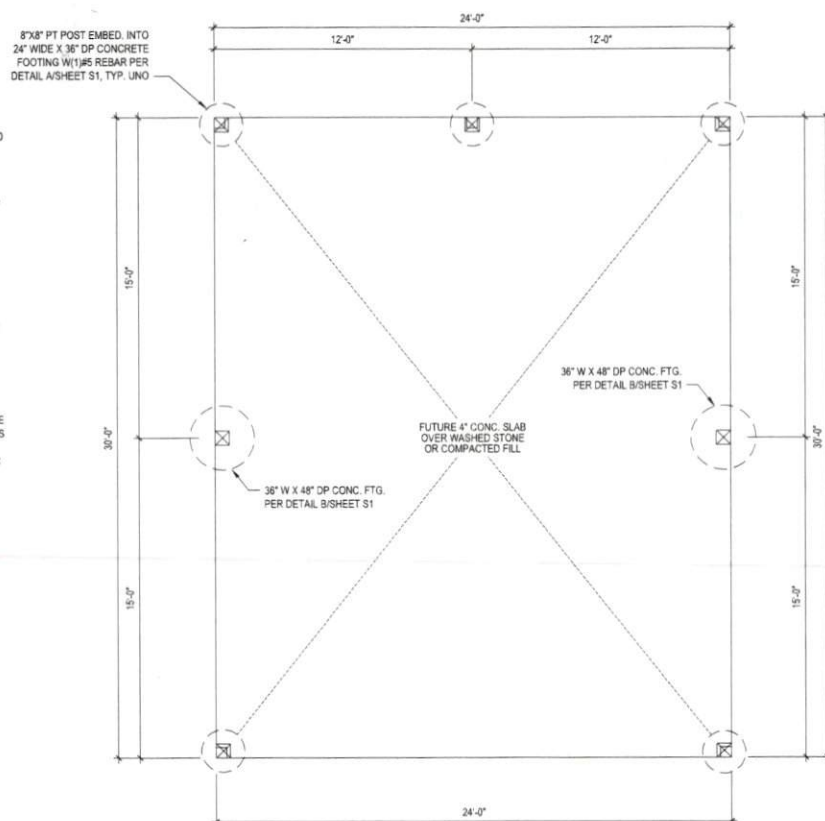


DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/960	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

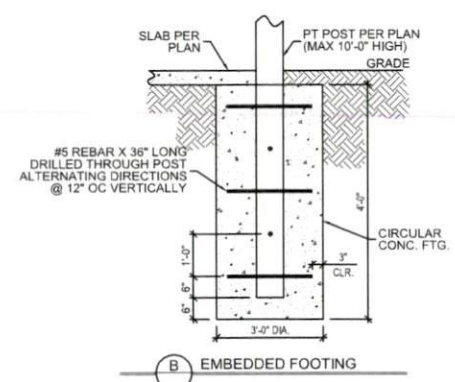
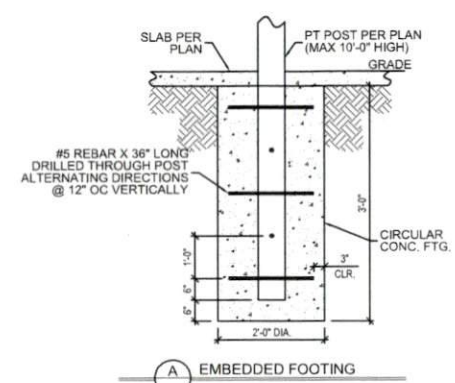
STRUCTURAL NOTES:

- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE, IN ADDITION TO ALL LOCAL CODES AND REGULATIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 2) ALL LUMBER SHALL BE SYP #2 (UNO). ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND $F_b = 2600$ PSI, $E = 1.9M$ PSI (OR GREATER) (I.E. LEVEL MICROLAM). ALL LSL LUMBER IS TO BE 1.58E ($F_b = 2325$ PSI) (OR GREATER). ALL PSL LUMBER IS TO BE 1.9E ($F_b = 2400$ PSI) (OR GREATER).
- 3) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-4", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 4) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO).
- 5) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 6) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 $F_y = 50$ KSI MIN. (UNO)
- 7) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 8) ALL CONCRETE, $f_c = 3000$ PSI MIN.
- 9) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 10) 12" O ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLT SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 11) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO). PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 12) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCR. MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION
- 13) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 14) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



