

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 24052574 BCTH-31

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by The Building Center.

Pages or sheets covered by this seal: I65584968 thru I65584973

My license renewal date for the state of North Carolina is December 31, 2024.

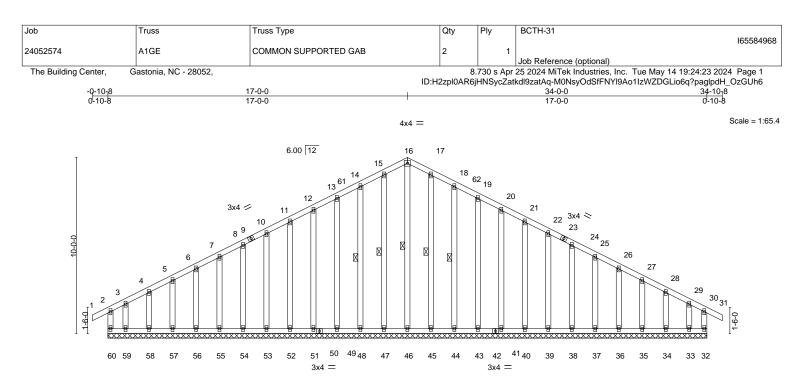
North Carolina COA: C-0844



May 16,2024

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		<u> </u>							
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.09 WB 0.10 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.00 -0.00	(loc) 31 31 32	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 310 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2			<b>ACING-</b> P CHORD	Structural			g directly app	plied or 6-0-0 oc purlins	δ,

WEBS

BOLCHORD	2X4 SP N0.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 16-46, 15-47, 14-48, 17-45, 18-44

REACTIONS. All bearings 34-0-0.

(lb) -Max Horz 60=-175(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except 60=-135(LC 14), 59=-136(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 60, 32, 46, 47, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 59, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-85/272, 13-14=-95/299, 14-15=-106/330, 15-16=-109/341, 16-17=-109/336,

17-18=-106/325, 18-19=-95/293, 19-20=-85/266

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 17-0-0, Corner(3) 17-0-0 to 20-4-13, Exterior(2) 20-4-13 to 34-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 1-4-0 oc.

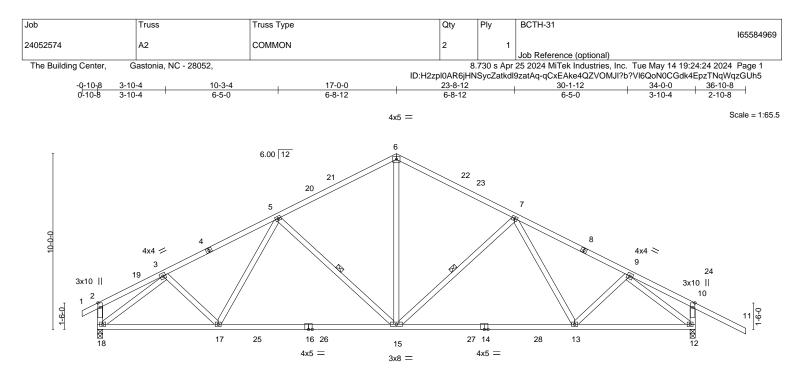
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 12) will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except (jt=lb) 60=135, 59=136.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



	6-10-6		17-0-	17-0-0 27-1-				1	34	34-0-0	
	6-10	)-6	10-1-1	10	1	10	-1-11	1	6-1	10-6	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 15 TCDL BCLL BCDL	20.0	SPACING Plate Grip Lumber D Rep Stres Code IRC	DOL 1.15 OL 1.15	<b>CSI.</b> TC BC WB Matri	0.71 0.83 0.84 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.24 13-15 -0.46 15-17 0.08 12	l/defl >999 >877 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 203 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2 WEBS 2 REACTIONS.	2x4 SP No.2 2x4 SP No.2 *Exc 14-16: 2x4 SP No.2 2x4 SP No.3 (size) 18=0- Max Horz 18=-1 Max Uplift 18=-3 Max Grav 18=14	3-8, 12=0-3-8 87(LC 14) 3(LC 16), 12=-8				BRACING- TOP CHORD BOT CHORD WEBS	Structural wood except end vert Rigid ceiling dir 1 Row at midpt	icals. ectly app	0 7 11		ns,
FORCES. (Ib) - TOP CHORD BOT CHORD WEBS	3-5=-1821/133, 17-18=-5/1538,	5-6=-1429/175 15-17=0/1606,	ces 250 (lb) or less exc , 6-7=-1429/164, 7-9=- 13-15=0/1507, 12-13= 9-13=0/277, 5-15=-495	1786/114, 10 0/1359	)-12=-273/	169					

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -0-10-8 to 2-6-5, Interior(1) 2-6-5 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13, Interior(1) 20-4-13 to 36-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60

& MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) All plates are 3x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 12.



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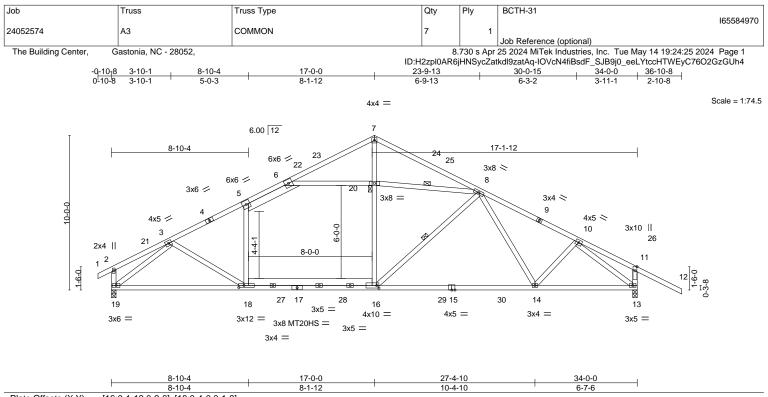


Plate Offsets (2	X,Y) [16:0-1-1	2,0-2-0], [18:0-4-0,0-1-8]							
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.77 BC 0.80 WB 0.92 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT		l/defl >999 >908 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 235 lb	<b>GRIP</b> 244/190 187/143 FT = 20%
LUMBER- TOP CHORD	2x4 SP No.1 *E: 1-4,9-12: 2x4 SI	1		BRACING- TOP CHORD	Structural wood except end vert		g directly app	plied or 2-5-6 oc purlins	,
BOT CHORD	2x4 SP No.2 *E	1		BOT CHORD	Rigid ceiling dire	ectly appl			
WEBS	2x4 SP No.3 *E	№.1, 15-17: 2x4 SP DSS xcept* 2x4 SP No.2, 5-6: 2x6 SP No.1		WEBS JOINTS	1 Row at midpt 1 Brace at Jt(s):		8-16, 8-20		
REACTIONS.	Max Horz 19=- Max Uplift 13=-						an	RTH CARO	No -
FORCES. (Ib TOP CHORD BOT CHORD WEBS	3-5=-2064/0, 5 18-19=0/1724 16-20=0/586,	1ax. Ten All forces 250 (lb) or less exi 5-61851/37, 6-7872/0, 7-8876/0, i , 16-18=0/1853, 14-16=0/1691, 13-14= 7-20=-30/541, 8-16=-243/381, 10-14=0 25, 8-20=-1295/125, 3-19=-1996/0	8-10=-1959/25, 11-13=- 0/1499					SEAL 036322	annan a
2) Wind: ASCE	7-10; Vult=120m	ave been considered for this design. nph Vasd–95mph; TCDL=6.0psf; BCDL al) and C-C Exterior(2) -0-10-8 to 2-6-5						C A. GILBE	ALINE

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -0-10-8 to 2-6-5, Interior(1) 2-6-5 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13, Interior(1) 20-4-13 to 36-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) 150.0lb AC unit load placed on the bottom chord, 12-10-4 from left end, supported at two points, 4-0-0 apart.

7) All plates are MT20 plates unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13.

May 16,2024



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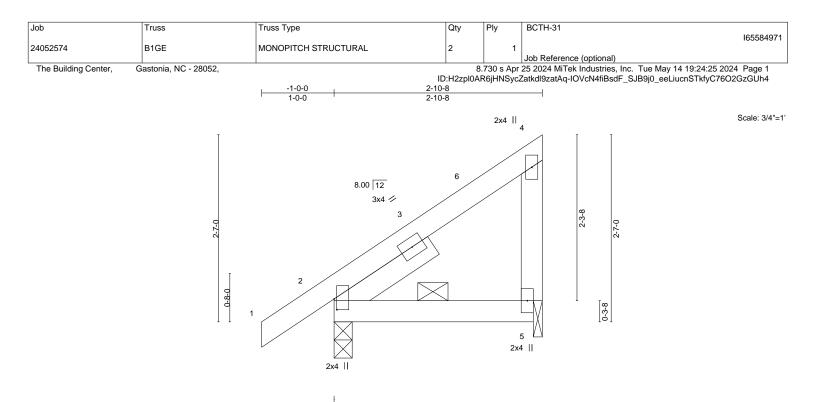


Plate Offsets (X,Y) [2:0-1-13	3,0-0-7]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.08 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.01 2-5 -0.01 2-5 0.00	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.	3 1-6-13	TC	e	Structural wood except end verti 1-6-0 oc bracing	cals.	g directly ap	oplied or 2-10-8 oc purl	ins,

REACTIONS. (size) 2=0-3-0, 5=0-1-8 Max Horz 2=65(LC 14) Max Uplift 2=-33(LC 14), 5=-40(LC 14) Max Grav 2=180(LC 2), 5=98(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-8-12 zone; cantilever left exposed ; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.



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A MiTek At 818 Soundside Road

Edenton, NC 27932

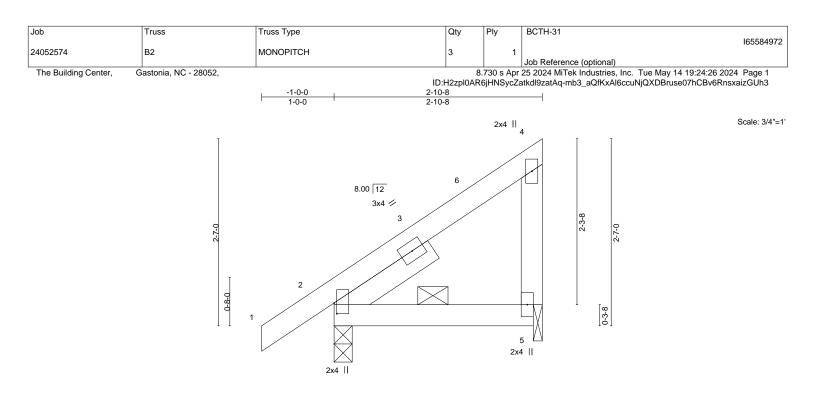


Plate Offsets (X,Y)	[2:0-1-13,	0-0-7]										
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 15.4 TCDL BCLL BCDL	20.0 1/20.0 10.0 0.0 * 10.0	<b>SPACING-</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.13 0.08 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 0.00	(loc) 2-5 2-5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x <sup>2</sup> WEBS 2x <sup>2</sup>	4 SP No.2 4 SP No.2 4 SP No.3 ft 2x4 SP No.3	1-6-13			Т		Structura except e 1-6-0 oc	nd verti	cals.	ng directly a	pplied or 2-10-8 oc purli	ins,

REACTIONS. (size) 2=0-3-0, 5=0-1-8 Max Horz 2=65(LC 14) Max Uplift 2=-33(LC 14), 5=-40(LC 14) Max Grav 2=180(LC 2), 5=98(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

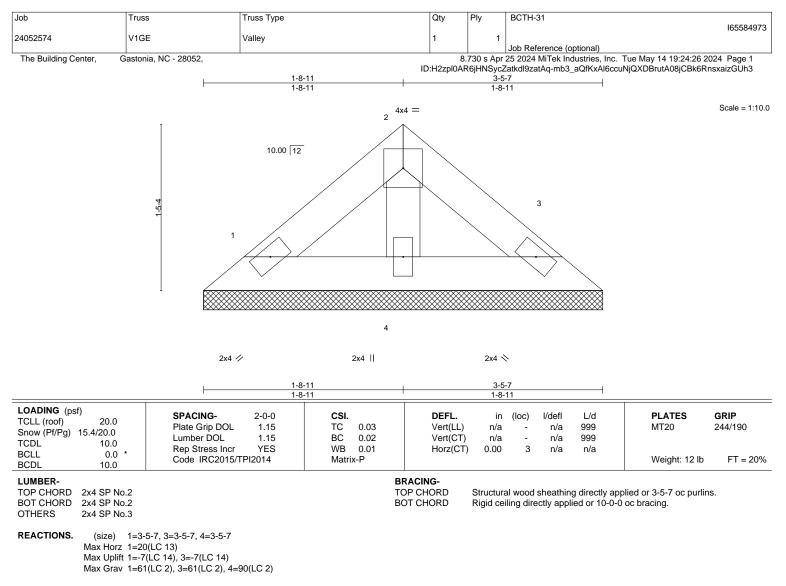
 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-8-12 zone; cantilever left exposed; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Edenton, NC 27932

