEEN MATING WALL COLUMN SUPPORTS	6 '	(9k) 30"x30"X11"	(9k) 30"x30"X11"				0 #
	8 '	(9k) 30"x30"X11"	(14k) 38"x38"X13"				9.95146 #
WN WN	10 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"				133.924 #
COLU	12 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"				257.897 #
ALL C	14 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"				381.87 #
G W	16 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"				505.843 #
ATIN	18 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"				629.816 #
Z Z	20 '	(14k) 38"x38"X13"	(14k) 38"x38"X13"	APPRO	ED BY		753.789 #
TWE	22 '	(14k) 38"x38"X13"	(20k) 44"x44"X14"		7/2/2010		877.762 #
. BE.	24 '	(14k) 38"x38"X13"	(20k) 44"x44"X14"		INIC		1001.73 #
SPA	26 '	(14k) 38"x38"X13"	(20k) 44"x44"X14"	Approval of the	is document does not authori	ze or	1125.71 #
IN I	28 '	(14k) 38"x38"X13"	(20k) 44"x44"X14"	requirements David R	of applicable State Laws. ichter		1249.68 #
I 9NI	30 '	(20k) 44"x44"X14"	(20k) 44"x44"X14"				1373.65 #
MAT	32 '	(20k) 44"x44"X14"	(20k) 44"x44"X14"				1497.63 #
MUM	34 '	(20k) 44"x44"X14"	(20k) 44"x44"X14"				1621.6 #
MAXII	36 '	(20k) 44"x44"X14"	(20k) 44"x44"X14"				1745.57 #
_	46 '	(20k) 44"x44"X14"	(30k) 54"x54"X17"				2365.44 #
		SUPPORTS (UNDER MATING OPE	NING AS CLEARSPA	ANS IN FEET		
POST	SPACING	7.9 '	7.9 ' 0/C				Girder beams
FOO	TER SIZE	(9k) 30"x30"X11"	(9k) 30"x30"X11"				construction to be (4)
		SUPPORT	S UNDER MATING W	ALLS- CLEARSPAN	S IN FEET		2X10 #2 SPF joists. Splices 6" X 8" MiTek
POST	SPACING	6.7 '	6.7 '				MT20 metal plates each
FOO	TER SIZE	(9k) 30"x30"X11"	(9k) 30"x30"X11"				side

Chart Key:

(Post Load)= Minimum allowable compression rating which post must be rated in kips (1000 lbs.).

(Post Capacity and Footer Size) Min. footer width (inches) x Min. footer length (inches) x Min. footer depth (inches)

Note: Steel piers must have a minimum steel base plate size of 4 inches x 5.5 inches which bears directly on footer sized per chart.

Minimum steel column top plate size of 4"x5.5"for 9000#; 6"x6"for 14000#; 6"x8"for 20000# & 6"x12"for 30000#

Minimum footer Reinforcement (Number of #4 bars each way):

Footer size	<u># of No. 4 bars</u>	<u>Footer size</u>	# of No. 4 bars
30"x30"	3	44"x44"	6
38"x38"	5	54"x54"	9

30' 1 STORY- W.O ATTIC OFF FRAME BASEMENT & CRAWL With Roof Pitch of 6/12 Min. to 6/12 Max.

NOTES: 1 DESIGNED FOR 100 MPH MAX. WIND SPEED.

2 DESIGNED FOR 1500 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.

3 DESIGN TO * Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd. All wind speeds are indicated as (V

4 MAX. MATING WALL OPENINGS LISTED IN CHART ASSUME OPENING IN BOTH HALVES. IF ANCHOR IS TIED TO ONLY ONE

COLUMN (ONE HALF) THEN HALF THE OPENING SIZE CAN BE USED WHEN LOOKING UP VALUE IN TABLE ABOVE. PIERS

SUPPORTS REQUIRED AT EACH SIDE OF DOOR OPENINGS AND ALL EXTERIOR WALL OPENINGS GREATER THAN 4'.

5 WHEN PIER CONFIGURATION IS NOT GIVEN IN CHART THE ACTUAL LOADS EXCEED

ALL PREDESIGNED FOOTERS AND A LOCAL ENGINEER MUST DESIGN THE SUPPORTS FOR THE GIVEN LOADS (- UPLIFT/ + GRAVITY LOADS).

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Support and anchorage for 16" Max. Recess

NON CORNER- SPANS ARE NOT LOCATED WITH 6' OF END OF HOME

			PIER	CONFIGUR	ATION AND	TION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1,4							
	ROUND SNOW		20) #	30	O #							
Max.	UPLIFT ™	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	
span ³	LOAD	Brk ²	anchors	anchors	anchors	anchors							
4	-201.86738#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"							
6	-302.80108#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"							
8	-403.73477#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"							
10	-504.66846#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"							
12	-605.60215#	-1	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"							

CORNER- SPANS ARE LOCATED WITH 6' OF END OF HOME

			PIER	CONFIGUR	ATION AND	MINIMUM F	OOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1,4					
G	GROUND SNOW		20	0 #	30	O #						
Max.	UPLIFT ™	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete
span ³	LOAD	Brk ²	anchors	anchors	anchors	anchors						
4	-168.36242 #	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
6	-252.54362#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
8	-336.72483 #	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
10	-420.90604#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
12	-505.08725#	-1	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"						

Support and anchorage for 48" Max. Porch Depth

NON CORNER- SPANS ARE NOT LOCATED WITH 6' OF END OF HOME

			PIER	PIER CONFIGURATION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1,4								
G	ROUND SNOW	/	20) #	30 #							
Max.	UPLIFT 10	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete
span ³	LOAD	Brk ²	anchors	anchors	anchors	anchors						
4	-99.339953#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
6	-149.00993#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
8	-198.67991 #	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
10	-248.34988#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
12	-298.01986#	-1	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"						

CORNER- SPANS ARE LOCATED WITHIN 6' OF END OF HOME

			PIER	PIER CONFIGURATION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1.4								
G	ROUND SNOV	/		0 #		O #						
Max.	UPLIFT ™	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete
span ³	LOAD	Brk ²	anchors	anchors	anchors	anchors						
4	-61.940481 #	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
6	-92.910721#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
8	-123.88096#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
10	-154.8512#	-1	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"	(S) 28"x28"X10"						
12	-185.82144#	-1	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"	(D) 40"x40"X12"						

NOTES:

- 1. Piers supports are required under all porch/ recess post and at intersection of sidewall (see key plan).
- 2. # Brk- Number of uplift brackets required under the support column. Brackets per Detail D6. Brackets maybe Installed individually or in pairs and must be tied to a ground anchor or concrete anchor with a minimum design capacity of 3150#. An alternate uplift connector may be used which has the required uplift load indicated above. NG- Indicates that uplift exceeds standard angle and tie down capacity and alternate design is require.
- 3. Max. Span- Maximum distance between adjacent porch post or supports as measure parallel to box length.
- 4. Piers- Indicates the minimum CMU block configuration (\$)ingle, (D)ouble, (T) Triple or (DR) (D)ouble (R)einforced Approval of this document does not authors of the RECESS and minimum footer size. See Detail D3 of D4 for pier configuration.

 5. w/ ground anchors- Minimum footer size for gravity load support at post. Uplift is taken to ground anchor anchors placed minimum so approve any deviation or deviations from the RECESS (TABLE P)
- 6. w/ concrete anchors- Minimum footer size based on gravity and uplift. Concrete anchors embedded into foot carry սխին Nichter
- 7. off frame basement & crawl foundation design for: 29' 8 " 2-section modular
- 8. designed for 100 mph max. wind speed.
- 9. Desgin for 1500 psf min. allowable soil bearing capacity.
- 10. Designed to the * Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd. All wind speeds are ind

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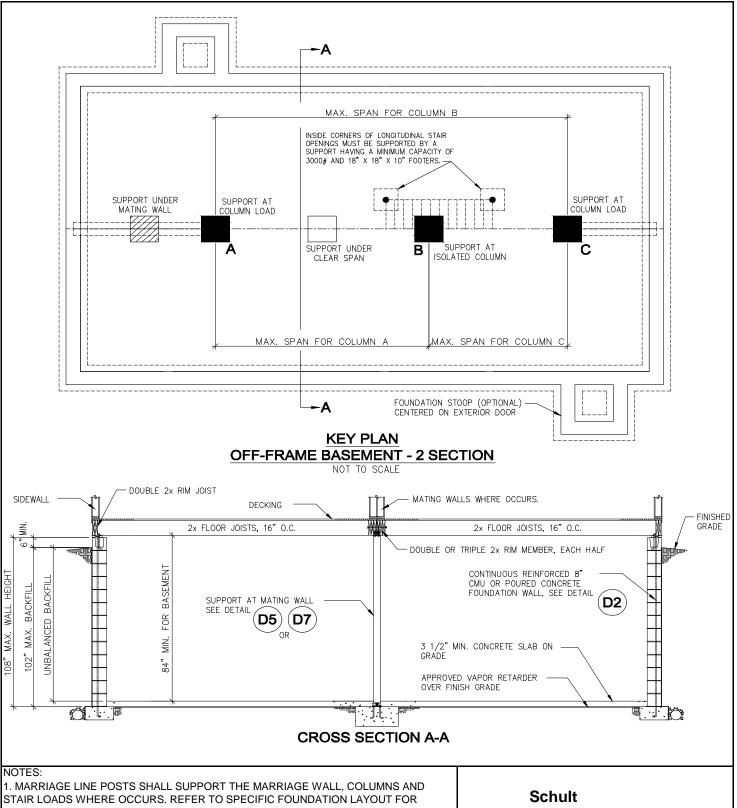
(TABLE P)

DATE: 3/27/07

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- LOCATION INFORMATION.
- 2. SEE GENERAL NOTES SECTION FOR VENTILATION, DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