FOUNDATION PLAN

1/4" = 1'-0"



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288 Shipwash Drive • Cary, NC 27513

CLIENT: LARRY WOOD

PROJECT: DETACHED CARPORT
581 SOUTH 164TH ST.
ERWIN, NC 28539

**FDN. PLAN
1ST FLR. FRMG.**

Drawn:	2401-010022
Date:	4/2/2024
Engineer:	JTI
Project/Location:	CSD
Notes:	SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number

S1

1 of 4

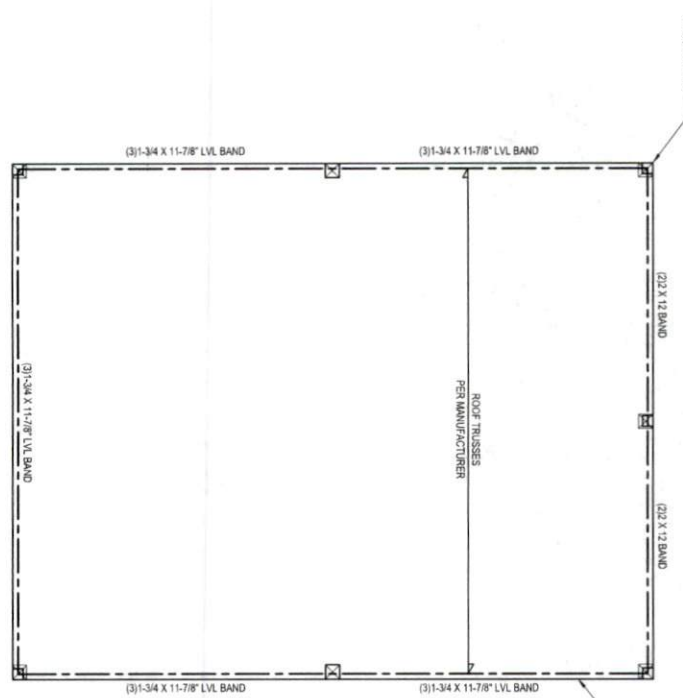
DRAWING: 2-DIMENSIONAL STRUCTURAL ENGINEERING/ARCHITECTURAL PROJECTS/PAID FOR BY: CLIENTS/PAID FOR BY: CLIENTS/PAID FOR BY: CLIENTS/PAID FOR BY: CLIENTS

DESIGN LOADS

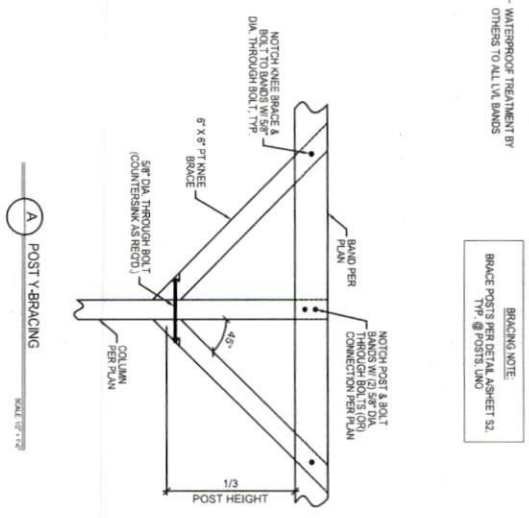
LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
		L _s	T _s
FLOOR (primary)	40	1/160	1/240
FLOOR (secondary)	40	1/160	1/240
ATTIC (w/ storage)	20	1/160	1/180
ATTIC (no storage)	10	1/160	1/240
EXTERNAL MASONRY	40	1/160	1/240
ROOF	20	1/160	1/180
ROOF TRUSS	20	1/160	1/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)		
SEISMIC	BASED ON SEISMIC ZONES A, B & C		

STRUCTURAL NOTES:

- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE. IN THE EVENT OF A CONFLICT BETWEEN THESE NOTES AND THE BUILDING CODE, THESE NOTES SHALL TAKE PRECEDENCE. ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION, TYPICAL, ENGINEERING & DESIGN. THIS IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 2) ALL LUMBER SHALL BE SYP #2 (UNO)
- 3) ALL LVL LUMBER TO BE 1 7/8" WIDE ACTUAL EACH SINGLE MEMBER AND (2) 1 7/8" DEEP (ON GRADE)
- 4) ALL LVL LUMBER IS TO BE 1 3/4" (F_y = 255 PSF) OR GREATER
- 5) ALL LVL LUMBER IS TO BE 1 3/4" (F_y = 240 PSF) OR GREATER
- 6) ALL LOAD BEARING EXTENSION WINDOW HEADERS ARE TO BE (2) 2x10 W/ (1) 2x4 JACK STUD (UNO) AND KING STUDS PER TABLE R802.7.3 AND JOISTERS W/ (2) 2x10S @ 0'-0" PROVIDED THAT THE TOP OF THE WINDOW HEADERS ARE TO BE 1'-0" ABOVE FINISH FLOOR HEIGHT IS 1'-0" OTHERWISE REFER TO TABLES R802.7.1) AND R802.7.2)
- 7) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (UNO) REFER TO TABLES R802.7.1) AND R802.7.2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 8) PER TO 3018 NC BUILDING CODE SECTION R802 FOR CONSTRUCTION OF WALLS OVER 10'-0" IN HEIGHT
- 9) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- 10) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 11) ALL CONCRETE F_c = 3000 PSI UNO
- 12) PRECASTIVE BEARING WALLS SHALL BE 8" OR GREATER PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 13) PSB COLUMNS DESIGNED WITH MAX HEIGHT OF 9'-0" (UNO)
- 14) PSB COLUMNS SHALL BE 12" DIA. WITH 4#4 REINFORCEMENT AT TOP AND BOTTOM OF EACH COLUMN (UNO)
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCBC.
- 16) MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 17) UPRIFT LOADS GREATER THAN 800# SHALL BE CONTINUOUSLY METAL HANGERS SHALL BE SHAWNSON OR APPROVED EQUAL.



FIRST FLOOR PLAN
 1/4" = 1'-0"
 CEILING HGT. = 10'-0"



BRACING NOTE:
 BRACE POSTS PER DETAIL SHEET S2
 1/4" @ 12" O.C. UNO

WATERPROOF TREATMENT BY OTHERS TO ALL LVL BANDS

NOTCH KNEE BRACE & BOLT TO BANDS W/ 5/8" DIA. THROUGH BOLT 1/4"

8" X 8" PT KNEE BRACE

5/8" DIA. THROUGH BOLT (COUNTERSINK AS REQD)

1/3 POST HEIGHT

A POST Y-BRACING

SCALE: 1/4" = 1'-0"

S2
 2 of 4

DATE	2401-01-0022
REVISED	03-2024
DESIGNED BY	JTT
CHECKED BY	CSD
SEE PLAN	

**1ST FLR. HEADER
 2ND FLR. FRMG.**

LARRY WOOD
 DETACHED CARPORT
 501 SOUTH 16TH ST.
 ERWIN, NC 28339

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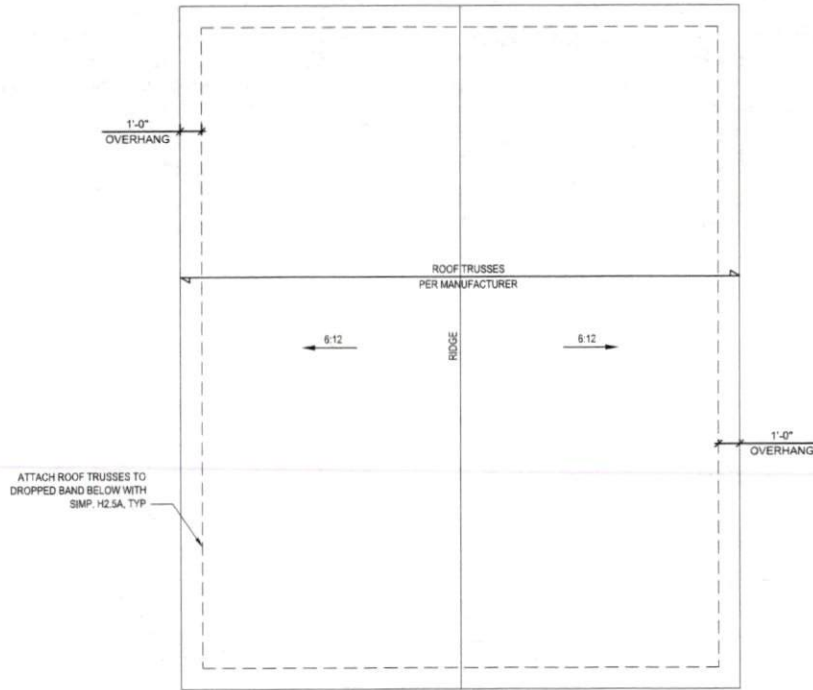
SEAL
 NORTH CAROLINA
 REGISTERED PROFESSIONAL ENGINEER
 CLAYTON S. BRYAN
 4107-2024

