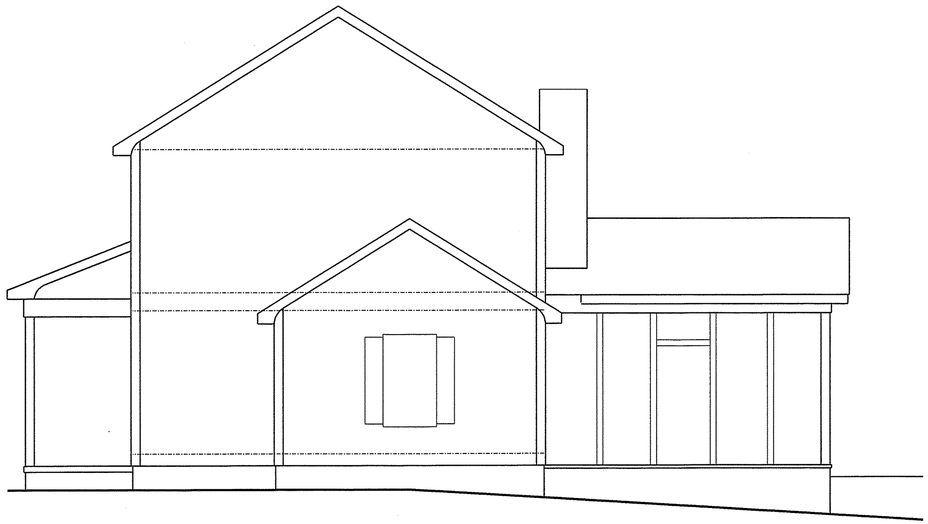
ANOTHER DESIGN BY THOM FOR THE

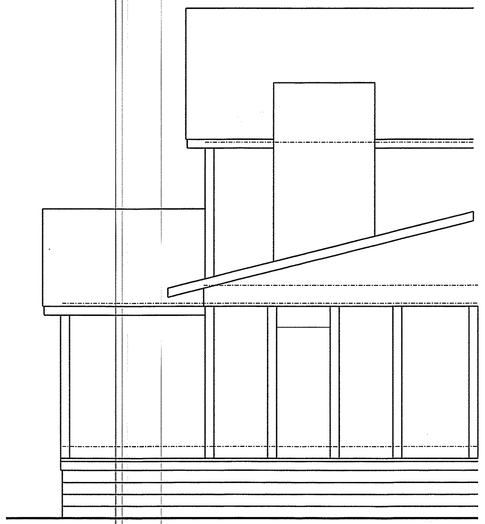
# EGERTON FAMILY

## MASTER SUITE ADDITION

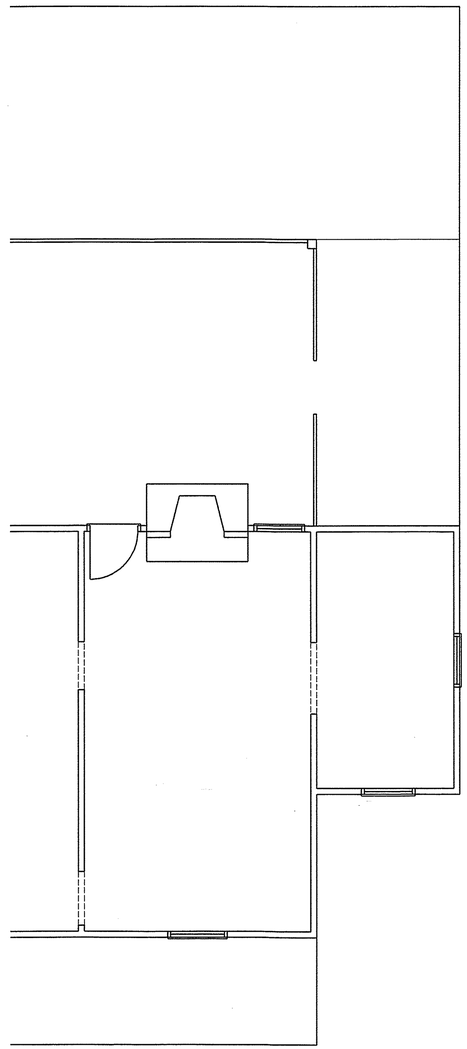
## 824 CREELVIEW LANE / LINDEN NORTH CAROLINA 28356



EXIST. SIDE ELEV.
1/4" = 1'-0"



EXIST. REAR ELEV.
1/4" = 1'-0"



EXISTING FLOOR PLAN
1/4" = 1'-0"


EXIST. FRONT ELEV.
1/4" = 1'-0"

