INDEX OF DRAWINGS				
A1	Cover Page			
A2	Floor Plans			
A3	Exterior Elevations & Cross Section			
D1	Details			

ALL WORK IS TO COMPLY WITH THE 2018 NORTH CAROLINA RESIDENTIAL CODE

DESIGN LOADS:

STRUCTURAL ATTICS WITHOUT STORAGE 10psf ATTICS WITH LIMITED STORAGE 20psf ASSUMED SOIL BEARING CAPACITY 1500psf CLIMATIC & GEOGRAPHIC DESIGN CRITERIA (TABLE R301.2(1))* **ROOF LIVE LOAD*** 20psf ULTIMATE WIND SPEED (V-ult)* 120mph

WIND EXPOSURE CATEGORY* A-B SEISMIC DESIGN CATEGORY* WEATHERING* MODERATE **FROST LINE DEPTH*** 12"

*Verify all climatic and geographic deign criteria with local building official.

B

ABBREVIATIONS					
A.F.F.	ABOVE FIRST FLOOR				
AT	ARCH TOP WINDOW				
AWN	AWNING WINDOW				
BD	BOARD				
CLG	CEILING				
CM	CASEMENT WINDOW				
COL.	COLUMN				
CT	CIRCLE TOP WINDOW				
DBO	DETERMINED BY OWNER				
DH	DOUBLE HUNG WINDOW				
DR	DOOR				
GL	GLIDING WINDOW				
HDR	HEADER				
HGT	HEIGHT				
LVL	LAMINATED VENEER LUMBER				
0.C.	ON CENTER				
O.H. DR.	OVERHEAD DOOR				
RCH	ROUGH CEILING HEIGHT				
SL	SIDELIGHT				
(T)	PRESSURE TREATED OR				
	NATURALLY RESISTANT TO DECAY				
T.O.W.	TOP OF WINDOW				
TEMP. GL.	TEMPERED GLASS				
TR	TRANSOM WINDOW				
U.N.O.	UNLESS NOTED OTHERWISE				
WDW	WINDOW				



FASTENER SCHEDULE FOR STRUCTURAL MEMBERS						
DESCRIPTION OF BUILDING ELEMENTS	NUMBER, TYPE & SPACING					
TOP OR SOLE PLATE TO STUD. END NAIL	2-16d					
STUD TO SOLE PLATE, TOENAIL	(3-8d), OR (2-16d)					
DOUBLE STUDS. FACE NAIL	10d AT 24" O.C.					
DOUBLE TOP PLATES. TYP. FACE NAIL	10d AT 24" O.C.					
,						
DOUBLE TOP PLATES, LAP SPLICE MIN 48"OFFSET OF	END JOINTS 8-16d					
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PL	ATE. TOENAIL 3-8d					
TOP PLATES, LAPS @ CORNERS AND INTERSECTION.	FACE NAIL 2-10d					
BUILT-UP OR CONT. HEADER. (2) PIECE w/ 1/2" SPACE	R 16d AT 16" O.C. ALONG EDGE					
CEILING JOIST TO PLATE. TOE-NAIL	3-8d					
CONTINUOUS HEADER TO STUD. TOE-NAIL	4-8d					
CEILING JOISTS LAPS OVER PARTITIONS FACE NAIL	3-10d					
CEILING IOISTS TO PARALLEL RAFTERS FACE NAIL	3-10d					
	3-16d					
BUILT-OF GIRDER AND BEAWS, 2 LUWBER LAYERS	2-10d AT EACH SPLICE & @ ENDS					
	2-16d AT EACH BEARING					
ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS	4-16d TOENAIL or 3-16d FACENAIL					
RAFTER TIES TO RAFTERS, FACE	3-8d					
WOOD STRUCTURAL PANELS AND PART	ICLE BOARD :					
ROOF AND WALL SHEATHING (TO FRAMI	NG)					
5/16"- 1/2" (SUBELOOR WALL) 6d COMMON	6"O.C. EDGES(I) 12"O.C. INTERMEDIATE(a)					
19/32"- 1" 8d COMMON	8d COMMON, 6"O.C. EDGES(I), 12"O.C. INTERMEDIATE(a)					
1/2" (b) 1 1/2" GA. ROOFING NAIL; 6d COMMON or STAPLE GA. 1 1/2";1 1/4 SCREWS, TYPE W OR S.(b,c,d,e) 4"O.C. EDGES(i), 8"O.C. INTERMEDIATE(c,e)						
5/8" (b) 1 3/4" GA. ROOFING NAIL; 8d COMMON or STAPLE GA. 1 5/8"; 1 1/4 SCREWS, TYPE W OR S.(b,c,d,e) 4"O.C. EDGES(i), 8"O.C. INTERMEDIATE(c,e)						
 a. For regions having basic wind speed of 100 mph or legable end wall framing shall be spaced 6 inches on centrattaching panel roof sheathing to intermediate supports a distance from ridges, eaves and gable end walls; and 4 is b. Gypsum sheathing shall conform to ASTM C 1396 and sheathing shall conform to ASTM C 208. General Notes: All nails are smooth-common, box or deformed shanks e sheathing connections shall have minimum average ben 0.192 inch (20d common nail), 90 ksi for shank diameter ksi for shank diameters of 0.142 inch or less. Staples are 16 gage wire and have a minimum 7/16-inch Nails shall be spaced at not more than 6 inches on center Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be ap Spacing of fasteners not included in this table shall be bar THIS TABLE PROVIDES THE MINIMUM NAILING REC 	es, nails for attaching wood structural panel roof sheathing to er. When basic wind speed is greater than 100 mph, nails for shall be spaced 6 inches on center for minimum 48-inch inches on center to gable end wall framing. I shall be installed in accordance with GA 253. Fiberboard except where otherwise stated. Nails used for framing and ding yield strengths as shown: 80 ksi for shank diameter of s larger than 0.142 inch but not larger than 0.177 inch, and 100 on diameter crown width. er at all supports where spans are 48 inches or greater. plied vertically. ased on Table R602.3(2). JIREMENTS FOR MOST APPLICATIONS. IF THERE ARE					