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND $F_b = 2600$ PSI, $E = 1,984$ PSI (OR GREATER) (I.E. LEVEL MICROLAM)
ALL LSL LUMBER IS TO BE 1.55E ($F_b = 2325$ PSI) (OR GREATER)
ALL PSL LUMBER IS TO BE 1.8E ($F_b = 2,400$ PSI) (OR GREATER)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 100 NAILS @ 8" O.C. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-4", OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 $F_y = 50$ KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, $f_c = 3000$ PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 12"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER, THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS, (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCR.
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



ROOF PLAN

1/4" = 1'-0"

* Engineer will draw and include construction notes, methods, materials, quantities, procedures or safety precautions. Any deviation or discrepancy in plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, or provisions in these documents were based on typical site conditions unless noted.



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386 Shipwash Drive • Cary, NC 27513

DATE: LARRY WOOD

DATE: DETACHED CAMPOR
ERWIN, N.C. 28539

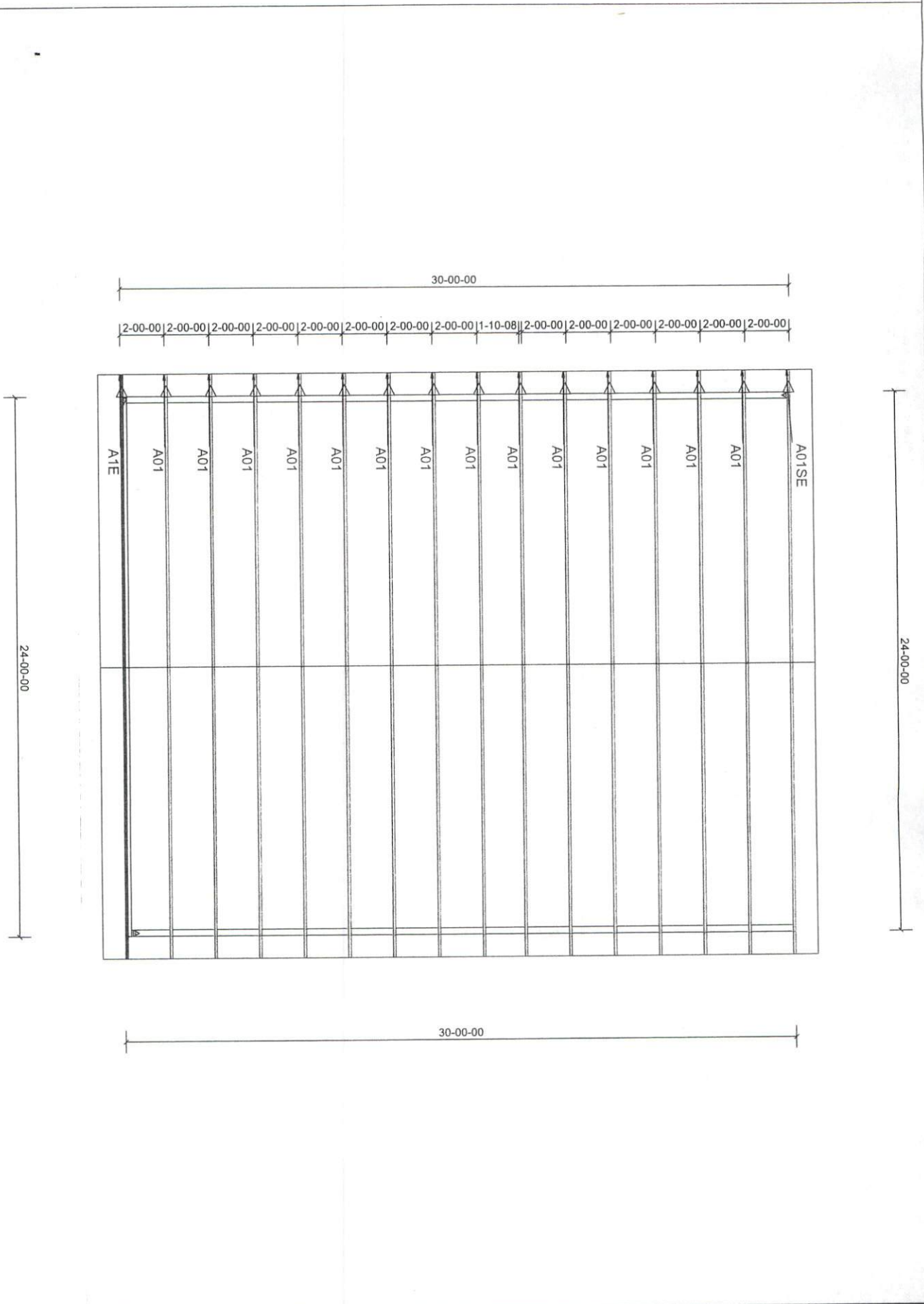
ROOF PLAN


Project #	2401-010022
Date	4/2/2024
Drawn By	JTT
Check By	CSD
Date	SEE PLAN

REVISIONS		
No.	Date	Remarks
△		
△		
△		

Sheet Number
S3

DRAWN: JTT/MSD; CHECKED: CSD; DATE: 4/2/2024; PROJECT: 2401-010022; SHEET: S3; SCALE: 1/4" = 1'-0"; TITLE: ROOF PLAN



2383- Dunn LARRY WOOD CARPORT		 84 Components 200 Emmett Rd Dunn NC 28334 United States Phone: 770.787.8715
LEE WOOD CARPORT Job# - 2301806		
Location	2383-Dunn	
Designer	RE	
<small>DO NOT CUT, NOTCH, OR BORE HOLES UNLESS SPECIFIC WRITTEN PERMISSION IS PROVIDED FOR ANY MODIFICATIONS TO THIS PLAN THESE INSTALLATION REQUIREMENTS APPLY TO ALL PERMANENT STRUCTURES IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS</small>		
Sheet # 1 of 1		
Roof Truss Placement Plan		
NOT TO SCALE DESIGNED DATE 12/26/2023		

Job 2301806-07081	Truss A1E	Truss Type Common Supported Gable	Qty 1	Ply 1	LARRY WOOD CARPORT Job Reference (optional)
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84 Components, Dunn, NC 28334

Run: 8.71 S Oct 4 2023 Print: 8.710 S Oct 4 2023 MiTek Industries, Inc. Tue Dec 26 05:25:50