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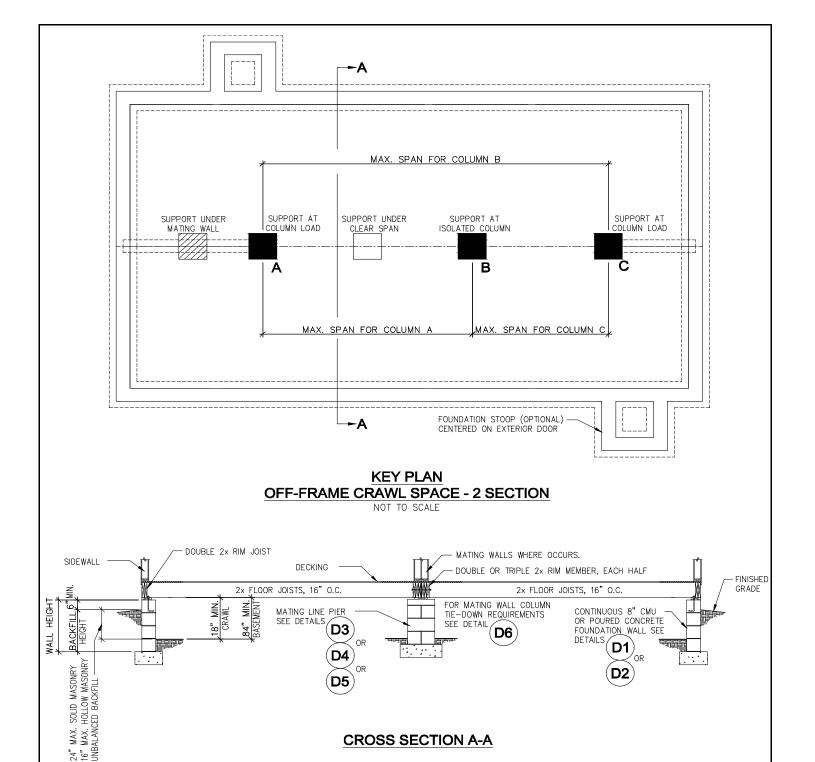
KEY PLAN 7 - OFF-FRAME / BASEMENT / 2 SECTION

DATE: 05/25/07

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CROSS SECTION A-A

NOTES:

- 1. MARRIAGE LINE POSTS SHALL SUPPORT THE MARRIAGE WALL, COLUMNS AND STAIR LOADS WHERE OCCURS. REFER TO SPECIFIC FOUNDATION LAYOUT FOR LOCATION INFORMATION.
- 2. SEE GENERAL NOTES SECTION FOR VENTILATION, DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.



Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws. **David Richter**

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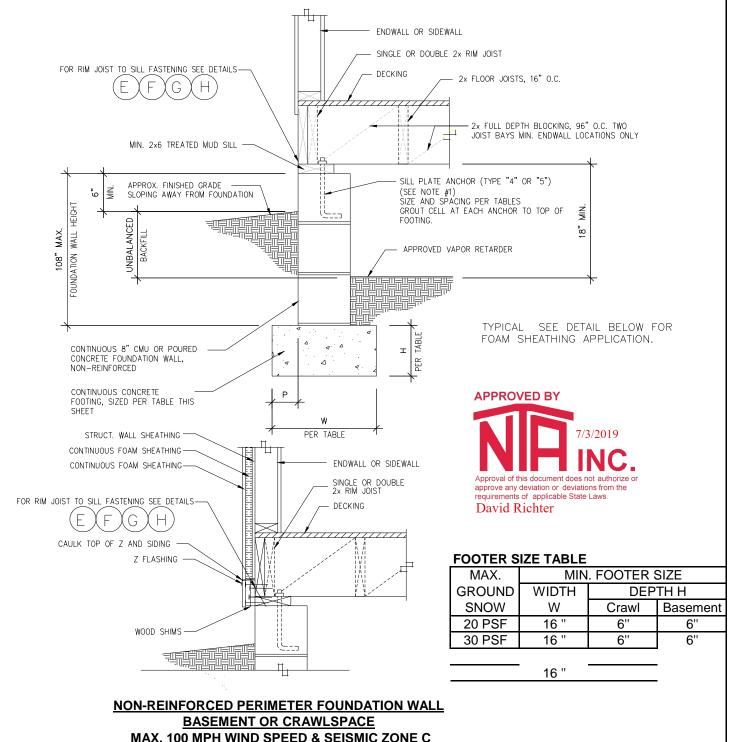
KEY PLAN 8 - OFF-FRAME / CRAWL SPACE / 2 SECTION

DATE: 05/25/07

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MAX. 100 MPH WIND SPEED & SEISMIC ZONE C 30 ' WIDE 1 STORY- W.O ATTIC

1) MUD SILL TO FOUNDATION ANCHORS:

TYPE 4: 1/2" DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH 2"x2"x1/8" WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.

TYPE 5: SIMPSON MAB OR MAS MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS.

- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
 - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED. a)
 - SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM b) BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.
- 3) DISTANCE FROM EDGE OF FOOTER TO FACE OF FOUNDATION WALL (P) SHALL NOT BE LESS THAN 2" AND SHALL NOT EXCEED THE FOOTER THICKNESS (H).
- 1500 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.

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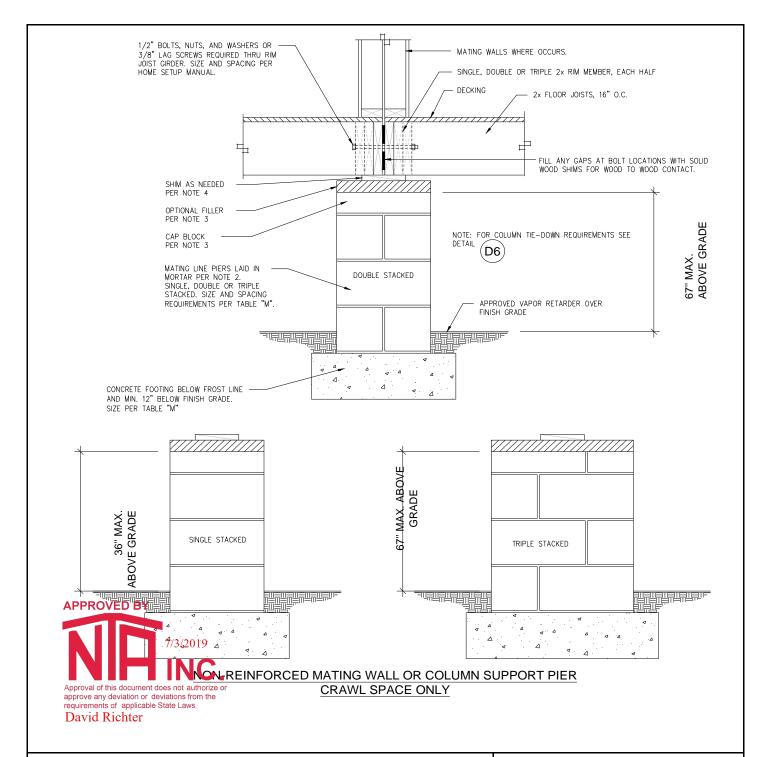
NON-REINFORCED PERIMETER FOUNDATION WALL - DETAIL - D1

DATE: 07/17/07

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NOTES:

- 1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. PIERS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE PIER IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. PIERS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.
- 2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTURED IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY.THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR OR APPROVED ALTERNATE (SEE GENERAL NOTE 12). SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE OR THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.
- 3. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2x NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF A HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2x NOMINAL HARDWOOD OR 2" OR 4" NOMINAL SOLID CONCRETE BLOCK. ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE PIERS THEY REST UPON. INDIVIDUAL LENGTHS OF CAP BLOCKS AND FILLER SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.
- 4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR FILLER AND MATE LINE RIM JOISTS.
- 5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M".
- 6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

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NON-REINFORCED MATING
WALL COLUMN SUPPORT PIER
- CRAWLSPACE ONLY - DETAIL

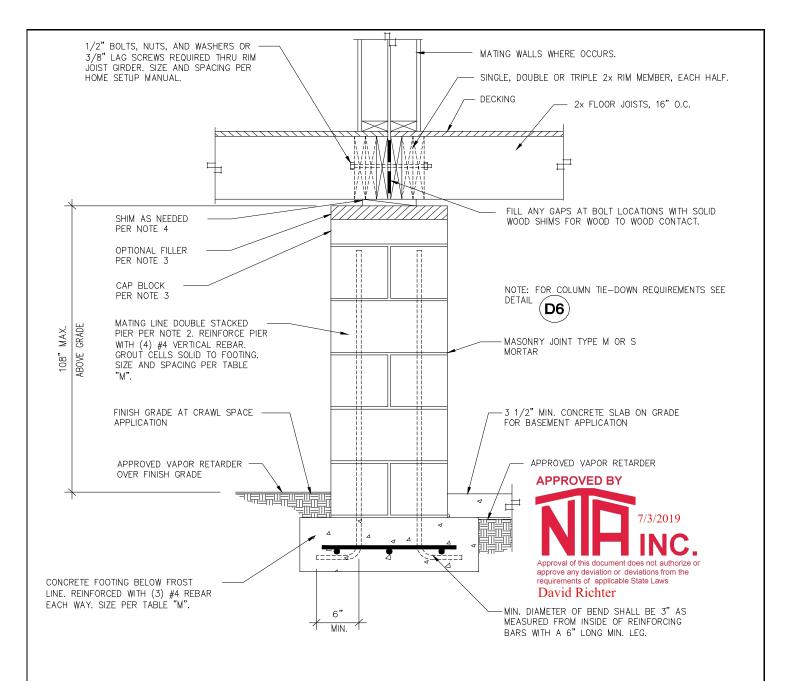
- D3

DATE: 06/13/07

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REINFORCED MATING WALL OR COLUMN SUPPORT PIER BASEMENT OR CRAWL SPACE (PIER SPACING AND FOOTER SIZE PER TABLE M)

NOTES

1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. PIERS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE PIER IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. PIERS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.

2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTURED IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY.THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR. SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE OR THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

3. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2X NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2X NOMINAL HARDWOOD OR 2" OR 4" NOMINAL SOLID CONCRETE BLOCK. ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE PIERS THEY REST UPON. INDIVIDUAL LENGTHS OF CAP BLOCKS AND FILLER SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR FILLER AND MATE LINE RIM JOISTS.

5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M".

6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

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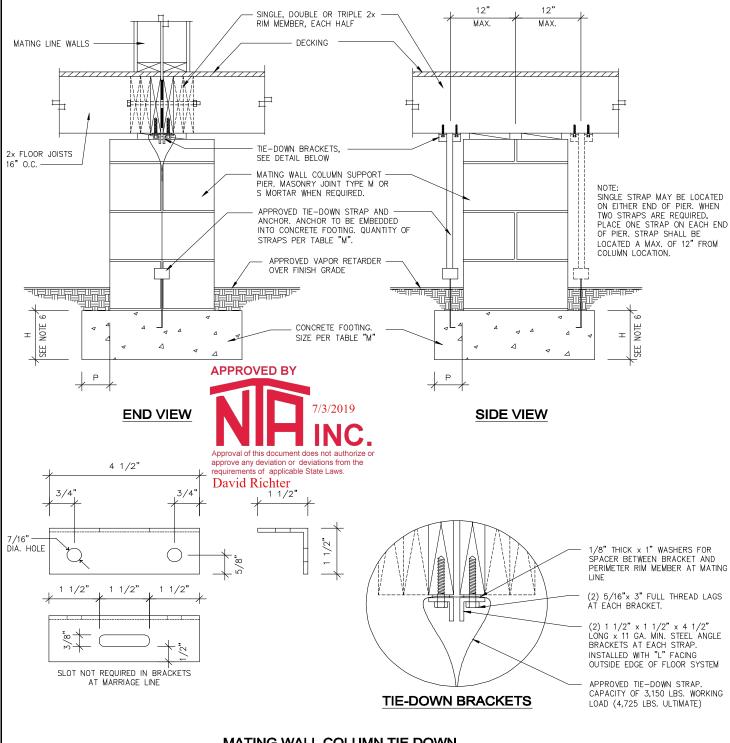
REINFORCED MATING WALL OR COLUMN SUPPORT PIER -BASEMENT OR CRAWL SPACE DETAIL - D5

DATE: 06/04/07

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MATING WALL COLUMN TIE DOWN

NOTES

- 1. ALL MARRIAGE WALL COLUMN LOCATIONS WITH OPENINGS 4 FEET OR GREATER MAY REQUIRE THE INSTALLATION OF COLUMN BRACKETS AND TIE-DOWNS. SEE TABLE "M" FOR REQUIREMENTS.
- 2. EACH BRACKET IS RATED FOR AN ALLOWABLE WORKING LOAD OF 1,719 LBS.
- 3. THE CAPACITY OF BOTH THE TIE-DOWN STRAP AND ANCHOR MUST BE 3,150 LBS. WORKING LOAD (4,725 LBS. ULTIMATE)
 4. USE A RADIUS CLIP FOR ALL BRACKET APPLICATIONS BY THREADING A PIECE OF STRAP OVER THE BRACKETS BEFORE
- LOOPING THE TIE-DOWN STRAP AROUND THE BRACKET.
- 5. GROUND ANCHORS WHICH ARE LISTED FOR THE REQUIRED CAPACITY ABOVE MAY BE USED IN LIEU OF CONCRETE ANCHOR.
- 6. DISTANCE FROM EDGE OF FOOTING TO FACE OF FOUNDATION WALL (P) SHALL NOT BE LESS THAN 2" AND SHALL NOT EXCEED THE FOOTING THICKNESS (H). FOOTING THICKNESS MAY BE 10" IF GROUND ANCHORS WITH AN UPLIFT CAPACITY OF 3,150 LBS. ARE USED IN PLACE OF CONCRETE ANCHORS.
- 7. FOOTING SIZES PER TABLE "M" HAVE BEEN DESIGNED ASSUMING CONCRETE ANCHORS WILL BE UTILIZED. IF GROUND ANCHORS ARE UTILIZED TO TRANSMIT UPLIFT INTO GROUND SOIL, THE DEPTH OF THE FOOTING MAY BE REDUCED TO (P). WHERE (P) IS EQUAL TO THE GREATEST DISTANCE FROM EDGE OF FOOTING TO EDGE OF PIER. MINIMUM DEPTH IS 9".

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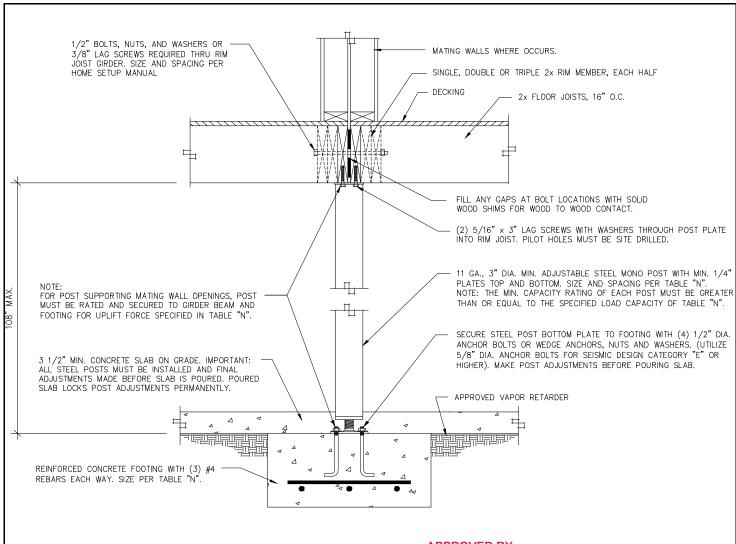
MATING WALL COLUMN TIE DOWN - DETAIL - D6

DATE: 06/29/07

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ALTERNATE POST INSTALLATION: STEEL POSTS MAY BE INSTALLED WITH SCREW JACK ASSEMBLY AT THE TOP OR BOTTOM. STEEL POSTS INSTALLED WITHOUT THE SCREW JACK ASSEMBLY AT THE BOTTOM AND ENCASED IN CONCRETE ARE SUBJECT TO LOAD REDUCTIONS. VERIFY THE CAPACITY OF THE STEEL POST BASED ON THE INSTALLATION METHOD PRIOR TO INSTALLATION OF THE POST.



ADJUSTABLE STEEL COLUMN POST BASEMENT OR CRAWL SPACE (MAXIMUM POST SPACING PER TABLE N)

NOTES:

- 1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. STEEL POSTS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE POST IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. COLUMN POSTS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.
- 2. MARRIAGE LINE STEEL POSTS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER
- 3. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS

Schult

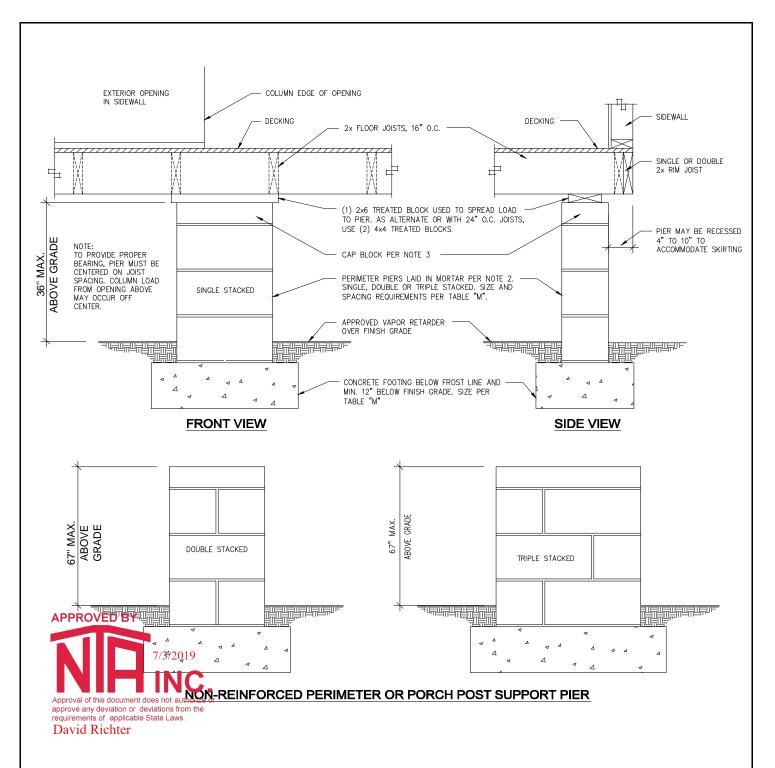
ADJUSTABLE STEEL COLUMN POST - BASEMENT OR CRAWL SPACE - <u>DETAIL - D7</u>

DATE: 06/08/07

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1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. PIERS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE PIER IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. PIERS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.

2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTURED IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY. SEE NOTE 7. FOR MORTAR REQUIREMENT. SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE OR

THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS. 3. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2x NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF A HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2x NOMINAL HARDWOOD OR 2' OR 4' NOMINAL SOLID CONCRETE BLOCK. ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE RIM JOISTS

KINI JOISTS. 4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR ILLER AND MATE LINE RIM JOISTS.

5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M". 6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR OR DRY STACKED ABOVE FIRST COARSE WITH SURFACE BONDING AGENT APPLIED THAT MEETS ASTM C887 WHEN ACCEPTABLE TO LOCAL AUTHORITY. BONDING AGENT MUST BE INTENDED FOR USE/APPLICATION AND SHALL BE INSTALLED PER MANUFACTURES SPECIFICATINS.