NOTICE TO CONTRACTOR:
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

01/03/2024

Harnett COUNTY NORTH CAROLINA

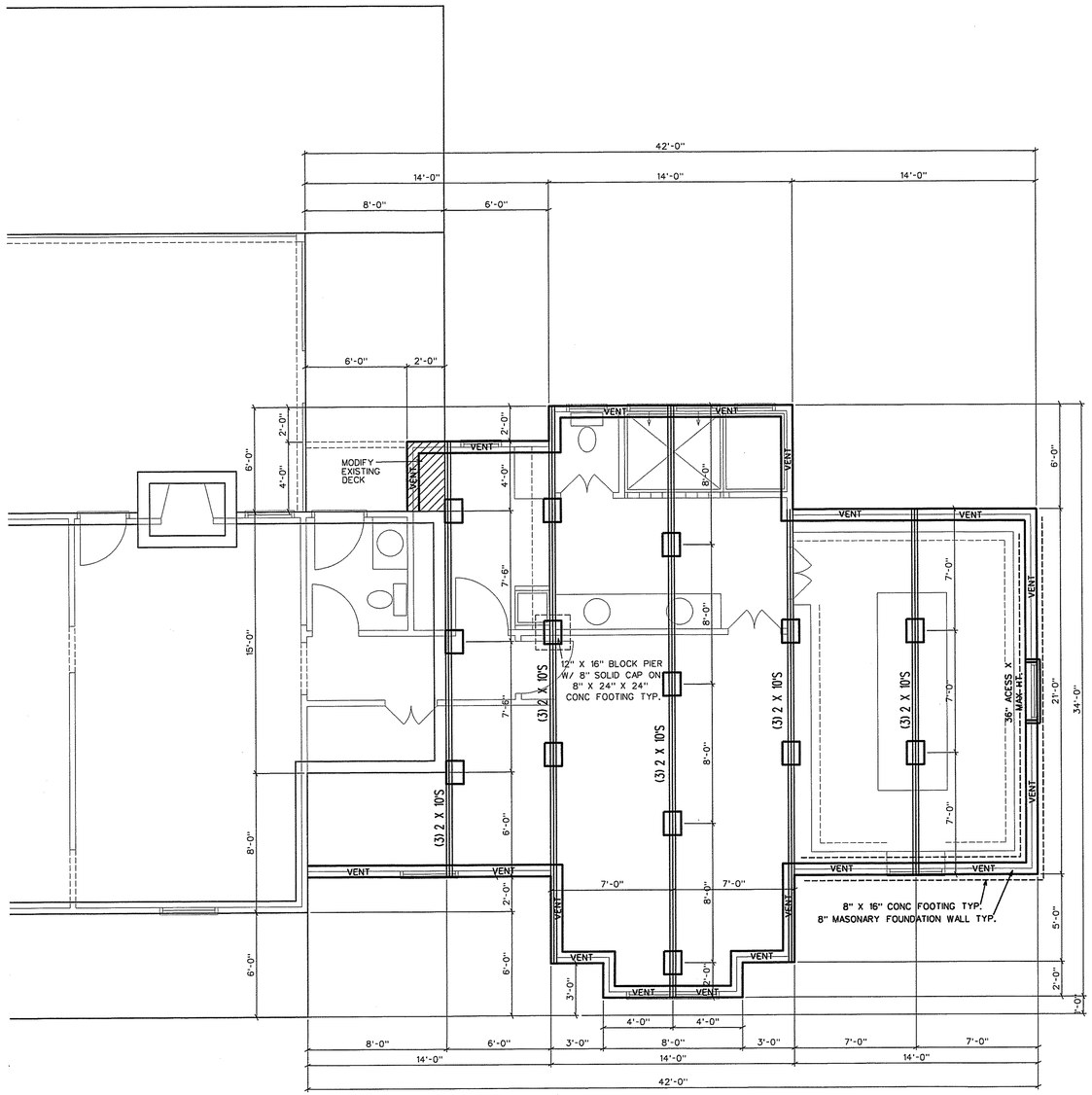

THOM STAHL DESIGNS
210 JUDD STREET
FAYETTEVILLE, NC 28305
(910) 286-2766


DO NOT SCALE DRAWINGS !! CALL THOM STAHL WITH ANY QUESTIONS!!