Page: 1

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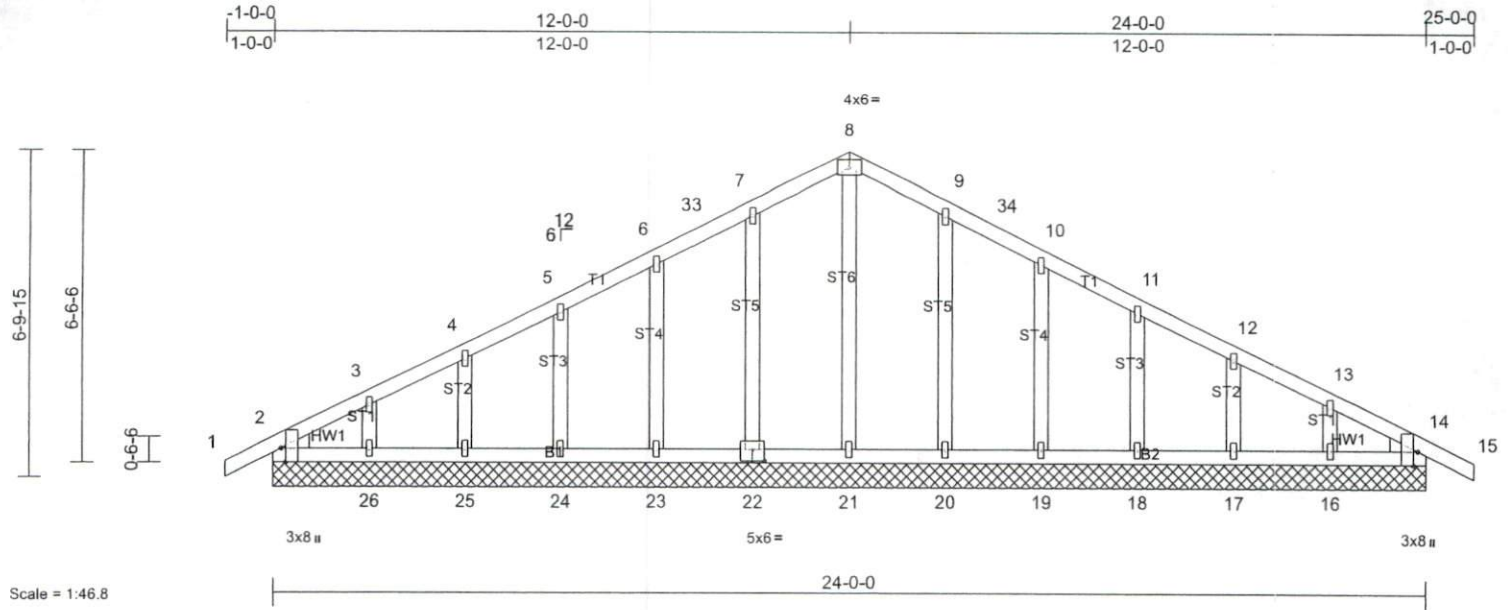


Plate Offsets (X, Y): [2:0-3-8,Edge], [14:0-3-8,Edge], [22:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	14	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MS								
											Weight: 134 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint (s) 2, 14, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16, 2, 14.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 14, 30.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

REACTIONS All bearings 24-0-0.
(lb) - Max Horiz 2=109 (LC 12), 27=109 (LC 12)
Max Uplift All uplift 100 (lb) or less at joint(s)
2, 14, 16, 17, 18, 19, 20, 22, 23,
24, 25, 26, 27, 30
Max Grav All reactions 250 (lb) or less at joint
(s) 2, 14, 16, 17, 18, 19, 20, 21, 22,
23, 24, 25, 26, 27, 30

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250
(lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) -1-0-0 to 2-0-0, Exterior (2) 2-0-0 to 12-0-0, Corner (3) 12-0-0 to 15-0-0, Exterior (2) 15-0-0 to 25-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - All plates are 1.5x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.

Job	Truss	Truss Type	Qty	Ply	LARRY WOOD CARPORT
2301806-07081	A01	Common	14	1	Job Reference (optional)

84 Components, Dunn, NC 28334

Run: 8.71 S Oct 4 2023 Print: 8.710 S Oct 4 2023 MiTek Industries, Inc. Tue Dec 26 05:25:52

