

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 24010078 BCTH-52

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by The Building Center.

Pages or sheets covered by this seal: I62907658 thru I62907669

My license renewal date for the state of North Carolina is December 31, 2024.

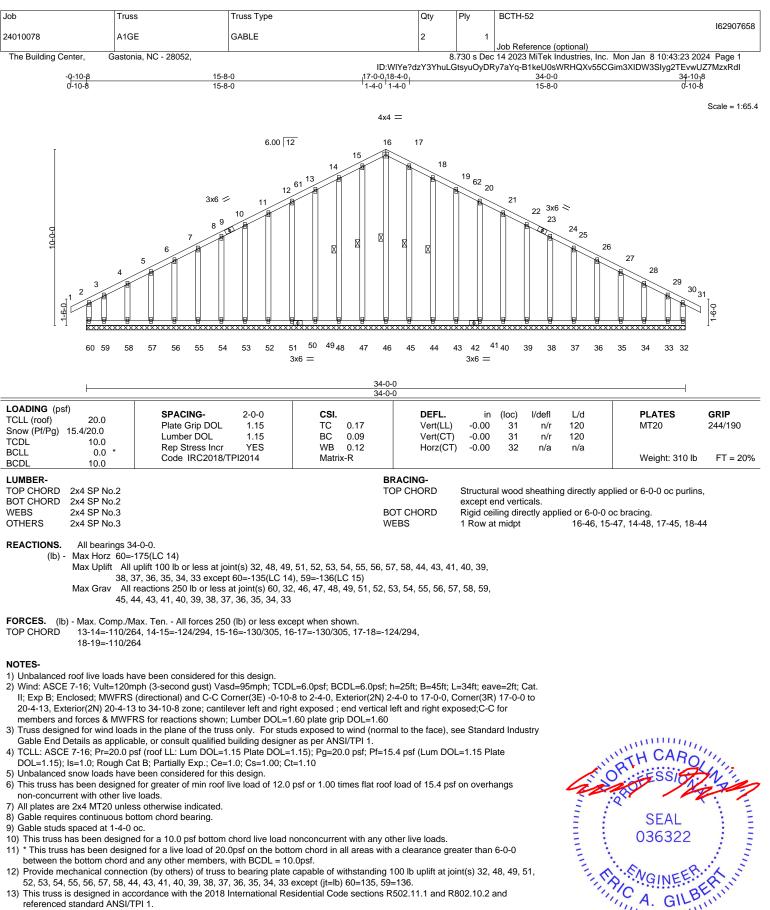
North Carolina COA: C-0844



January 9,2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



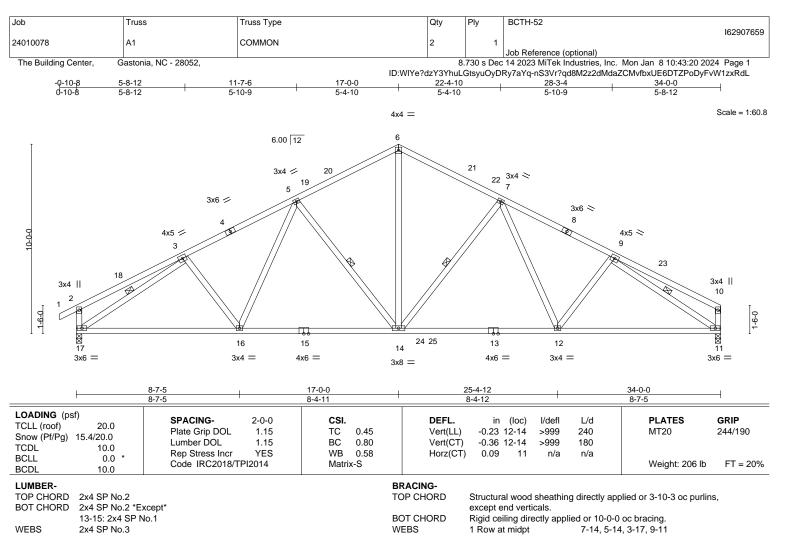
referenced standard ANSI/TPI 1.