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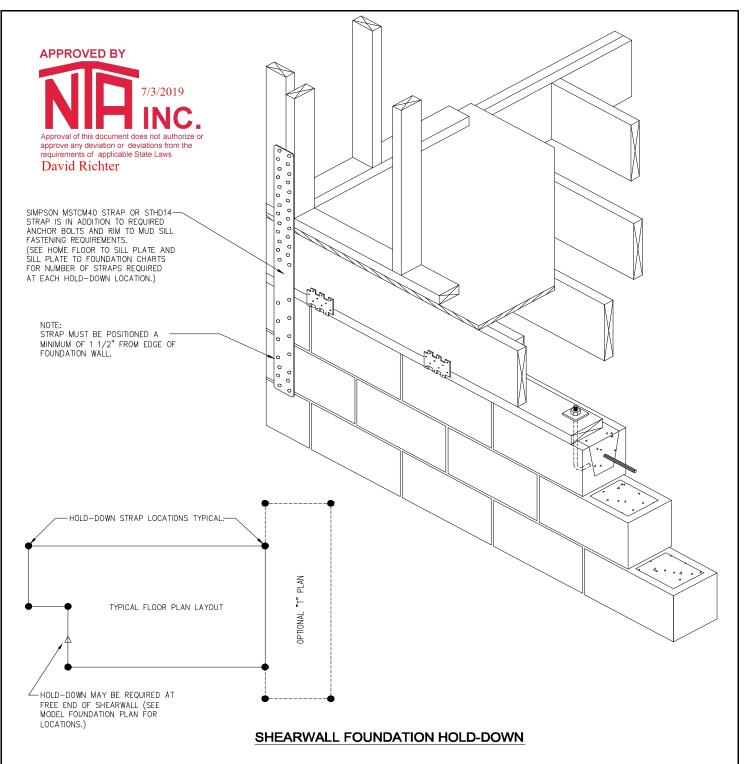
NON-REINFORCED PERIMETER/ **PORCH POST SUPPORT PIER -DETAIL - D15**

DATE: 07/18/07

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NOTES:

- 1. WHERE REQUIRED AT FREE-END HOLD-DOWNS (AS LOCATED ON THE FOUNDATION LAYOUT) OR AT BUILDING CORNERS PER THE FASTENING TABLES INCLUDED WITHIN THIS FOUNDATION DESIGN PACKAGE, THE FOUNDATION HOLD-DOWN STRAPS ARE THE RESPONSIBILITY OF OTHERS AND ARE NOT PROVIDED BY CLAYTON HOME BUILDING GROUP OR SUBSIDIARIES.
- 2. SIMPSON MSTCM40 SHALL BE FASTENED TO WALL STUD WITH (26) 16d NAILS AND TO FOUNDATION WALL WITH (14) 1/4" x 2 1/4" TITAN SCREWS.
- 3. SIMPSON MSTCM40 OR STHD14 STRAP MAY BE PLACED ON ENDWALL OR SIDEWALL.
 MINIMUM EDGE DISTANCE OF TITAN SCREW TO CONCRETE OR MASONRY BLOCK CORNER OF 1
 1/2" MUST BE MAINTAINED.
- 4. SIMPSON MSTCM40 STRAP IS IN ADDITION TO THE REQUIRED ANCHOR BOLTS AND RIM TO MUD SILL FASTENING REQUIREMENTS.
- 5. SIMPSON STHD14 STRAP (POURED WALLS) MUST BE FASTENED TO WALL STUD WITH (38) .148X 3 1/4" NAILS.)
- 6. DESIGN STRAP CAPACITY: MSTCM40=4250# AND STHD14= 5025#

Schult

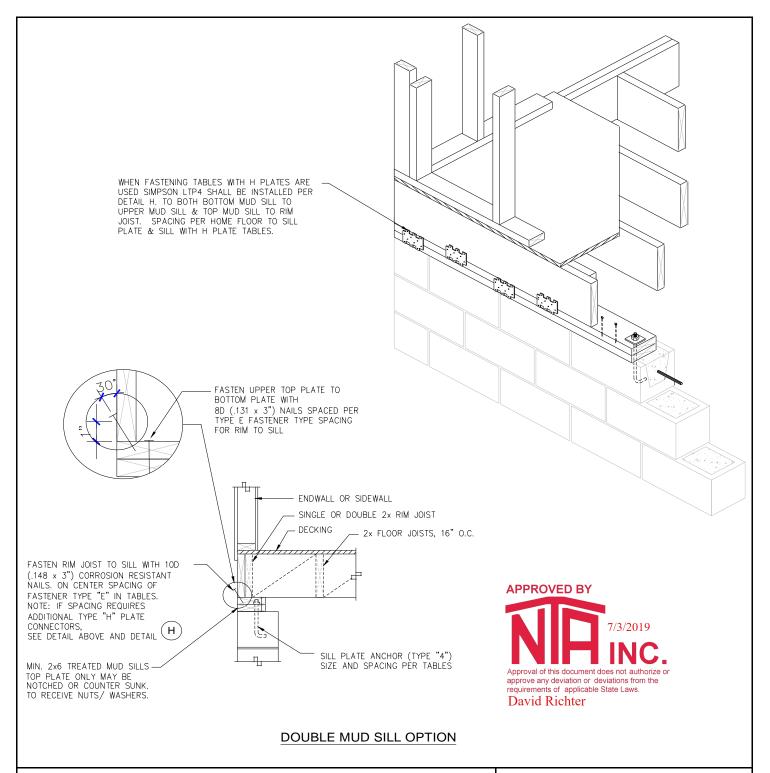
SHEARWALL FOUNDATION HOLD-DOWN - DETAIL - D18

DATE: 06/13/07

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NOTES:

- 1. MUD SILL TO FOUNDATION ANCHORS:
 - TYPE 4:1/2" DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH 2"x2"x1/8" WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO BOTTOM MUD SILL PLATE.
- UPPER MUD SILL MUST BE FASTENED TO LOWER MUD SILL WITH .131"X3" NAILS SPACED PER RIM JOIST TO MUD SILL SPACING TABLE FOR TYPE E FASTENERS.
- 4. WHEN FASTENING TABLES WITH H PLATES ARE USED, SIMPSON LPT4 PLATES MUST BE INSTALLED FROM LOWER MUD SILL TO UPPER MUD SILL AND FROM UPPER MUD SILL TO RIM JOIST PER FASTENING SPACED PER RIM TO MUD SILL SPACING TABLES.

Schult

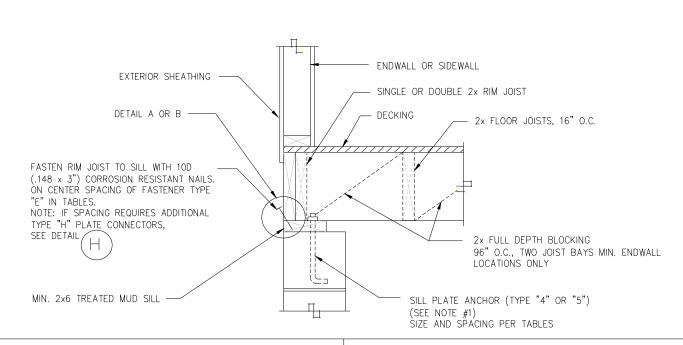
DOUBLE MUD SILL FOUNDATION WALL <u>DETAIL - D34</u>

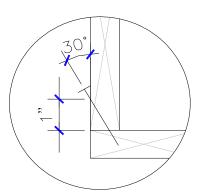
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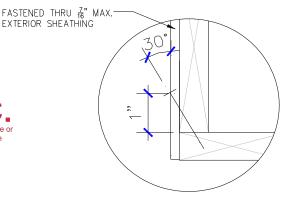
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ALTERNATE FASTENER:

THE FOLLOWING ALTERNATE FASTENERS MAY BE USED WHEN SPACING IN CHART IS MULTIPLIED BY THE FOLLOWING FASTENER MULTIPLIER.

8D (.131 x 3") NAIL = .82 16D (.162 x 3 1/2") NAIL = 1.2 #8 x 3" WOOD SCREW = .78 ALTERNATE FASTENER:

FASTENERS MAY BE INSTALLED THROUGH $\frac{7}{16}$ " MAXIMUM THICK WALL SHEATHING WHEN SPACING IN CHARTS ARE REDUCED BY MULTIPLYING BY THE FOLLOW:

10d (.148"X3") NAIL = .68 8D (.131 x 3") NAIL = .55 16D (.162 x 3 1/2") NAIL = .816 #8 x 3" WOOD SCREW = .53

DETAIL A- DIRECT RIM TO SILL FASTENING

DETAIL B- THRU SHEATHING RIM TO SILL FASTENING

FLOOR TO SILL PLATE FASTENING -TYPE "E" -ENDWALL OR SIDEWALL

NOTES:

1) MUD SILL TO FOUNDATION ANCHORS:

TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.

TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS

- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
 - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
 - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

Schult

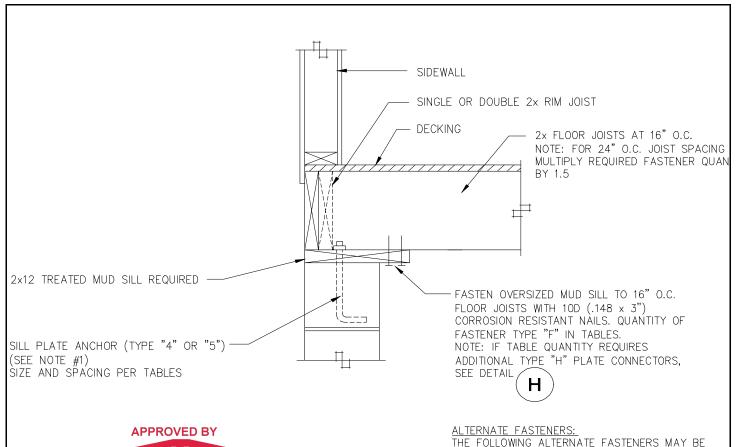
FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL DETAIL - E

DATE: 04/17/07

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7/3/2019

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David Richter

THE FOLLOWING ALTERNATE FASTENERS MAY BE USED WHEN QUANTITY IN TABLE IS MULTIPLIED BY THE FOLLOWING FASTENER MULTIPLIER.

8D (.131 x 3") NAIL = 1.21 16D (.162 x 3 1/2") NAIL = .83 #8 x 3" WOOD SCREW = 1.28

NOTE: THIS DETAIL FOR TYPE "F" FASTENING IS APPLICABLE TO SIDEWALL CONNECTIONS ONLY AND CAN NOT BE USED FOR ENDWALLS. SEE FASTENING TYPE "E" OR TYPE "G" FOR ENDWALL APPLICATION.