Page: 1

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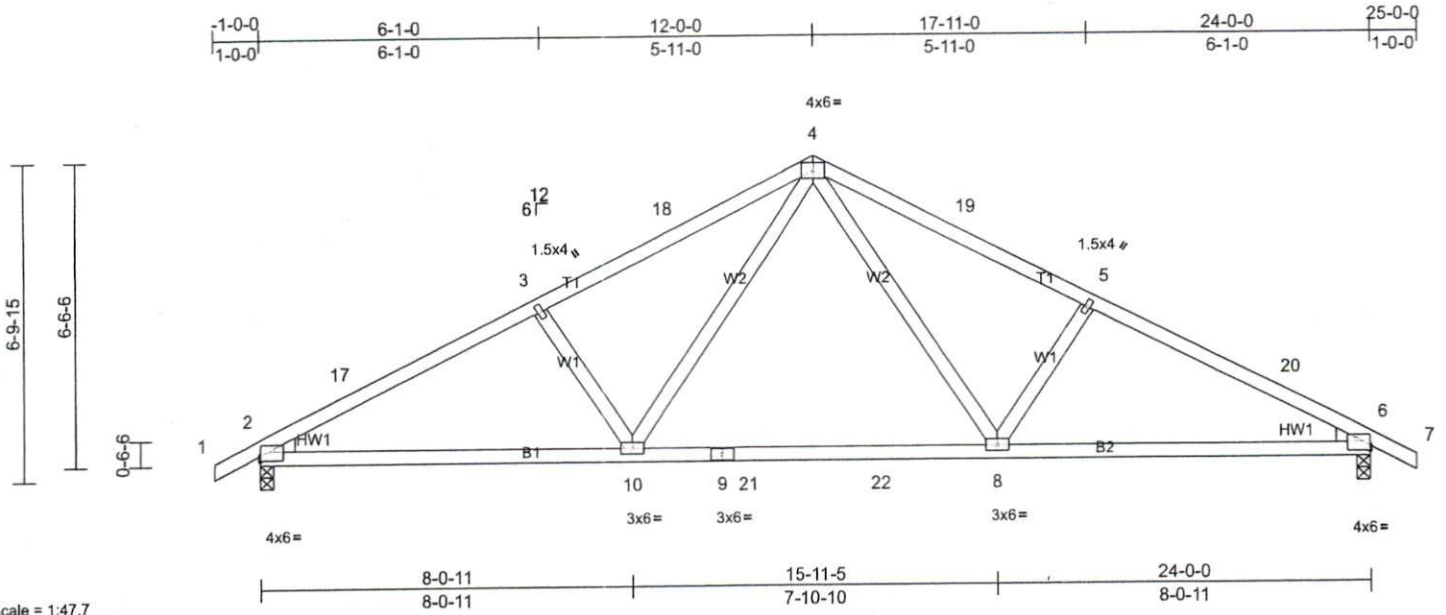


Plate Offsets (X, Y): [2:Edge,0-1-4], [6:Edge,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.44	Vert(LL)	-0.17	8-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.28	8-10	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MS								
											Weight: 113 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2 *Except* W1:2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 2 and 134 lb uplift at joint 6.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

REACTIONS (lb/size) 2=1020/0-3-8, (min. 0-1-8),
6=1020/0-3-8, (min. 0-1-8)
Max Horiz 2=109 (LC 12)
Max Uplift 2=-134 (LC 12), 6=-134 (LC 13)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-17=-1619/184, 3-17=-1478/212,
3-18=-1429/214, 4-18=-1343/230,
4-19=-1343/230, 5-19=-1429/214,
5-20=-1478/212, 6-20=-1619/184

BOT CHORD 2-10=-203/1378, 9-10=-38/927,
9-21=-38/927, 21-22=-38/927, 8-22=-38/927,
6-8=-113/1378

WEBS 4-8=-99/531, 5-8=-345/210, 4-10=-99/531,
3-10=-345/210

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 12-0-0, Exterior (2) 12-0-0 to 15-0-0, Interior (1) 15-0-0 to 25-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Job 2301806-07081	Truss A01SE	Truss Type Common	Qty 1	Ply 1	LARRY WOOD CARPORT Job Reference (optional)
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84 Components, Dunn, NC 28334

Run: 8.71 S Oct 4 2023 Print: 8.710 S Oct 4 2023 MiTek Industries, Inc. Tue Dec 26 05:25:52

