

Trenco 818 Soundside Rd Edenton, NC 27932

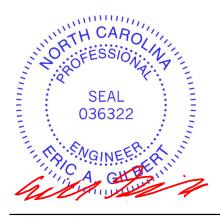
Re: 24031274 BCTH-36

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by The Building Center.

Pages or sheets covered by this seal: I64319425 thru I64319436

My license renewal date for the state of North Carolina is December 31, 2024.

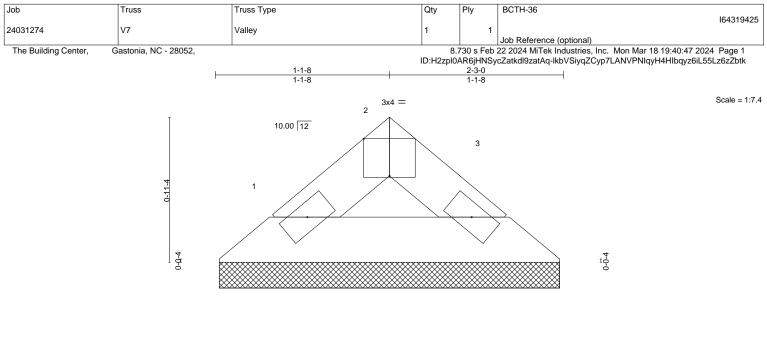
North Carolina COA: C-0844



March 20,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



2x4 1/

2x4 🚿

0-<u>0-5</u> 0-0-5 2-3-0 2-2-11

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.01 BC 0.02 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	n/a n/a	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 6 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2		TC						pplied or 2-3-0 oc pur 0 oc bracing.	lins.

Plate Offsets (X,Y)-- [2:0-2-0,Edge]

REACTIONS. (size) 1=2-2-6, 3=2-2-6 Max Horz 1=-11(LC 12)

Max Grav 1=58(LC 2), 3=58(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

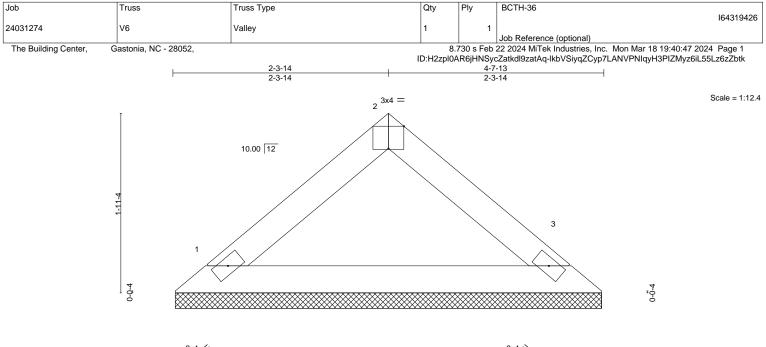
2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a fuss system. Derive use, the building designer host verify the applications of design had been and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





2x4 🥢

2x4 🚿

4-7113

Rigid ceiling directly applied or 10-0-0 oc bracing.

		4-7-8				0-0-5		
Plate Offsets (X,Y) [2:0-2-0,E	dge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.06 BC 0.18 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			CING- CHORD	Structural wood	sheathin	g directly app	lied or 4-7-13 oc purl	ins.

BOT CHORD

4-7-8

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=4-7-3, 3=4-7-3

Max Horz 1=-29(LC 12) Max Uplift 1=-1(LC 14), 3=-1(LC 14)

Max Grav 1=154(LC 2), 3=154(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

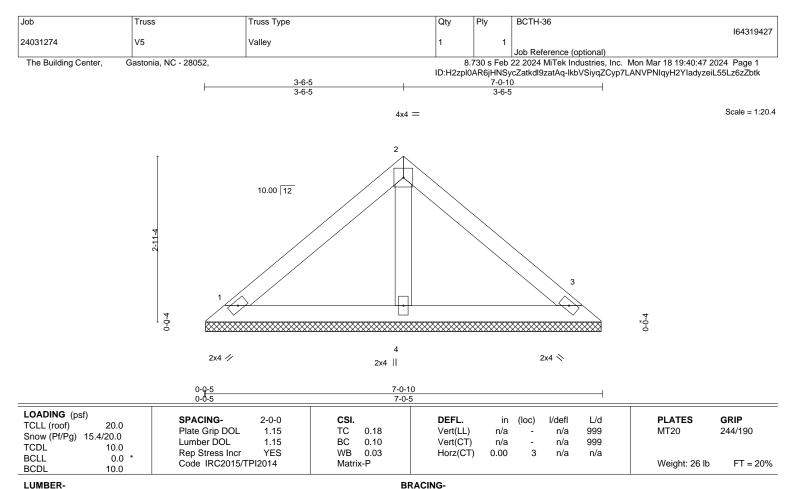
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design and the second way the approximation of design and the second and and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=7-0-0, 3=7-0-0, 4=7-0-0

Max Horz 1=47(LC 13)

Max Uplift 1=-17(LC 14), 3=-17(LC 14)

Max Grav 1=143(LC 2), 3=143(LC 2), 4=213(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

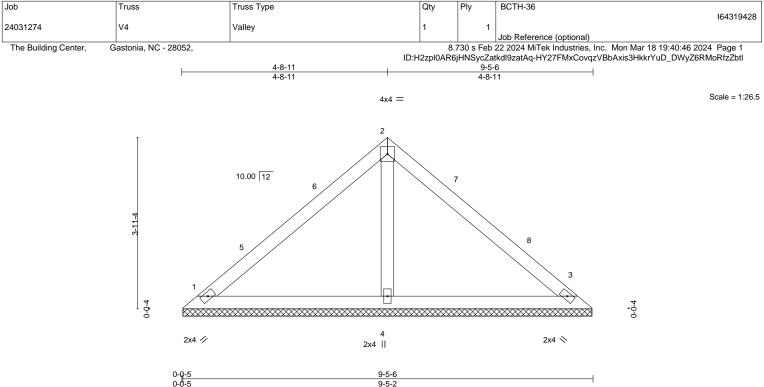
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



	0-0-5	9-5-2							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.19 WB 0.06 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 36 lb	GRIP 244/190 FT = 20%
BCDL 10.0			ACING-					Weight. 30 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=9-4-13, 3=9-4-13, 4=9-4-13 Max Horz 1=-65(LC 12)

Max Uplift 1=-15(LC 14), 3=-15(LC 14)

Max Grav 1=183(LC 2), 3=183(LC 2), 4=325(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-8-11, Exterior(2) 4-8-11 to 7-8-11, Interior(1) 7-8-11 to 9-0-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

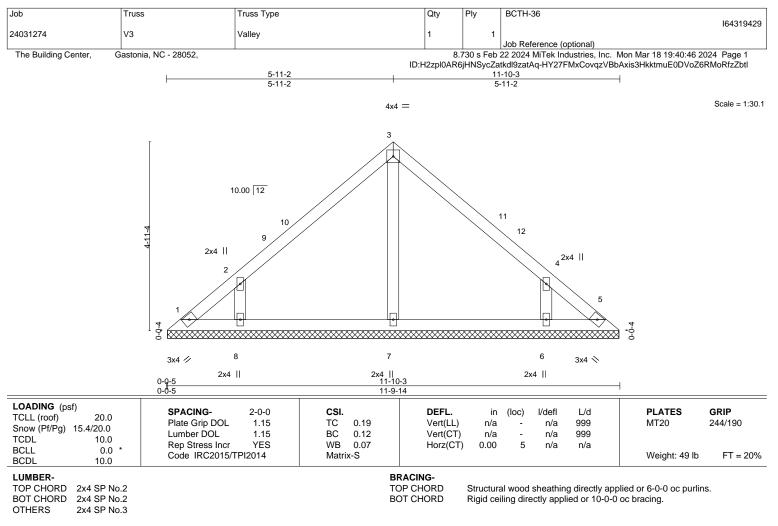
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a fuss system. Derive use, the building designer index very the applications of design had very the applications of design index very the applications of design index very the application of the applicat and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



REACTIONS. All bearings 11-9-10.

(lb) - Max Horz 1=83(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=301(LC 23), 6=301(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 5-11-2, Exterior(2) 5-11-2 to 8-11-2, Interior(1) 8-11-2 to 11-5-6 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

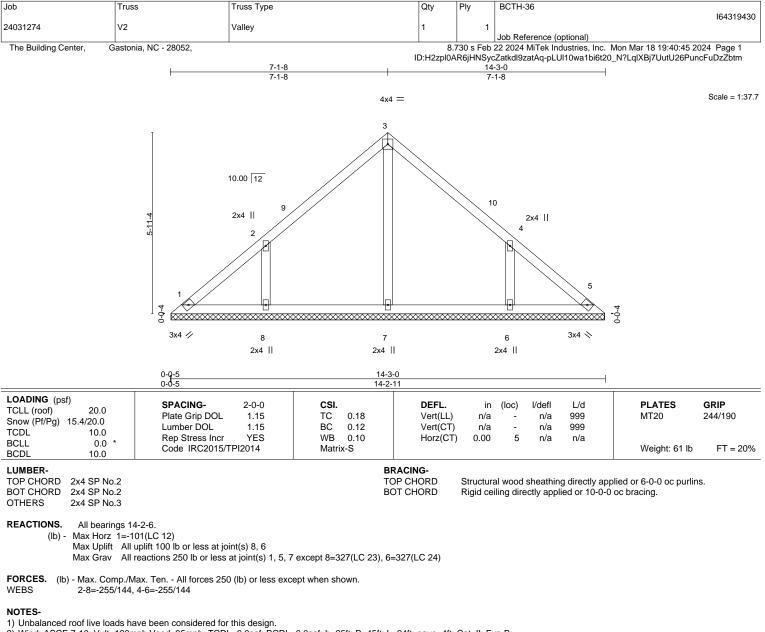
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



818 Soundside Road



- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-1-8, Interior(1) 3-1-8 to 7-1-8, Exterior(2) 7-1-8 to 10-1-8, Interior(1) 10-1-8 to 13-10-3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

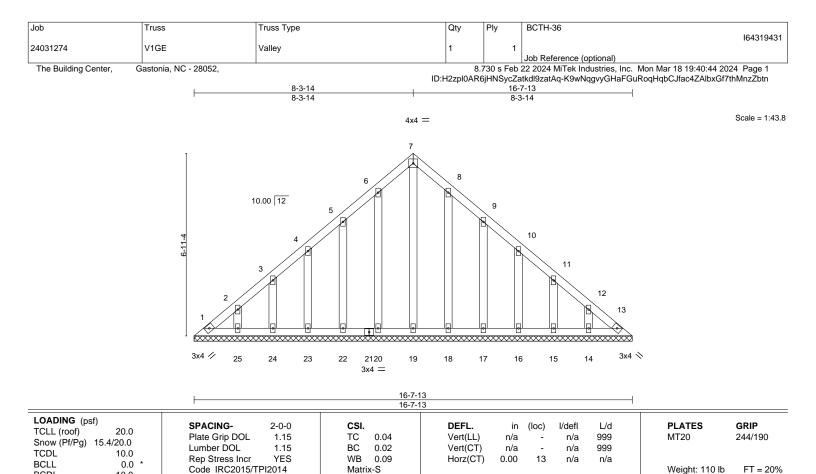
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





BRACING-

TOP CHORD

BOT CHORD

L	UM	BE	R-
-			

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. All bearings 16-7-13.

Max Horz 1=-119(LC 12) (lb) -

10.0

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14 Max Grav All reactions 250 lb or less at joint(s) 1, 13, 19, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) 0-4-13 to 3-4-13, Exterior(2) 3-4-13 to 8-3-14, Corner(3) 8-3-14 to 11-3-14, Exterior(2) 11-3-14 to 16-2-15 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14.



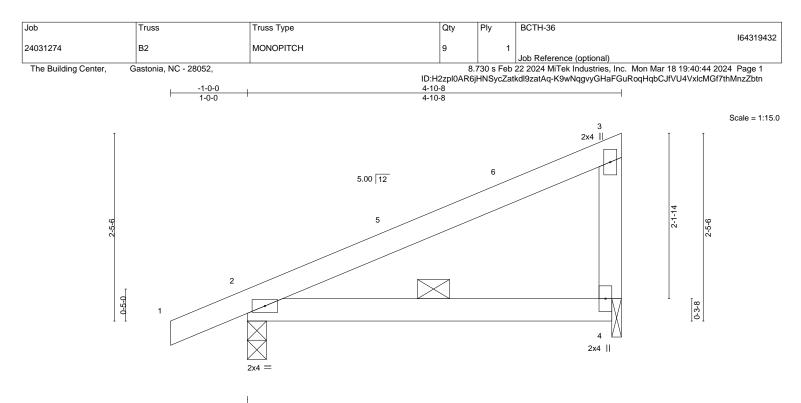
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 20.2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.36 BC 0.29 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190 FT = 20%
LUMBER-		BF	RACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16) Max Grav 2=260(LC 2), 4=176(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

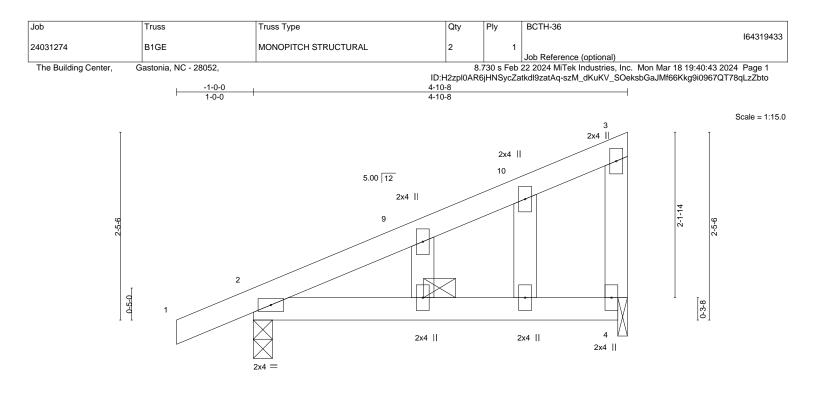


Structural wood sheathing directly applied or 4-10-8 oc purlins,

except end verticals.

3-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.36 BC 0.29 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%
---	---	---	--	------------------------------	--------------------------	-------------------------------	--------------------------	---------------------------------	------------------------------------

LOWIDEN-		DIVACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-10-8 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	3-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16) Max Grav 2=260(LC 2), 4=176(LC 2)

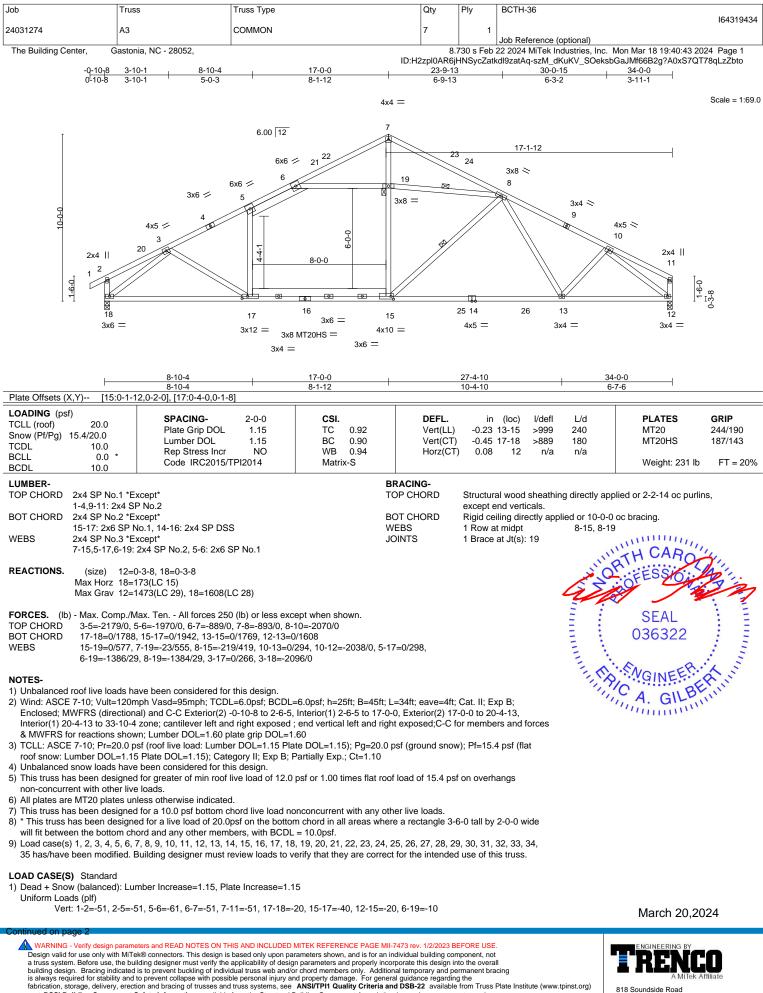
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

