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#### STRUCTURAL NOTES

#### GENERAL

1, DESIGN CODE DATA

2018 INTERNATIONAL BUILDING CODE ASCE 7-16: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. AISC 360-05: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS

2. DESIGN LOADS:

ROOF DEAD LOADS: 10 PSF ROOF LIVE LOADS: 20 PSF

3, WIND DESIGN CRITERIA

WIND LOAD: 150 MPH 3 SECOND GUST NOMINAL DESIGN WIND SPEED: 116 MPH INTERNAL PRESSURE COEFFICIENT: 0.0 (OPEN) RISK CATEGORY 1 BUILDING (THIS STRUCTURE IS DESIGNED AS AN UNINHABITABLE STRUCTURE AND SHALL NOT BE CONVERTED TO A HABITABLE WITHOUT PRIOR APPROVAL FROM ENGINEER OF RECORD) EXPOSURE CATEGORY 8 BASE VELOCITY PRESSURE: 34.3 PSF

4. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON OR EXISTING STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.

	COMPONE	NTS AND CLADDIN	G	
ROOFS!	DESIGN PRE	SSURE (UET)	DESIGN PRE	SSURE (ASO)
TRIBUTARY AREA 10 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
C ZONE That,	- \$33,4"	30(7 ***	20,0	18.4
ZONE 2 max	51.6	47,6	30,9	28.5
· . ZONE 3 from	66,8	-61.5	40.1 / "	36,9
TRIBUTARY AREA 100 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
C SZONE I MAIL I	+ 33.4"	30,7.6	120,0	18,4 7
ZONE 2 max	33.4	47.6	20.0	28,5
* ZONE TIMAL TO	324	Se. 717.60	* ** * 20.0	** 28.5 .1
WALLS	DESIGN P	RESSURE	DESIGN	PRESSURE
TRIBUTARY AREA 10 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSE)
ZONE 4	34.3	37.7	20.6	22.6
- ZONE'5"	34/3	78.0	20.6	28.85 7
TRIBUTARY AREA 100 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
- ZONE'4"	₹ 28.2	* 31.7	" 15.9	3 19.0
ZONE 5	28.2	35,9	16.9	21,5

BUILDING RISK CATEGORY 1 - AGRICULTURAL STORAGE STRUCTURES INTENDED ONLY FOR INCIDENTAL HUMAN OCCUPANCY, OR A DETACHED ONE & TWO FAMILY DWELLING ASSIGNED SDC A, B, OR C, OR WITH A Ss, IS LESS THAN 0.4g.

5, SNOW LOAD (S)

19.00psf = Roof Snow Load 25psf = Ground Snow Load

Thermal Factor = 1.00 Snow Exposure Factor = 1.00 Snow Importance Factor = 0.80

6. SEISMIC LOAD (E)

Equivalent Lateral Force Procedure

0.35 = 0.2s Short Period Spectral Response Acceleration S(s)

0.12 = 1.0s Spectral Response Acceleration S(1)

Site Classification =0

Seismic Importance Factor =1.00

Seismic Design Category = C

Seismic Design Short Period Acceleration, Sds = 0.355

Seismic Design 1 Sec Period Acceleration, SD1 = 1.189

Cantilever Columns for Timber Frames, R=1.5 Transverse & Longitudinal.

Analysis Procedure: Under Section 1613 of the IBC 2018, Earthquake Loads are not required to be considered for a Risk Category 1 Structure.



7. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.