EGERTON FAMILY
ANOTHER DESIGN BY THOM FOR THE
MASTER SUITE ADDITION
824 CREELVIEW LANE / LINDEN NORTH CAROLINA 28356

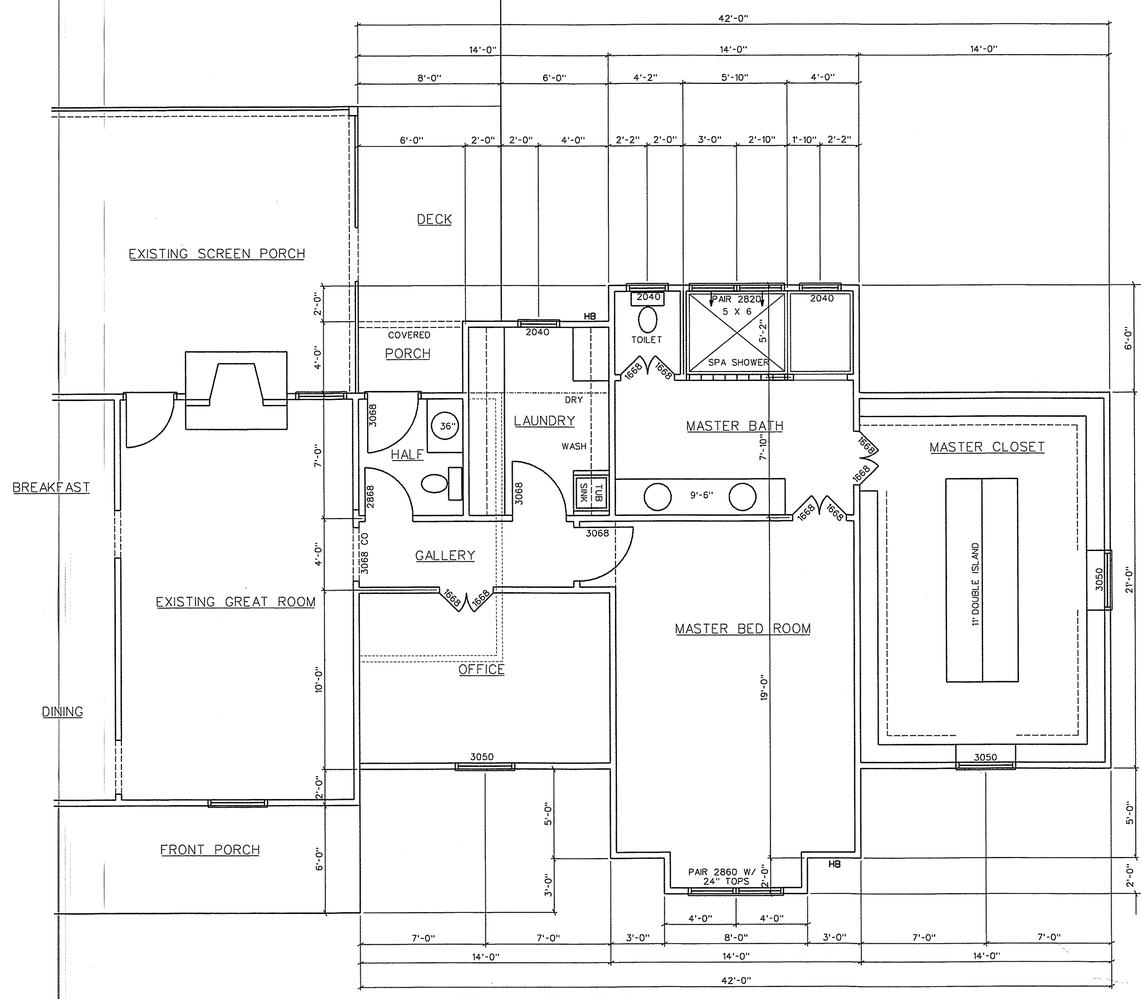
EXISTING
DATE: 2-14-23
REVISED:
CODE: EDGEADD1-22

SHEET NO.
A1
OF 3




# FOUNDATION PLAN

1/4" = 1'-0"




# NEW FLOOR PLAN 1085 SF HEATED

1/4" = 1'-0"

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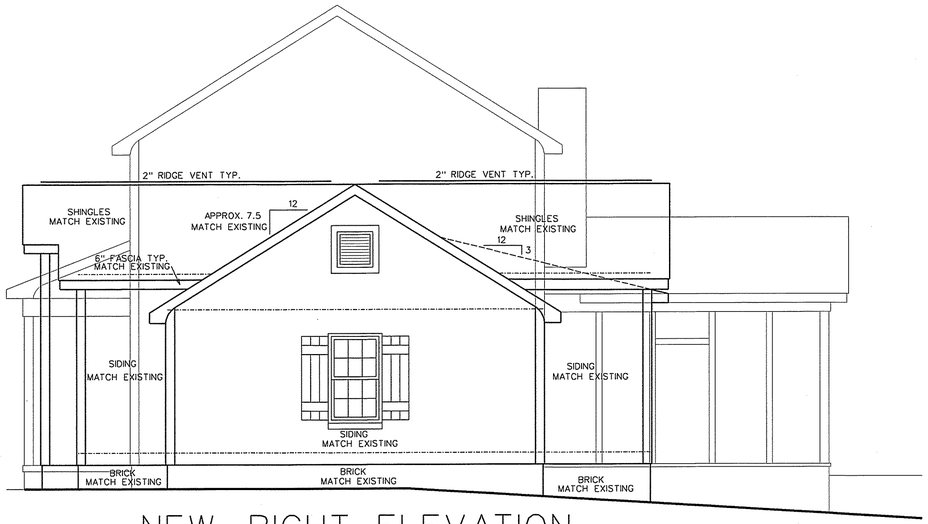
ANOTHER DESIGN BY THOM FOR THE

# EGERTON FAMILY

MASTER SUITE ADDITION
824 CREEKVIEW LANE / LINDEN NORTH CAROLINA 28356

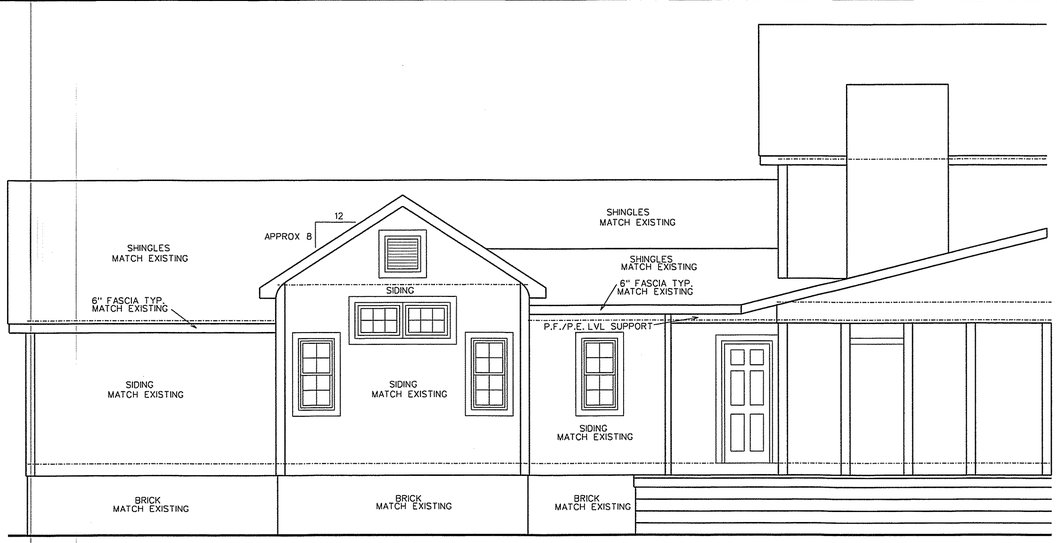
FLOOR PLANS
DATE: 2-14-23
REVISED:
CODE: EDGEADD1-22

