

REACTIONS. (lb/size) 2=1120/0-3-8 (min. 0-1-8), 8=1120/0-3-8 (min. 0-1-8)

Max Horz 2=-185(LC 6)

Max Uplift2=-41(LC 8), 8=-41(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3= 3671/0, 3-4=-2683/0, 4-5=-2574/0, 5-6=-2574/0, 6-7=-2684/0,

7-8=-3671/0

BOT CHORD 2-12+0/3179, 11-12=0/3205, 10-11=0/3205, 8-10=0/3176

WEBS 5-11=0/2672, 7-11=-869/234, 3-11=-869/234

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=20ft; B=29ft; L=26ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

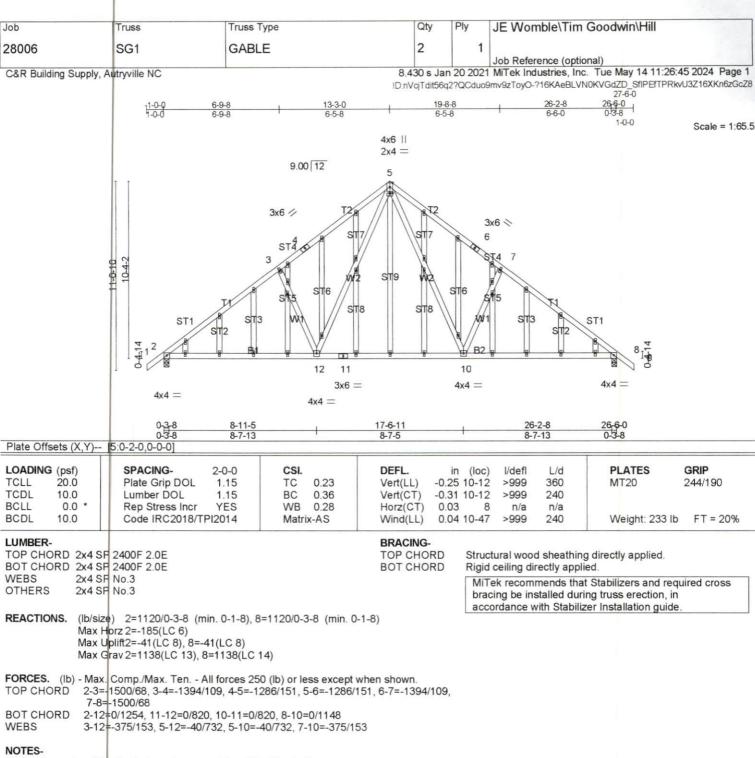
5) Bearing at joint(s) \$\frac{1}{2}\$, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=20ft; B=29ft; L=26ft; eave=4ft; Cat. II; Ekp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber ΦOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

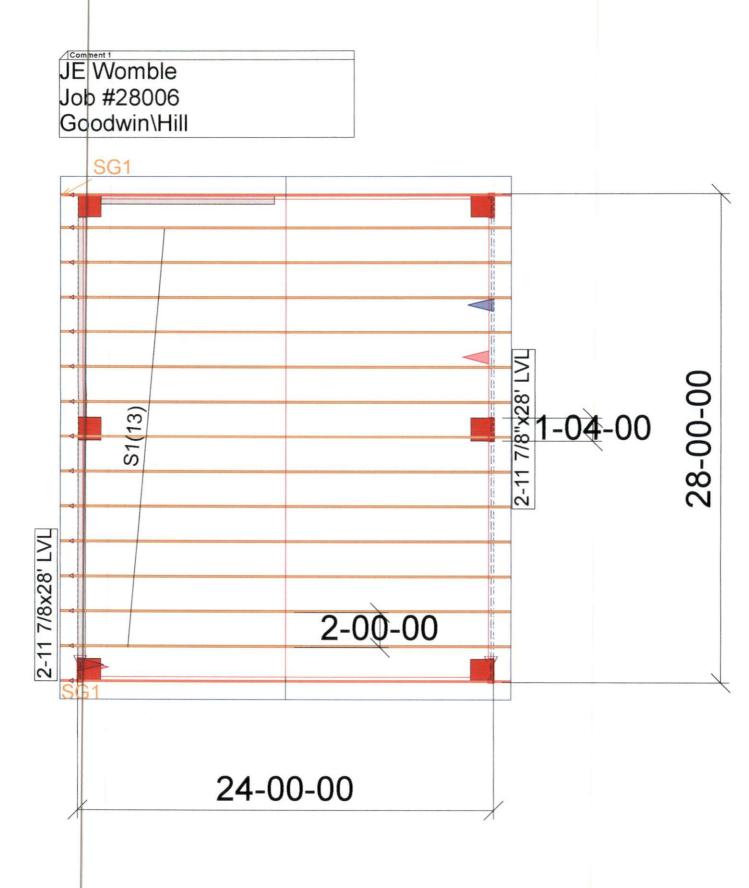
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



Client: C&R Building Supply Date: 5/14/2024 Project: Input by: Nicholas DeGuzman isDesign Address Job Name Project # Level: Level 1.750" X 11.875" Murphy 2.0E-3100F LVL **B1** 2-Ply - PASSED 1 SPF End Grain 0-3-8 2 SPF End Grain 0-3-8 Member Information Reactions PATTERNED Ib (Uplift) Type Girde Brg **Application** Floor Direction Live Dead Snow Wind Plies: Design Method: ASD 1870 0 1785 1 Vertical 0 Moisture Condition: Dry Building Code: IRC 2021 Vertical 0 1870 1785 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F General Load Bearings 40 PSF Floor Live: Dir. Bearing Length Cap. React D/L lb Total Ld. Case 15 PSF Dead: 1 - SPF 3.500" Vert 40% 1870 / 1785 3655 1 End Grain **Analysis Results** 2 - SPF 3.500" 40% 1870 / 1785 3655 L Vert Analysis Actual Location Allowed Capacity Comb Case End 11968 ft-I 7' 24484 ft-lb Moment 49% D+S L Grain Unbraced 11968 ft-Ib 7' 11976 ft-lb 100% D+S Shear 2986 lb 1'3 3/8" 9241 lb D+S LL Defl inch 0.198 (L/823) 7' 1/16" 0.451 (L/360) 44% S TL Defl inch 0.404 (L/402) 7' 1/16" 0.677 (L/240) 60% D+S **Design Notes** 1 Provide support to prevent lateral movement and rotation at the end bearings. 2 Fasten all plies using prows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6" 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported on the bottom edge only. 5 Top loads must be supported equally by all plies 6 Top must be laterally braced at a maximum of 7'1 11/16" o.c. 7 Bottom must be laterally braced at end bearings.

o Lateral sienderness ratio based on single ply width.											
ID	Load Type	e Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	255 PLF	0 PLF	255 PLF	0 PLF	0 PLF	S1 Truss	
1	Self Weigh	nt			12 DI E						

Calculated Structured Designs is resignsible only of the structural adequacy of this component based on the structural adequacy of this component based on the design criteria and loadings sillown. It is the responsibility of the customer and/of the contractor to 2. Refer to manufacturer's regarding installation regarding installation refeated application, and to verify the dimensions and loads.

Lumber

- LVL beams must not be cut or drilled
 Refer to manufacturer's product informat regarding installation requirements, multi-fastening details, beam strength values, and co
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restral
 Provide lateral support at bearing points
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Murphy Engineered Wood Products 412 West Central Sutherlin, OR 97479 (541) 459-4545 www.murphyplywood.com APA: PR-L283, ICC-ES: ESR-2913

Eastern Engineered Wood Products 1245 Easton Road, PA 18015



Page 1 of 2

11 7/8"

Const

Ld. Comb.

D+S

D+S

0

0

3 1/2"

This design is valid until 2/14/2027





Client: C&R Building Supply Date: 5/14/2024 Page 2 of 2 Project: Input by: Nicholas DeGuzman isDesign Address Job Name: Project # Level: Level Murphy 2.0E-3100F LVL 1.750" X 11.875" **B1** 2-Ply - PASSED 11 7/8" 3 1/2" 1 SPF End Grain 0-3-8 2 SPF End Grain 0-3-8 Multi-Ply Analysis Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6". 0.0 % 0.0 PLF Load Yield Limit per Foot 181.1 PLF Yield Limit per Fastener 90.5 lb. CM Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00 Manufacturer Info Eastern Engineered Wood Notes chemicals For flat roofs provide proper drainage to prevent ponding Murphy Engineered Wood Products 412 West Central Sutherlin, OR 97479 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the irlended application, and to verify the dimensions and loads. **Products** Handling & Installation and ling. & Installation

LVL beams must not be cut or drilled.

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

Damaged Beams must not be used.

Design assumes top edge is laterally restrained.

Provide lateral support at bearing points to avoid lateral displacement and rotation. 1245 Easton Road, PA 18015 (541) 459-4545 www.murphyplywood.com APA: PR-L283, ICC-ES: ESR-2913 Lumber Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

This design is valid until 2/14/2027