FLOOR TO SILL PLATE FASTENING - TYPE "F" - SIDEWALL ONLY

NOTES:

- 1) MUD SILL TO FOUNDATION ANCHORS:
 - TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.
 - TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS
- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
 - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
 - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

Schult

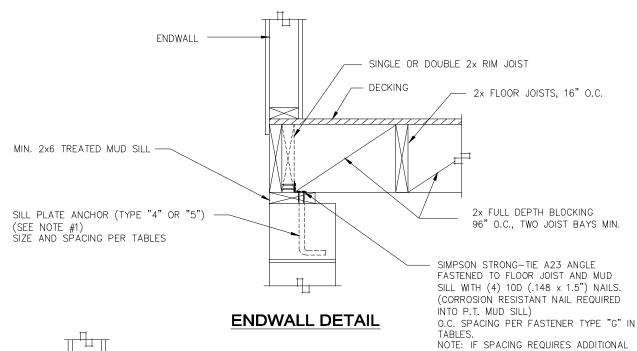
FLOOR TO SILL PLATE FASTENING - SIDEWALL ONLY <u>DETAIL - F</u>

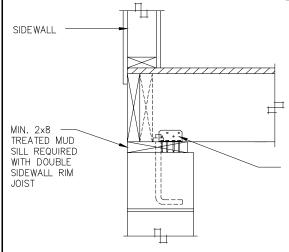
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SIDEWALL DETAIL

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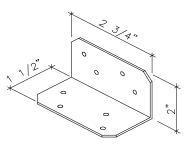
7/3/2019

INC

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David Richter

SIMPSON STRONG-TIE A23 ANGLE FASTENED TO FLOOR JOIST AND MUD SILL WITH (4) 10D (.148 x 1.5") NAILS. (CORROSION RESISTANT NAIL REQUIRED INTO P.T. MUD SILL) USE (1) OR (2) ANGLES EA. JOIST PER FASTENER TYPE "G" IN TABLES. NOTE: IF SPACING REQUIRES ADDITIONAL TYPE "H" PLATE CONNECTORS, SEE DETAIL



TYPE "H" PLATE CONNECTORS,

SEE DETAIL

SIMPSON STRONG-TIE 'A23' ANGLE

FLOOR TO SILL PLATE FASTENING - TYPE "G" -ENDWALL OR SIDEWALL

NOTES:

- 1) MUD SILL TO FOUNDATION ANCHORS:
 - TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.
 - TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS
- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
 - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
 - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

Schult

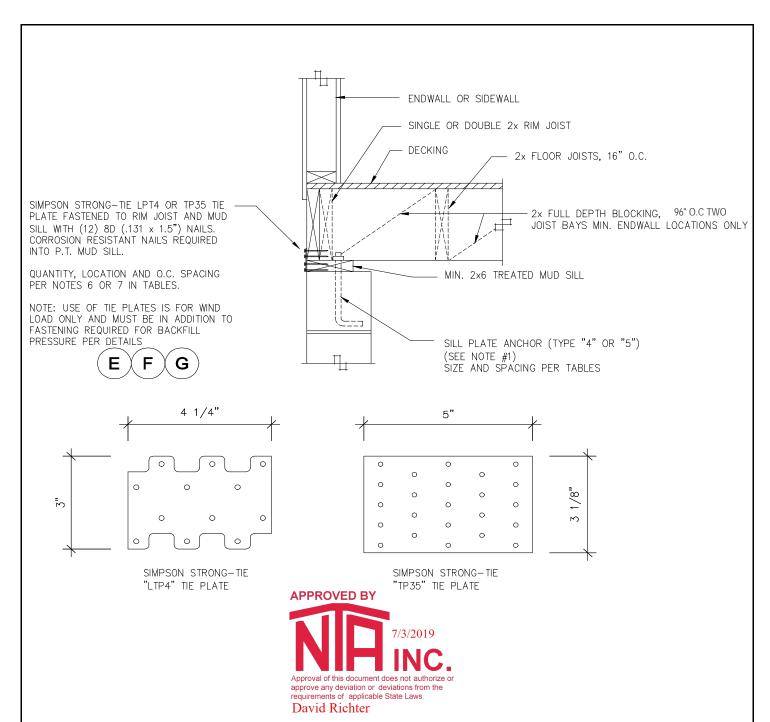
FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL - DETAIL - G

DATE: 05/25/07

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FLOOR TO SILL PLATE FASTENING - TYPE "H" - ENDWALL OR SIDEWALL

NOTES:

- 1) MUD SILL TO FOUNDATION ANCHORS:
 - TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.
 - TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS
- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
 - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
 - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

Schult

FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL DETAIL - H

DATE: 04/17/07

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Home Floor to Sill Plate & Sill Plate to Foundation WITH TYPE H PLATE CONNECTORS (See note 6 & 7)

SOIL CLASSES SC, ML-CL AND INORGANIC CL SOILS [Allowable bearing capacity of 1500 psf or less]

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max.

Roof Pitch: 6/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

*Wind Speed (3s): 100 Seismic Zone C



		N	IAXIMUM I	ASTENER	SPACING	OR FASTE	NERS PER	R JOIST SP	ACING 2,3 8	5	# REQ'D
		SI	DEWALL I	ASTENIN	G SPACINO	3 ¹	E	ND WALL	FASTENIN	G	S/W HDS
Foundati	on Wall ¹⁰		Rim to Sill	6	Sill to F	nd. Wall	Rim t	o Sill ⁷	Sill to F	SEE	
Wall	Backfill	F	astener Typ		Anchor	Spacing	Fasten	er Type	Anchor	Spacing	D18
Height	Depth	Е	F ⁴	Ğ⁴	4	5	Е	G	4	5	/CORNER
24 "	16 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	40" o.c.	492" o.c.	56" o.c.	30" o.c.	1
32 "	24 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	16" o.c.	194" o.c.	54" o.c.	29" o.c.	1
40 "	32 "	8.4" o.c.	2	1	72" o.c.	72" o.c.	8" o.c.	102" o.c.	48" o.c.	28" o.c.	1
3.833 '	3.33 '	4.9" o.c.	2	1	42" o.c.	47" o.c.	5" o.c.	61" o.c.	38" o.c.	25" o.c.	0
7 '	4 '	5.2" o.c.	2	1	45" o.c.	49" o.c.	5" o.c.	64" o.c.	39" o.c.	26" o.c.	0
7'	5 '	NA	4	1	23" o.c.	25" o.c.	NA	33" o.c.	23" o.c.	20" o.c.	0
7'	6 '	NA	6	2	13" o.c.	15" o.c.	NA	19" o.c.	13" o.c.	13" o.c.	0
8 '	4 '	5.9" o.c.	2	1	51" o.c.	56" o.c.	6" o.c.	73" o.c.	42" o.c.	27" o.c.	0
8 '	5 '	3.0" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	37" o.c.	26" o.c.	21" o.c.	0
8 '	6 '	NA	6	2	15" o.c.	17" o.c.	NA	22" o.c.	15" o.c.	15" o.c.	0
8 '	7 '	NA	9	2	10" o.c.	11" o.c.	NA	14" o.c.	10" o.c.	10" o.c.	0
9'	3 '	15.3" o.c.	1	1	72" o.c.	72" o.c.	16" o.c.	194" o.c.	54" o.c.	29" o.c.	1
9 '	4 '	6.7" o.c.	2	1	57" o.c.	63" o.c.	7" o.c.	82" o.c.	44" o.c.	27" o.c.	0
9 '	5'	3.4" o.c.	3	1	29" o.c.	32" o.c.	3" o.c.	42" o.c.	29" o.c.	22" o.c.	0
9 '	6'	NA	5	2	17" o.c.	19" o.c.	NA	24" o.c.	17" o.c.	16" o.c.	0
9 '	7 '	NA	8	2	11" o.c.	12" o.c.	NA	15" o.c.	11" o.c.	11" o.c.	0
9'	8'	NA	11	NA	7" o.c.	8" o.c.	NA	10" o.c.	7" o.c.	8" o.c.	0

NOTES:

- 1. Fastener Types A,B,C & D are not reflected in charts and are available prescriptively per table R404.1(1) in 2006 IRC.
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- " Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
 - "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
 - "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners are in addition to (2) Type H tie plates spaced within 6' of corners & 96" oc. elsewhere along sidewalls.(See note 3)
- 7. Fasteners are in addition to Type H tie plates spaced at 33" oc. along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage

have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.

- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

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Home Floor to Sill Plate & Sill Plate to Foundation WITHOUT TYPE H PLATE CONNECTORS (See note 6 & 7)

SOIL CLASSES SC, ML-CL AND INORGANIC CL SOILS [Allowable bearing capacity of 1500 psf or less]

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max. Roof Pitch: 6/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

*Wind Speed (3s): 100 Seismic Zone C



		N	IAXIMUM F	ASTENER	SPACING	OR FASTE	NERS PE	R JOIST SF	PACING 2,3 8	£ 5	# REQ'D	
		SI	DEWALL I	FASTENIN	G SPACINO	3 ¹	E	ND WALL	FASTENIN	G	S/W HDS	
Foundati	on Wall ¹⁰		Rim to Sill	6	Sill to F	nd. Wall	Rim t	o Sill ⁷	I ⁷ Sill to Fnd. Wall			
Wall	Backfill	F	astener Typ		Anchor	Spacing	Fasten	er Type	Anchor	Spacing	D18	
Height	Depth	Е	F ⁴	G⁴	4	5	Е	G	4	5	/CORNER	
24 "	16 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	56" o.c.	30" o.c.	1	
32 "	24 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	28" o.c.	54" o.c.	29" o.c.	1	
40 "	32 "	8.4" o.c.	2	1	72" o.c.	72" o.c.	7" o.c.	24" o.c.	48" o.c.	28" o.c.	1	
3.833 '	3.33 '	4.9" o.c.	2	1	42" o.c.	47" o.c.	5" o.c.	18" o.c.	38" o.c.	25" o.c.	1	
7 '	4 '	5.2" o.c.	2	1	45" o.c.	49" o.c.	5" o.c.	19" o.c.	39" o.c.	26" o.c.	1	
7 '	5 '	NA	4	1	23" o.c.	25" o.c.	NA	10" o.c.	23" o.c.	20" o.c.	1	
7'	6 '	NA	6	2	13" o.c.	15" o.c.	NA	6" o.c.	13" o.c.	13" o.c.	0	
8 '	4 '	5.9" o.c.	2	1	51" o.c.	56" o.c.	6" o.c.	20" o.c.	42" o.c.	27" o.c.	1	
8 '	5 '	3.0" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	12" o.c.	26" o.c.	21" o.c.	1	
8 '	6'	NA	6	2	15" o.c.	17" o.c.	NA	6" o.c.	15" o.c.	15" o.c.	1	
8 '	7'	NA	9	2	10" o.c.	11" o.c.	NA	4" o.c.	10" o.c.	10" o.c.	0	
9	3 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	28" o.c.	54" o.c.	29" o.c.	1	
9 '	4 '	6.7" o.c.	2	1	57" o.c.	63" o.c.	6" o.c.	22" o.c.	44" o.c.	27" o.c.	1	
9	5 '	3.4" o.c.	3	1	29" o.c.	32" o.c.	4" o.c.	13" o.c.	29" o.c.	22" o.c.	1	
9 '	6'	NA	5	2	17" o.c.	19" o.c.	NA	7" o.c.	17" o.c.	16" o.c.	1	
9'	7'	NA	8	2	11" o.c.	12" o.c.	NA	4" o.c.	11" o.c.	11" o.c.	0	
9'	8 '	NA	11	NA	7" o.c.	8" o.c.	NA	3" o.c.	7" o.c.	8" o.c.	0	

NOTES:

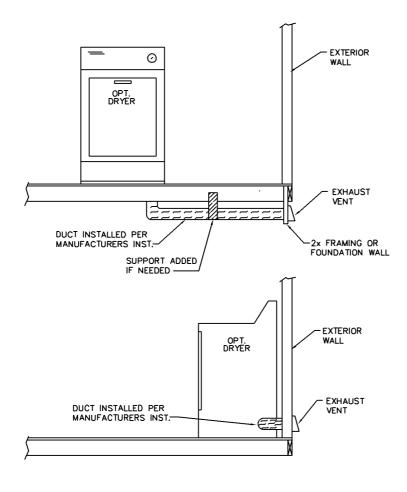
- 1. RESERVED
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- " Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H)

Anchor Types:

- "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
- "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along sidewall.
- 7. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage

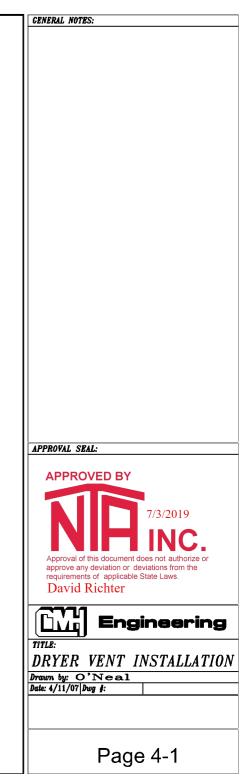
have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.

- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.



INSTALLTION INSTRUCTIONS:

EXHAUST DUCTS FOR DOMESTIC CLOTHES DRYERS SHALL BE CONSTRUCTED OF METAL OR NONCOMBUSTIBLE MATERIAL OF EQUAL STRENGTH AND CORROSION RESISTANCE AND SHALL HAVE A SMOOTH INTERIOR FINISH. NO PART OF THE DUCT TO BE IN CONTACT WITH THE GROUND. THE DUCT TO RUN TO THE OUTSIDE OF THE UNIT AND SHALL NOT TERMINATE UNDERNEATH THE UNIT. A APPROVED DAMPER TO BE INSTALLED ON THE END OF THE DUCT.



ELECTRICAL FURNACE DESCRIPTION CHART

Nortek							nded Wire zes	
Model						NM-B	SEU*	Low Voltage
E Series				Max Over-	Min. Circuit	60°C	60°C	Thermostat Wire
	Supply C	Circuit	Total Amperes	Current Rating	Ampacity	Copper	Copper	Size
010	Single		44.6	60	56	4-2	4-4-6	
012	Single		51.2	70	64	4-2	4-4-6	2-Wire
	Dual	"A"	27.1	40	34	8-2	6-6-10	system max wire
		"B"	24.2	30	30	10-2	8-8-10	lengths:
015	Single		N/A	N/A	N/A			24 Ga. = 55'
	Dual	"A"	44.6	60	56	4-2	4-4-6	22 Ga. = 90'
		"B"	20.8	30	26	10-2	8-8-10	20 Ga. = 140'
017	Single		N/A	N/A	N/A			24 Ga. = 55'
	Dual	"A"	47.9	60	60	4-2	4-4-6	22 Ga. = 90'
		"B"	22.5	30	28	10-2	8-8-10	20 Ga. = 140'
020	Single		N/A	N/A	N/A			18 Ga. = 225'
	Dual	"A"	44.6	60	56	4-2	4-4-6	
		"B"	41.7	60	52	4-2	4-4-6	4 or more-Wire
023	Single		N/A	N/A	N/A			system max wire
	Dual	"A"	45.5	60	57	4-2	4-4-6	lengths:
		"B"	48.0	60	60	4-2	4-4-6	24 Ga. = 25'
								22 Ga. = 45'
								20 Ga. = 70'
								18 Ga. = 110'

ELECTRIC FURNACE MODEL NUMBER	OUTPUT CAPACITY (BTU)
E#EB-010H	35,000
E#EB-012H	41,000
E#EB-015H	53,000
E#EB-017H	57,000
E#EB-020H	70,000
E#EB-023H	75,000

= Series Version

*- NEC Section 338.10(B)(4)(a)



	ELE(CTRICAL LEGEN	D (NO7	TO SCALE)						
	\Diamond	LIGHT		PANEL BOX						
	-CAN-	CAN LIGHT	\bigcirc	THERMOSTAT						
	- (P) -	PULL CHAIN LIGHT	- ()-	SWITCH						
	9	BATH FAN	.ن س	3-WAY SWITCH						
APPROVED BY		FLUORESCENT LIGHT	∇	PHONE JACK						
7/3/2019	TV	CABLE JACK	© CO	CEILING MOUNT C.O. & SMOKE DETECTOR						
Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.		15 AMP RECEPT FLOOR LEVEL	\otimes_{co}	CEILING MOUNT C.O. DETECTOR						
David Richter		15 AMP RECEPT CABINET LEVEL	(SD)	WALL MOUNT SMOKE DETECTOR						
	F	15 AMP RECEPT SIDEWAYS	(SD)	CEILING MOUNT SMOKE DETECTOR						
		20 AMP RECEPT FLOOR LEVEL		SWITCH LEG						
		20 AMP RECEPT Cabinet Level	GEN	JUNCTION BOX						
		20 AMP RECEPT SIDEWAYS		CEILING FAN						
	+	240 VOLT RECEPT		CETEINO T AN						
	₩P GFI	15 AMP Waterproof Recept	() 1/1	POT & PAN RACK						
	∰ _{WP} GFI	20 AMP Waterproof Recept		HEAT TAPE RECEPT						
		FURNACE WH WATER HEATER								
	A D	ASHED SYMBOL RE	PRESENT	TS AN OPTION						
	GFI-I	NDICATES A GROUND	FAULT P	ROTECTED RECEPT						

APPLIANCE	MANUFACTURER	MODEL#	ANSI/ASME STANDARD
TOILET	BRISTOL BAY	VCEFB-03B	
SINKS	LYONS	KS01P4-TB	
	EL MUSTICE & SON	#610 UTILITY	
	PREMIUM FLOW	SINGLE BOWL	
	CORESTONE & TEKA	DOUBLE BOWL	
	REVERE	BAR SINK	
LAVITORIES	BRISTOL BAY	VCL-10	
		7112	
TUB SHOWER	BAYMONT BATHWARE	5118	UL
		5100 5109	
		3109	
SHOWER	BAYMONT BATHWARE	3309	UL
		3308	
		3304	
TUB	BAYMONT BATHWARE	2205	UL





Trenco

818 Soundside Rd Edenton, NC 27932

Re: WPL-913-0315-014_(16W)
CMH MANUFACTURING - SCHULT (Rich-NC)

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Wood Perfect, Ltd.

Pages or sheets covered by this seal: I33865413 thru I33865426

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844





July 3,2018

Galinski, John

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

7.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:37:12 2018 Page

Structural wood sheathing directly applied or 6-0-0 oc purlins,

APPROVED BY

David Richter

Rigid ceiling directly applied or 7-0-1 oc bracing.

except end verticals.

1 Brace at Jt(s): 12

1 Row at midpt

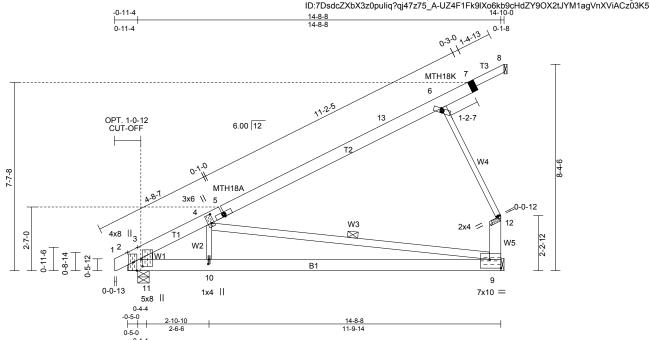


Plate Offset	ts (X,Y) [2:0)-4-0,0-1-12], [4	1:0-4-4,0-1-4], [5:0-0-5,0-1-2], [6:0-0	-11,0-1-2], [9:0-8-5,1	-2-12], [9:	Edge,0-4-4], [1	0:0-2-4	0-0-8],	[11:0-3-6	,0-0-12]		
SPACING-: LOADING		SPACING-: LOADING		SPACING-	2-0-0	CSI.	0.70	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	23.1	TCLL	34.7	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.70 0.55	Vert(LL) Vert(CT)		9-10 9-10	>999 >551	240 180	MT20 MT18HS	197/144 197/144
(Ground Sn	ow=30.0)	(Ground Sn	,	Rep Stress Incr	YES	WB	0.55	Horz(CT)	0.01	9-10	>ɔɔ ı n/a	n/a	IVII IONS	197/144
TCDL	11.0	TCDL	16.5	Code IBC2015/T		(Mati		11012(01)	0.01	3	11/G	TI/ CI	Weight: 80	lh
BCLL	0.0 *	BCLL	0.0 *	Code IBC2013/1	F12014	(iviali	ix)						FT = 0%	ID
BCDL	10.0	BCDL	15.0										F1 - 0%	

TOP CHORD

BOT CHORD

WEBS

JOINTS

I UMRFR-**BRACING-**

TOP CHORD 2x6 SPF No.2 *Except*

7-8: 2x4 SPF No.2 2x6 SPF No.2

BOT CHORD 2x3 SPF Stud *Except* WEBS

4-9: 2x4 SPF No.2, 9-12: 2x6 SPF Stud, 3-11: 2x6 SP No.2

(lb/size) 9=627/Mechanical, 8=0/Mechanical, 2=735/0-5-8 REACTIONS.

Max Horz 8=-103(LC 19), 2=464(LC 12) Max Uplift 9=-506(LC 12), 2=-321(LC 12) Max Grav 9=730(LC 19), 2=771(LC 19)

FORCES. (lb) - Maximum Compression/Maximum Tension

1-2=-1/0, 2-3=-1340/530, 3-4=-1188/355, 4-5=-503/24, 5-13=-474/42, 6-13=-279/57, 6-7=-201/76, 7-8=-118/88, TOP CHORD

9-12=-463/492

2-11=-792/929, 10-11=-792/929, 9-10=-792/929 **BOT CHORD**

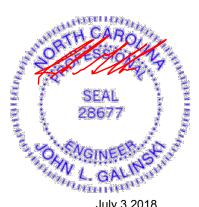
4-10=0/439, 4-9=-779/569, 6-12=-511/543, 3-11=-273/252 **WEBS**

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

7=148/84/60/0, 12=511/545/0/0

NOTES-

- 1) Wind: ASCE 7-10; Vult=152mph (3-second gust) Vasd=120mph @24in o.c.; TCDL=4.4psf; BCDL=4.0psf; (Alt. 180mph @16in o.c.; TCDL=6.6psf; BCDL=6.0psf); h=22ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pg=30.0 psf (ground snow); Ps=23.1 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 18.0 psf or 2.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 506 lb uplift at joint 9 and 321 lb uplift at joint 2. Continued on page 2



Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

July 3,2018

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	CMH MANUFACTURING - SCHULT (Rich-NC)	133865413
WPL-913-0315-014_(16W)	9529-15B	HINGED TRUSS	1		M9529 : 6/12 32 WIDE MOD/HUD	133603413

T.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:37:12 2018 Page 2 ID:7DsdcZXbX3z0puliq?qj47z75_A-UZ4F1Fk9lXo6kb9cHdZY9OX2tJYM1agVnXViACz03K5

NOTES-

- 14) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 15) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



7.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:39:07 2018 Page

Structural wood sheathing directly applied or 6-0-0 oc purlins,

APPROVED BY

David Richter

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

1 Brace at Jt(s): 13

1 Row at midpt

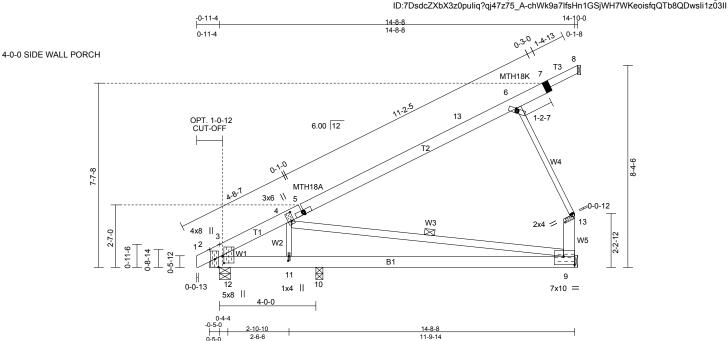


Plate Offse	ts (X,Y) [2:	<u>0-4-0,0-1-12], [</u> 4	<u>4:0-4-4,0-1-4], [</u>	<u>[5:0-0-5,0-1-2], [6:0-0</u>	<u>-11,0-1-2], [</u>	9:0-8-5 <u>,</u> 1	<u>-2-12], [9:</u>	Edge,0-4-4], [11:0-2-4,	<u>0-0-8],</u>	[12:0-3-6	<u>,0-0-12]</u>		
SPACING-: LOADING		SPACING- LOADING		SPACING- Plate Grip DOL	2-0-0 1.15	CSI.	0.70	DEFL. Vert(LL)	in -0.14	(loc) 9-10	l/defl >900	L/d 240	PLATES MT20	GRIP 197/144
TCLL (Ground Sn	23.1 ow=30.0)	TCLL (Ground Sr	34.7	Lumber DOL	1.15	BC	0.70	Vert(TL)			>679	180	MT18HS	197/144
TCDL	11.0	TCDL	16.5	Rep Stress Incr Code IBC2012/T	YES	WB (Mat	0.64	Horz(TL)	-0.01	9	n/a	n/a	Weight: 80	lh
BCLL BCDI	0.0 *	BCLL	0.0 * 15.0	Code IBC2012/1	F12007	(IVIAL	11.						FT = 0%	ID

BRACING-

TOP CHORD

BOT CHORD

WEBS

JOINTS

LUMBER-

REACTIONS.

TOP CHORD 2x6 SPF No.2 *Except*

7-8: 2x4 SPF No.2

2x6 SPF No.2 **BOT CHORD** 2x3 SPF Stud *Except* **WEBS**

4-9: 2x4 SPF No.2, 9-13: 2x6 SPF Stud, 3-12: 2x6 SP No.2

(lb/size) 9=516/Mechanical, 8=0/Mechanical, 2=460/0-3-0, 10=389/0-3-8

Max Horz 8=-103(LC 19), 2=464(LC 12)

Max Uplift 9=-501(LC 12), 2=-302(LC 12), 10=-24(LC 12) Max Grav 9=625(LC 19), 2=511(LC 19), 10=570(LC 5)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1/0, 2-3=-747/718, 3-4=-562/453, 4-5=-503/24, 5-14=-474/42, 6-14=-279/57, 6-7=-201/76, 7-8=-118/88,

9-13=-463/492

BOT CHORD 2-12=-893/400, 11-12=-893/400, 10-11=-893/400, 9-10=-893/400 4-11=-365/281, 4-9=-189/672, 6-13=-511/543, 3-12=-432/251

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in) 7=148/84/60/0, 13=511/545/0/0

NOTES-

- 1) Wind: ASCE 7-10; Vult=152mph (3-second gust) Vasd=120mph @24in o.c.; TCDL=4.4psf; BCDL=4.0psf; (Alt. 180mph @16in o.c.; TCDL=6.6psf; BCDL=6.0psf); h=22ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pg=30.0 psf (ground snow); Ps=23.1 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 18.0 psf or 2.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2



Approval of this document does not authorize or

approve any deviation or deviations from the

requirements of applicable State Laws

July 3,2018

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	CMH MANUFACTURING - SCHULT (Rich-NC)	133865423
WPL-913-0315-014_(16W)	9529-15L	HINGED TRUSS	1		M9529-P4 : 6/12 32 WIDE MOD/HUD	133603423

Job Reference (optional)
7.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:39:07 2018 Page 2
ID:7DsdcZXbX3z0puliq?qj47z75_A-chWk9a7lfsHn1GSjWH7WKeoisfqQTb8QDwsli1z03ll

NOTES-

- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 501 lb uplift at joint 9, 302 lb uplift at joint 2 and 24 lb uplift at joint 10.
 14) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Trenco

818 Soundside Rd Edenton, NC 27932

Re: WPL-913-0315-014_(16W)
CMH MANUFACTURING - SCHULT (Rich-NC)

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Wood Perfect, Ltd.

Pages or sheets covered by this seal: I33865459 thru I33865460

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844





July 3,2018

Galinski, John

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

7.640 s Apr 22 2016 MiTek Industries, Inc. Tue Jul 03 07:50:37 2018 Page

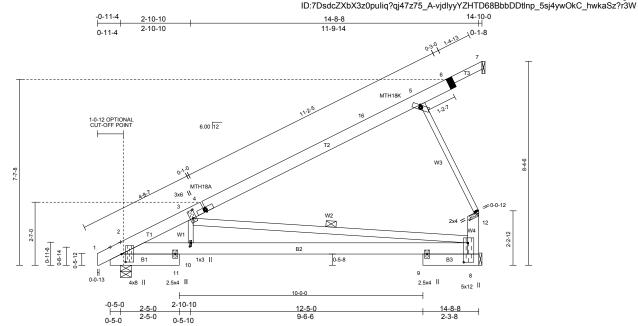


Plate Offsets (X,Y)--SPACING-: 1-4-0 **SPACING-:** 1-0-0 SPACING-1-4-0 CSI. **DEFL** in I/defl L/d **PLATES GRIP** (loc) LOADING (psf) LOADING (psf) Plate Grip DOL 1.15 TC 0.49 Vert(LL) -0.18 8-10 >939 240 MT20 197/144 **TCLL** 23 1 TCLL 30 B 0.58 Vert(CT) -0.37 MT18HS 197/144 Lumber DOL 1.15 BC 8-10 >457 180 (Ground Snow=30.0) (Ground Snow=40.0) Rep Stress Incr YES WB 0.42 Horz(CT) 0.05 8 n/a n/a TCDL 11.0 TCDL 14.7 Code IBC2015/TPI2014 (Matrix-M) Weight: 87 lb **BCLL** 0.0 **BCLL** 0.0

LUMBER-

BCDL

NOTES-

TOP CHORD 2x6 SPF No.2 *Except*

BCDI

6-7: 2x4 SPF No.2 2x6 SPF No.2

BOT CHORD WEBS 2x3 SPF Stud *Except*

10.0

3-8: 2x4 SPF No.2, 8-12: 2x6 SPF Stud

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

APPROVED BY

except end verticals.