SHEET NO.
A2
OF 3




# NEW RIGHT ELEVATION

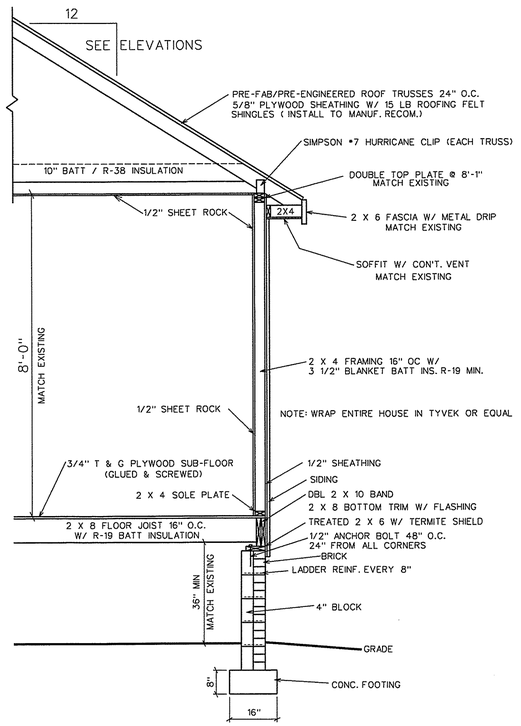
1/4" = 1'-0"




# NEW REAR ELEVATION

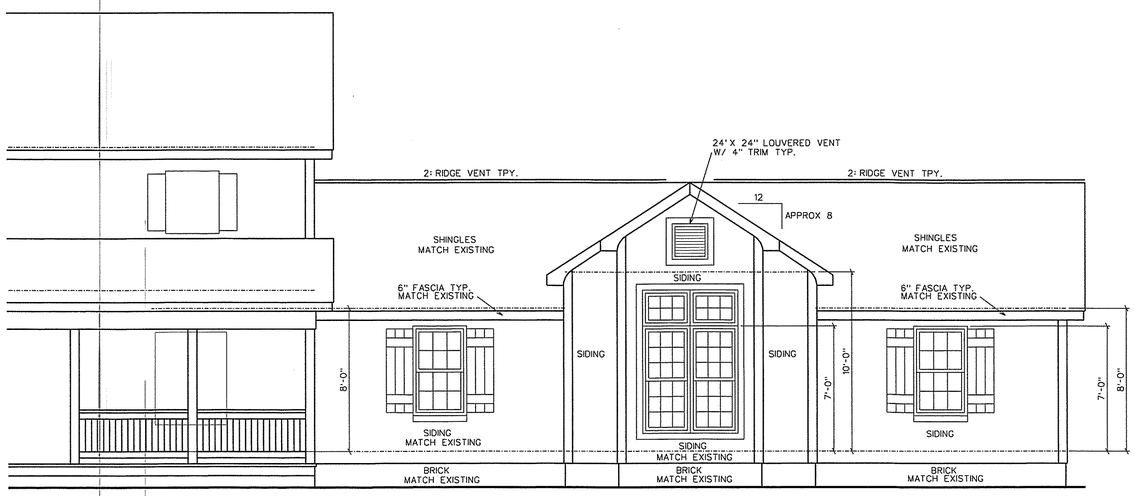
1/4" = 1'-0"

THOM STAHL TO REVIEW ALL FLOOR AND ROOF TRUSS SHOP DRAWINGS BEFORE ANY ARE MADE !!
EMAIL TRUSS PLANS FOR REVIEW TO : thomtanic1912@aol.com




# TYP. WALL SECTION DETAIL

1/2" = 1'-0"




# NEW FRONT ELEVATION

1/4" = 1'-0"

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210 JUDD STREET
FAYETTEVILLE, NC 28305
(910) 286-2766