Page: 1

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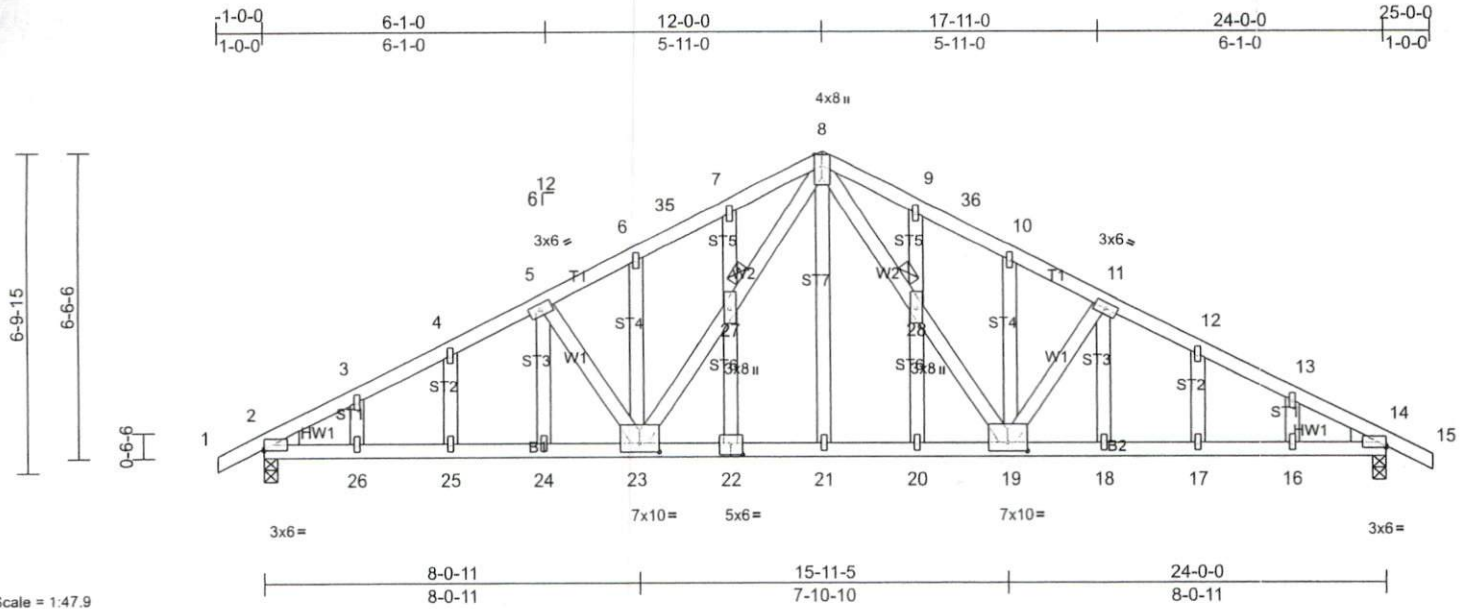


Plate Offsets (X, Y): [2:Edge,0-0-8], [14:Edge,0-0-8], [19:0-5-0,0-2-4], [22:0-3-0,0-3-0], [23:0-5-0,0-2-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	-0.06	19-20	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.65	Vert(CT)	-0.12	19-20	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.19	Horz(CT)	0.04	14	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MS								
											Weight: 164 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.2 *Except* W1:2x4 SP No.3
 OTHERS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 JOINTS 1 Brace at Jt(s): 27, 28

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size)

2=1020/0-3-8, (min. 0-1-8),
 14=1020/0-3-8, (min. 0-1-8)
 Max Horiz 2=108 (LC 16)
 Max Uplift 2=-133 (LC 12), 14=-135 (LC 13)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1590/151, 3-4=-1557/181,
 4-5=-1527/210, 5-6=-1390/213,
 6-35=-1386/237, 7-35=-1352/243,
 7-8=-1359/281, 8-9=-1338/276,
 9-36=-1352/242, 10-36=-1383/237,
 10-11=-1390/213, 11-12=-1527/210,
 12-13=-1557/181, 13-14=-1590/151
 BOT CHORD 2-26=-182/1365, 25-26=-182/1365,
 24-25=-182/1365, 23-24=-182/1365,
 22-23=-29/943, 21-22=-29/943,
 20-21=-29/945, 19-20=-29/945,
 18-19=-91/1365, 17-18=-91/1366,
 16-17=-91/1366, 14-16=-91/1366
 WEBS 8-28=-164/505, 19-28=-161/498,
 11-19=-314/122, 23-27=-159/497,
 8-27=-173/540, 5-23=-315/122

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 11-10-14, Exterior (2) 11-10-14 to 14-10-14, Interior (1) 14-10-14 to 25-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 2 and 135 lb uplift at joint 14.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard