

THOMPSON AND ASSOCIATES

1149 Executive Circle
Suite D-2
Cary, North Carolina 27511
(919) 465-1566



December 8, 2023

To: Mr. Brian Davis

Re: Bryant Residence
1048 Jackson Road
Fuquay-Varina, North Carolina 27526


Dear Mr. Davis:

As per your request, a field inspection was conducted at 1048 Jackson Road in Fuquay-Varina, North Carolina on December 8, 2023. The purpose of this field inspection was to investigate the pad construction located on the property associated with the construction of a proposed metal out building.

Upon initial inspection, the earthen area of the building pad was excavated. The organic material of the pad area had been stripped from the pad area and placed to the rear, outside of the pad area. The pad area was constructed out of earthen fill material borrowed on site adjacent to the pad area. The fill material, via visual inspection was found to be adequate. The area of the constructed building pad was probed with a 1/2" steel rod. All areas of the constructed building pad were found to be adequate. Based upon this inspection, it has been determined that the building pad area is adequately compacted and is ready to have the concrete slab poured.

I trust that this information will assist you in the completion of your project. If I can be of any further assistance please feel free to contact me.

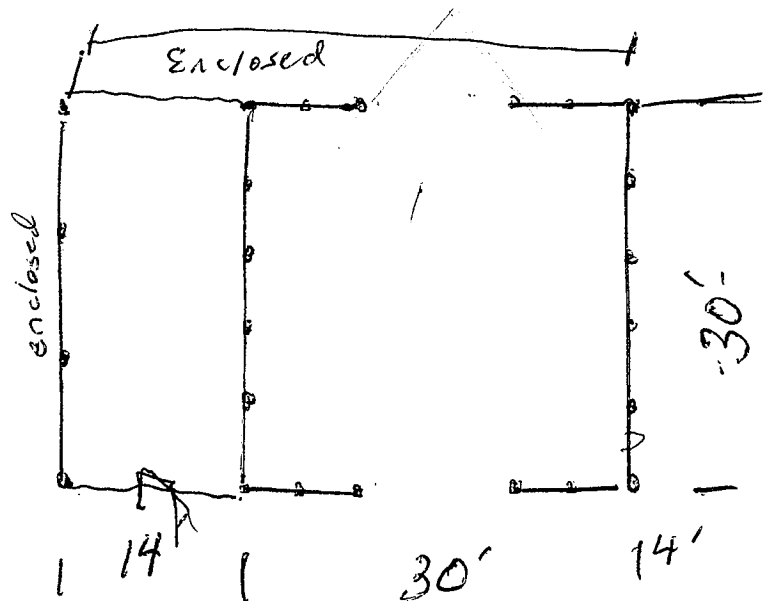
Sincerely,

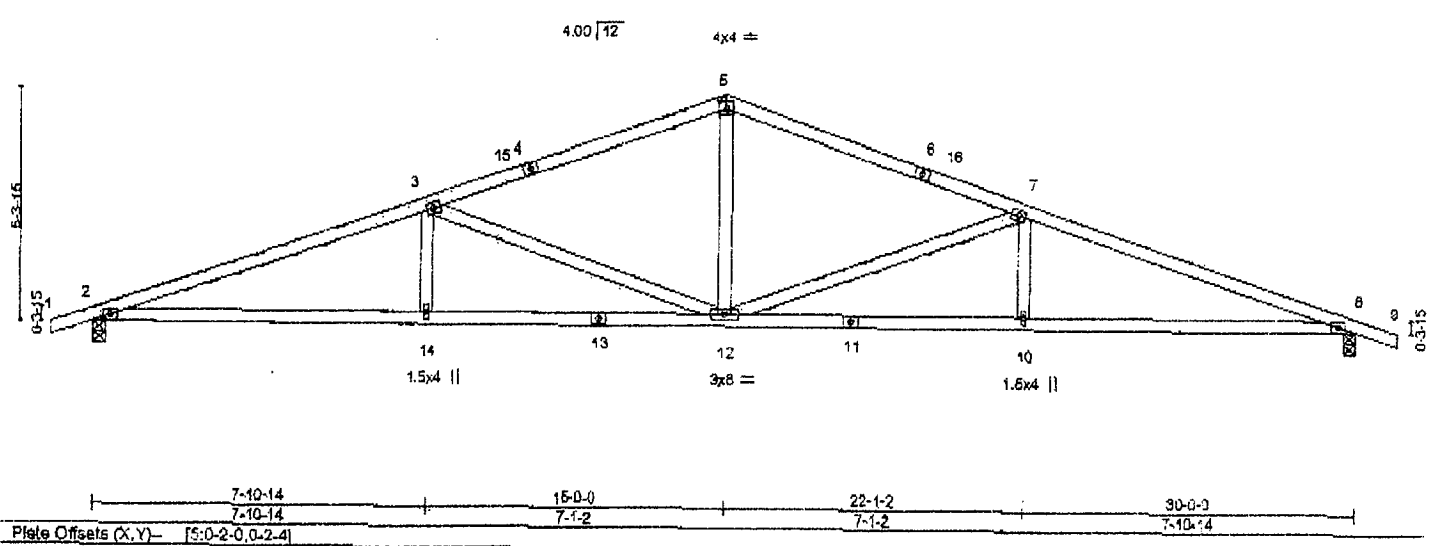
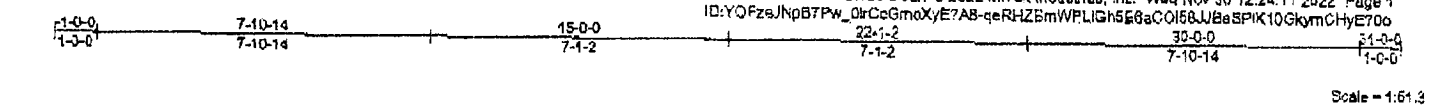