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# EGERTON FAMILY

ANOTHER DESIGN BY THOM FOR THE
MASTER SUITE ADDITION
2824 CREELVIEW LANE / LINDEN, NORTH CAROLINA 28356

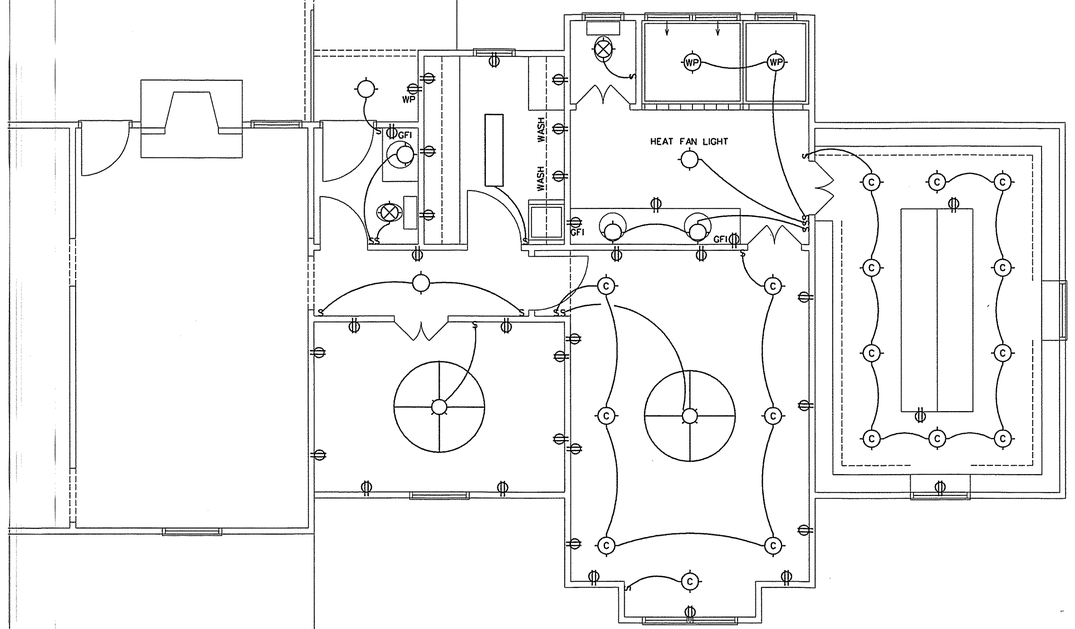
ELEVATIONS
DATE: 2-14-23
REVISED:
CODE: EDGEADD1-22

SHEET NO.
A3
OF 3

THOM STAHL DESIGNS
210 JUDD STREET
FAYETTEVILLE, NC 28305
(910) 286-2766

## GENERAL NOTES

1. WIRE ALL FANS AND LIGHT SEPERATE
2. ALL CAN LIGHTS ON A DIMMER SWITCH
3. SEE OWNER FOR TV, COMPUTER AND PHONE LOCATIONS AND NEEDS
4. MECHANICAL CONTRACTOR TO CONFIRM W/ OWNER
   NUMBNER OF UNITS, UNIT SIZES, ZONES AND PLACEMENT
   OF UNITS AND DUCTWORK




ELEC PLAN
1/4" = 1'-0"

DO NOT SCALE DRAWINGS !! CALL THOM STAHL WITH ANY QUESTIONS!!

ANOTHER DESIGN BY THOM FOR THE

# EGERTON FAMILY

MASTER SUITE ADDITION
324 CREELVIEW LANE / LINDEN NORTH CAROLINA 28356

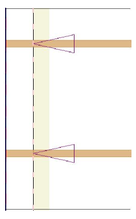
ELEC PLAN
DATE: 2-14-23
REVISED:
CODE: EDGEADD1-22

