

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 24031270 BCTH-34

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by The Building Center.

Pages or sheets covered by this seal: I64319377 thru I64319388

My license renewal date for the state of North Carolina is December 31, 2024.

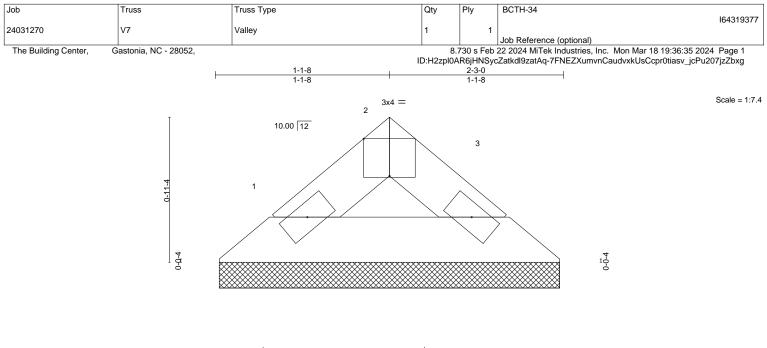
North Carolina COA: C-0844



March 20,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



2x4 🥢

2x4 🚿

0-<u>0-5 2-3-0</u> 0-0-5 2-2-11

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCDL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.01 BC 0.02 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 6 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			RACING- OP CHORD	Structura	ıl wood	sheathin	g directly ap	oplied or 2-3-0 oc purl	lins.

BOT CHORD

Structural wood sheathing directly applied or 2-3-0 oc purlin Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=2-2-6, 3=2-2-6 Max Horz 1=-11(LC 12) Max Grav 1=58(LC 2), 3=58(LC 2)

Plate Offsets (X,Y)-- [2:0-2-0,Edge]

BOT CHORD 2x4 SP No.2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

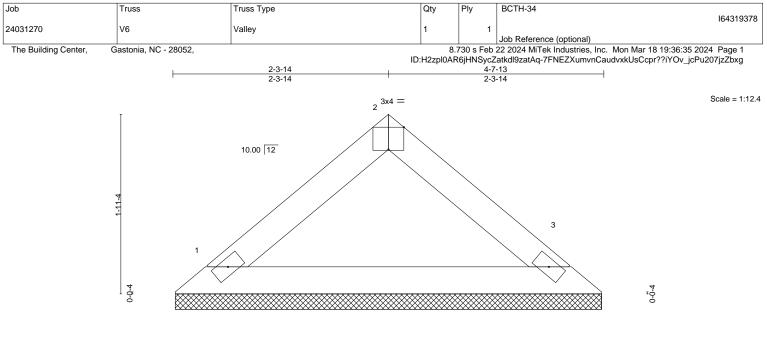
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



2x4 🥢

2x4 🚿

4-7113

Rigid ceiling directly applied or 10-0-0 oc bracing.

		4-7-8			0-0-5		
Plate Offsets (X,Y) [2:0-2-0,E	dge]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.06 BC 0.18 WB 0.00 Matrix-P	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - a -	/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2		BRACII TOP CH		ıral wood sh	eathing directly app	blied or 4-7-13 oc purl	ins.

BOT CHORD

4-7-8

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=4-7-3, 3=4-7-3

Max Horz 1=-29(LC 12) Max Uplift 1=-1(LC 14), 3=-1(LC 14)

Max Grav 1=154(LC 2), 3=154(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

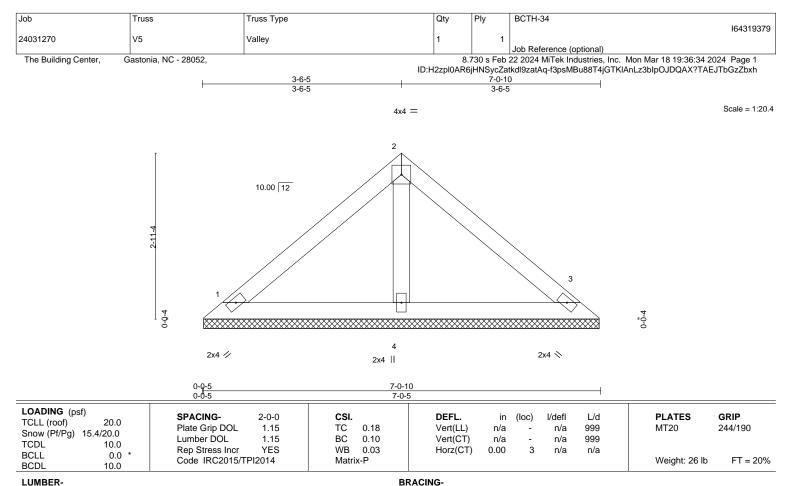
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=7-0-0, 3=7-0-0, 4=7-0-0

Max Horz 1=47(LC 13)

Max Uplift 1=-17(LC 14), 3=-17(LC 14)

Max Grav 1=143(LC 2), 3=143(LC 2), 4=213(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

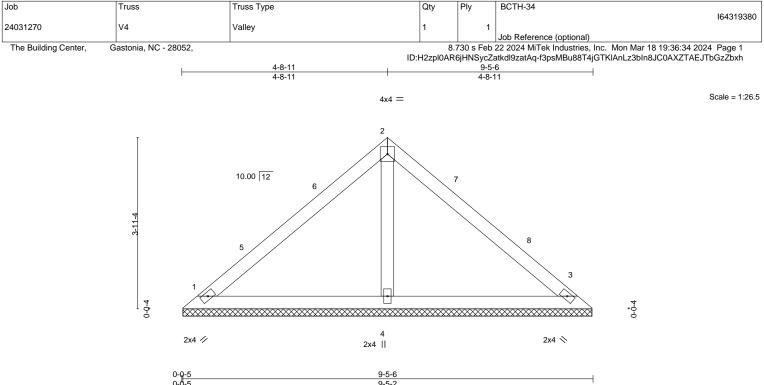


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



	0-0-5	9-5-2							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.26 BC 0.19 WB 0.06	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 36 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=9-4-13, 3=9-4-13, 4=9-4-13 Max Horz 1=-65(LC 12)

Max Uplift 1=-15(LC 14), 3=-15(LC 14)

Max Grav 1=183(LC 2), 3=183(LC 2), 4=325(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-8-11, Exterior(2) 4-8-11 to 7-8-11, Interior(1) 7-8-11 to 9-0-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

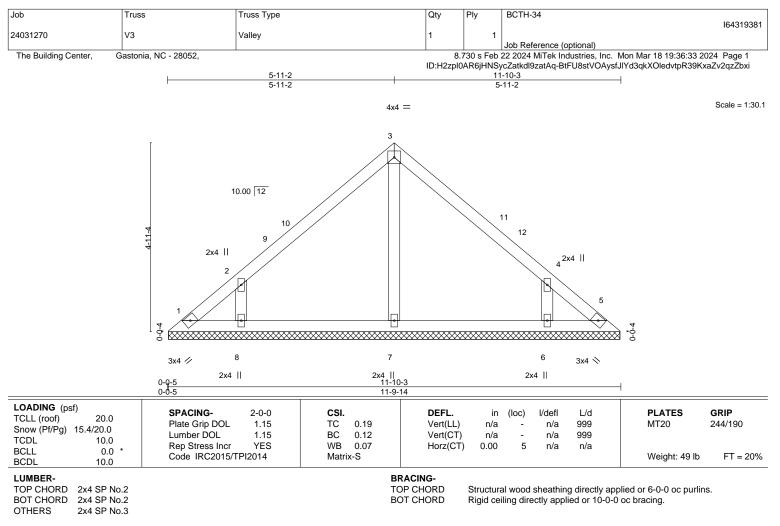
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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REACTIONS. All bearings 11-9-10.

(lb) - Max Horz 1=83(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=301(LC 23), 6=301(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

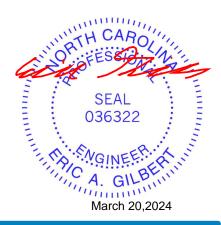
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 5-11-2, Exterior(2) 5-11-2 to 8-11-2, Interior(1) 8-11-2 to 11-5-6 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

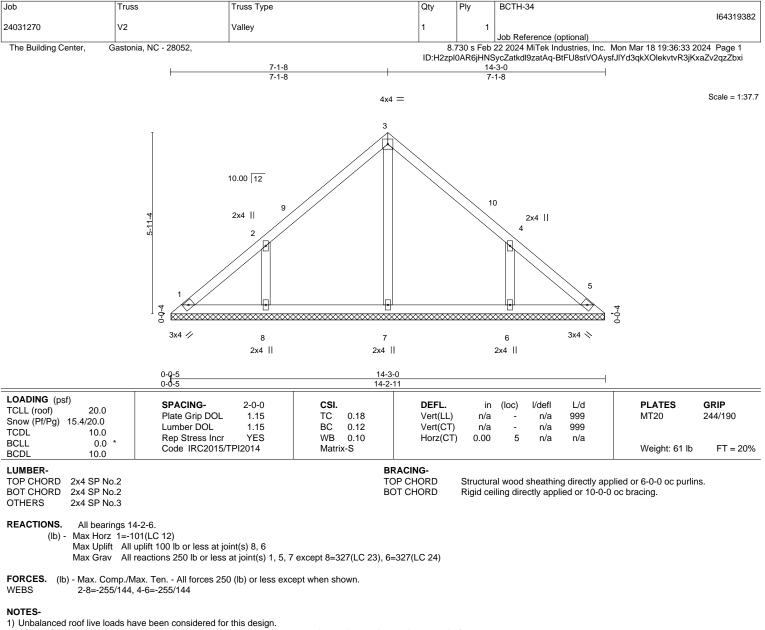
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



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- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-13 to 3-1-8, Interior(1) 3-1-8 to 7-1-8, Exterior(2) 7-1-8 to 10-1-8, Interior(1) 10-1-8 to 13-10-3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

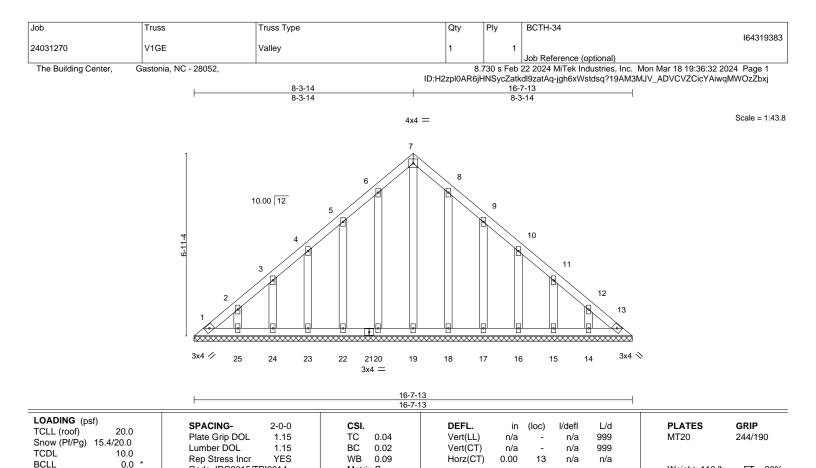
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.



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818 Soundside Road



Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LU	JME	BER-	

BCDL

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

REACTIONS. All bearings 16-7-13.

(lb) - Max Horz 1=-119(LC 12)

10.0

Max Uplift All uplift 100 b or less at joint(s) 1, 13, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14 Max Grav All reactions 250 lb or less at joint(s) 1, 13, 19, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) 0-4-13 to 3-4-13, Exterior(2) 3-4-13 to 8-3-14, Corner(3) 8-3-14 to 11-3-14, Exterior(2) 11-3-14 to 16-2-15 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 20, 22, 23, 24, 25, 18, 17, 16, 15, 14.



Weight: 110 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

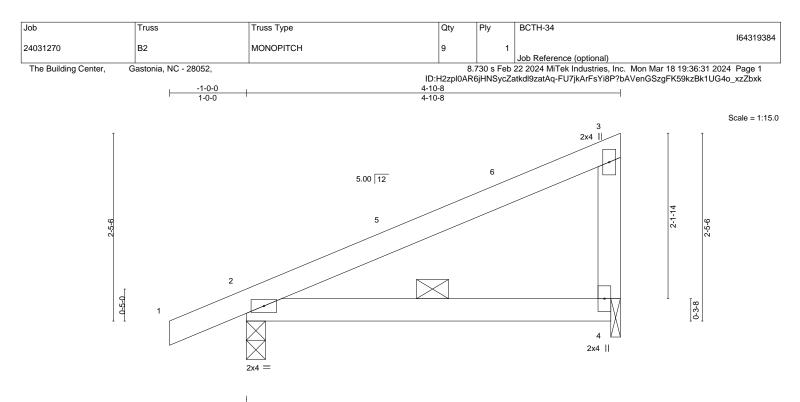
Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

March 20,2024

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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.36 BC 0.29 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190 FT = 20%
LUMBER-		BF	RACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16) Max Grav 2=260(LC 2), 4=176(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.



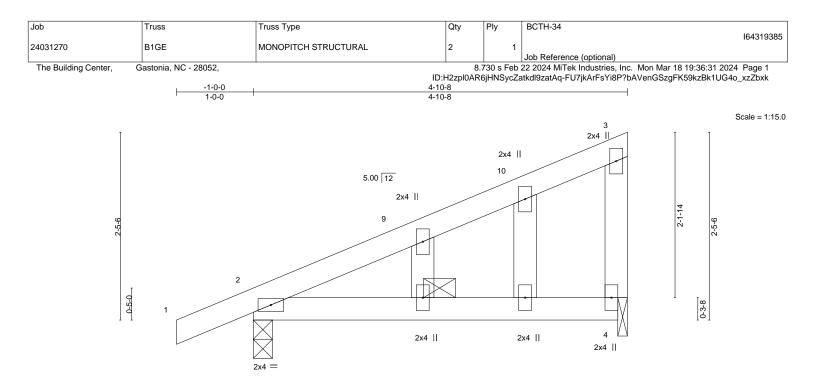
Structural wood sheathing directly applied or 4-10-8 oc purlins,

except end verticals.

3-0-0 oc bracing.

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818 Soundside Road



LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.36 BC 0.29 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190 FT = 20%
BCDL 10.0	Code IRC2015/1P12014	Matrix-P						Weight: 23 lb	FI = 20%

LOWIDEN-		DIVACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-10-8 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	3-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16) Max Grav 2=260(LC 2), 4=176(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

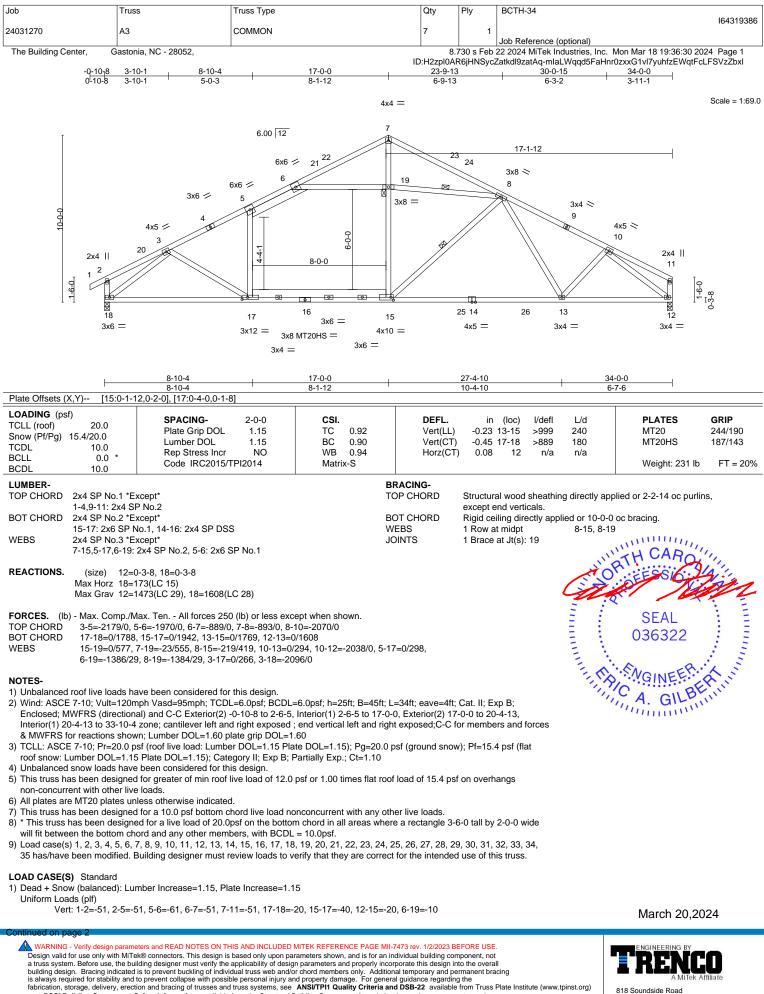
- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.



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818 Soundside Road



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



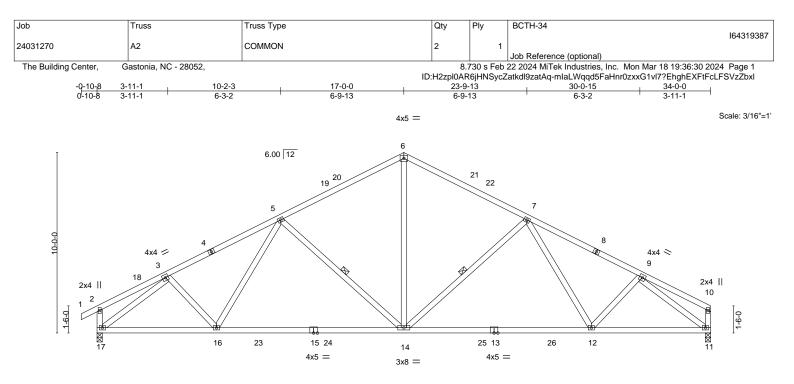
Job	Truss	Truss Type	Qty	Ply	BCTH-34
04004070			7		I64319386
24031270	A3	COMMON	/	1	Job Reference (optional)
The Building Center, G	astonia, NC - 28052,				b 22 2024 MiTek Industries, Inc. Mon Mar 18 19:36:30 2024 Page 2
			ιυ.π22ριυΑ	којпизус	cZatkdl9zatAq-mlaLWqqd5FaHnr0zxxG1vl7yuhfzEWqtFcLFSVzZbxl
LOAD CASE(S) Standard					
 Dead + Roof Live (balar Uniform Loads (plf) 	iced): Lumber Increase=1.15	, Plate Increase=1.15			
Vert: 1-2=-60, 2		11=-60, 17-18=-20, 15-17=-40, 12-15=-20, 6-			
 Dead + 0.75 Roof Live (Uniform Loads (plf) 	balanced) + 0.75 Uninhab. A	ttic Storage: Lumber Increase=1.15, Plate Inc	rease=1.15	5	
	-5=-50, 5-6=-60, 6-7=-50, 7-	11=-50, 17-18=-20, 15-17=-70, 15-25=-20, 25	-26=-50, 12	2-26=-20,	, 6-19=-10
	nced) + 0.75 Uninhab. Attic	Storage: Lumber Increase=1.15, Plate Increase	se=1.15		
Uniform Loads (plf) Vert: 1-2=-43 2	-5=-43 5-6=-53 6-7=-43 7-	11=-43, 17-18=-20, 15-17=-70, 15-25=-20, 25	-26=-50 12	2-26=-20	6-19=-10
		c Storage: Lumber Increase=1.15, Plate Incre		- 20 20,	
Uniform Loads (plf)	-513 5-653 6-2113 7	/-21=-62, 7-11=-27, 17-18=-20, 15-17=-70, 15	25-20 24	5-2650	12-2620 6-1910
		tic Storage: Lumber Increase=1.15, Plate Inc		5-2050,	, 12-2020, 0-1310
Uniform Loads (plf)					
		24=-63, 11-24=-43, 17-18=-20, 15-17=-70, 15 Increase=1.25, Plate Increase=1.25	-25=-20, 25	5-26=-50,	, 12-26=-20, 6-19=-10
Uniform Loads (plf)	Ū				
		11=-20, 17-18=-40, 15-17=-60, 12-15=-40, 6- r Increase=1.60, Plate Increase=1.60	19=-10		
Uniform Loads (plf)	OS. Internal) Case T. Lumber	increase=1.00, Flate increase=1.00			
		=14, 7-23=24, 11-23=14, 17-18=-12, 15-17=-3	2, 12-15=-1	12, 6-19=-	10
		5, 11-23=26, 2-18=14, 11-12=26 r Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf)					
		24, 7-10=14, 10-11=24, 17-18=-12, 15-17=-32 5, 10-11=36, 2-18=-26, 11-12=-14	2, 12-15=-12	2, 6-19=-1	10
		er Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf)	2 - 22 - 6 - 42 - 6 7 - 22 - 7	7 11 - 22 17 18 - 20 15 17 - 40 12 15 - 20 6	10 10		
	2-5=-33, 5-6=-43, 6-7=-33, 7 2-7=13, 7-11=-13, 2-18=-17,	7-11=-33, 17-18=-20, 15-17=-40, 12-15=-20, 6 11-12=-24	-19=-10		
11) Dead + 0.6 C-C Wind (er Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf) Vert: 1-2=-28	2-5=-33 5-6=-43 6-7=-33 7	7-11=-33, 17-18=-20, 15-17=-40, 12-15=-20, 6	-19=-10		
Horz: 1-2=8, 2	-7=13, 7-11=-13, 2-18=24, 1	1-12=17	/ 10- 10		
,	ind (Pos. Internal) Left: Luml	ber Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf) Vert: 1-2=12, 2	2-5=-0, 5-6=-10, 6-7=-0, 7-11	=5, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=	-10		
	, 2-7=-12, 7-11=17, 2-18=13,				
13) Dead + 0.6 MWFRS W Uniform Loads (plf)	and (Pos. Internal) Right: Lur	nber Increase=1.60, Plate Increase=1.60			
Vert: 1-2=1, 2-		, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=-10			
	, 2-7=-17, 7-11=12, 2-18=-15	5, 11-12=-13 ber Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf)	ind (Neg. Internal) Lett. Luti	bei increase=1.00, Flate increase=1.00			
	2-5=-27, 5-6=-37, 6-7=-27, 7 2-7=7, 7-11=8, 2-18=22, 11-1	7-11=-12, 17-18=-20, 15-17=-40, 12-15=-20, 6	6-19=-10		
		z=o mber Increase=1.60, Plate Increase=1.60			
Uniform Loads (plf)			40 40		
	-5=-12, 5-6=-22, 6-7=-12, 7- , 2-7=-8, 7-11=-7, 2-18=-6, 1	11=-27, 17-18=-20, 15-17=-40, 12-15=-20, 6- 1-12=-22	19=-10		
16) Dead + 0.6 MWFRS W		el: Lumber Increase=1.60, Plate Increase=1.6	0		
Uniform Loads (plf) Vert: 1-2=25.2	2-5=13. 5-6=3. 6-7=13. 7-11=	=13, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=	-10		
Horz: 1-2=-37	, 2-7=-25, 7-11=25, 2-18=-19	9, 11-12=19			
17) Dead + 0.6 MWFRS W Uniform Loads (plf)	(ind (Pos. Internal) 2nd Paral	lel: Lumber Increase=1.60, Plate Increase=1.	60		
Vert: 1-2=16, 2		, 17-18=-12, 15-17=-32, 12-15=-12, 6-19=-10	1		
	, 2-7=-16, 7-11=16, 2-18=-19 (ind (Neg. Internal) 1st Parall	9, 11-12=19 el: Lumber Increase=1.60, Plate Increase=1.6	80		
Uniform Loads (plf)	ind (Neg. Internal) 13t1 arai				
		7-11=-21, 17-18=-20, 15-17=-40, 12-15=-20, 6	6-19=-10		
	2-7=1, 7-11=-1, 2-18=-10, 11 /ind (Neg. Internal) 2nd Paral	Ilel: Lumber Increase=1.60, Plate Increase=1.	60		
Uniform Loads (plf)					
	2-5=-21, 5-6=-31, 6-7=-21, 7 2-7=1, 7-11=-1, 2-18=-10, 11	7-11=-21, 17-18=-20, 15-17=-40, 12-15=-20, 6 -12=10	5-19=-10		
20) Dead + Snow on Overl	nangs: Lumber Increase=1.1				
Uniform Loads (plf) Vert: 1-2=-51	2-5=-20 5-630 6-720 7	7-11=-20, 17-18=-20, 15-17=-40, 12-15=-20, 6	-1910		
	_eft): Lumber Increase=1.15,		, 13-10		
Uniform Loads (plf)		7 91- 76 7 11- 90 17 10- 90 45 47 40 4	2 15 - 20 4	s 10 - 40	
	2-5=-51, 5-6=-61, 6-21=-51, Right): Lumber Increase=1.1	7-21=-76, 7-11=-29, 17-18=-20, 15-17=-40, 1 5, Plate Increase=1.15	2-10=-20, 6	5-19=-10	
Uniform Loads (plf)			0.45 00	240.40	
vert: 1-2=-29,	2-3=-29, 5-6=-39, 6-7=-29, 7	7-24=-77, 11-24=-51, 17-18=-20, 15-17=-40, 1	∠-15=-20, 6	o-19=-10	

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ob	Truss	Truss Type	Qty	Ply	BCTH-34	16431938
4031270	A3	COMMON	7	1		10431930
					Job Reference (optional)	
The Building Center,	Gastonia, NC - 28052	2,			22 2024 MiTek Industries, Inc. Mon Mar Zatkdl9zatAq-mIaLWqqd5FaHnr0zxxG1v	
-OAD CASE(S) Stan	dard					
		ber Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf		0 7 00 7 44 00 47 40 00 45 47 00 4		10.00.00	0.40.40	
		6-7=-20, 7-11=-20, 17-18=-20, 15-17=-80, 1 Attic Storage + 0.75(0.6 MWFRS Wind (Ner				
Uniform Loads (plf)	.	. , ,			
		6-7=-49, 7-11=-37, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	, 6-19=-10	
	=2, 2-7=6, 7-11=6, 2-18 / (bal.) + 0.75 Uninhab.	Attic Storage + 0.75(0.6 MWFRS Wind (New	a. Int) Right): Lumber	Increase=1	.60. Plate Increase=1.60	
Uniform Loads (plf			5·····)···5····			
	, , ,	6-7=-37, 7-11=-49, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	, 6-19=-10	
	=-10, 2-7=-6, 7-11=-6, 2 ((bal) + 0 75 Uninbab	2-18=-5, 11-12=-16 Attic Storage + 0.75(0.6 MWFRS Wind (Ne	n Int) 1st Parallel): Lu	mber Incre	ase=1.60. Plate Increase=1.60	
Uniform Loads (plf						
		6-7=-44, 7-11=-44, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	6-19=-10	
	=-3, 2-7=1, 7-11=-1, 2-1	18=-8, 11-12=8 Attic Storage + 0.75(0.6 MWFRS Wind (Ne	n Int) 2nd Parallel): L	umber Incre	assa-1.60. Plate Increase-1.60	
Uniform Loads (plf					ase=1.00, 1 late increase=1.00	
	, , ,	6-7=-44, 7-11=-44, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	, 6-19=-10	
	=-3, 2-7=1, 7-11=-1, 2-1		(Niger light) oft),		1.00 Plate Increase 1.00	
Uniform Loads (plf		hab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) Leit): Lumb	erincrease	=1.60, Plate Increase=1.60	
u u	/	6-7=-56, 7-11=-44, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	6-19=-10	
	=2, 2-7=6, 7-11=6, 2-18	-, -				
(29) Dead + 0.75 Root Uniform Loads (plf	()	hab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) Right): Lum	ber Increas	e=1.60, Plate Increase=1.60	
		6-7=-44, 7-11=-56, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50,	12-26=-20	6-19=-10	
	=-10, 2-7=-6, 7-11=-6, 2					
30) Dead + 0.75 Roof Uniform Loads (plf		hab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) 1st Parallel)	: Lumber Ir	crease=1.60, Plate Increase=1.60	
N N	/	6-7=-51, 7-11=-51, 17-18=-20, 15-17=-70, 1	5-25=-20. 25-26=-50.	12-26=-20	6-19=-10	
Horz: 1-2=	=-3, 2-7=1, 7-11=-1, 2-	18=-8, 11-12=8	, ,			
,	()	hab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int) 2nd Paralle	I): Lumber I	ncrease=1.60, Plate Increase=1.60	
Uniform Loads (plf Vert: 1-2=	/	6-7=-51, 7-11=-51, 17-18=-20, 15-17=-70, 1	5-25=-20, 25-26=-50.	12-26=-20	6-19=-10	
	=-3, 2-7=1, 7-11=-1, 2-1		, ,			
		per Increase=1.15, Plate Increase=1.15				
Uniform Loads (plf Vert: 1-2=		6-7=-60, 7-11=-20, 17-18=-20, 15-17=-40, 1	2-15=-20 6-19=-10			
		ber Increase=1.15, Plate Increase=1.15	2 10- 20, 0 10- 10			
Uniform Loads (plf						
		6-7=-20, 7-11=-60, 17-18=-20, 15-17=-40, 1 + 0.75 Uninhab. Attic Storage: Lumber Incre		200-1 15		
Uniform Loads (plf	· · · · · · · · · · · · · · · · · · ·	1 0.7 0 Ominiao. Allo Storage. Lumber Incle	usu=1.15, Flate IIICIE	435-1.10		
Vert: 1-2=	-50, 2-5=-50, 5-6=-60,	6-7=-50, 7-11=-20, 17-18=-20, 15-17=-70, 1			6-19=-10	
35) 4th Dead + 0.75 R Uniform Loads (plf	(,	+ 0.75 Uninhab. Attic Storage: Lumber Incre	ase=1.15, Plate Increa	ase=1.15		





	6-7-6 6-7-6	7-0-0)-4-10		27-4-10 10-4-10			34-0-0 6-7-6	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	CSI. TC 0.71 BC 0.85 WB 0.85 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.26 14-16 -0.50 12-14 0.08 11	l/defl >999 >810 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 198 lb	GRIP 244/190 FT = 20%

LUMBER-		BRACING-			
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheat	thing directly applied or 3-7-0 oc purlins,	
BOT CHORD	2x4 SP No.2 *Except*		except end verticals.		
	13-15: 2x4 SP No.1	BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 oc bracing.	
WEBS	2x4 SP No.3	WEBS	1 Row at midpt	7-14, 5-14	
			·		

REACTIONS. (size) 17=0-3-8, 11=0-3-8 Max Horz 17=173(LC 15) Max Uplift 17=-35(LC 16), 11=-10(LC 16) Max Grav 17=1411(LC 2), 11=1347(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-5=-1832/134, 5-6=-1448/175, 6-7=-1448/178, 7-9=-1837/146

BOT CHORD 16-17=-81/1543, 14-16=-55/1604, 12-14=-48/1538, 11-12=-93/1456

WEBS 6-14=-25/896, 7-14=-493/121, 5-14=-491/121, 3-17=-1816/72, 9-11=-1829/106

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) -0-10-8 to 2-6-5, Interior(1) 2-6-5 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13, Interior(1) 20-4-13 to 33-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

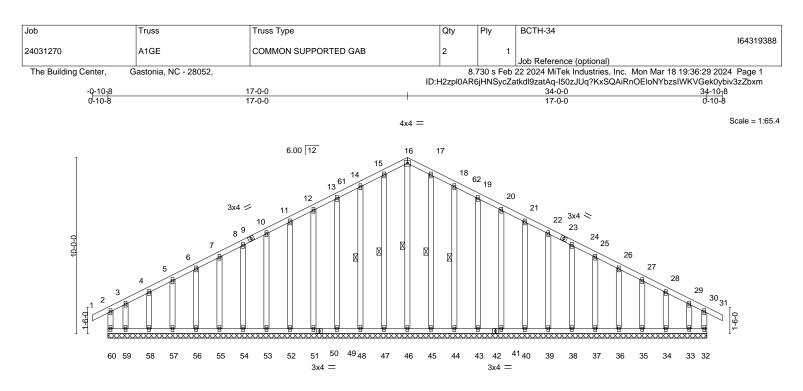
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) All plates are 3x4 MT20 unless otherwise indicated.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 11.



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		<u> </u>						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.09 WB 0.10 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.00 -0.00	oc) l/defl 31 n/r 31 n/r 32 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 310 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2			ACING- P CHORD	Structural w		g directly app	plied or 6-0-0 oc purlins	δ,

LOWIDER		BIULONICO			
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,		
BOT CHORD	2x4 SP No.2		except end verticals.		
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.		
OTHERS	2x4 SP No.3	WEBS	1 Row at midpt	16-46, 15-47, 14-48, 17-45, 18-44	

REACTIONS. All bearings 34-0-0.

(lb) - Max Horz 60=-175(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except 60=-135(LC 14), 59=-136(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 60, 32, 46, 47, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 59, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-85/272, 13-14=-95/299, 14-15=-106/330, 15-16=-109/341, 16-17=-109/336,

17-18=-106/325, 18-19=-95/293, 19-20=-85/266

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 17-0-0, Corner(3) 17-0-0 to 20-4-13, Exterior(2) 20-4-13 to 34-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 1-4-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32, 48, 49, 51, 52, 53, 54, 55, 56, 57, 58, 44, 43, 41, 40, 39, 38, 37, 36, 35, 34, 33 except (jt=lb) 60=135, 59=136.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