Randall L. Miller, PE
Thompson and Associates, PA
Lisc# C-0343



30X30X12'H BUILDING
WITH 14X30 SIDE SHELTER
RAY BRYANT: BUILDER

6X6 POST: 6' O.C. ON BUILDING
6X6 POST: 10' O.C. ON SHELTER
ALL HOLES 12" X 36" DEEP
6" CONMIX FOOTING + BACK FILL
TRUSS 2' O.C.
2X8 SHELTER RAFTERS 2' O.C.
2X4 PURLINS TOP + SIDES
2- 2X10 PLATES ON BUILDING
2- 2X12 PLATES ON SHELTER
THROUGH BOLTS ON PLATES
2.5 HUR. TIES ON TRUSS + RAFTERS
29 GA. METAL TOP + SIDES





LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.56	in (loc) l/def L/d	MT20	244/190
Snow (P/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.60	Vert(LL) -0.17 12 >999 240		
TCDL 5.0	Lumber DOL 1.15	WB 0.83	Vert(CT) -0.26 12-14 >999 180		
BCLL 0.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.09 8 n/a n/a		
BCDL 5.0	Code IRC2018/TP12014			Weight: 131 lb	FT = 20%

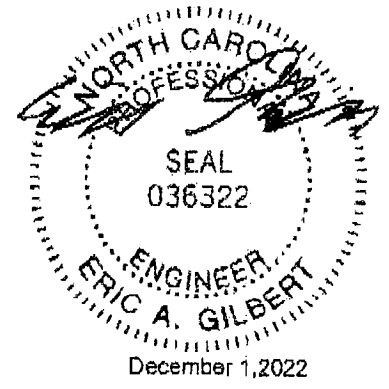
LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3

BRACING-
 TOP CHORD Sheathed or 3-5-4 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=0-3-8
 Max Horz 2=55(LC 11)
 Max Uplift 2=154(LC 12), 8=154(LC 12)
 Max Grav 2=949(LC 2), 8=949(LC 2)

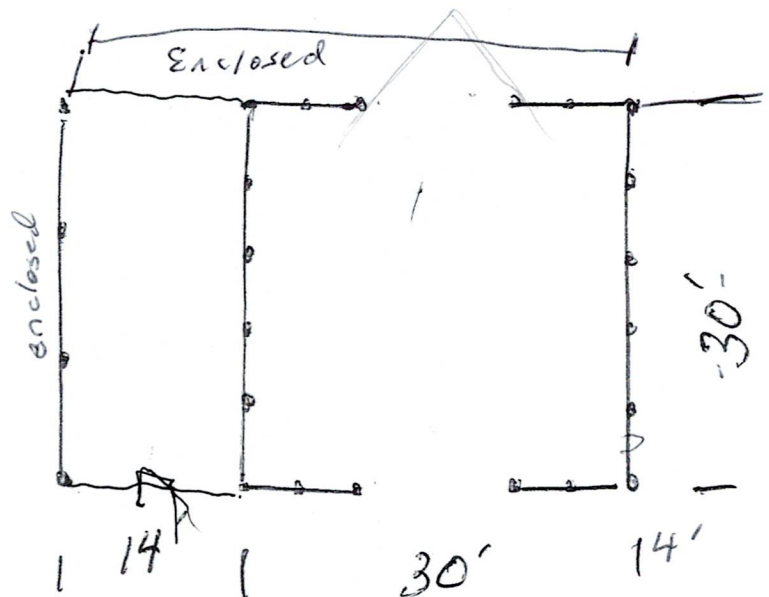
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2196/307, 3-5=-1473/231, 5-7=-1473/231, 7-8=-2196/307
 BOT CHORD 2-14=-248/2024, 12-14=-248/2024, 10-12=-248/2024, 8-10=-248/2024
 WEBS 5-12=-64/584, 3-12=-757/134, 7-12=-757/134

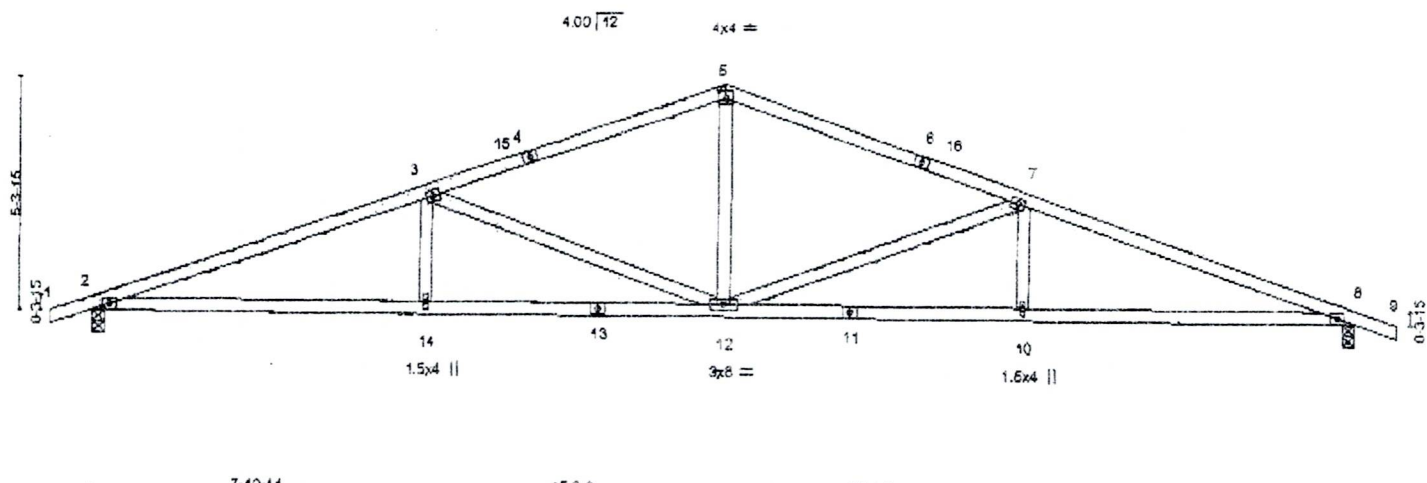
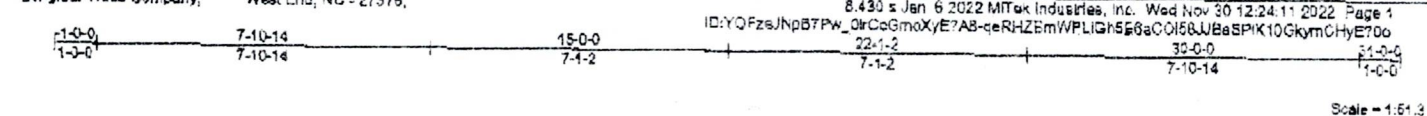
- NOTES.**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=3.0psf, BCCL=3.0psf, h=12ft, B=45ft, L=30ft, eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; P=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=16.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Ia=1.0; Rough Cat B; Partially Exp.; Ce=1.0, Cs=1.00; Cl=1.0
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
 - All plates are 3x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (1=1b) 2=154, 8=154.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1.



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TODL	5.0	Rep Stress Incr	YES	WB	0.83	Vert(CT)	-0.26	12-14	>999	190	
BCLL	0.0	Code IRC2018/TPI2014		Matrix-S		Horz(CT)	0.08	8	n/a	n/a	
BCDL	5.0										Weight: 131 lb FT = 20%

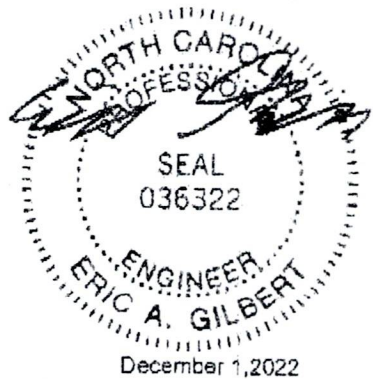
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 - TOLL ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); le=1.0; Rough Cat B; Partially Exp.; Ce=1.0 Cs=1.00; Ct=1.0
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December 1, 2022