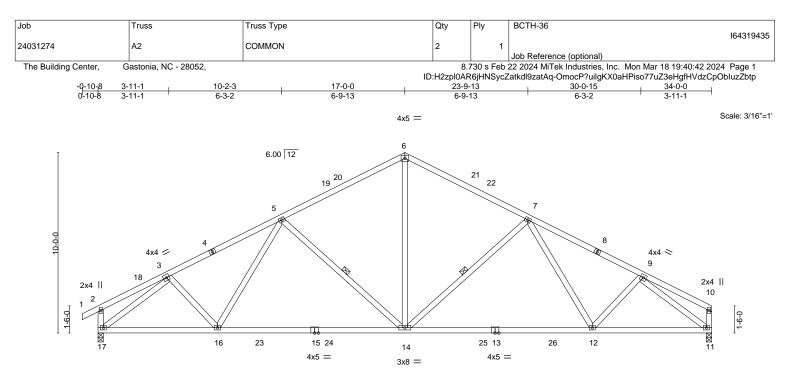
Job	Truss	Truss Type	Qty	Ply	BCTH-36	
24031274	A3	COMMON	7	1		164319434
					Job Referenc	
The Building Center,	Gastonia, NC - 28052,	ID:				<pre>k Industries, Inc. Mon Mar 18 19:40:43 2024 Page 2 _dKuKV_SOeksbGaJMf66B2g?A0xS7QT78qLzZbto</pre>
			•			
 LOAD CASE(S) Standar 2) Dead + Roof Live (bala 	a Inced): Lumber Increase=1.15	5, Plate Increase=1.15				
Uniform Loads (plf)	2 5 - 60 5 6 - 70 6 7 - 60 7	11_ 60_17_19_ 20_15_17_ 40_12_15_ 20_6	0- 10			
		·11=-60, 17-18=-20, 15-17=-40, 12-15=-20, 6- \ttic Storage: Lumber Increase=1.15, Plate Inc		5		
Uniform Loads (plf)		.11=-50, 17-18=-20, 15-17=-70, 15-25=-20, 25	26 50 1	2 26- 20	6 10 - 10	
		Storage: Lumber Increase=1.15, Plate Increas	,	2-26=-20,	6-19=-10	
Uniform Loads (plf)	25-1256-5267-127	.11=-43, 17-18=-20, 15-17=-70, 15-25=-20, 25	26- 50 1	2 26- 20	6 10- 10	
		c Storage: Lumber Increase=1.15, Plate Incre		2-20=-20,	0-19=-10	
Uniform Loads (plf)	2-543 5-653 6-2143 7	7-21=-62, 7-11=-27, 17-18=-20, 15-17=-70, 15	25-20 2	5-2650	12-2620 6-	1910
		ttic Storage: Lumber Increase=1.15, Plate Incr			12 20- 20, 0	10-10
Uniform Loads (plf) Vert: 1-2=-27	2-5=-27 5-6=-37 6-7=-27 7-	24=-63, 11-24=-43, 17-18=-20, 15-17=-70, 15	-25=-20 2	5-26=-50	12-26=-20 6-	19=-10
7) Dead + Uninhabitable		r Increase=1.25, Plate Increase=1.25	20- 20, 2	0 20- 00,	12 20- 20, 0	
Uniform Loads (plf) Vert: 1-2=-20	2-5=-20 5-6=-30 6-7=-20 7-	.11=-20, 17-18=-40, 15-17=-60, 12-15=-40, 6-	9=-10			
8) Dead + 0.6 C-C Wind (r Increase=1.60, Plate Increase=1.60	10-10			
Uniform Loads (plf) Vert: 1-2=46.2	2-20=24, 5-20=14, 5-6=4, 6-7;	=14, 7-23=24, 11-23=14, 17-18=-12, 15-17=-3	2. 12-15=-	12.6-19=	-10	
Horz: 1-2=-58	2-20=-36, 7-20=-26, 7-23=36	6, 11-23=26, 2-18=14, 11-12=26	_,	, • · •		
 Dead + 0.6 C-C Wind (Uniform Loads (plf) 	Pos. Internal) Case 2: Lumbe	r Increase=1.60, Plate Increase=1.60				
Vert: 1-2=9, 2-		24, 7-10=14, 10-11=24, 17-18=-12, 15-17=-32	, 12-15=-1	2, 6-19=-1	0	
		6, 10-11=36, 2-18=-26, 11-12=-14 per Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf))) E -)) E C - 42 C Z -)) ⁻	7 11 22 17 19 20 15 17 10 12 15 20 6	10 10			
	, 2-7=13, 7-11=-13, 2-18=-17,	7-11=-33, 17-18=-20, 15-17=-40, 12-15=-20, 6 11-12=-24	-19=-10			
	(Neg. Internal) Case 2: Lumb	per Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-2=-28	3, 2-5=-33, 5-6=-43, 6-7=-33, 7	7-11=-33, 17-18=-20, 15-17=-40, 12-15=-20, 6	-19=-10			
	2-7=13, 7-11=-13, 2-18=24, 1	1-12=17 ber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	Mind (FOS. Internal) Leit. Luin	Der increase=1.00, Flate increase=1.00				
	, 2-5=-0, 5-6=-10, 6-7=-0, 7-1 ⁻ 4, 2-7=-12, 7-11=17, 2-18=13	1=5, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=- 11-12-15	10			
		mber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf) Vert: 1-2=1	2-5=5 5-6=-5 6-7=5 7-11=-0	, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=-10				
Horz: 1-2=-1	3, 2-7=-17, 7-11=12, 2-18=-15	5, 11-12=-13				
14) Dead + 0.6 MWFRS Uniform Loads (plf)	Wind (Neg. Internal) Left: Lum	ber Increase=1.60, Plate Increase=1.60				
Vert: 1-2=-23		7-11=-12, 17-18=-20, 15-17=-40, 12-15=-20, 6	-19=-10			
	2-7=7, 7-11=8, 2-18=22, 11-1 Nind (Neg. Internal) Right: Lu	2=6 mber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)			10 10			
	2-5=-12, 5-6=-22, 6-7=-12, 7- 3, 2-7=-8, 7-11=-7, 2-18=-6, 1	-11=-27, 17-18=-20, 15-17=-40, 12-15=-20, 6- 1-12=-22	19=-10			
/	Nind (Pos. Internal) 1st Parall	el: Lumber Increase=1.60, Plate Increase=1.6	0			
Uniform Loads (plf) Vert: 1-2=25	2-5=13, 5-6=3, 6-7=13, 7-11	=13, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=	10			
	7, 2-7=-25, 7-11=25, 2-18=-19 Nind (Pos. Internal) 2nd Para	9, 11-12=19 Ilel: Lumber Increase=1.60, Plate Increase=1.6	80			
Uniform Loads (plf)						
	, 2-5=4, 5-6=-6, 6-7=4, 7-11=4 8, 2-7=-16, 7-11=16, 2-18=-19	4, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=-10 9 11-12=19				
18) Dead + 0.6 MWFRS		lel: Lumber Increase=1.60, Plate Increase=1.6	0			
Uniform Loads (plf) Vert: 1-2=-16	6. 2-5=-21. 5-6=-31. 6-7=-21. 7	7-11=-21, 17-18=-20, 15-17=-40, 12-15=-20, 6	-19=-10			
Horz: 1-2=-4	, 2-7=1, 7-11=-1, 2-18=-10, 11	1-12=10				
19) Dead + 0.6 MWFRS Uniform Loads (plf)	Wind (Neg. Internal) 2nd Para	Ilel: Lumber Increase=1.60, Plate Increase=1.	50			
Vert: 1-2=-16		7-11=-21, 17-18=-20, 15-17=-40, 12-15=-20, 6	-19=-10			
	, 2-7=1, 7-11=-1, 2-18=-10, 11 rhangs: Lumber Increase=1.1					
Uniform Loads (plf)	2-520 5 6- 20 6 7- 00 -	7-11=-20, 17-18=-20, 15-17=-40, 12-15=-20, 6	-10- 10			
	Left): Lumber Increase=1.15		-19=-10			
Uniform Loads (plf)	2-551 5-661 6 21- 51	, 7-21=-76, 7-11=-29, 17-18=-20, 15-17=-40, 1	2-1520	6-1010		
22) Dead + Snow (Unbal.	Right): Lumber Increase=1.1		<u>∠</u> -ıJ= - ∠U,	0-1 <i>9</i> = - 10		
Uniform Loads (plf) Vert: 1-2=-29	2-5=-29 5-6=-39 6-729	7-24=-77, 11-24=-51, 17-18=-20, 15-17=-40, 1	2-15=-20	6-19=-10		
von. 1 223	., _ = _ = _0, 0 0 = 00, 0 7 = 20, 1	, = 0 . , 20, . 0 40, 1	_ 10- 20,	0 10- 10		
Continued on page 2						

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



lob	Truss	Truss Type	Qty	Ply	BCTH-36	10404040
24031274	A3	COMMON	7	1		l6431943
					Job Reference (optional)	
The Building Center,	Gastonia, NC - 28052,	,	ID:H2zpl0AF		22 2024 MiTek Industries, Inc. Mo kdl9zatAq-szM_dKuKV_SOeksbG	
LOAD CASE(S) Stand	lard					
23) Dead + Uninhabita	ble Attic Storage: Lumb	per Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert: 1-2=-		6-7=-20, 7-11=-20, 17-18=-20, 15-17=-80, 1	5-25=-20 25-26=-60	12-26=-20	6-19=-10	
24) Dead + 0.75 Snow	(bal.) + 0.75 Uninhab.	Attic Storage + 0.75(0.6 MWFRS Wind (Ne				
Uniform Loads (plf)		6-7=-49, 7-11=-37, 17-18=-20, 15-17=-70, 1	5 25- 20 25 26- 50	12 26- 20	6 10- 10	
	2, 2-7=6, 7-11=6, 2-18		5-25=-20, 25-26=-50	, 12-20=-20	, 0-19=-10	
25) Dead + 0.75 Snow	(bal.) + 0.75 Uninhab.	Attic Storage + 0.75(0.6 MWFRS Wind (Ne	g. Int) Right): Lumbe	r Increase=1	.60, Plate Increase=1.60	
Uniform Loads (plf)				12.26 20	6 10 - 10	
	-34, 2-5=-37, 5-6=-47, 6 -10, 2-7=-6, 7-11=-6, 2	6-7=-37, 7-11=-49, 17-18=-20, 15-17=-70, 1 -18=-5, 11-12=-16	5-25=-20, 25-26=-50	J, 12-26=-20	, 0-19=-10	
		Attic Storage + 0.75(0.6 MWFRS Wind (Ne	g. Int) 1st Parallel): L	umber Incre	ase=1.60, Plate Increase=1.60	
Uniform Loads (plf)						
	40, 2-5=-44, 5-6=-54, 6 -3, 2-7=1, 7-11=-1, 2-1	6-7=-44, 7-11=-44, 17-18=-20, 15-17=-70, 1 88 11-12-8	5-25=-20, 25-26=-50), 12-26=-20	, 6-19=-10	
		Attic Storage + 0.75(0.6 MWFRS Wind (Ne	g. Int) 2nd Parallel): I	umber Incr	ease=1.60, Plate Increase=1.60	
Uniform Loads (plf)						
	·40, 2-5=-44, 5-6=-54, 6 ·-3, 2-7=1, 7-11=-1, 2-1	6-7=-44, 7-11=-44, 17-18=-20, 15-17=-70, 1 88, 11-12-8	5-25=-20, 25-26=-50), 12-26=-20	, 6-19=-10	
		ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) Left): Lum	per Increase	=1.60, Plate Increase=1.60	
Uniform Loads (plf)		2 .	, ,			
		6-7=-56, 7-11=-44, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50	, 12-26=-20	, 6-19=-10	
	2, 2-7=6, 7-11=6, 2-18 _ive (bal.) + 0.75 Uninh	=16, 11-12=5 ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Nea. Int) Right): Lur	nber Increas	se=1.60. Plate Increase=1.60	
Uniform Loads (plf)	()		(
		6-7=-44, 7-11=-56, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50	, 12-26=-20	, 6-19=-10	
	-10, 2-7=-6, 7-11=-6, 2 ive (bal.) + 0.75 Uninb	-18=-5, 11-12=-16 ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) 1st Paralle	I): Lumber l	ncrease=1.60 Plate Increase=1	60
Uniform Loads (plf)	()		(riog. m) for a and			
		6-7=-51, 7-11=-51, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50	, 12-26=-20	, 6-19=-10	
	-3, 2-7=1, 7-11=-1, 2-1	8=-8, 11-12=8 ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) 2nd Parall	al): Lumber	Increase-1.60 Plate Increase-1	1 60
Uniform Loads (plf)	()	ab. Alle Glorage + 0.75(0.0 MWFRO Wind				1.00
	, , ,	6-7=-51, 7-11=-51, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50	, 12-26=-20	, 6-19=-10	
	-3, 2-7=1, 7-11=-1, 2-1	8=-8, 11-12=8 er Increase=1.15, Plate Increase=1.15				
Uniform Loads (plf)	· · · · ·	el liclease=1.15, Plate liclease=1.15				
Vert: 1-2=-	60, 2-5=-60, 5-6=-70, 6	6-7=-60, 7-11=-20, 17-18=-20, 15-17=-40, 1	2-15=-20, 6-19=-10			
,	(/	ber Increase=1.15, Plate Increase=1.15				
Uniform Loads (plf) Vert: 1-2=-		6-7=-20, 7-11=-60, 17-18=-20, 15-17=-40, 1	2-15=-20, 6-19=-10			
	, , ,	- 0.75 Uninhab. Attic Storage: Lumber Incre	,	ease=1.15		
Uniform Loads (plf)				40.00		
		6-7=-50, 7-11=-20, 17-18=-20, 15-17=-70, 1 • 0.75 Uninhab. Attic Storage: Lumber Incre			, 6-19=-10	
Uniform Loads (plf)	(,	erre enimale. Auto ellerage. Euriber mere		1.10		
	20.25.20.50.20.0	6-7=-20, 7-11=-50, 17-18=-20, 15-17=-70, 1				





	-7-6 -7-6	17-0-0 10-4-10	27-4-10 10-4-10	34-0-0 6-7-6	1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0-0Plate Grip DOL1.11Lumber DOL1.11Rep Stress IncrYESCodeIRC2015/TPI2014	TC 0.71 BC 0.85 WB 0.85	DEFL. in (loc) I/defl Vert(LL) -0.26 14-16 >999 Vert(CT) -0.50 12-14 >810 Horz(CT) 0.08 11 n/a	L/d PLATES 240 MT20 180 n/a Weight: 198 II	GRIP 244/190

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-7-0 oc purlins,
BOT CHORD	2x4 SP No.2 *Except*		except end verticals.
	13-15: 2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt 7-14, 5-14

REACTIONS. (size) 17=0-3-8, 11=0-3-8 Max Horz 17=173(LC 15) Max Uplift 17=-35(LC 16), 11=-10(LC 16) Max Grav 17=1411(LC 2), 11=1347(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-5=-1832/134, 5-6=-1448/175, 6-7=-1448/178, 7-9=-1837/146

BOT CHORD 16-17=-81/1543, 14-16=-55/1604, 12-14=-48/1538, 11-12=-93/1456

WEBS 6-14=-25/896, 7-14=-493/121, 5-14=-491/121, 3-17=-1816/72, 9-11=-1829/106

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -0-10-8 to 2-6-5, Interior(1) 2-6-5 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13, Interior(1) 20-4-13 to 33-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

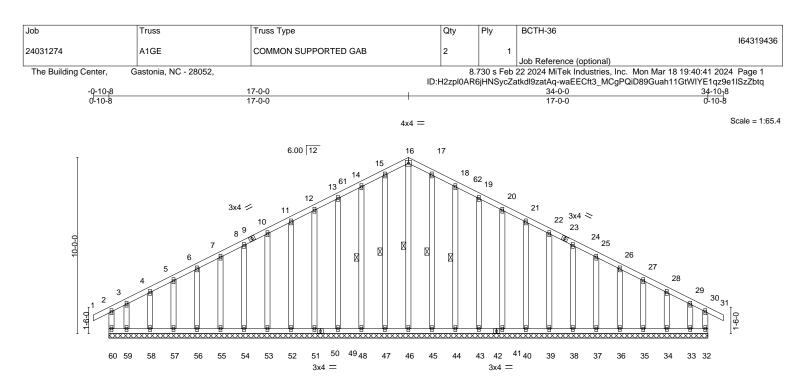
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) All plates are 3x4 MT20 unless otherwise indicated.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 11.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



		34-0-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.09 WB 0.10 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 31 -0.00 31 -0.00 32	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 310 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2				Structural wood		g directly app	plied or 6-0-0 oc purlins	3,

LOWRER	-	BRACING-			
TOP CHORD 2x4 SP No.2		TOP CHORD	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,		
BOT CHO	DRD 2x4 SP No.2		except end verticals		
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.		
OTHERS	2x4 SP No.3	WEBS	1 Row at midpt	16-46, 15-47, 14-48, 17-45, 18-44	

REACTIONS. All bearings 34-0-0.

(lb) -Max Horz 60=-175(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except 60=-135(LC 14), 59=-136(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 60, 32, 46, 47, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 59, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-85/272, 13-14=-95/299, 14-15=-106/330, 15-16=-109/341, 16-17=-109/336,

17-18=-106/325, 18-19=-95/293, 19-20=-85/266

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 17-0-0, Corner(3) 17-0-0 to 20-4-13, Exterior(2) 20-4-13 to 34-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 1-4-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 12) will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except (jt=lb) 60=135, 59=136.



818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a futs system: Denote use, the building designer inder very the applications of design had needed an intervent with a policitation of the system of the state of the system of the syste and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