BOT CHORD Rigid ceiling directly applied or 9-3-4 oc bracing.

WEBS 1 Row at midpt **JOINTS** 1 Brace at Jt(s): 12

REACTIONS. (lb/size) 2=497/0-5-8, 8=406/Mechanical, 7=0/Mechanical

> Max Horz 2=306(LC 12), 7=-68(LC 19) Max Uplift 2=-234(LC 12), 8=-334(LC 12) Max Grav 2=521(LC 19), 8=475(LC 19)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-966/314, 3-4=-347/25, 4-16=-328/38, 5-16=-195/48, 5-6=-134/51, 6-7=-79/59, 8-12=-293/312

2-11=-613/799, 10-11=-607/798, 9-10=-589/806, 8-9=-577/778 BOT CHORD

3-10=0/323, 3-8=-683/432, 5-12=-341/362 **WEBS**

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws. **David Richter**

FT = 0%

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

6=99/56/40/0, 12=341/363/0/0

- Wind: ASCE 7-10; Vult=152mph (3-second gust) Vasd=120mph @16in o.c.; TCDL=4.4psf; BCDL=4.0psf; (Alt. 176mph @12in o.c.; TCDL=5.9psf; BCDL=5.3psf); h=22ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pg=30.0 psf (ground snow); Ps=23.1 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design
- 5) This truss has been designed for greater of min roof live load of 18.0 psf or 2.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 2 and 319 lb uplift at joint 8.



July 3,2018

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	CMH MANUFACTURING - SCHULT (Rich-NC)	133865459
WPL-913-0315-014_(16W)	9529-15	HINGED TRUSS	1		T9529 - 6/12 9' FLAT (RICH) - 15	133603439

7.640 s Apr 22 2016 MiTek Industries, Inc. Tue Jul 03 07:50:38 2018 Page 2 ID:7DsdcZXbX3z0puliq?qi47z75_A-NvB79HZB2mLzlLAnnxP_J1XGc7QBfrzMDLfH6vz?r3V

NOTES-

- 14) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 15) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

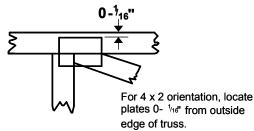


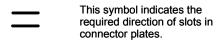
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





^{*} Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89: Design Standard for Bracing.

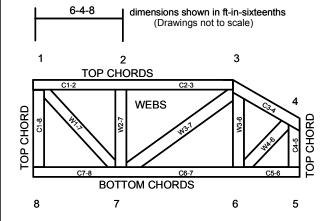
BCSI:

Building Component Safety Information, Guide to Good Practice for Handling,

Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR

truss unless otherwise shown.

APPROVED BY

9. L

7/3/20 19

ESR-3282
Approval of this document does not authorize or approve any deviation or deviations from that applicable State Laws.

David Richter

Trusses are designed for wind loads in the plane of the

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- approve any deviation or deviations from the Plate type, size, orientation and location dimensions requirements of applicable State Laws indicated are minimum plating requirements.
 - Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
 - Top chords must be sheathed or purlins provided at spacing indicated on design.
 - Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
 - 15. Connections not shown are the responsibility of others.
 - Do not cut or alter truss member or plate without prior approval of an engineer.
 - 17. Install and load vertically unless indicated otherwise.
 - Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
 - Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
 - Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

MODULAR PLANS REVIE	EW CHECKLIST
	PAGE 1 of 3 revised May 20
Manufacturer	CMH MANUFACTUING INC.
Model number/name	24
ard Party	NTA INC.
Review Date	7/3/19
Reviewer	DAVID RICHTER
	Plan Sheet Page # and NOTES
QC MANUAL (current and complete)	
APPENDIX B (required and attached)	single family dwelling - not required
PLAN SHEETS	
Each plan sheet third-party stamped with approver's name	
Each plan sheet is numbered and/or indexed	IX-1
GENERAL (cover sheet)	
Code References	1-0
Statement regarding connection to public utilities	1-0
Statement regarding bathrooms if not included	1-0
Construction type	1-0
Occupancy classification	1-0
Fire resistance ratings (if required)	1-0
Floor live load	1-0
Roof live load	1-0
Design wind velocity	1-0
Seismic information (commercial projects)	1-0
Thermal zones	1-0, HDD on REScheck (attached)
Notice to inspections department regarding items to be site	
installed	1-0
FLOOR PLANS	
Interior and exterior wall layouts	1-1
Door and window schedule	1-0.2
Light and Ventilation requriements	TS-1
Attic access (size and locaiton)	1-1
Non-prescriptive headers	Charts on 1-0, calc ref on 1-0
Safety glazing requirements	1-1
Fire rating of Exterior walls (if applicable)	
EXTERIOR ELEVATIONS	
Exterior materials	20-1, 20-2, 1-0.2
Attic ventilation requirements	20-1, 20-2
PLUMBING	
Plan	locations on floor plan 1-1
All fixtures furnished by mfg. shown on plans	1-1
Materials (water supply & distribution, DWV, storm	
drainage)	DWV: 8-1; Supply: 9-1
Supply and waste risers, including DWV system (generic)	
beneath the building	DWV: 8-1; Supply: 9-1
Water heater (type and capacity)	ref to electrical appliances on 1-0
Tracer (type and capacity)	To to otoothou appliances on 1-0

MODULAR PLANS RE	PAGE 2 of 3	revised May
	PAGE 2 013	l Tevised May
	Plan Sheet Page #	and NOTES
<u>MECHANICAL</u>		
Design calculations	attached	
Installed unit capacity	attached	
Supply and returns (locations and sizes)	4-1A	
Duct sizes	4-1A	
Specifications (units, ducts)	1-1, 4-1A	
All appliances furnished by mfg. shown on plans	1-1, exhaust fans 11-1	
ELECTRICAL		
Plan	11-1	
Location of all electrical boxes	11-1	
Electrical panel location	11-1	
Note regarding main disconnect (if applicable)		
Exterior lighting and receptacles	11-1	
Ground level receptacles (if applicable)	11-1	
Smoke detector location(s)	11-1	
Electrical load calculations	TS-5	
Electrical panel layout (breaker and wire sizes, circuit schedule)	11-1	
Panel and service entrance sizes	Panel: 1-0a, SE ref in set-up or	n 1-∩
All fixtures furnished by mfg. shown on plans	11-1	
ACCESSIBILITY		
(for other than 1 & 2 family dwellings)		
Entrances and means of egress		
Doors, doorways, and door hardware		
Stairs and handrails		
Toilet rooms, plumbing fixtures, grab bars, etc		
Bathrooms and shower rooms		
Occupancy specific requirements		
Multi-family dwellings: Type A and B units		
FLOOR X-SECTION		
Joist and beam sizes and spacing	1-0.2	
Materials species and grade	1-0.2	
Sheathing, decking, and concrete as applicable	1-0.2	
astening instructions	1-0.2	
nsulation	1-0.2	
Details as required for clarification	1-0.2, other details ref manual	on 1-0.2
WALL X-SECTION		
Stud and column sizes and spacing	studs: 1-0.2; column charts: 1-0	12
Materials species and grade	1-0.2	·
Sheathing and bracing	1-0.2	
Headers and lintels	header charts: 1-0.2	
Finishes	1-0.2	
Fastening instructions	1-0.2	
nsulation	1-0.2	

MODULAR PLANS REVI	PAGE 3 of 3	revised l
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	Plan Sheet Page # and NOTES	
CEILING/ROOF X-SECTION		
Truss, rafter, and beam spacing	1-0.2	
Lumber species and grade	1-0.2	
Sheathing and decking	1-0.2	
Finishes	1-0.2	
Fastening instructions	1-0.2	
Insulation	1-0.2	
Details including NC sealed truss designs or manual		
reference	man ref to trusses 1-0.2, other deta	ils man ref
FOUNDATION PLAN		
Footings, pier, and curtain wall locations and specifications	21-30 PSF (OFF FRAME) / 21-PF (ON FRAME	
X-sections with dimensions	21-30 PSF (OFF FRAME) / 21-PF (ON FRAME	
Anchorage - sill plate to piers and curtain wall	21-30 PSF (OFF FRAME) / 21-PF (ON FRAM	
Anchorage - building to sill plate	21-30 PSF (OFF FRAME) / 21-PF (ON FRAM	
Anchorage - tie downs (lateral and longitudinal)	21-30 PSF (OFF FRAME) / 21-PF (ON FRAM	
Soil bearing capacity	21-30 PSF (OFF FRAME) / 21-PF (ON FRAM	
Minimum concrete compressive strength	21-30 PSF (OFF FRAME) / 21-PF (ON FRAM	
Motar type	21-30 PSF (OFF FRAME) / 21-PF	
Ventilation requirements (with and without vapor barrier)	24 20 DOE (OFF FDAME) / 04 DE	· /ON EDAN
Crawl space access requirements	21-30 PSF (OFF FRAME) / 21-PF 21-30 PSF (OFF FRAME) / 21-PF	ON FRAIN
ENERGY COMPLIANCE		
Demonstrate compliance	PDECORIDE IN	
Demonstrate compliance	PRESCRIPTIVE	
SET-UP INSTRUCTIONS		
Floor and ceiling connections	ref to set-up manual on 1-0.2	
Marriage wall connections	ref to set-up manual on 1-0.2	
Roof set-up connections	ref to set-up manual on 1-0.2	
Plumbing connections	ref to set-up manual on 1-0.2	
Mechanical connections	ref to set-up manual on 1-0.2	
Electrical connections	ref to set-up manual on 1-0.2	
Fire stopping	1-0.2	
Air infiltration elimination	ref to set-up manual on 1-0.2	
Notice to inspections department attachment if set-up		
instructions are by attachment	1-0.2	
TEMS NOT INSPECTED IN PLANT		
List of items not inspected by 3rd. Party	1-0.2	
Notice to inspections department	1-0.2	