THOMPSON AND ASSOCIATES

1149 Executive Circle Suite D-2 Cary, North Carolina 27511 (919) 465-1566



December 8, 2023

To: Mr. Brian Davis

Re: Bryant Residence 1048 Jackson Road Fuguay-Varina, North Carolina 27526

Dear Mr. Davis:

As per your request, a field inspection was conducted at 1048 Jackson Road in Fuquay-Varina, North Carolina on December 8, 2023. The purpose of this field inspection was to investigate the pad construction located on the property associated with the construction of a proposed metal out building.

Upon initial inspection, the earthen area of the building pad was excavated. The organic material of the pad area had been stripped from the pad area and placed to the rear, outside of the pad area. The pad area was constructed out of earthen fill material borrowed on site adjacent to the pad area. The fill material, via visual inspection was found to be adequate. The area of the constructed building pad was probed with a 1/2" steel rod. All areas of the constructed building pad were found to be adequate. Based upon this inspection, it has been determined that the building pad area is adequately compacted and is ready to have the concrete slab poured.

I trust that this information will assist you in the completion of your project. If I can be of any further assistance please feet free to contact me.

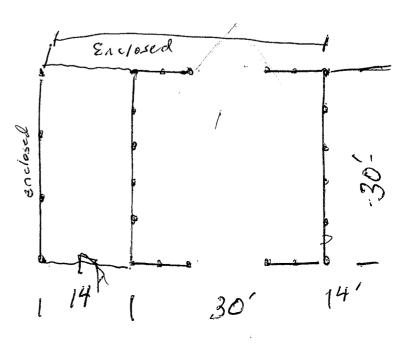
Sincerely

Randall L. Miller, PE 9793 Thompson and Associates, F

Lisc# C-0343

30×30×12'H BUILDING WITH 14×30 SIDE SHELTER RAY BRYANT: BUILDER

6x6 POST: 6'O.C. ON BUILDING 6x6 POST: 10'O.C. ON SHELTER ALL HOLES 12"X36" DEEP 6" CONMIX FOOTING + BACK FILL TRUSS 2'O.C. 2X8 SHELTER RAFTERS 2'0.C. 284 AURLINS TOP + SIDES 2-2X10 PLATES ON BUILDING 2-2×12 PLATES ON SHELTER THROUGH BOLTS ON PLATES 2.5 HUR. TIES ON TRUSS + RAFTERS 29 GA. METAL TOP + SIDES





RE: T22-11059 - BRYANT

Site Information:

Project Customer:

Project Name:

Lot/Block:

Subdivision:

Model:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014

Wind Code: ASCE 7-16 [AVIIII Spirits ii: 130 mph

Roof Load: 40.0 psf

Mean Roof Height (feet): 12

No. Seal#

Truss Name Date 155500986 T01 12/1/22 Design Program: MiTek 20/20 8.3

Design Method: MWFRS (Directional) ASCE 7-16 [All Heights]

Trenco

818 Soundside Rd

Edenton, NC 27932

Floor Load: N/A psf

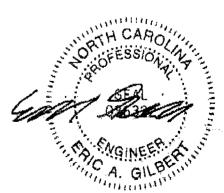
Exposure Category: B

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Longleaf Truss Company.

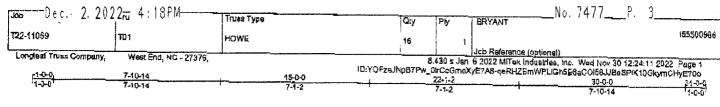
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022.

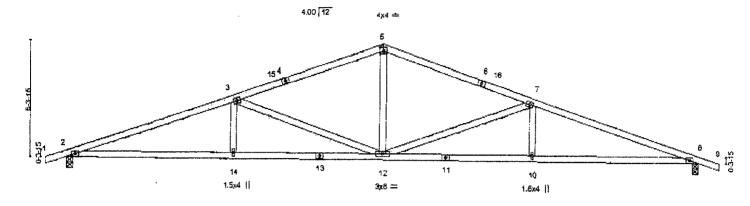
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters. shown (e.g., loads; supports, dimensions, shapes and design codes), which were given to MTek or TRENCO. Any project specific information included is for MTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



December 1,2022



Scale - 1:61.3



- 7-10 7-10		15-0-0 7-1-2	22-1-2 7-1-2	· · · · · · · · · · · · · · · · · · ·	30-0-0 7-10-14	
Plate Offsets (X,Y)- [5:0-2-0,0-	2-4			·····	(510474	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 5.0 BCLL 0,0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.56 BC 0.50 WB 0.83	DEPL. in () Vert(LL) -0.17 Vert(CT) -0.26 12 Horz(CT) 0.09	loc) l/defi L/d 12 >999 240 -14 >999 190 B n/a n/a	PLATES MT20	GR(P 244/190
BCDL 5.0	Code IRC2018/TPI2014	Metrix-S			Weight: 131 lb	FT = 209

BRACING.

TOP CHORD

BOY CHORD

Sheathed or 3-5-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER.

