

1 **SECOND FLOOR FRAMING PLAN**
SCALE : 1/4"=1'-0"

CONSULTANT INFO:

PROJECT INFO:

OWNER INFO:

CONTRACTOR:

No.	Date	Description

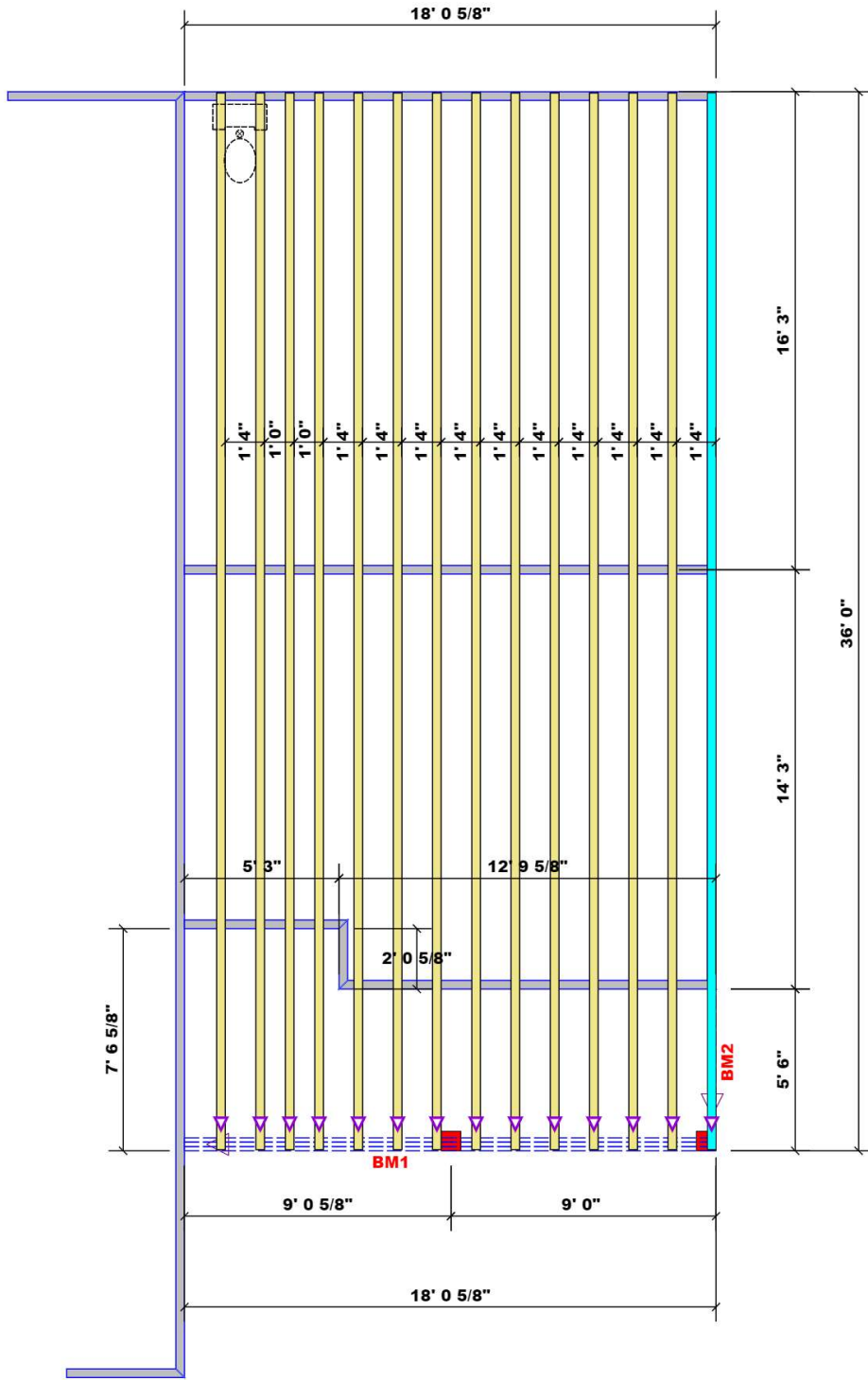
Project No.	Date
	08-MAR-2024
Drawn By	Checked By

DRAWING TITLE:
SECOND FLOOR FRAMING PLAN

SHEET NUMBER:
A-3.0

- Issued for Construction
- Not Issued for construction

SCALE: **AS SHOWN**



▲ = Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS
 (BASED ON TABLES H001-203 & 03)
 NUMBER OF JACK STUDS REQUIRED @ EA END OF
 HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) 2X4 HEADERS	END REACTION (UP TO)	REQ'D STUDS FOR (2) 2X4 HEADERS
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

BUILDER	Branden Toth	CITY / CO.	Lillington / Harnett
JOB NAME	92 Parkview Ln. / Harnett Co.	ADDRESS	92 Parkview Ln.
PLAN	Plan	MODEL	Floor
SEAL DATE	Seal Date	DATE REV.	03/19/24
QUOTE #	80324-1551	DRAWN BY	Dwayne Naylor
JOB #	Order #	SALES REP.	Dwayne Naylor

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature _____ *Dwayne Naylor*
 Dwayne Naylor

Truss Placement Plan
SCALE: 1/4" = 1'-0"

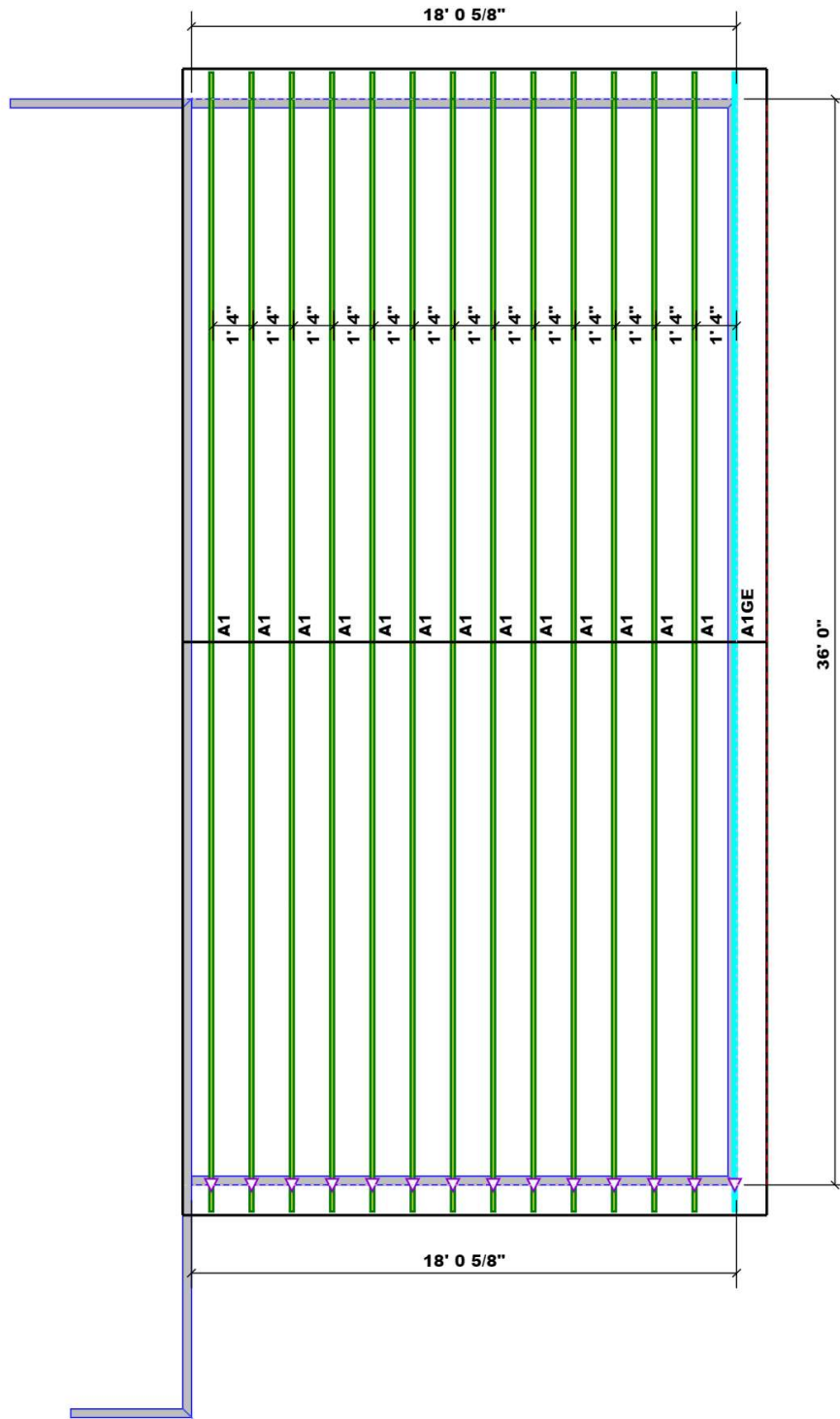
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ stcindustry.com



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444



▲ = Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS
 (BASED ON TABLES 6001.2(1) & (2))
 NUMBER OF JACK STUDS REQUIRED @ EA END OF
 HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (TO) 1" X 4" HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (TO) 1" X 4" HEADER
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

BUILDER	Branden Toth	CITY / CO.	Lillington / Harnett
JOB NAME	92 Parkview Ln. / Harnett Co.	ADDRESS	92 Parkview Ln.
PLAN	Plan	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	03/19/24
QUOTE #	80324-1624	DRAWN BY	Sales Area
JOB #	Order #	SALES REP.	Dwayne Naylor

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature _____ Sales Area

Truss Placement Plan
SCALE: NTS

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ stcindustry.com



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 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444

R o b e r t C h a r l e s E v a n s
A r c h i t e c t

5 4 5 P e a r l S t r e e t
F a y e t t e v i l l e , N o r t h C a r o l i n a 2 8 3 0 3

8 March 2024

Harnett County
Building Inspections
420 McKinney Parkway
Lillington, North Carolina 27546
attn: Brad Sutton

re: An Addition for:
Kester Residence
92 Parkview Lane
Lillington , North Carolina 27546

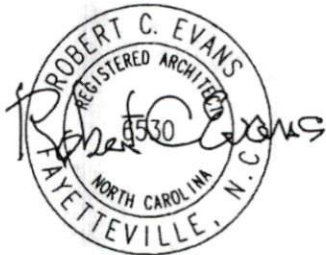
Mr. Sutton,

I have inspected the construction at the above referenced address and can say that the existing monolithic slab and footings meet the minimum width required for 2,000psf load bearing soil and a 2 story residential structure based on Table R403.1(1) , footnote b of the 2018 NC Residential Code.

The existing headers over the windows and doors are double 2x10's and are sufficient to support the new second floor.

If you have any questions concerning this letter, please call me at 910.624.9259.

Sincerely,



Robert Charles Evans
NC Registered Architect #6530

R o b e r t C h a r l e s E v a n s
A r c h i t e c t

5 4 5 P e a r l S t r e e t
F a y e t t e v i l l e , N o r t h C a r o l i n a 2 8 3 0 3

8 March 2024

Harnett County
Building Inspections
420 McKinney Parkway
Lillington, North Carolina 27546
attn: Brad Sutton

re: An Addition for:
Kester Residence
92 Parkview Lane
Lillington , North Carolina 27546

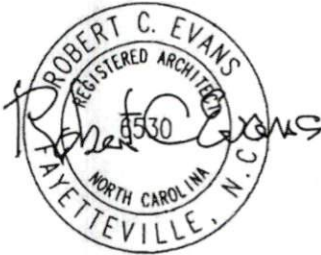
Mr. Sutton,

I have inspected the construction at the above referenced address and can say that the existing monolithic slab and footings meet the minimum width required for 2,000psf load bearing soil and a 2 story residential structure based on Table R403.1(1) , footnote b of the 2018 NC Residential Code.

The existing headers over the windows and doors are double 2x10's and are sufficient to support the new second floor.

If you have any questions concerning this letter, please call me at 910.624.9259.

Sincerely,



Robert Charles Evans
NC Registered Architect #6530

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln. / Harnett Co.
B0324-1551	ET1	GABLE	1	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor

Run: 8.430 s May 12 2021 Print: 8.430 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:45 2024 Page 1
ID:8DY79ggOhK30H1DEIz8UqMzalSr-YpbKKiCSTw0JDg9etXIO8IGHq6IMLdEZgFmTNRzZNI0

0-1-8



0-1-8
Scale = 1:58.4

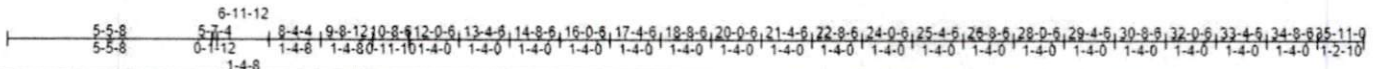
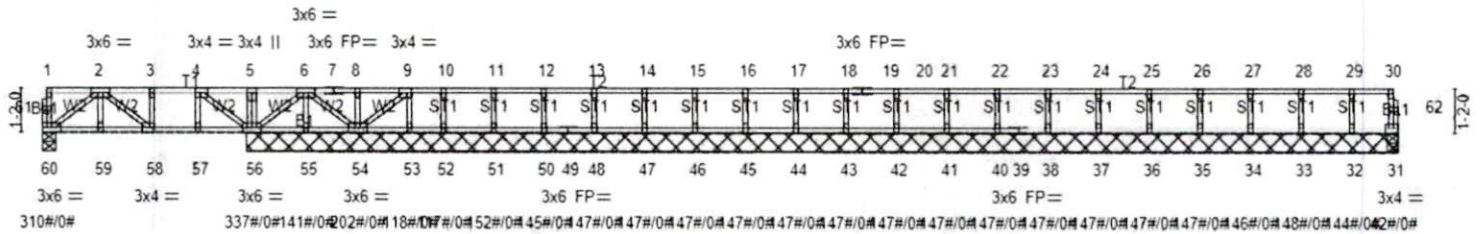


Plate Offsets (X,Y)- [4 0-1-8 Edge], [9 0-1-8 Edge], [58 0-1-8 Edge]

LOADING (psf)	SPACING-	CSI	DEFL.	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.12	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.00	BC 0.15	Vert(LL) -0.01 58 >999 480		
BCLL 0.0	Lumber DOL 1.00	WB 0.10	Vert(CT) -0.02 58 >999 360		
BCDL 5.0	Rep Stress Incr YES	Matrix-S	Horz(CT) -0.00 60 n/a n/a		
	Code IRC2015/TPI2014			Weight 162 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat)
 BOT CHORD 2x4 SP No.1(flat)
 WEBS 2x4 SP No.3(flat)
 OTHERS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 30-5-8 except (jt=length) 60=0-4-4.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 31, 31, 52, 51, 50, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 55, 54, 53 except 60=310(LC 1), 56=337(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-409/0, 3-4=-409/0
 BOT CHORD 59-60=0/353, 58-59=0/353, 57-58=0/409, 56-57=0/409
 WEBS 2-60=-432/0, 4-56=-397/0

JOINT STRESS INDEX

1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 44 = 0.00, 45 = 0.00, 46 = 0.00, 47 = 0.00, 48 = 0.00, 49 = 0.00, 50 = 0.00, 51 = 0.00, 52 = 0.00, 53 = 0.00, 54 = 0.00, 55 = 0.00, 56 = 0.00, 57 = 0.00, 58 = 0.00, 59 = 0.00, 60 = 0.00, 61 = 0.00, 62 = 0.00 and 62 = 0.00

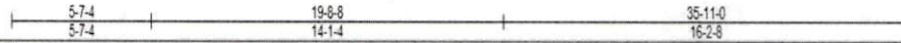
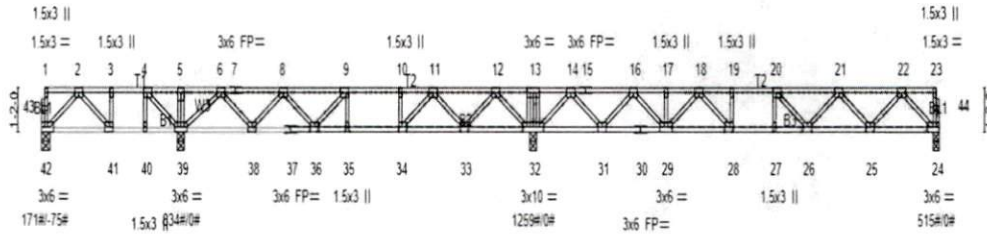
NOTES-

- All plates are 1.5x3 MT20 unless otherwise indicated.
- Plates checked for a plus or minus 1 degree rotation about its center.
- Truss to be fully sheathed on one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 1-4-0 oc.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln. / Harnett Co.
B0324-1551	FA	Floor	9	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run 8:43:05 May 12 2021 Print 8:43:05 May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:46 2024 Page 1
 ID:8DY79ggChK30H1DEIz8UqMzaiSr-109IyeD5EE8ArqkqRFGdgvNYV8K40PvVv1wzN7?



LOADING (psf)	SPACING-	1-4-0	CSI	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL	1.00	TC 0.44	Vert(LL)	-0.10	27	>999	480	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.49	Vert(CT)	-0.14	27	>999	360		
BCLL 0.0	Rep Stress Incr	YES	WB 0.36	Horz(CT)	0.02	24	n/a	n/a		
BCDL 5.0	Code IRC2015/TP12014		Matrix-S							
									Weight: 182 lb	FT = 20%F, 11%E

LUMBER-
 TOP CHORD 2x4 SP No. 1(flat)
 BOT CHORD 2x4 SP No. 1(flat)
 WEBS 2x4 SP No. 3(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 0-3-8 except (if=length) 42=0-4-4, 24=0-3-0.
 (lb) - Max Uplift All uplift 100 lb or less at joint(s) 42
 Max Grav All reactions 250 lb or less at joint(s) 42 except 39=834(LC 3), 32=1259(LC 11), 24=515(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-162/342, 3-4=-162/342, 4-5=0/698, 5-6=0/698, 6-7=-447/20, 7-8=-447/20,
 8-9=-940/54, 9-10=-1014/187, 10-11=-1014/187, 11-12=-344/611, 12-13=0/1429,
 13-14=0/1429, 14-15=-371/263, 15-16=-371/263, 16-17=-1289/0, 17-18=-1289/0,
 18-19=-1751/0, 19-20=-1751/0, 20-21=-1605/0, 21-22=-1047/0
 BOT CHORD 40-41=-342/162, 39-40=-342/162, 37-38=-1/810, 36-37=-1/810, 35-36=-187/1014,
 34-35=-187/1014, 33-34=-417/732, 32-33=-795/0, 31-32=-522/0, 30-31=-90/908,
 29-30=-90/908, 28-29=0/1570, 27-28=0/1751, 26-27=0/1751, 25-26=0/1437, 24-25=0/636
 WEBS 2-41=-283/0, 4-39=-616/0, 12-32=0/573, 11-33=-603/0, 11-34=0/568,
 8-38=-474/0, 14-32=-1104/0, 14-31=0/761, 16-31=-736/0, 16-29=0/524, 18-29=-400/0,
 18-28=0/437, 22-24=-796/0, 22-25=0/535, 21-25=-507/0, 6-38=0/501, 6-39=-854/0

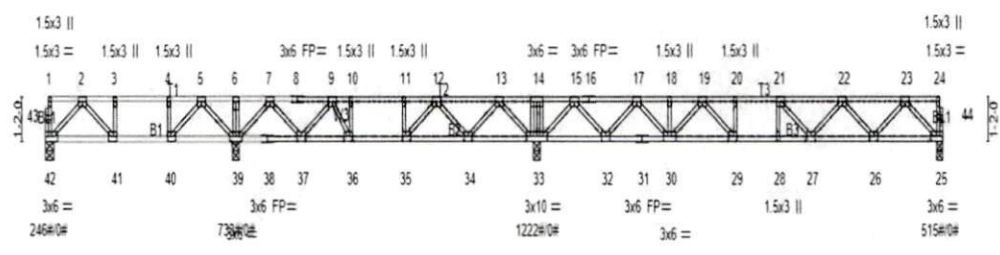
JOINT STRESS INDEX
 1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 43 = 0.00, 44 = 0.00 and 44 = 0.00

- NOTES-**
- Unbalanced floor live loads have been considered for this design.
 - All plates are 3x4 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 1 degree rotation about its center.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 42.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0 131" X 3") nails Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln / Hamett Co.
B0324-1551	FB	Floor	4	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run 8:430 s May 12 2021 Print 8:430 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:47 2024 Page 1
 ID: 8DY79ggOnK30H1DEtz8UqMzaiSr-VCj49_Dj?XG1Tz1?yinsD7MZVvUspSt%8ZFAsKzZNI



7-7-14	19-6-8	35-11-0
7-7-14	12-0-10	16-2-8
Plate Offsets (X,Y) - [21 0-1-8 Edge] [29 0-1-8 Edge] [35 0-1-8 Edge] [38 0-1-8 Edge] [40 0-1-8 Edge] [41 0-1-8 Edge]		

LOADING (psf)	SPACING-	CSI	DEFL	PLATES	GRIP
TCLL 40.0	1-4-0	TC 0.36	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.00	BC 0.47	Vert(LL) -0.10 28 >999 480		
BCLL 0.0	Lumber DOL 1.00	WB 0.36	Vert(CT) -0.14 28 >999 360		
BCDL 5.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.02 25 n/a n/a		
	Code IRC2015/TPI2014				Weight: 181 lb FT = 20%F, 11%E

LUMBER-
 TOP CHORD 2x4 SP No.1(flat)
 BOT CHORD 2x4 SP No.1(flat)
 WEBS 2x4 SP No.3(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 0-3-8 except (l=length) 42=0-4-4, 25=0-3-0.
 (lb) - Max Grav. All reactions 250 lb or less at joint(s) 42 except 39=733(LC 3), 33=1222(LC 11), 25=515(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-385/33, 3-4=-385/33, 4-5=-385/33, 5-6=0/444, 6-7=0/444, 7-8=-460/141,
 8-9=-460/141, 9-10=-750/276, 10-11=-750/276, 11-12=-750/276, 12-13=-250/651,
 13-14=0/1419, 14-15=0/1419, 15-16=-360/252, 16-17=-360/252, 17-18=-1260/0,
 18-19=-1260/0, 19-20=-1745/0, 20-21=-1745/0, 21-22=-1601/0, 22-23=-1045/0
 BOT CHORD 41-42=0/266, 40-41=-33/385, 36-37=-181/701, 35-36=-276/750, 34-35=-477/571,
 33-34=-824/0, 32-33=-513/0, 31-32=-80/898, 30-31=-80/898, 29-30=0/1562, 28-29=0/1745,
 27-28=0/1745, 26-27=0/1434, 25-26=0/635
 WEBS 13-33=-852/0, 13-34=0/525, 12-34=-529/0, 12-35=0/452, 7-39=-605/0, 7-37=0/353,
 9-37=-323/52, 2-42=-331/0, 5-39=-455/0, 5-40=0/393, 15-33=-1103/0, 15-32=0/760,
 17-32=-734/0, 17-30=0/523, 19-30=-399/0, 19-29=0/435, 23-25=-796/0, 23-26=0/534,
 22-26=-506/0

JOINT STRESS INDEX
 1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 44 = 0.00 and 44 = 0.00

- NOTES-**
- Unbalanced floor live loads have been considered for this design.
 - All plates are 3x4 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 1 degree rotation about its center.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION. Do not erect truss backwards.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln. / Hamett Co
B0324-1551	ET1	GABLE	1	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor

Run: 8.430 s May 12 2021 Print: 8.430 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:45 2024 Page 1
ID:8DY79ggOhK30H1DEIz8UqMzalSr-YpbKKICSTw0JDg9etXIO8IGHq6tMLdEZgFmTNRzZNI0

0-1-8



0-1-8
Scale = 1/58.4

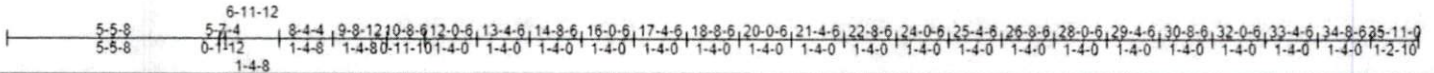
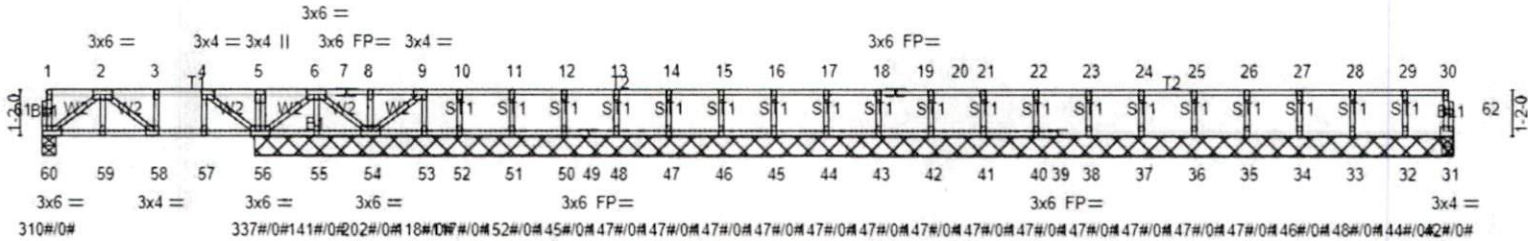


Plate Offsets (X,Y)-- [4:0-1-8,Edge], [9:0-1-8,Edge], [58:0-1-8,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	2-0-0	TC 0.12	Vert(LL)	-0.01	58	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.00	BC 0.15	Vert(CT)	-0.02	58	>999		
BCLL 0.0	Lumber DOL 1.00	WB 0.10	Horz(CT)	-0.00	60	n/a		
BCDL 5.0	Rep Stress Incr YES	Matrix-S						
	Code IRC2015/TPI2014							
							Weight: 162 lb	FT = 20%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	
OTHERS 2x4 SP No.3(flat)	

REACTIONS. All bearings 30-5-8 except (jt=length) 60=0-4-4.
(lb) - Max Grav All reactions 250 lb or less at joint(s) 31, 31, 52, 51, 50, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 55, 54, 53 except 60=310(LC 1), 56=337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-409/0, 3-4=-409/0
BOT CHORD 59-60=0/353, 58-59=0/353, 57-58=0/409, 56-57=0/409
WEBS 2-60=-432/0, 4-56=-397/0

JOINT STRESS INDEX
1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 44 = 0.00, 45 = 0.00, 46 = 0.00, 47 = 0.00, 48 = 0.00, 49 = 0.00, 50 = 0.00, 51 = 0.00, 52 = 0.00, 53 = 0.00, 54 = 0.00, 55 = 0.00, 56 = 0.00, 57 = 0.00, 58 = 0.00, 59 = 0.00, 60 = 0.00, 61 = 0.00, 62 = 0.00 and 62 = 0.00

- NOTES-**
- All plates are 1.5x3 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 1 degree rotation about its center.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 1-4-0 oc.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.

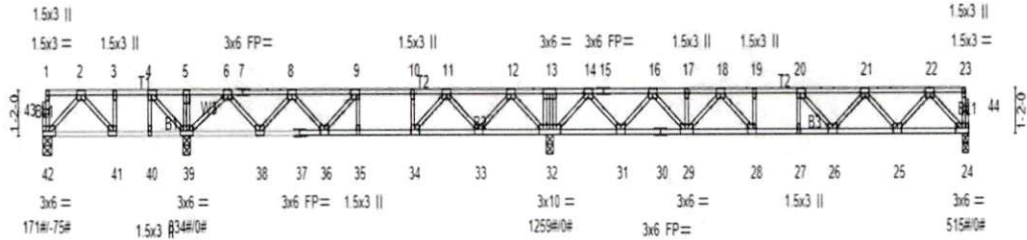
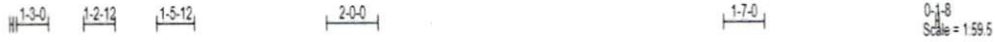
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln / Hammett Co
B0324-1551	FA	Floor	9	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor

Run: 8:43:05 May 12 2021 Print: 8:43:05 May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:46 2024 Page 1
ID:8DY79ggOnK30H1DEIz8UqMzalsr-109yeD5EE8ArqkqRFgDgvpNYV8K40PwV1wtzZNI?

0-1-8



5-7-4	19-8-8	35-11-0
5-7-4	14-1-4	16-2-8
Plate Offsets (X,Y) - [4 0-1-8 Edge] [9 0-1-8 Edge] [20 0-1-8 Edge] [28 0-1-8 Edge] [34 0-1-8 Edge] [41 0-1-8 Edge]		

LOADING (psf)	SPACING-	1-4-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.44	Vert(LL)	-0.10	27	>999	480	MT20 244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.49	Vert(CT)	-0.14	27	>999	360	
BCLL 0.0	Rep Stress Incr	YES	WB 0.36	Horz(CT)	0.02	24	n/a	n/a	
BCDL 5.0	Code IRC2015/TPI2014		Matrix-S						Weight 182 lb FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat)
BOT CHORD 2x4 SP No.1(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

All bearings 0-3-8 except (if=length) 42=0-4-4, 24=0-3-0.
(lb) - Max Uplift All uplift 100 lb or less at joint(s) 42
Max Grav All reactions 250 lb or less at joint(s) 42 except 39=834(LC 3), 32=1259(LC 11), 24=515(LC 5)

FORCES.

(lb) - Max Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-162/342, 3-4=-162/342, 4-5=0/698, 5-6=0/698, 6-7=-447/20, 7-8=-447/20, 8-9=-940/54, 9-10=-1014/187, 10-11=-1014/187, 11-12=-344/611, 12-13=0/1429, 13-14=0/1429, 14-15=-371/263, 15-16=-371/263, 16-17=-1289/0, 17-18=-1289/0, 18-19=-1751/0, 19-20=-1751/0, 20-21=-1605/0, 21-22=-1047/0
BOT CHORD 40-41=-342/162, 39-40=-342/162, 37-38=-1/810, 36-37=-1/810, 35-36=-187/1014, 34-35=-187/1014, 33-34=-417/732, 32-33=-795/0, 31-32=-522/0, 30-31=-90/908, 29-30=-90/908, 28-29=0/1570, 27-28=0/1751, 26-27=0/1751, 25-26=0/1437, 24-25=0/636
WEBS 2-41=-283/0, 4-39=-616/0, 12-32=-911/0, 12-33=0/573, 11-33=-603/0, 11-34=0/568, 8-38=-474/0, 14-32=-1104/0, 14-31=0/761, 16-31=-738/0, 16-29=0/524, 18-29=-400/0, 18-28=0/437, 22-24=-796/0, 22-25=0/535, 21-25=-507/0, 6-38=0/501, 6-39=-854/0

JOINT STRESS INDEX

1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 43 = 0.00, 44 = 0.00 and 44 = 0.00

NOTES-

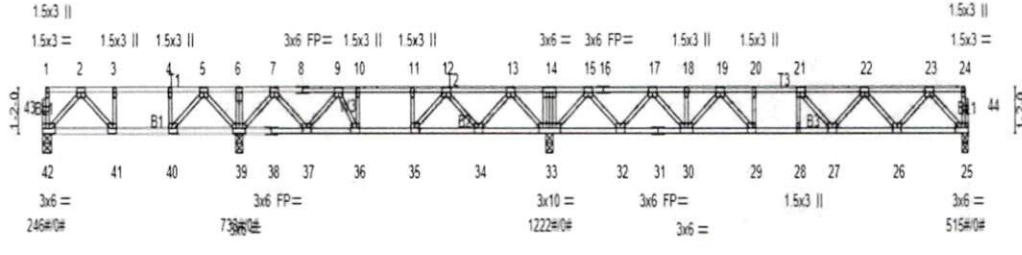
- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 42.
- 5) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln / Hamett Co.
B0324-1551	FB	Floor	4	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor

Run: 8:43:05 s May 12 2021 Print: 8:43:05 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:16:47 2024 Page 1
ID: 8DY79ggOhK30H1DEtZ8UqMzaiSr-VCj49_Dj7XG1TzJ1?ysD7MZwUspSts6ZFaSKzZNI



	7-7-14	19-8-8	35-11-0
	7-7-14	12-0-10	16-2-8
Plate Offsets (X,Y)-	[21 0-1-8 Edge]	[29 0-1-8 Edge]	[35 0-1-8 Edge]
		[38 0-1-8 Edge]	[40 0-1-8 Edge]
			[41 0-1-8 Edge]

LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.36	Vert(LL) -0.10 28 >999 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.47	Vert(CT) -0.14 28 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.36	Horz(CT) 0.02 25 n/a n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			
					Weight 181 lb FT = 20%F, 11%E

LUMBER-
 TOP CHORD 2x4 SP No.1(flat)
 BOT CHORD 2x4 SP No.1(flat)
 WEBS 2x4 SP No.3(flat)

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 0-3-8 except (if=length) 42=0-4-4, 25=0-3-0.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 42 except 39=733(LC 3), 33=1222(LC 11), 25=515(LC 5)

FORCES. (lb) - Max Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD
 2-3=-385/33, 3-4=-385/33, 4-5=-385/33, 5-6=0/444, 6-7=0/444, 7-8=-460/141,
 8-9=-460/141, 9-10=-750/276, 10-11=-750/276, 11-12=-750/276, 12-13=-250/651,
 13-14=0/1419, 14-15=0/1419, 15-16=-360/252, 16-17=-360/252, 17-18=-1280/0,
 18-19=-1280/0, 19-20=-1745/0, 20-21=-1745/0, 21-22=-1901/0, 22-23=-1045/0

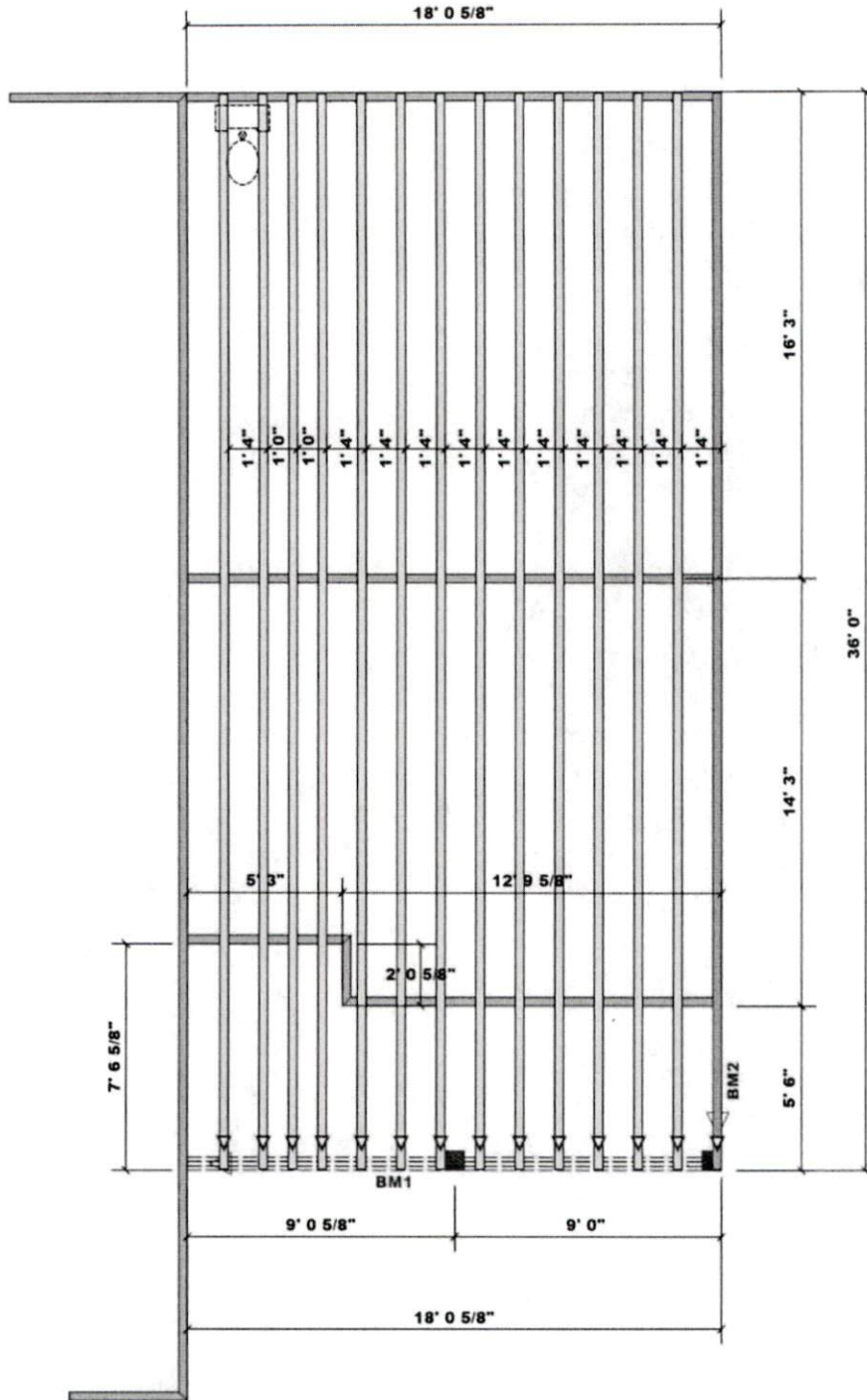
BOT CHORD
 41-42=0/266, 40-41=-33/385, 36-37=-181/701, 35-36=-276/750, 34-35=-477/571,
 33-34=-824/0, 32-33=-513/0, 31-32=-80/898, 30-31=-80/898, 29-30=0/1582, 28-29=0/1745,
 27-28=0/1745, 26-27=0/1434, 25-26=0/635

WEBS
 13-33=-852/0, 13-34=0/525, 12-34=-529/0, 12-35=0/452, 7-39=-605/0, 7-37=0/353,
 9-37=-323/52, 2-42=-331/0, 5-39=-455/0, 5-40=0/393, 15-33=-1103/0, 15-32=0/760,
 17-32=-734/0, 17-30=0/523, 19-30=-399/0, 19-29=0/435, 23-25=-795/0, 23-26=0/534,
 22-26=-506/0

JOINT STRESS INDEX
 1 = 0.00, 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00, 41 = 0.00, 42 = 0.00, 43 = 0.00, 44 = 0.00 and 44 = 0.00

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x4 MT20 unless otherwise indicated.
 - 3) Plates checked for a plus or minus 1 degree rotation about its center.
 - 4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard



▲ Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS

BASED ON TABLE 1001.1-1.1
 NUMBER OF JACK STUDS REQUIRED IS DEPENDENT ON JOIST SPACING

TRUSS SPACING (ON CENTER)	TRUSS DEPTH (INCHES)	MAXIMUM JOIST SPACING (ON CENTER)	MAXIMUM JOIST WEIGHT (LBS/FT)
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

BUILDER	Brunden Teth	CITY / CO.	Lillington / Harnett
JOB NAME	92 Parkview Ln. / Harnett Co.	ADDRESS	92 Parkview Ln.
PLAN	Plan	MODEL	Floor
SEAL DATE	Self Date	DATE REV.	03/19/24
QUOTE #	80324-1951	DRAWN BY	Dwayne Naylor
JOB #	Order #	SALES REP.	Dwayne Naylor

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables / derived from the prescriptive Code requirements 1) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Dwayne Naylor
 Dwayne Naylor

Truss Placement Plan
 SCALE: 1/4" = 1'-0"

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls and columns is the responsibility of the building designer. For general guidance regarding bracing, consult ICC-ES E1 and E2-6.8.2 provided with the truss delivery package or online @ stcindustry.com



ROOF & FLOOR TRUSSES & BEAMS

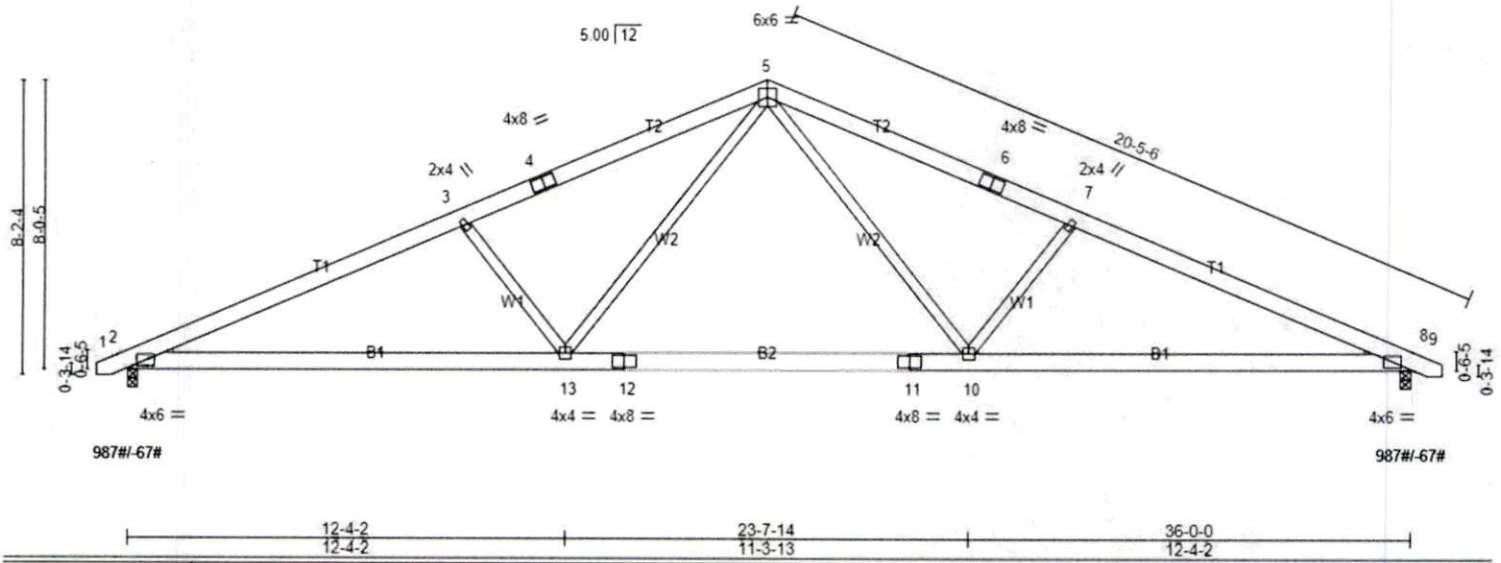
Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln. / Harnett Co.
B0324-1624	A1	COMMON	13	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run: 8.430 s May 12 2021 Print: 8.430 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:15:33 2024 Page 1
 ID: 8DY79ggOhk30H1DEIz8UqMzaiSr-fzF6dhKIQoyXacfggQSVdWlyZ4F7U2Uz72JqsczZNJ8



Scale = 1:61.9



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.21	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.38	Vert(LL) -0.19 10-13 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.16	Vert(CT) -0.27 10-13 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.05 8 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.06 13-16 >999 240		
				Weight: 221 lb	FT = 25%

LUMBER-
 TOP CHORD 2x6 SP No. 1
 BOT CHORD 2x6 SP No. 1
 WEBS 2x4 SP No. 2

BRACING-
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (size) 2=0-3-8 (min. 0-1-8), 8=0-3-8 (min. 0-1-8)
 Max Horz 2=62(LC 16)
 Max Uplift 2=-67(LC 12), 8=-67(LC 13)
 Max Grav 2=987(LC 1), 8=987(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-20=-1922/400, 3-20=-1887/418, 3-4=-1729/375, 4-21=-1675/385, 5-21=-1670/399,
 5-22=-1670/399, 6-22=-1675/385, 6-7=-1729/375, 7-23=-1887/418, 8-23=-1922/400
 BOT CHORD 2-13=-304/1742, 12-13=-146/1161, 12-24=-146/1161, 24-25=-146/1161, 11-25=-146/1161,
 10-11=-146/1161, 8-10=-311/1742
 WEBS 5-10=-81/647, 7-10=-393/195, 5-13=-81/647, 3-13=-393/195

JOINT STRESS INDEX
 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00 and 13 = 0.00

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf; h=15ft, Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-8-0 to 3-8-13, Interior(1) 3-8-13 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13, Interior(1) 22-4-13 to 36-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln / Hamett Co.
B0324-1624	A1GE	GABLE	1	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run 8:43:05 May 12 2021 Print: 8:43:05 May 12 2021 MITek Industries, Inc. Tue Mar 19 13:15:34 2024 Page 1
 ID:8DY79ggChK30H1DEIz6UqMzaiSr-89pJr1LwB64OCIE7D7zkmkqA1UgwDVCSL3DO3zZNU7



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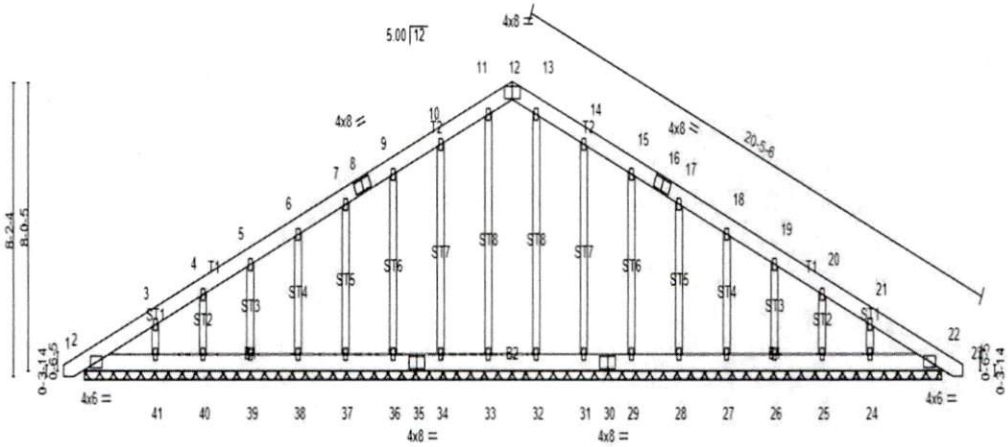


Plate Offsets (X,Y) - [12 0-4-0 Edge]

LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.4-0	TC 0.04	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(LL) 0.00 22 n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.07	Vert(CT) 0.00 22 n/r 120		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Horz(CT) 0.00 22 n/a n/a		
				Weight 271 lb FT = 25%	

LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 36-0-0
 (lb) - Max Horz 2=105(LC 16)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 34, 36, 37, 38, 39, 40, 41, 31, 29, 28, 27, 26, 25, 24
 Max Grav All reactions 250 lb or less at joint(s) 2, 22, 33, 34, 36, 37, 38, 39, 40, 41, 32, 31, 29, 28, 27, 26, 25, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

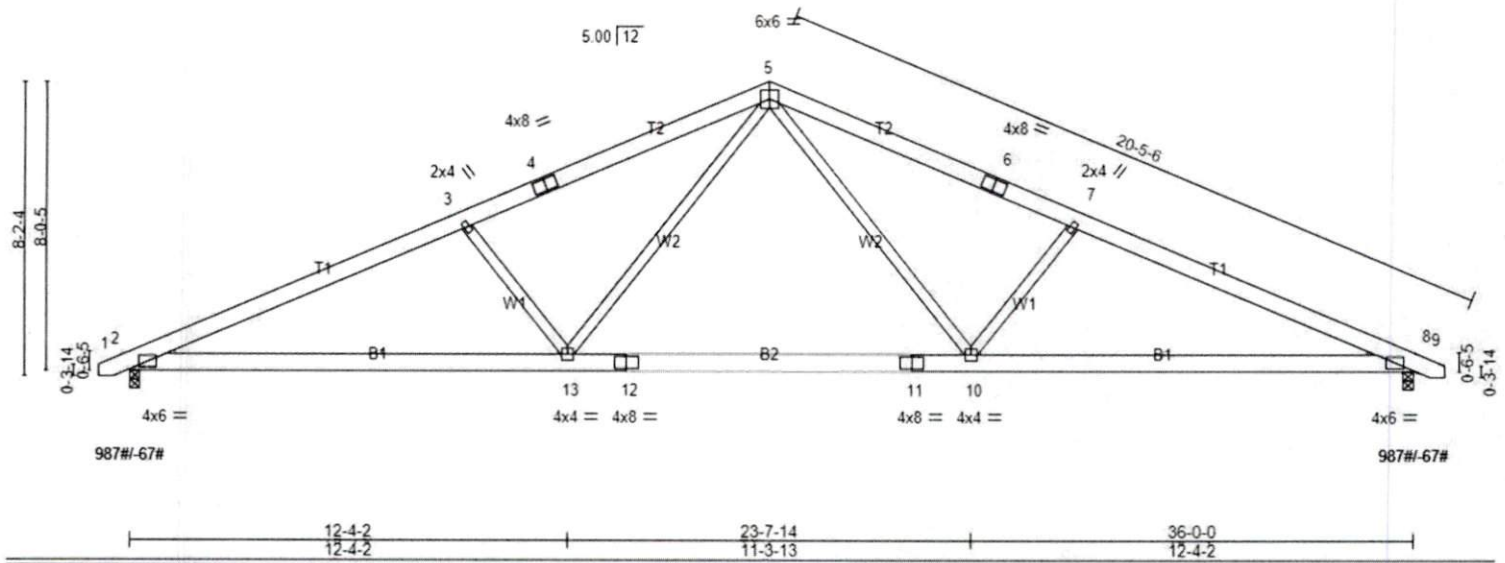
JOINT STRESS INDEX
 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00,
 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00,
 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00 and 41 = 0.00

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grp DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 22, 34, 36, 37, 38, 39, 40, 41, 31, 29, 28, 27, 26, 25, 24.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln. / Harnett Co.
B0324-1624	A1	COMMON	13	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run 8.430 s May 12 2021 Print: 8.430 s May 12 2021 MiTek Industries, Inc. Tue Mar 19 13:15:33 2024 Page 1
 ID: 8DY79ggOhK30H1DEIz8UqMzaiSr-fzF6dhKIQoyXacfpqQSVOWlyZ4F7U2Uz72JgsczZNJ8



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.21	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.38	Vert(LL) -0.19 10-13 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.16	Vert(CT) -0.27 10-13 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.05 8 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.06 13-16 >999 240		
				Weight 221 lb	FT = 25%

LUMBER-
 TOP CHORD 2x6 SP No. 1
 BOT CHORD 2x6 SP No. 1
 WEBS 2x4 SP No. 2

BRACING-
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (size) 2=0-3-8 (min. 0-1-8), 8=0-3-8 (min. 0-1-8)
 Max Horz 2=62(LC 16)
 Max Uplift 2=-67(LC 12), 8=-67(LC 13)
 Max Grav 2=987(LC 1), 8=987(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-20=-1922/400, 3-20=-1887/418, 3-4=-1729/375, 4-21=-1675/385, 5-21=-1670/399,
 5-22=-1670/399, 6-22=-1675/385, 6-7=-1729/375, 7-23=-1887/418, 8-23=-1922/400
 BOT CHORD 2-13=-304/1742, 12-13=-146/1161, 12-24=-146/1161, 24-25=-146/1161, 11-25=-146/1161,
 10-11=-146/1161, 8-10=-311/1742
 WEBS 5-10=-81/647, 7-10=-393/195, 5-13=-81/647, 3-13=-393/195

JOINT STRESS INDEX
 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00 and 13 = 0.00

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf, BCDL=6.0psf, h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-8-0 to 3-8-13, Interior(1) 3-8-13 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13, Interior(1) 22-4-13 to 36-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
 - 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	92 Parkview Ln / Harnett Co.
B0324-1624	A1GE	GABLE	1	1	Job Reference (optional)

Comtech, Inc., Fayetteville, NC 28309, Dwayne Naylor Run 8:430 s May 12 2021 Print 8:430 s May 12 2021 MITek Industries, Inc. Tue Mar 19 13:15:34 2024 Page 1
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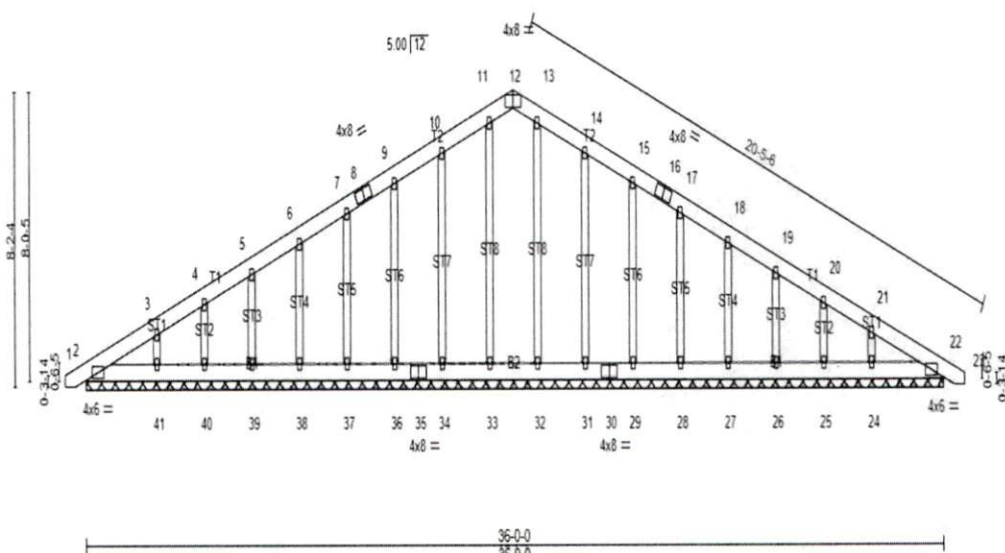


Plate Offsets (X,Y) - [12'-0.4'-0 Edge]

LOADING (psf)	SPACING-	1-4-0	CSI	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15	TC 0.04	Vert(LL)	0.00	22	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.01	Vert(CT)	0.00	22	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	22	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S							Weight 271 lb FT = 25%

LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

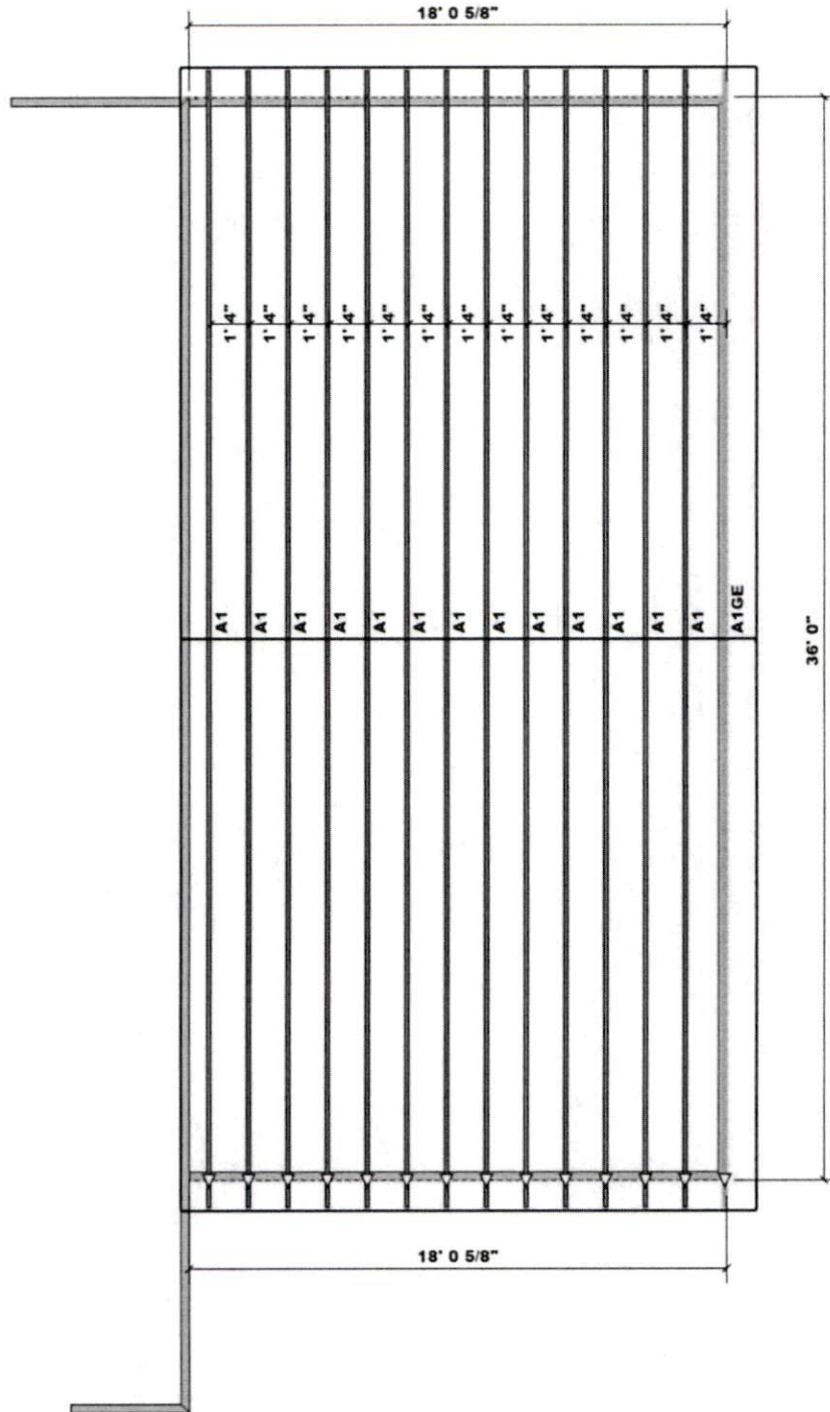
REACTIONS. All bearings 36'-0-0
 (lb) - Max Horz 2=105(LC 16)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 34, 36, 37, 38, 39, 40, 41, 31, 29, 28, 27, 26, 25, 24
 Max Grav All reactions 250 lb or less at joint(s) 2, 22, 33, 34, 36, 37, 38, 39, 40, 41, 32, 31, 29, 28, 27, 26, 25, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

JOINT STRESS INDEX
 2 = 0.00, 3 = 0.00, 4 = 0.00, 5 = 0.00, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00, 28 = 0.00, 29 = 0.00, 30 = 0.00, 31 = 0.00, 32 = 0.00, 33 = 0.00, 34 = 0.00, 35 = 0.00, 36 = 0.00, 37 = 0.00, 38 = 0.00, 39 = 0.00, 40 = 0.00 and 41 = 0.00

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=15ft, Cat. II; Exp C, Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - All plates are 2x4 MIT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2'-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 22, 34, 36, 37, 38, 39, 40, 41, 31, 29, 28, 27, 26, 25, 24.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



▲ = Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS

BASED ON TABLES 400 TO 410
 NUMBER OF JACK STUDS REQUIRED IS IN THE SP
 HEADERS COLUMN

TRUSS SPACING	TRUSS HEIGHT	TRUSS WEIGHT	TRUSS AREA	TRUSS PERIOD	TRUSS PERIOD	TRUSS PERIOD
1700	1	2550	1	3400	1	
3400	2	5100	2	6800	2	
5100	3	7650	3	10200	3	
6800	4	10200	4	13600	4	
8500	5	12750	5	17000	5	
10200	6	15300	6			
11900	7					
13600	8					
15300	9					

BUILDER	Brandon Toth	CITY / CO.	Litlington / Harnett
JOB NAME	92 Parkview Ln. / Harnett Co.	ADDRESS	92 Parkview Ln.
PLAN	Plan	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	03/19/24
QUOTE #	80324-1624	DRAWN BY	Sales Area
JOB #	Order #	SALES REP.	Dwayne Naylor

Beating reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: _____ Sales Area: _____
 Sales Area

Truss Placement Plan
 SCALE: NTS

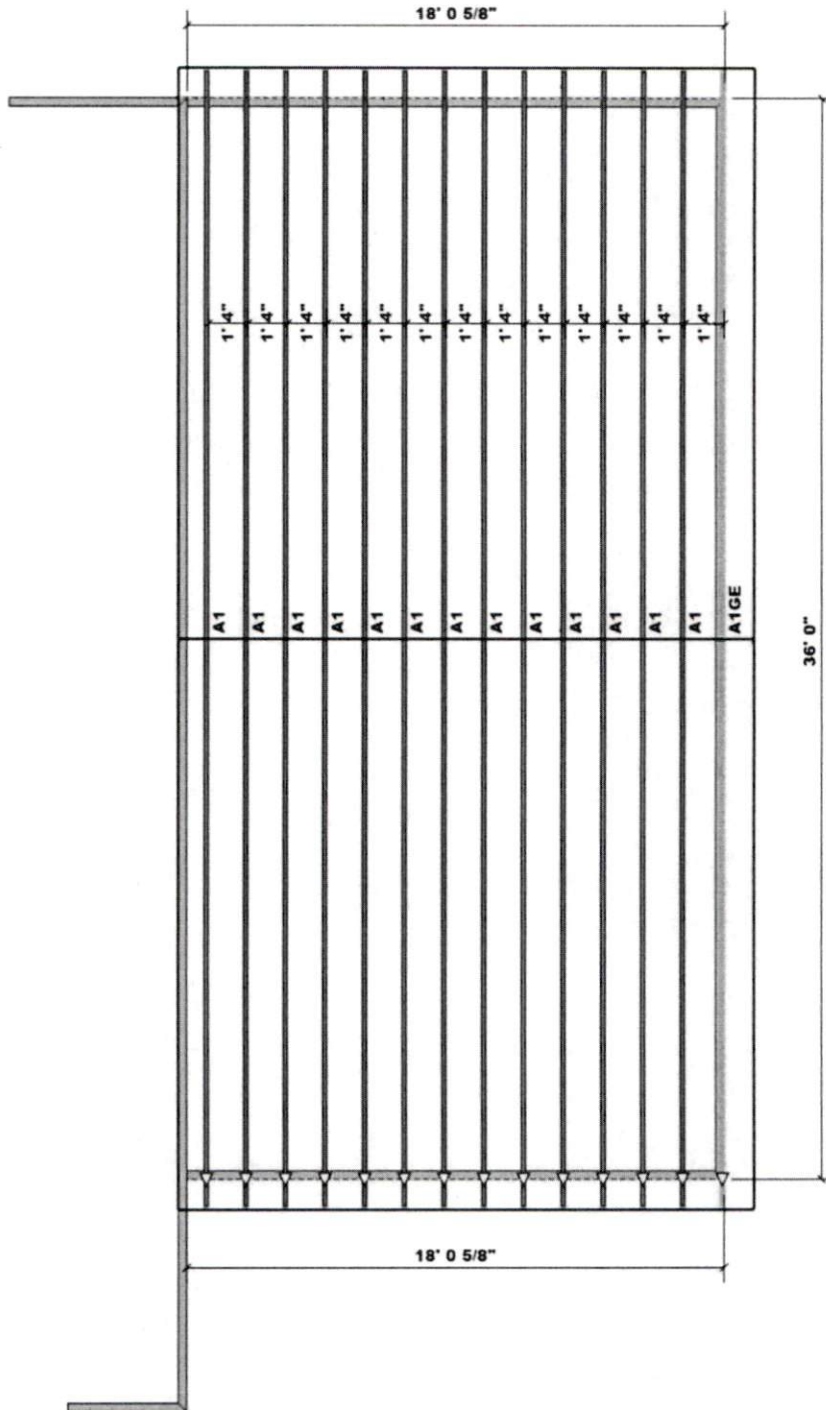
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls and columns is the responsibility of the building designer. For general guidance regarding bracing, consult ICC-ES E1 and E20-03 provided with the truss delivery package or online @ trussindustry.com



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444



▲ = Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS

BASED ON TABLE 402.101-1.1
 NUMBER OF JACK STUDS REQUIRED IS 24 END OF HEADS-ESSES

SPAN LENGTH (FEET)	MAXIMUM UNIFORM LOAD (PSF)	MAXIMUM POINT LOAD (KIP)	MAXIMUM UNIFORM LOAD (PSF)	MAXIMUM POINT LOAD (KIP)
1700	1	2500	1	3400
3400	2	5100	2	6800
5100	3	7650	3	10200
6800	4	10200	4	13600
8500	5	12750	5	17000
10200	6	15300	6	
11900	7			
13600	8			
15300	9			

BUILDER	Brandon Toth	CITY / CO.	Litlington / Harnett
JOB NAME	92 Parkview Ln. / Harnett Co.	ADDRESS	92 Parkview Ln.
PLAN	Plan	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	03/19/24
QUOTE #	80324-1624	DRAWN BY	Solix Area
JOB #	Order #	SALES REP.	Dwayne Naylor

Beaming reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. This contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 1500#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 1500#.

Signature: _____ Sales Area: _____
 Sales Area

Truss Placement Plan
 SCALE: NTS

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult ICC-ES and ECR-83 provided with the truss delivery package or online @ vtrussindustry.com



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