8.THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF CONSTRUCTION AND ALL JOB SITE

9. VERIFY ALL DIMENSIONS PRIOR TO THE START OF CONSTRUCTION - DO NOT SCALE DRAWINGS.

FOOTING AND FOUNDATION WALL 3,000 PSI @ 28 DAYS SLAB ON GRADE 3.000 PSI @ 28 DAYS ALL OTHER CIP CONCRETE 3.000 PSI @ 28 DAYS CONCRETE REINFORCING STEEL **60 KSI, ASTM A615** 

1. ALLOWABLE SOIL BEARING CAPACITY = 2,000 PSF FOR FOOTINGS (PRESUMPTIVE)

2. GRADE AREAS IN ACCORDANCE WITH ELEVATIONS AND GRADES SHOWN ON THE SITE DRAWINGS AND AS REQUIRED FOR DRAINAGE.

3. ALL SLAB ON GRADE AREAS SHALL BE PROOF ROLLED. ALL SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH COMPACTABLE FILL.

4. SLAB ON GRADE TO BE CONSTRUCTED ON A MINIMUM OF 6" OF COMPACTED GRANULAR FILL

5, ALL FILL MATERIAL USED IN GRADING OPERATIONS SHALL CONSIST OF EARTH, WHICH IS FREE OF DEBRIS, BOULDERS OR ORGANIC MATERIAL, FILL SHALL BE PLACED IN MAXIMUM OF 12' LIFTS AND COMPACTED TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY.

6. ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR COMPACTED FILL HAVING A MINIMUM ALLOWABLE BEARING CAPACITY AS INDICATED ABOVE.

7. THE ENGINEER SHALL BE NOTIFIED IF ACTUAL FIELD CONDITIONS DO NOT MEET BEARING REQUIREMENTS OR, IF QUESTIONABLE SOIL CONDITIONS ARE DISCOVERED INCLUDING BUT NOT LIMITED TO PEAT AND OTHER HIGH ORGANIC SOILS.

8. ANY FOUNDATION UNDER THE BASE FLOOD ELEVATION SHALL COMPLY W/ R322.2.2 OF THE FLORIDA BUILDING CODE & WILL PROVIDE FLOOD VENTS TO MEET THESE REQUIREMENTS.

ANGLES, PLATES, AND CHANNELS 36 KSI, ASTM A36 SQUARE AND RECTANGULAR HSS 46 KSI, ASTM A500 GRADE B,

(2) GRADE A5, 5/8" W/ A325 BOLTS BOLTS @ RIDGE (PEAK):

(4) FLAT & (2) LOCK WASHERS

(2) 5/8" X 12" CARRIAGE BOLTS & BOLTS @ TRUSS TO POST CONN.

(4) 2"x2"x1/8" FLAT & LOCK

WASHERS

WELDING ELECTRODES (ALL WELDS ARE FULL PENETRATION WELDS U.N.O.)

13. WOOD

SYP NO. 2 OR BETTER 2x8 AND SMALLER

MINIMUM DESIGN VALUES

1,500 PSI

825 PSI

175 PSI

565 PSI

1,600 PS 1,600,000 PSI

Emin 580,000 PSI

(5) 4

WIND ZONES

WALL \*3'-0" EDGE DISTANCE



Gable and Flat Roofs 7° < Ø < 45°

6x6, 8X8 OR 10X10 WD POST TREATED SOUTHERN YELLOW PINE (SYP) MINIMUM DESIGN VALUES

Fb 1,350 PSI

1,500,000 PSI

14. ROOF & WALL SHEATHING ROOF METAL PANELS SHALL BE 26 GA. x 3'-0" WIDE w/ 5 RIBS (RECOMMENDED)

OR 29 GA. x 3'-0" WIDE w/ 5 RIBS

1	9	0	Ø	Ø	0	8
	9	0	8	3	0	3
<u> </u>	0	0	0	3	0	09

S Tube -50' HSS sckwater 12'-50 Black 8736 I JOB NUMBER RAWN BY CHECKED BY LOT DATE SHEET TITLE GENERAL NOTES DRAWMING NUMBER

S100

3200 W. 23RD STREET

PANAMA CITY, FL 32405

MIKWEBER.COM

EOR: MICHAEL K. WEBER P.E

STRUC TURAL ENGINEER
NORTH CAROLINA PE.#043557

Drawings

Truss Dra Systems

ANTED COMES OF TH

THIS BUILDING MAY BE USED FOR RESIDENTIAL USE ONLY

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Blackwater Truss Systems
R7875 Hwy 87 N.
Milton, FL 32570 40'-0" 39-1" OVERALL TRUSS LENGTH 12 CHECKED BY PLOT DATE 2 40' TRUSS w/ BOBTAIL STHEET TITLE DRAWMING NUMBER S111

# GENERAL NOTES

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THE - HALS SKEE SHEEKS IN HE DAWNES, HE SHEETS SALS SHEEKS IN HIS THAN AND THAN ARREST FROM IN SALSSE HARRIST IN HIS THAN A KARDES SHALL HET HE LESS HAVE SHEETS IN HIS THAN A FAMILY OF THE COST AND THE DECEMBER SHEETCHEEKS. KINNERS THE SHOPS IN IN TROOP OF

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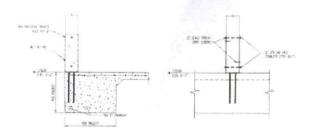
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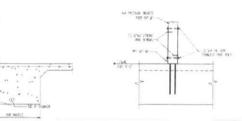
STANDARD FOUNDATION FOR THE STATE OF NORTH CAROLINA FOR BLACKWATER TRUSS SYSTEM 8736 FL-87 MILTON, FL. 32570





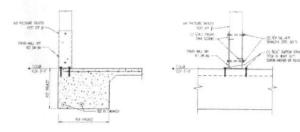




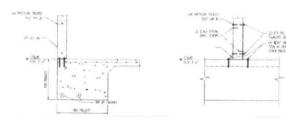


# BLACKWATER TRUSS SYSTEM ANCHOR BRACKET D SOUL LEVEL ST. 1

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UNE HOSE	POST SQT (MIN)	THE	INSIDE DIREASON 5/D.	LAE HESE	MAKEMUM SPILIS	TYPE	2057 SIZE (WM)
10-0	fieli	BMF06	56	10'-0"	40"	BWH4	hit
17-0	Ged:	BWF56	25,	12 -0*	40"	ENP66	fix5.
14'-0"	bd	HMF98	76	14"-9"	45	34988	Ball
16'-0"	bd	[MF88	35	16'-0"	35"	39756	545
18-0"	545	(86.60	75'	15'-0"	25'	94966	há
	MONG IN POST 8		ME B. KIMED HT GROWLUNG		WOME OF FOST IS HEQUIPES ARTHO		



BLACKWATER TRUSS SYSTEM ANCHOR BRACKET D



BLACKWATER TRUSS SYSTEM ANCHOR BRACKET D WORE FETTOW 811

	SET POST A	MC HE/KB			SET POST AN	CHURS	
ENE HEAR	POST SIZE (MIN)	TOPE	NSCE DMCKSON STE.	DAY HONN	MAZZEON SPAN	TPE	P257 523 (MP6
10'-0"	616	RM16	24"	10'-0"	40	3951	- Surfi
17'-0"	5x6	EWS6	267	12'-0"	40'	SWIS	0.6
14'-0"	345	£9633	74"	14'-0"	40	9465	Siñ
16'-0"	345	£905	76"	16'-0"	30	0465	Self
18'-0"	545	DWS.	76	15'-0"	7.	THE	816
NGNEER MORES. MICHAEL SPA 12'-0" MICH DIGNEER.	ONE OF POST E REQUIRES ABOVE	15'-6' O.C. MALENGNELR	ALL SPACING OMER NO IN LIPENSED		ON, OF POST ; REQUIES ACTION		





DATE: 09-13-2023 CHANGE DISCREPAND LL

STANDARD FOUNDATION FOR THE STATE OF NORTH CAROLINA FOR BLACKWATER TRUSS SYSTEM 8736 FL-87 MILTON, FL. 32570



BLACKWATER TRUSS
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# STANDARD FOUNDATION FOR THE STATE OF NORTH CAROLINA FOR BLACKWATER TRUSS SYSTEM

8736 FL-87

MILTON, FL. 32570





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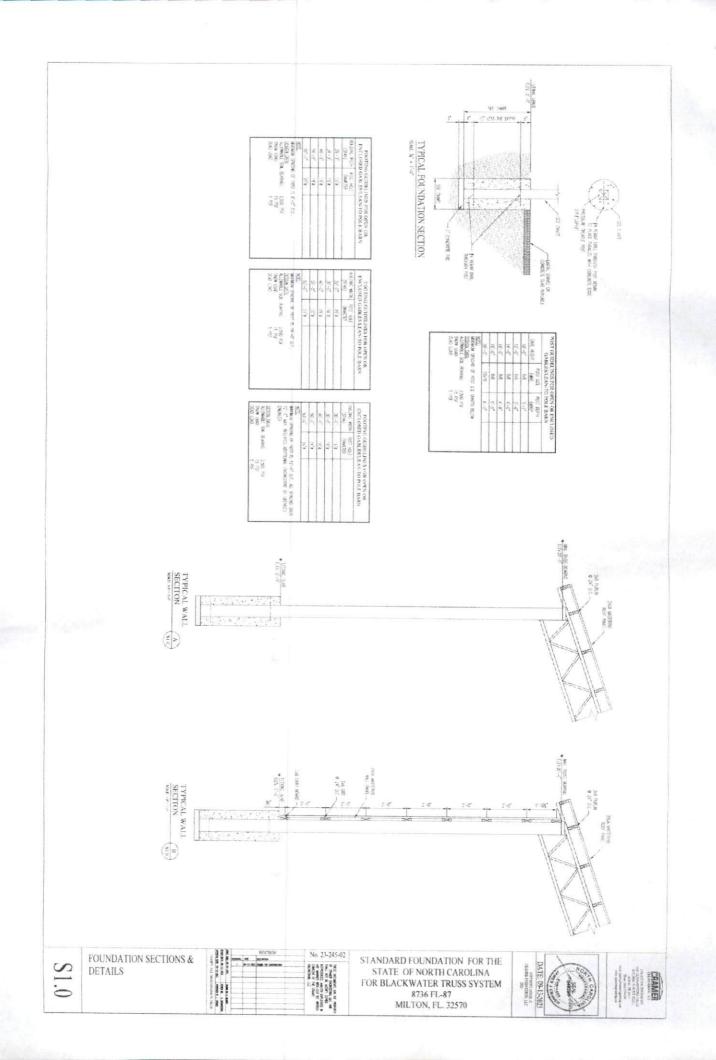
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	ABBREV	IATIO	NS
TYPS	DESCRIPTION	TYPE	DESCRIPTION
A.B.	ANCHOR BOLT	1.	LENGTH
ALT	ALTERNATE	LLH	LONG LEG HORIZONT AL
ARCH	ARCHITECTURAL	LLV.	LONG LEG VERTICAL
AVG	AVERAGE	I. P.	LOW POINT
BCDL	BOTTOM CHORD DEAD LOAD	MAX	MAXIMUM
BCLL	BOTTOM CHORD LIVE LOAD	MECH	MECHANICAL
BFF	BELOW FINISH FLOOR	MEZZ	MEZZANINE
BLDG	BUILDING	MFR.	MANUFACTURER
BM	BEAM	MIN.	MENIMUM
BO	BOTTOM OF	MISC	MISCELLANEOUS
BOT	BOTTOM	MWFRS	MAIN WIND FORCE.
BRG	BEARING		RESISTING SYSTEM
		M.O.	MASONRY OFFINING
CI	CONTROL JOINT		
CIR	CLEAR	N.S.	NEAR SIDE
CMU	CONCRETE MASONRY UNIT		44479-7411-7714-7
CONC	CONCRETE	o.c.	ON CENTER
CONST. JT.	CONSTRUCTION JOINT	O.D	OUTSIDE DIAMETER
CONT	CONTINUOUS	O.H	OVER HEAD
D	DEPTH	0/0	OUT TO OUT
DET.	DETAIL	PSF	Lancaca and a contract of
DIA	DIAMETER	P.S.I	POUNDS PER SQUARE FOOT
101.75	DIASIETER	P.S.1	POLINDS PER SQUARE INCH
ELEV	ELEVATION	R.	RADIUS
EQ.	EQUAL.	REINF	REINFORCEMENT
E W	EACH WAY		0.0000000000000000000000000000000000000
EXIST	EXISTING	SIM.	SIMILAR
FIN.	FINISH	T&B	TOP AND BOTTOM
FLR.	FLOOR	TCDL	TOP CHORD DEAD LOAD
TND	FOUNDATION	TCLL	TOP CHORD LIVE LOAD
F.S.	FAR SIDE	T.O.	TOP OF
FTG.	FOOTING	TOS	TOP OF STEEL
		TYP	TYPICAL
GA.	GAUGE	U.N.O.	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL	C.14.0.	CACCOS MOTED OTHERWISE
HP.	HIGH POINT	VERT.	VERTICAL
2772	1001003003	VIE	VERIFY IN FIELD
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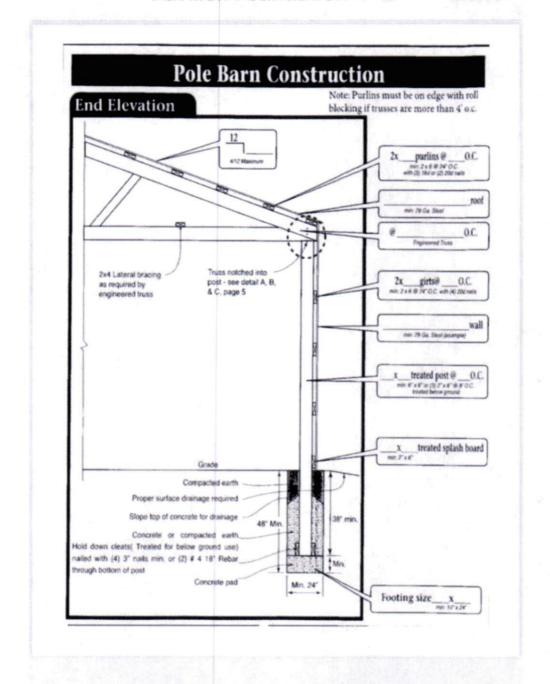




# Pavilion Foundation-1



## Done













# Pavilion Foundation-2



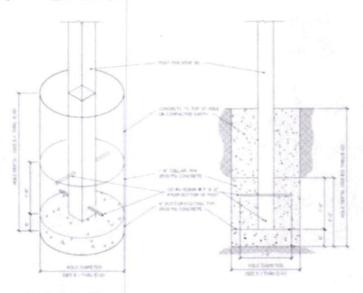


#### SECTION F: FOUNDATION REQUIREMENTS

Bottom of Footing shall be excavated to depth required, and inspected. Inspection will be measuring physical depth of hole and width across to ensure holes meet requirements of this manual. Pour 6" Concrete footing at bottom of hole, allow 7 days before applying loads to concrete footing. Pour collar section once post is plumb & square. The post may be within 4" of edge of hole (out of plane center) for up to 6" x 6" Posts, and within 3" for 8" X 8" Post.

Refer to Tables E-1 thru E-10 for sizing of posts and foundation holes.

Figure F-1: Typical Hole Diagram



### Required Inspections:

- Holes only no concrete
- 2. 6" concrete fully cured, with post braced in place. No additional concrete poured prior to this inspection.

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