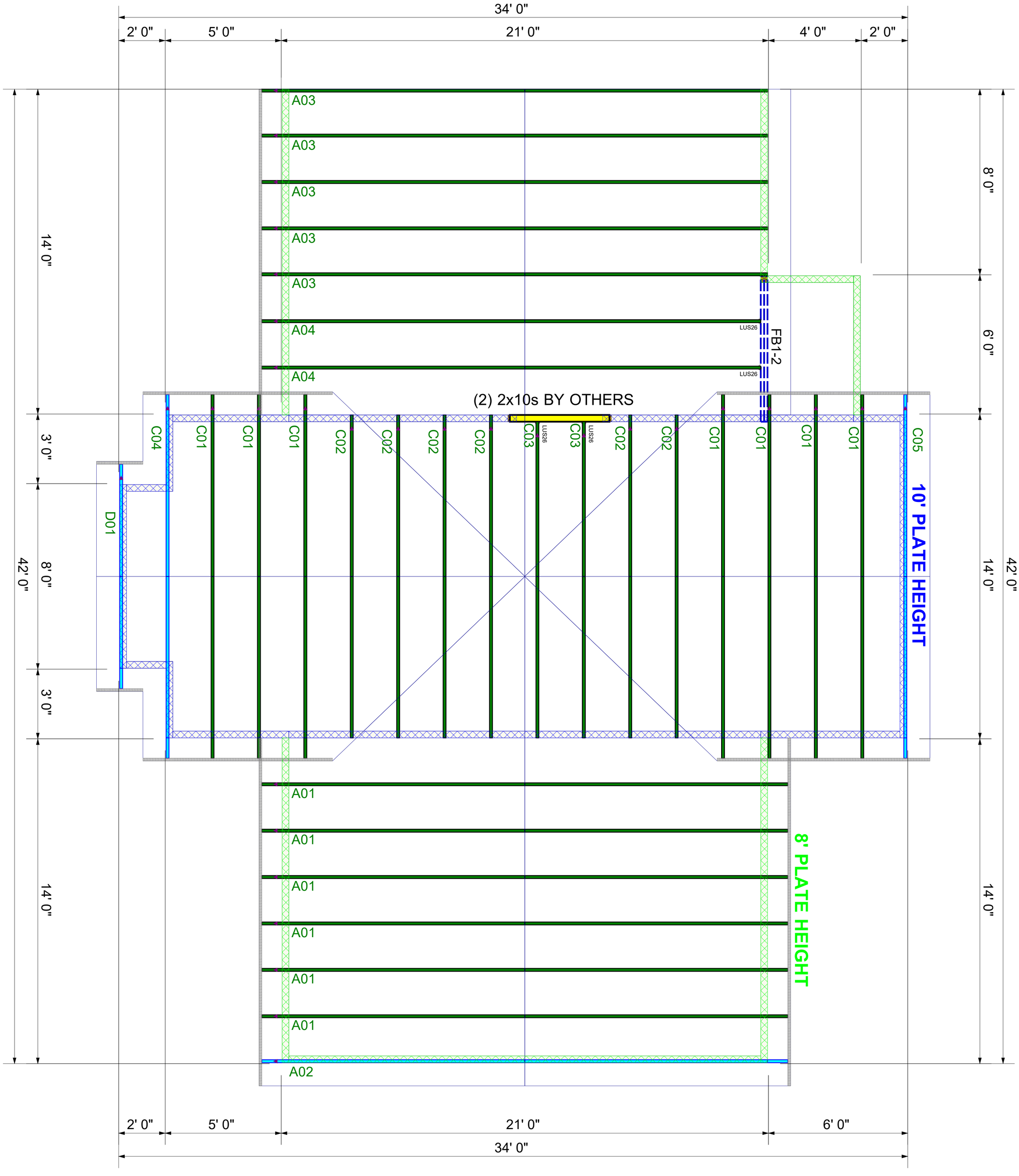
SHEET NO.
E1
OF 1

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

Truss Drawing Left End Indicator






## General Notes:

** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

Scale: NTS
Date: 11/14/2023
Designer: ND
Project Number: **2390042**
Sheet Number: **1/1**

CRITICAL PATH SOLUTIONS

824 CREEKVIEW
EDGARTON ADDITION

# COMPONENT PLACEMENT PLAN

Kempsville Building Materials
A Division of the Carter Lumber Company

**THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.** These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179

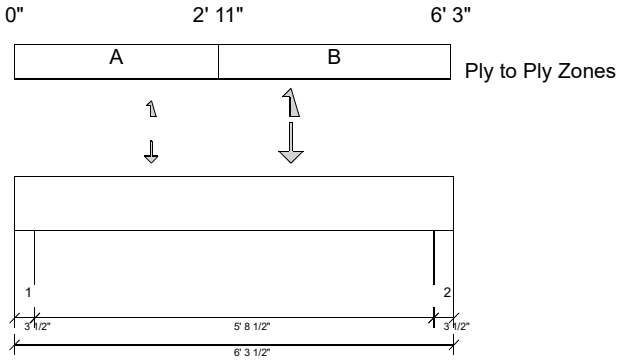
| Revisions | | |
|---|---|---|
| 00/00/00 | Name | |
| 00/00/00 | Name | |
| 00/00/00 | Name | |
| 00/00/00 | Name | |
| 00/00/00 | Name | |

| | | | | | |
|---|---|---|---|---|---|
| MiTek | Customer: **[Building Permit Number]**<br>Street 1:<br>City:<br>Customer Ph... | Job Name: **23090042**<br>Level: **1st FLOOR**<br>Label: **FB1-2 - i84**<br>Type: **Beam** | **2 Ply Member**<br>**2.0 RigidLam DF LVL 1-3/4 x 9-1/4** | Status:<br>**Design Passed** |

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.6.3.353.Update10.11

Report Version: 2021.03.26 11/14/2023 13:02




## DESIGN INFORMATION

| | |
|---|---|
| Building Code: | IRC 2018 |
| Design Methodology: | ASD |
| Risk Category: | II (General Construction) Residential |
| Service Condition: | Dry |
| LL Deflection Limit: | L/360, 0.75" (absolute) |
| TL Deflection Limit: | L/240, 1.00" (absolute) |

**Lateral Restraint Requirements:**

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 1'- 11 3/4" Bottom: 1'- 11 3/4"

**Bearing Stress of Support Material:**

- 425 psi Wall @ 0'- 2 1/2"
- 1323 psi Wall @ 6'- 1"

## ANALYSIS RESULTS

| Design Criteria | Location | Load Combination | LDF | Design | Limit | Result |
|---|---|---|---|---|---|---|
| Max Pos. Moment: | 3'- 11 1/2" | D + Lr | 1.15 | 3167 lb ft | 15231 lb ft | Passed - 21% |
| Max Neg. Moment: | 3'- 11 1/2" | 0.6D + 0.6W | 1.60 | 1177 lb ft | 21136 lb ft | Passed - 6% |
| Max Shear: | 5'- 2 3/4" | D + Lr | 1.15 | 1492 lb | 7198 lb | Passed - 21% |
| Live Load (LL) Neg. Defl.: | 3'- 3 3/16" | 0.6W | | 0.024" | L/360 | Passed - L/999 |
| Total Load (TL) Pos. Defl.: | 3'- 2 13/16" | D + Lr | | 0.039" | L/240 | Passed - L/999 |

## SUPPORT AND REACTION INFORMATION

| ID | Input Bearing Length | Controlling Load Combination | LDF | Downward Reaction | Uplift Reaction | Resistance of Member | Resistance of Support | Result |
|---|---|---|---|---|---|---|---|---|
| 1 | 3 1/2" | D + Lr | 1.15 | 1308 lb | | 9188 lb | 5206 lb | Passed - 25% |
| 1 | 3 1/2" | 0.6D + 0.6W | 1.60 | | -382 lb | - | - | |
| 2 | 3 1/2" | D + Lr | 1.15 | 1501 lb | | 9188 lb | 16207 lb | Passed - 16% |
| 2 | 3 1/2" | 0.6D + 0.6W | 1.60 | | -547 lb | - | - | |