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818 Soundside Road

Edenton, NC 27932

The Giran January 9,2024



REACTIONS. (size) 11=0-3-8, 17=0-3-8 Max Horz 17=173(LC 15) Max Uplift 11=-10(LC 16), 17=-35(LC 16) Max Grav 11=1448(LC 29), 17=1503(LC 28)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-5=-2042/114, 5-6=-1618/155, 6-7=-1617/156, 7-9=-2048/118, 2-17=-304/99

BOT CHORD 16-17=-70/1827, 14-16=-27/1738, 12-14=-16/1666, 11-12=-63/1729

7-14=-505/94, 7-12=0/295, 6-14=-39/1144, 5-14=-504/94, 5-16=0/291, 3-17=-1959/43, 9-11=-1998/69

NOTES-

WFBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior(1) 2-6-5 to 17-0-0, Exterior(2R) 17-0-0 to 20-4-13, Interior(1) 20-4-13 to 33-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

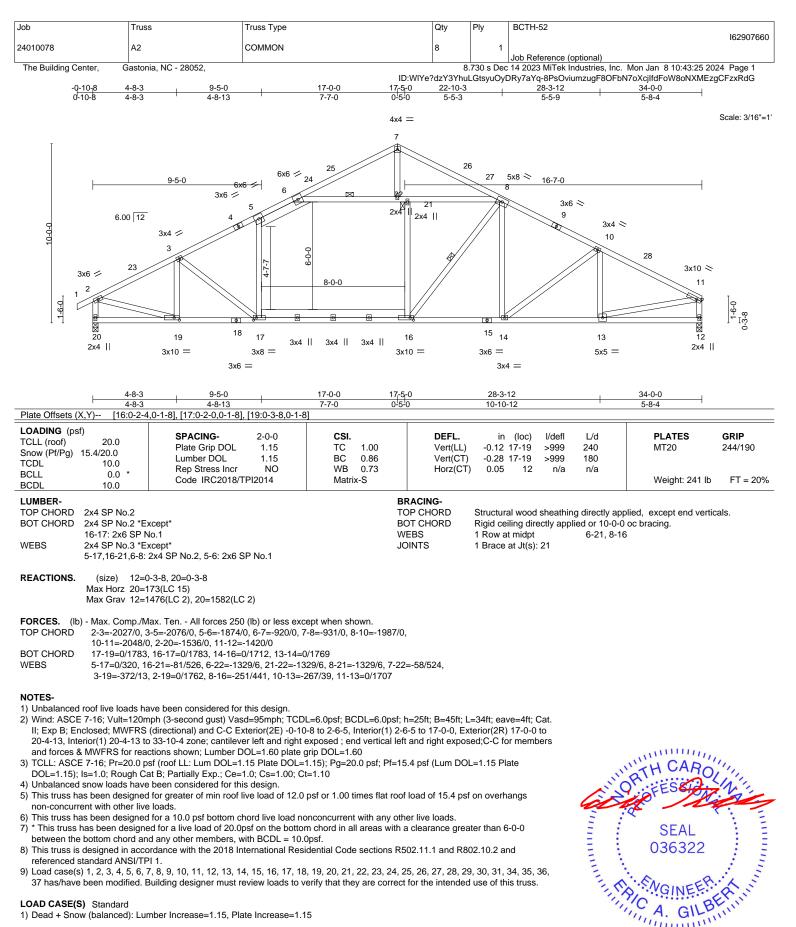
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 17.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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Edenton, NC 27932

11111111 January 9,2024

lob	Truss	Truss Type	Qty	Ply	BCTH-52	
24010078	A2	COMMON	8	1		162907660
The Building Center,	Gastonia, NC - 28052,			8 730 s Dei	Job Reference (optional) c 14 2023 MiTek Industries, Inc. Mor	n Jan 8 10:43:25 2024 Page 2
···· ····· · · · · · · · · · · · ·	,				DRy7aYq-8PsOviumzugF8OFbN7o	
LOAD CASE(S) Stan	dard					
Uniform Loads (plf)			40.40.00			
		=-61, 8-11=-51, 17-20=-20, 16-17=-40, ' se=1.15, Plate Increase=1.15	12-16=-20			
Uniform Loads (plf)	,					
		=-70, 8-11=-60, 17-20=-20, 16-17=-40, ' nhab. Attic Storage: Lumber Increase=1		5		
Uniform Loads (plf)		mab. / mile eterage. Europer mereade-	1.10, 1 100 11010000-1.1	•		
		=-60, 8-11=-50, 17-20=-20, 16-17=-40, ⁻ b. Attic Storage: Lumber Increase=1.15,				
Uniform Loads (plf)	balanced) i 0.70 omma	. Alle Glorage. Europer increase - 1.15,				
		=-53, 8-11=-43, 17-20=-20, 16-17=-40, ' ab. Attic Storage: Lumber Increase=1.1				
Uniform Loads (plf)		ab. Alle Slorage. Lumber increase=1.1	5, Flate Inclease=1.15			
		24=-75, 7-8=-37, 8-11=-27, 17-20=-20, <i>1</i>				
Uniform Loads (plf)	Undal. Right) + 0.75 Unir	hab. Attic Storage: Lumber Increase=1.	.15, Plate Increase=1.1t	0		
Vert: 1-2=-2		7=-75, 8-27=-53, 8-11=-43, 17-20=-20,		2-14=-20		
 Dead + Uninhabitat Uniform Loads (plf) 	le Attic Without Storage:	Lumber Increase=1.25, Plate Increase=	=1.25			
u ,	20, 2-5=-20, 5-7=-30, 7-8	=-30, 8-11=-20, 17-20=-40, 16-17=-60, ⁻	12-16=-40			
 Dead + 0.6 C-C Wir Uniform Loads (plf) 	nd (Pos. Internal) Case 1:	Lumber Increase=1.60, Plate Increase=	=1.60			
u /	8, 2-23=18, 5-23=13, 5-7	=3, 7-26=13, 8-26=3, 8-11=13, 17-20=-	12, 16-17=-32, 12-16=-1	2		
		7-26=35, 11-26=25, 2-20=21, 11-12=32				
9) Dead + 0.6 C-C Wir Uniform Loads (plf)	id (Pos. Internal) Case 2:	Lumber Increase=1.60, Plate Increase=	=1.60			
Vert: 1-2=8		3, 7-8=3, 8-28=13, 11-28=18, 17-20=-12		2		
		7-28=25, 11-28=30, 2-20=-32, 11-12=-2 1: Lumber Increase=1.60, Plate Increase				
Uniform Loads (plf		. Lumber increase=1.00, 1 late increase	e=1.00			
		8=-42, 8-11=-32, 17-20=-20, 16-17=-40,	, 12-16=-20			
	=-8, 2-7=12, 7-11=-12, 2-2 ind (Neg. Internal) Case 2	20=30, 11-12=23 2: Lumber Increase=1.60, Plate Increase	e=1.60			
Uniform Loads (plf)					
	-28, 2-5=-32, 5-7=-42, 7- =8, 2-7=12, 7-11=-12, 2-2	8=-42, 8-11=-32, 17-20=-20, 16-17=-40, 0=-23 11-12=-30	, 12-16=-20			
		oft: Lumber Increase=1.60, Plate Increas	se=1.60			
Uniform Loads (plf	,	-5, 8-11=5, 17-20=-12, 16-17=-32, 12-1	6- 12			
	=-24, 2-7=-12, 7-11=17, 2		0-12			
,	()	ght: Lumber Increase=1.60, Plate Increa	ase=1.60			
Uniform Loads (plf Vert: 1-2=		, 8-11=-0, 17-20=-12, 16-17=-32, 12-16	6=-12			
	-13, 2-7=-17, 7-11=12, 2					
14) Dead + 0.6 MWFR Uniform Loads (plf	,	eft: Lumber Increase=1.60, Plate Increas	se=1.60			
Vert: 1-2=	-23, 2-5=-27, 5-7=-37, 7-	8=-22, 8-11=-12, 17-20=-20, 16-17=-40,	, 12-16=-20			
	=3, 2-7=7, 7-11=8, 2-20=2 S Wind (Neg. Internal) R	22, 11-12=6 ight: Lumber Increase=1.60, Plate Incre	ase=1.60			
Uniform Loads (plf						
	-7, 2-5=-12, 5-7=-22, 7-8 -13, 2-7=-8, 7-11=-7, 2-2	=-37, 8-11=-27, 17-20=-20, 16-17=-40, ⁻	12-16=-20			
		t Parallel: Lumber Increase=1.60, Plate	Increase=1.60			
Uniform Loads (plf	,	0 44 40 47 00 40 40 47 00 40 40	10			
	25, 2-5=13, 5-7=3, 7-8=3 =-37, 2-7=-25, 7-11=25, 2	, 8-11=13, 17-20=-12, 16-17=-32, 12-16 -20=-19, 11-12=19	p=-12			
17) Dead + 0.6 MWFR	S Wind (Pos. Internal) 2r	d Parallel: Lumber Increase=1.60, Plate	e Increase=1.60			
Uniform Loads (plf Vert: 1-2=	,	, 8-11=4, 17-20=-12, 16-17=-32, 12-16=	=-12			
Horz: 1-2=	=-28, 2-7=-16, 7-11=16, 2	-20=-19, 11-12=19				
 Dead + 0.6 MWFR Uniform Loads (plf 		st Parallel: Lumber Increase=1.60, Plate	e Increase=1.60			
N N	,	8=-31, 8-11=-21, 17-20=-20, 16-17=-40,	, 12-16=-20			
	=-4, 2-7=1, 7-11=-1, 2-20					
Uniform Loads (plf		nd Parallel: Lumber Increase=1.60, Plate	e increase=1.60			
Vert: 1-2=	-16, 2-5=-21, 5-7=-31, 7-	8=-31, 8-11=-21, 17-20=-20, 16-17=-40,	, 12-16=-20			
	=-4, 2-7=1, 7-11=-1, 2-20:)verhangs: Lumber Increa	=-10, 11-12=10 ase=1.15, Plate Increase=1.15				
Uniform Loads (plf	•					
		8=-30, 8-11=-20, 17-20=-20, 16-17=-40,	, 12-16=-20			
Uniform Loads (plf		e=1.15, Plate Increase=1.15				
Vert: 1-2=	-51, 2-5=-51, 5-24=-61, 7	-24=-90, 7-8=-39, 8-11=-29, 17-20=-20,	, 16-17=-40, 12-16=-20			
22) Dead + Snow (Unl	bai. Right): Lumber Increa	se=1.15, Plate Increase=1.15				

Continued on page 3

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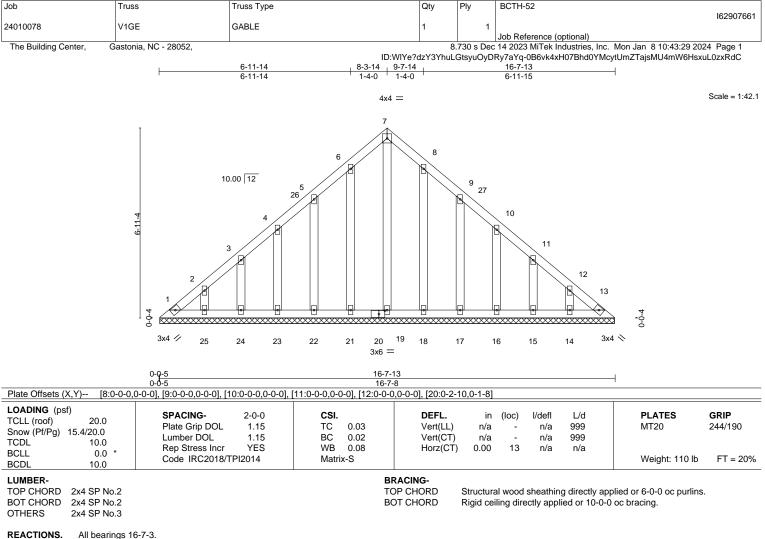


Job	Truss	Truss Type	Qty	Ply	BCTH-52	100007000
24010078	A2	COMMON	8	1		162907660
					Job Reference (optional)	
The Building Center,	Gastonia, NC - 2805	2,	ID:WIYe?dz		c 14 2023 MiTek Industries, Inc. M OyDRy7aYq-ccQm62vOjCo6mYqn	
LOAD CASE(S) S	tandard					
Uniform Loads						
		7-27=-90, 8-27=-61, 8-11=-51, 17-20=-20,	16-17=-40, 12-16=-2	0		
Uniform Loads		ber Increase=1.25, Plate Increase=1.25				
		7-8=-30, 8-11=-20, 17-20=-20, 16-17=-40,	14-16=-60, 12-14=-2	0		
		. Attic Storage + 0.75(0.6 MWFRS Wind (N			60, Plate Increase=1.60	
Uniform Loads						
	, , , ,	7-8=-47, 8-11=-37, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
	-2=2, 2-7=6, 7-11=6, 2-2		a a Just) Diabt), Juanha		CO Plata Increase 1 CO	
25) Dead + 0.75 Sn Uniform Loads		. Attic Storage + 0.75(0.6 MWFRS Wind (N	eg. Int) Right): Lumbe	er increase="	1.60, Plate Increase=1.60	
		7-8=-59, 8-11=-49, 17-20=-20, 16-17=-40,	14-16=-50 12-14=-2	0		
	-2=-10, 2-7=-6, 7-11=-6,			•		
		. Attic Storage + 0.75(0.6 MWFRS Wind (N	eg. Int) 1st Parallel): I	umber Incre	ase=1.60, Plate Increase=1.60	
Uniform Loads	M /					
		7-8=-54, 8-11=-44, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
	-2=-3, 2-7=1, 7-11=-1, 2-	20=-8, 11-12=8 . Attic Storage + 0.75(0.6 MWFRS Wind (N	ag Int) 2nd Darallal);	lumbor loor	anan 1.60 Blata Ingranga 1.60	
Uniform Loads		. Alle Storage + 0.75(0.6 WWFRS Wind (N	eg. Int) zhu Parallel).	Lumber inci	ease=1.00, Flate Increase=1.00	
		7-8=-54, 8-11=-44, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
	-2=-3, 2-7=1, 7-11=-1, 2-		,			
		hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) Left): Lum	ber Increase	=1.60, Plate Increase=1.60	
Uniform Loads	M /					
		7-8=-54, 8-11=-44, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
	-2=2, 2-7=6, 7-11=6, 2-2	0=16, 11-12=5 hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Nog. Int) Dight): Lu	mbor Incroa	a-1.60 Plate Increase-1.60	
Uniform Loads	()	Tab. Alle Storage + 0.75(0.0 MWFRS Wind	u (Neg. III.) Kigili). Lu		se=1.00, Flate Inclease=1.00	
	M /	7-8=-66, 8-11=-56, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
Horz: 1	-2=-10, 2-7=-6, 7-11=-6,	2-20=-5, 11-12=-16				
/	()	hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) 1st Paralle	el): Lumber I	ncrease=1.60, Plate Increase=1.	60
Uniform Loads	M /			•		
	-2=-47, 2-5=-51, 5-7=-61, -2=-3, 2-7=1, 7-11=-1, 2-	7-8=-61, 8-11=-51, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
	, , ,	hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) 2nd Paral	el). Lumber	Increase=1.60 Plate Increase=1	60
Uniform Loads	()					
	M /	7-8=-61, 8-11=-51, 17-20=-20, 16-17=-40,	14-16=-50, 12-14=-2	0		
Horz: 1	-2=-3, 2-7=1, 7-11=-1, 2-	20=-8, 11-12=8				
		ber Increase=1.15, Plate Increase=1.15				
Uniform Loads		7 9 20 9 44 20 47 20 20 40 47 40	10.10 00			
		7-8=-30, 8-11=-20, 17-20=-20, 16-17=-40, hber Increase=1.15, Plate Increase=1.15	12-16=-20			
Uniform Loads	· · · · · ·	1.10, 1 ale morease=1.10				
		7-8=-70, 8-11=-60, 17-20=-20, 16-17=-40,	12-16=-20			
36) 3rd Dead + 0.75	5 Roof Live (unbalanced)	+ 0.75 Uninhab. Attic Storage: Lumber Inci		ease=1.15		
Uniform Loads						
		7-8=-30, 8-11=-20, 17-20=-20, 16-17=-40,				
/	()	+ 0.75 Uninhab. Attic Storage: Lumber Incr	ease=1.15, Plate Incl	ease=1.15		
Uniform Loads		7-860 8-1150 17-2020 16-1740	14.40 50 40.44 0	0		

Vert: 1-2=-20, 2-5=-20, 5-7=-30, 7-8=-60, 8-11=-50, 17-20=-20, 16-17=-40, 14-16=-50, 12-14=-20

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All bearings 16-7-3.

Max Horz 1=119(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 21, 22, 23, 24, 25, 18, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 19, 21, 22, 23, 24, 25, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 8-3-14, Exterior(2R) 8-3-14 to 11-3-14, Interior(1) 11-3-14 to 16-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 21, 22, 23, 24, 25, 18, 17, 16, 15, 14.

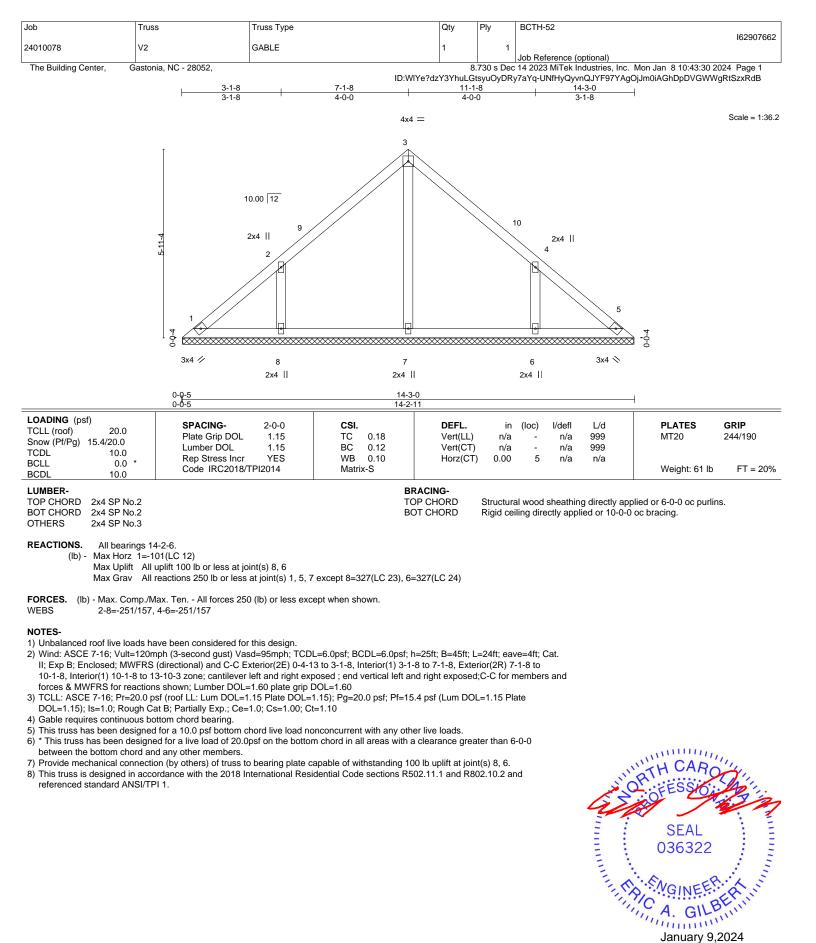
8) Non Standard bearing condition. Review required.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



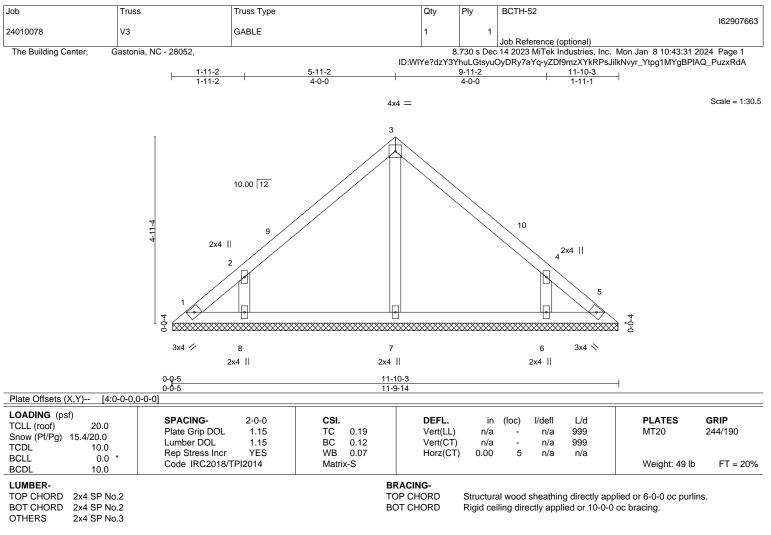
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818 Soundside Road Edenton, NC 27932



REACTIONS. All bearings 11-9-10.

(lb) - Max Horz 1=-83(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=301(LC 23), 6=301(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 5-11-2, Exterior(2R) 5-11-2 to 8-11-2, Interior(1) 8-11-2 to 11-5-6 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.
 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

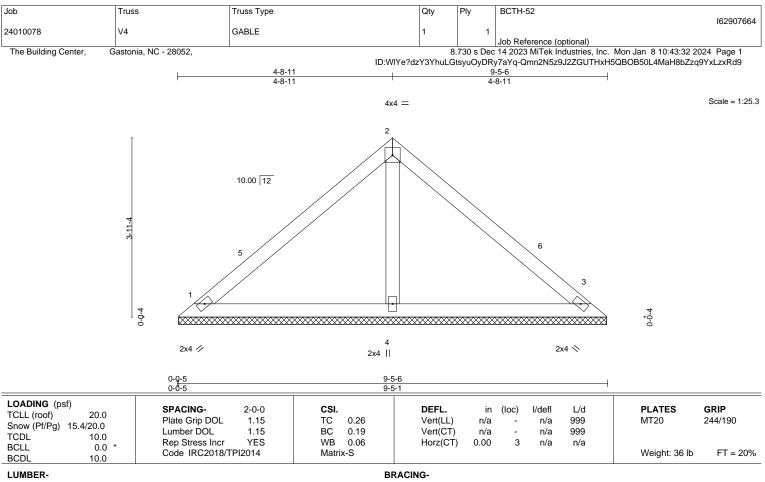
referenced standard ANSI/TPI 1.



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818 Soundside Road



TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=9-4-13, 3=9-4-13, 4=9-4-13 Max Horz 1=65(LC 13) Max Uplift 1=-15(LC 14), 3=-15(LC 14) Max Grav 1=183(LC 2), 3=183(LC 2), 4=325(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-8-11, Exterior(2R) 4-8-11 to 7-8-11, Interior(1) 7-8-11 to 9-0-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

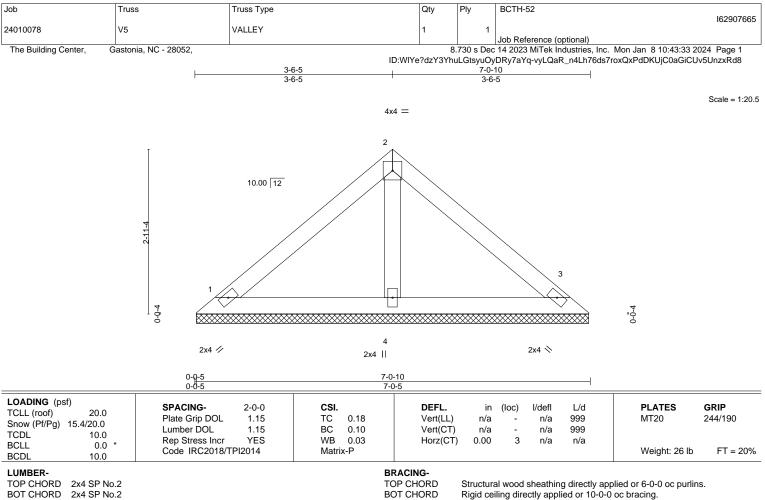
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. 1=7-0-0, 3=7-0-0, 4=7-0-0 (size) Max Horz 1=-47(LC 12) Max Uplift 1=-17(LC 14), 3=-17(LC 14) Max Grav 1=143(LC 2), 3=143(LC 2), 4=213(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

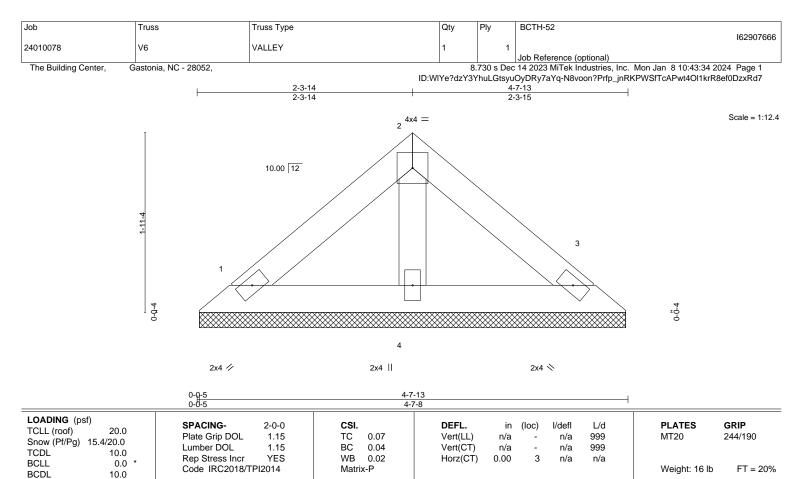
7) Non Standard bearing condition. Review required.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. (size) 1=4-7-3, 3=4-7-3, 4=4-7-3 Max Horz 1=29(LC 13) Max Uplift 1=-11(LC 14), 3=-11(LC 14) Max Grav 1=88(LC 2), 3=88(LC 2), 4=131(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) Non Standard bearing condition. Review required.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



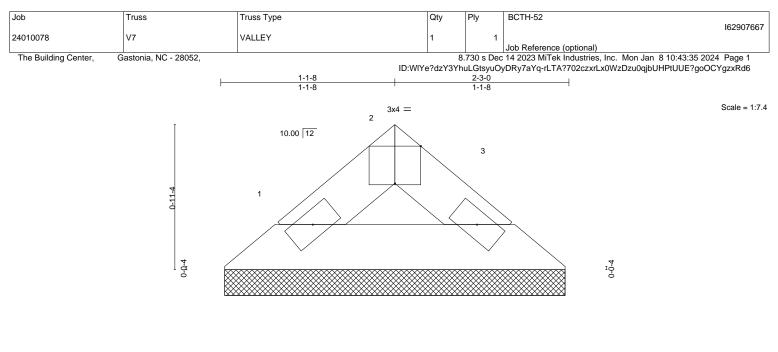
Structural wood sheathing directly applied or 4-7-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



2x4 🥢

2x4 📎

BRACING-

TOP CHORD

BOT CHORD

		2-2-11 2-2-11			<u>2-3</u> -0 0-0-5		
Plate Offsets (X,Y) [2:0-2-0,							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.01 BC 0.02 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P				Weight: 6 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=2-2-6, 3=2-2-6 Max Horz 1=-11(LC 12) Max Grav 1=58(LC 2), 3=58(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

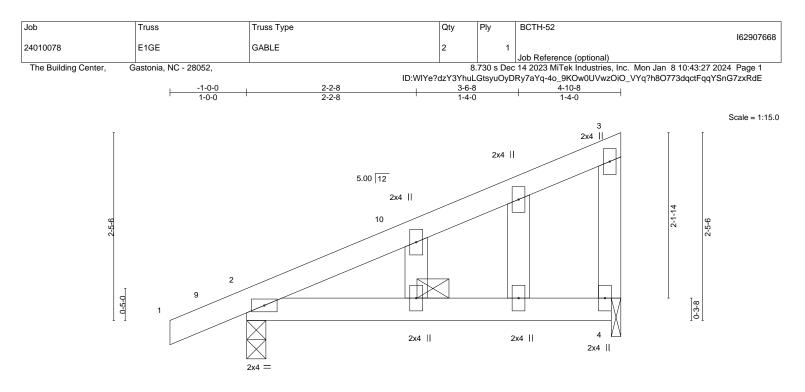


Structural wood sheathing directly applied or 2-3-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.49 BC 0.29 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P	1012(01)	0.00	7	n/a	n/a	Weight: 23 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

REACTIONS.

(size) 2=0-3-0, 4=0-1-8 Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16)

Max Grav 2=301(LC 21), 4=216(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 4-10-8 oc purlins,

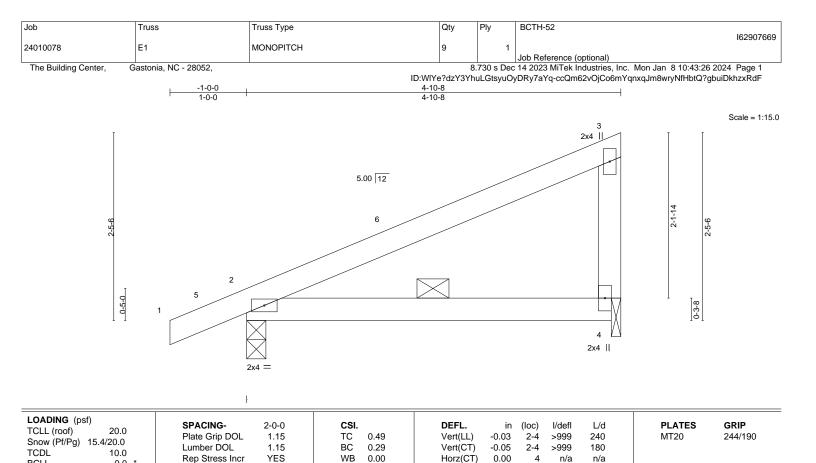
except end verticals.

3-0-0 oc bracing.

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818 Soundside Road



BRACING-

TOP CHORD

BOT CHORD

Matrix-P

n	-/	"	

WEBS

BCLL

BCDL

LUMBER-

TOP CHORD

BOT CHORD

REACTIONS. (size) 2=0-3-0, 4=0-1-8

0.0

10.0

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

Max Horz 2=66(LC 13) Max Uplift 2=-31(LC 16)

Max Grav 2=301(LC 21), 4=216(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2018/TPI2014

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-8-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Weight: 19 lb

Structural wood sheathing directly applied or 4-10-8 oc purlins,

except end verticals.

3-0-0 oc bracing.

FT = 20%

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