REACTIONS.

TOP CHORD BOT CHORD 2x4 SP No.1 2x4 SP No.1

WEBS 2x4 SP No.3

(size) 2=0-3-8, 8=0-3-8

Max Horz 2=58(LC 11) Max Uplift 2=154(LC 12), B=-154(LC 12) Max Grev 2=949(LC 2), 8=949(LC 2)

FORCES. (lb) - Mex. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2196/307, 3-5=-1473/281, 5-7=-1473/231, 7-8=-2198/307 TOP CHORD BOT CHORD 2-14=-248/2024, 12-14=-248/2024, 10-12=-248/2024, 8-10=-248/2024

WEBS 5-12=-64/584, 3-12=-757/134, 7-12=-757/134

1) Unbalanced roof live loads have been considered for this design.

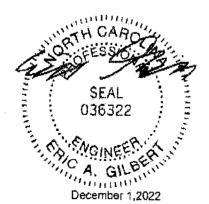
- 2) Wind: A3CE 7-16; Vuli=120mph (3-second gust) Vasd=85mph; TCDL=3.0psf; BCDL=3.0psf; h=12ft; B=45ft; L=30ft; eqvs=4ft; Cat. il; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 clate orio DOL≃1.60
- 3) TCLL: ASCE 7-15; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOC=1.15); le=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Ce=1.00; Cl=1.70 4) Unbalanced snow loads have been considered for his design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs hon-concurrent with other live loads.
- 6) Dead leads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adecuacy of top chord dead load.

Ti All plates are 3x4 MT20 unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chand live load nonconcurrent with any other live loads.

- 9) * This truss has been designed for a live load of 20,0ps/ on the bottom chord in all areas where a rectangle 3-8-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plats capable of withstanding 100 to uplift at joint(s) except ((t=b) 2=154, 8=154.
- 11) This truss is designed in accordance with the 2016 international Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



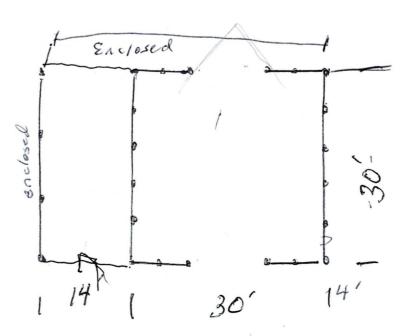
A WARNING . VOID Code: Assumptions and READ NOTES ON THIS AND INCLUDED MITCH REFERENCE PAGE M1-7473 (etc. \$158,2020 BEFORE USE. Owings will for use only with MT-effe connectors. This design is bessed only upon personnectors shown, and is for an individual building component, not a most system. Before use, the building design of most yet to design be most only upon personnectors shown, and is for an individual building component, not a most system. Before use, the building design of most verify the applicability of design personalists and properly incorporate byte design personal building design for the overall building design design. Building design of individual truss with another only. Additional temporary and personal brinding a shivey explains for stability and to provide for design personal register of the design personal success and properly despised. For general guidance regarding the fathication, shrape, delivery, erection and bracker of trustees and trust systems, see

ARRIVED Caralty Criteria, DSB-6\$ and BCSI Building Component Safety Information evaluable from Truss Flate Institute, 267 or out Highway, Suite 203 Weldorf, MD 20601



30×30×12'H BUILDING WITH 14×30 SIDE SHELTER RAY BRYANT: BUILDER

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No. Seal# Truss Name Date 1 155500986 T01 12/1/22

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Truss Design Engineer's Name: Gilbert, Eric

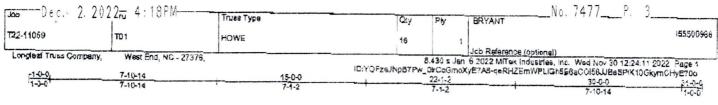
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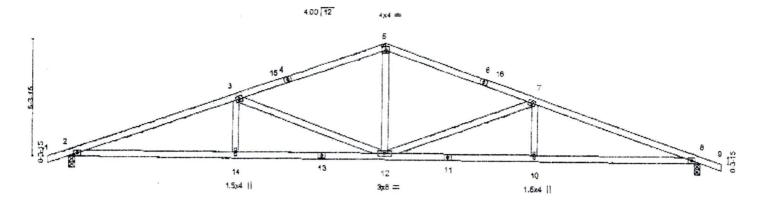


December 1,2022

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Scale - 4:54 3



7-10 7-10 Plete Offsets (X,Y) [5:0-2-0,0	14	15-0-0 7-1-2	22-1-2 7-1- 2		30-0-0 7-10-14	
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BOT CHORD

Sheathed or 3-5-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER.

REACTIONS.

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2x4 SP No.1 BOT CHORD

WEBS 2x4 SP No.3

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NOTES.

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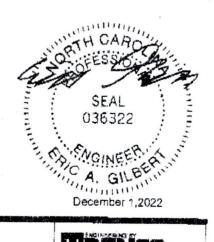
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