## LOADING

| Type | Start Loc | End Loc | Source | Face | Dead (D) | Live (L) | Snow (S) | Roof Live (Lr) | Wind (W) |
|---|---|---|---|---|---|---|---|---|---|
| Self Weight | 0' | 6'- 3 1/2" | Self Weight | Top | 9 lb/ft | - | - | - | - |
| Point | 1'- 11 1/2" | 1'- 11 1/2" | A04(c01) | Front | 415 lb | - | 291 lb | 420/-1 lb | 126/-537 lb |
| Point | 3'- 11 1/2" | 3'- 11 1/2" | A04(c02) | Front | 776 lb | - | 792 lb | 1144/-1 lb | 344/-1788 lb |

## UNFACTORED REACTIONS

| ID | Start Loc | End Loc | Source | Dead (D) | Live (L) | Snow (S) | Roof Live (Lr) | Wind (W) |
|---|---|---|---|---|---|---|---|---|
| 1 | 0' | 0'- 3 1/2" | E17(i4) | 599 lb | - | 491 lb | 709/-1 lb | 382 lb/ -1236 lb |
| 2 | 6' | 6'- 3 1/2" | 2(i19) | 646 lb | - | 592 lb | 855/-1 lb | 382 lb/ -1236 lb |

## DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.99

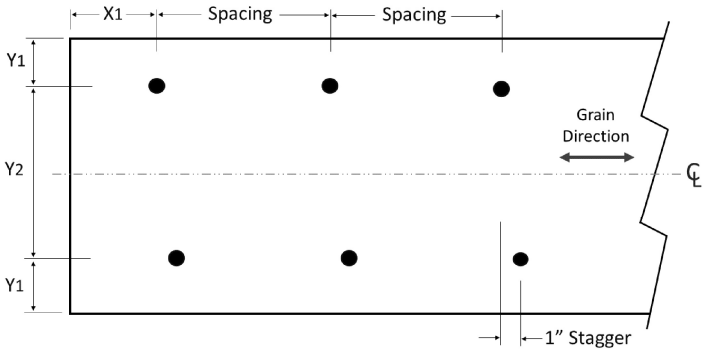
## PLY TO PLY CONNECTION

- Zone A: Factored load = 418 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 6. Row = 2, Spacing = 12"
  Zone B: Factored load = 960 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 18. Row = 2, Spacing = 5"
  12d (0.148"x3.25") nails properties: D = 0.148" , L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25" , Y1 = 0.75", Y2 = 1.5"
  Install fasteners from one face.
  X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.

| | | | | |
|---|---|---|---|---|
| MiTek | Customer: [Building Permit Number]<br>Street 1:<br>City:<br>Customer Ph... | Job Name: 23090042<br>Level: 1st FLOOR<br>Label: FB1-2 - i84<br>Type: Beam | 2 Ply Member<br>2.0 RigidLam DF LVL 1-3/4 x 9-1/4 | Status:<br>Design Passed |

## PLY TO PLY CONNECTION

**FASTENER INSTALLATION – 2 ROWS (FROM ONE FACE)**



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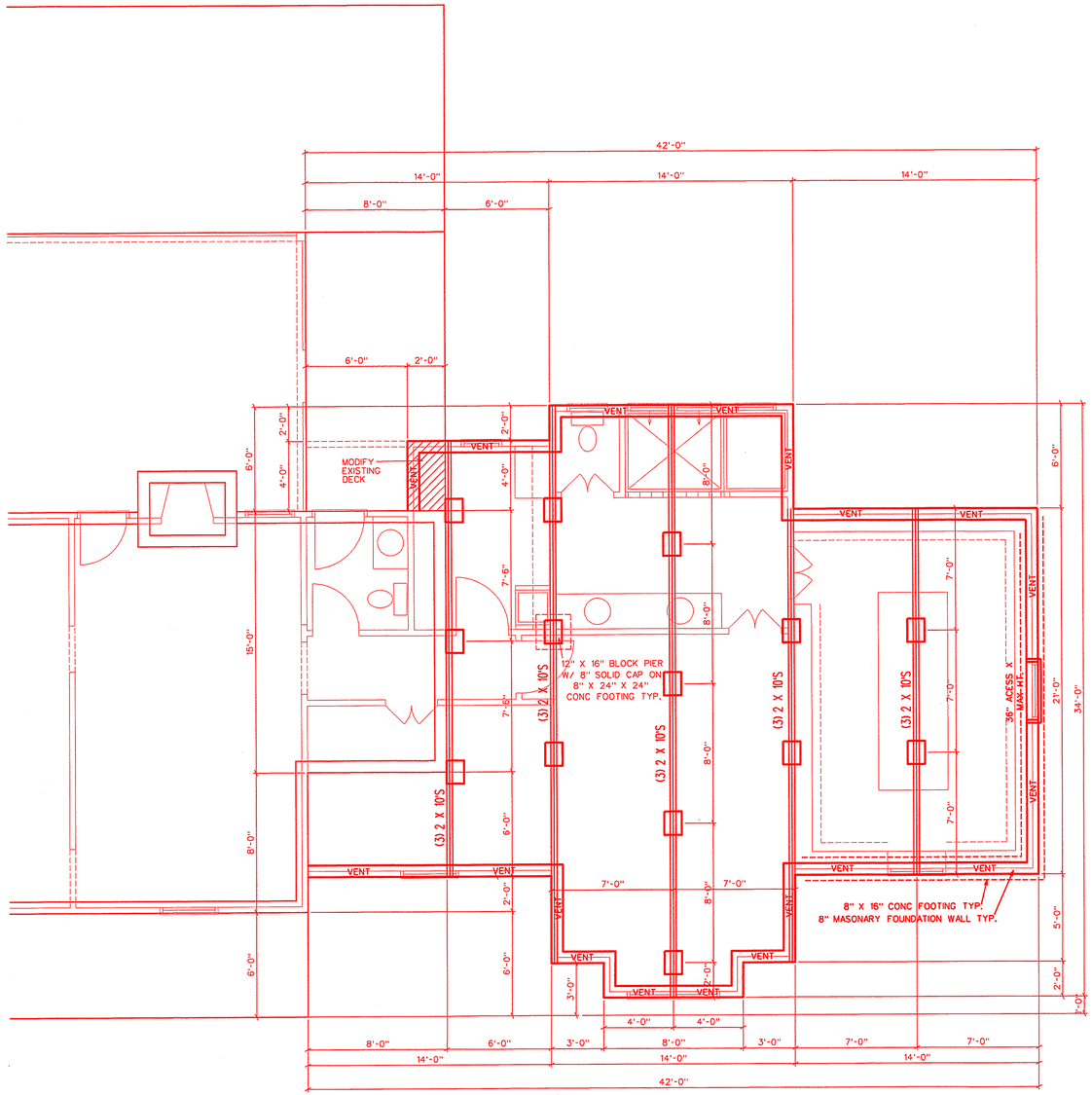
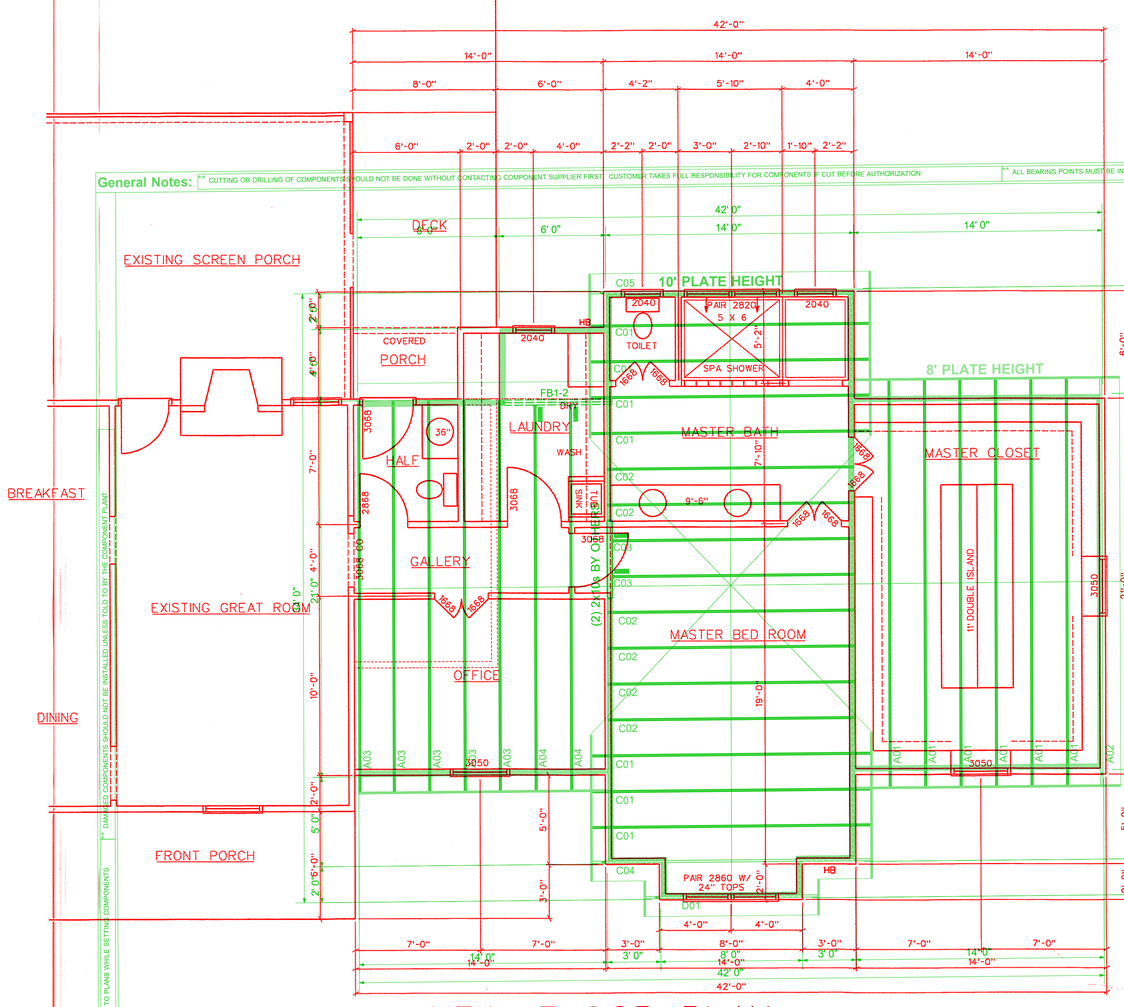



# FOUNDATION PLAN
1/4" = 1'-0"




# NEW FLOOR PLAN 1085 SF HEATED
1/4" = 1'-0"

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# EGERTON FAMILY

MASTER SUITE ADDITION
824 CREELVIEW LANE / LINDEN, NORTH CAROLINA 28356

FLOOR PLANS
DATE: 2-14-23
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CODE: EDGEADD1-22

SHEET NO. A2
OF 3