ANY DISCREPANCIES BETWEEN THIS TABLE AND WHAT IS SHOWN ELSEWHERE ON THESE PLANS, THE MORE STRINGENT REQUIREMENT SHALL APPLY UNLESS OTHERWISE APPROVED.

PLOT PLAN SCALE: N.T.S

SEPTIC TANK SYSTEM

GENERAL NOTES

GENERAL

Contractor shall provide the requirements of all detail callouts denoted as "TYPICAL" or "TYP." at specifically noted conditions and at all like conditions throughout the project, unless otherwise noted. All details on detail sheets titled as "TYPICAL", and not directly referenced on plans, shall be incorporated at occurring locations throughout the project. Requirements of details not denoted or titled as "TYPICAL" or "TYP" shall be provided at the specific location shown on the plan and adjacent areas as applicable. requirements of details denoted as "SIMILAR" or "SIM" shall be provided with differences as indicated or implied on referenced details and plans.

Details may be depicted diagrammatically. For example, roof pitches, floor/ roof/ wall thickness, framing members, etc., may differ in scale from actual proposed conditions. Details shall be understood in context with other drawings conveying structural and architectural design intent. Dimensions and notes shall take preference over the scaling of the drawing. It is recommended that contractors/ sub-contractors do not scale dimensions from the drawings but use the written dimensions instead.

Where conflicting information exists between these plans and other referenced requirements, the more stringent requirement shall apply unless otherwise approved.

Contractor shall be responsible for providing adequate structural support of construction loads during all phases of construction including, but not limited to, foundation backfill, bracing of wall framing to resist construction floor loads and lateral building loading, bracing of trusses during installation and subsequent construction loading, and other conditions as dictated by the contractor's construction practice.

On-site verification of all dimensions and conditions shall be the responsibility of the contractor and subcontractors. The contractor shall compare and coordinate all drawings prior to construction and when in the opinion of the contractor a discrepancy or error exists, he shall promptly report it to the designer for proper adjustment before proceeding with the work.

All work shall comply with the manufacturer's or fabricator's instructions or recommendations for the preparation of substrates and installation and use of material. Manufacturer's installation instructions/ specifications shall be available on the job site at the time of inspection

SITE WORK landscaping.







Work performed shall comply with these general notes unless otherwise noted on plans. The work shall comply with all applicable local and state codes, ordinances, regulations and amendments and all other authorities having jurisdiction.

All conditions and dimensions shown on the plans shall be verified by the contractor, any discrepancies that require clarification or revisions shall be brought to the attention of the designer before commencing with the work. Ryan Moe Home Design (RMHD), it's owners and affiliates are not responsible for any loss resulting from an error or omission on these plans

These drawings do not cover site work, excavation, grading and

VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION

EXCAVATION

Shall be sufficient to provide full design dimensions or to allow for forming as required.

BACKFILL AND COMPACTION

Use clean material containing no organic material, trash, muck, roots, logs, stumps, concrete, asphalt or other deleterious substances. Do not backfill against masonry walls until super structure is in place, or adequate bracing is provided. Prior to placing fill, the existing surface shall be cleared of all refuse or organic materials. Equivalent fluid pressure of soil backfill not to exceed 30 P.C.F. uniform class SM or better.

FOUNDATIONS

Soil bearing value assumed to be 1,500 PSF minimum unless otherwise noted on drawings. Builder to be notified immediately should bearing capacity of less than 1,500 psf or high water table be encountered. Foundation walls of masonry and concrete are to be constructed as per plan and in accordance with the applicable building codes.

CONCRETE CONCRETE SHALL REACH THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH OF(28 days psi) UNLESS NOTED OTHERWISE:

Basement walls, foundations and other concrete not exposed to the weather 2500psi

Basement slabs and interior slabs on grade, except garage floor slabs: 2500psi

Basement walls, foundation walls, exterior walls and other vertical concrete work exposed to the weather: 3000psi and air-entrained.

Porches, carport slabs and steps exposed to the weather, and garage floor slabs: 3000psi and air-entrained.

All concrete to be poured in accordance with ACI 318 specification. REINFORCING RODS

Shall conform to ASTM A-615 grade 60. WWF shall conform to ASTM A-185

STEEL All metal anchors, fasteners, joist hangers, etc. to be galvanized. All structural steel to conform to ASTM-36. Pipe to be A53. Tube to be A500 of A501. Detailing to be in accordance with AISC structural steel detailing manual. Connections shall be capable of supporting an allowable uniform load stress of 24 ksi. Steel columns and bases to be given a shop coat of rust inhibitive paint or equivalent. Bottom of steel columns shall be anchored in concrete.

LUMBER EXPOSED TO WEATHER

All exposed exterior lumber or lumber in contact with masonry or concrete shall be pressure preservative treated in and bear quality mark of approved inspection agency. And complies w/ the requirements of the American Lumber Standard Committee Treated Wood Program. Provide fire retardant sheathing and lumber where indicated on drawings. All wood shall be a minimum of 8" above finish grade or pressure preservative treated less than 8" above finish grade.

MAXIMUM MOISTURE CONTENT Lumber shall be 19%, kiln dried in accordance with AWPA standards.

PLYWOOD & O.S.B.

Plywood & O.S.B. used structurally shall meet the performance standards and all other requirements of applicable commercial standards for the type, grade and species of plywood and shall be so identified by an approved testing agency.

FLASHING

Flash and counter flash at roof and wall intersections, valleys, crickets and saddles, and sidewalls per code. Flashing at vent pipes, soil stacks, vertical front walls to be applied in accordance with manufacturer's printed instructions. Flashing material to be min. .019 corrosion resistant metal, mineral surfaced roll roofing, or approved equal.

Flash all exterior openings and all building corners as required. Flash and caulk wood beams and other projections through exterior walls and roof surfaces.

CAULKING

Fill all joints of different materials and all penetrations as required. ALL OPERABLE WINDOWS

All operable windows shall have noncorrosive screens and sash locks. Screens are provided to prevent the entry of insects and are not intended to prevent children from falling out of open windows. Windows having the lowest part of their clear opening less than 24" above finished floor and more than 72" above finished grade shall: (1) be allowed to open to such a distance such that a sphere of 4" cannot passs through, or, (2) shall be provided with guards complying with ASTM F 2090, or (3) provided with window opening control devices that comply with ASTM F 2090

WEATHERPROOFING

All sliding, swinging doors, and window openings to the exterior shall be fully weatherstripped, caulked, gasketed or otherwise treated to limit air infiltration. Doors and windows shall meet air infiltration and other performance factors as required by code.

HEADERS All headers over 48" in length to have a minimum (2) jack studs at each end.

STRENGTH OF FRAMING MATERIALS All framing lumber shall be AF&PA/NDS (or other approved

agency) rated lumber of the following (or as noted on plans): Joists and rafters shall be southern pine, grade 2 or better.

Wood studs and posts shall be SPF stud grade or better

Headers shall be #2 Douglas fir or #1 Southern Pine, #1 Hem fir or #1 SPF

LVL beams shall have the following minimum properties: -Bending stress "Fb" = 2600 PSI

-Horizontal shear "Fv" = 285 PSI

-Compression perpendicular to grain "Fc" = 750 PSI -Compression parallel to grain "Fc" = 2510 PSI

-Modulus of elasticity "E" - 2,000,000 PSI

ENGINEERED TRUSSES

Trusses shown on this plan are for design and illustrative purposes. The truss manufacturer is to provide truss design drawings which may differ from what is shown on this plan. Notify RMHD if the truss design drawings affect any aspect of the design.

Truss design drawings shall be provided to the building official and approved prior to installation. Truss design drawings shall include, at a minimum, the information specified below. Truss design drawings shall be provided with the shipment of trusses delivered to the jobsite.

1. Slope or depth, span and spacing.

2. Location of all joints.

3. Required bearing widths. 4. Design loads as applicable.

4.1. Top chord live load (as determined from Section R301.6). 4.2. Top chord dead load.

4.3. Bottom chord live load.

4.4. Bottom chord dead load.

4.5. Concentrated loads and their points of application.

4.6. Controlling wind and earthquake loads. 5. Adjustments to lumber and joint connector design values for conditions of use.

6. Each reaction force and direction. 7. Joint connector type and description (e.g., size, thickness or gage) and the dimensioned location of each joint connector except where symmetrically located relative to the joint interface. 8. Lumber size, species and grade for each member.

9. Connection requirements for: 9.1. Truss to girder-truss.

9.2. Truss ply to ply.

9.3. Field splices.

10. Calculated deflection ratio and/or maximum description for live and total load. 11. Maximum axial compression forces in the truss members to

enable the building designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces shall be shown on the truss design drawing or on supplemental documents

12. Required permanent truss member bracing location.

Trusses shall be braced to prevent rotation and provide lateral stability in accordance with the requirements of the individual truss design drawings. In the absence of specific bracing requirements, trusses shall be braced in accordance with accepted industry practice such as the SBCA Building Component Safety Information (BCSI) Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Truss Members shall not be cut, notched, drilled, spliced or otherwise altered in any way without the approval of a registered design professional. Alterations resulting in the addition of load (e.g., HVAC equipment, water heater) that exceeds the design load for the truss shall not be permitted without verification that the truss is capable of supporting such additional loading.

ROOF TRUSSES SHALL BE BRACED IN ACCORDANCE WITH THE BUILDING COMPONENT SAFETY INFORMATION GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES (BCSI-B3)

TO PRINT TO SCALE, DRAWING SHALL BE PRINTED ON ARCH D (24" x 36") SIZED PAPER

DRAWING IS PROPERLY PRINTED TO SCALE IF EACH BOX MEASURES ONE INCH BY ONE INCH ALL OTHER PAGES WILL HAVE MARKS IN THIS CORNER SPACED ONE INCH APART.

Ryan <u>Carvajal Garage</u>

Design

[Moe Home]



REVISIONS







WALL BRACING PLAN

BRACED WALL LINE SCHEDULE							
	WALL LINE 1	WALL LINE 2	WALL LINE A	WALL LINE B			
BRACED-WALL-LINE LOCATION	1ST OF 1-STORY	1ST OF 1-STORY	1ST OF 1-STORY	1ST OF 1-STORY			
EAVE TO RIDGE HEIGHT	6 FT	6 FT	6 FT	6 FT			
BRACED-WALL-LINE SPACING	50.25 FT	50.25 FT	22.75 FT	22.75 FT			
WALL HEIGHT	12 FT	12 FT	12 FT	12 FT			
BRACING METHOD	CS-WSP	CS-WSP	CS-WSP	CS-WSP			
GB CONSTRUCTION TYPE	N/A	N/A	N/A	N/A			
GYPSUM WALL BOARD ON INSIDE	NO	YES	NO	NO			
HORIZONTAL JOINTS BLOCKED	YES	YES	YES	YES			
HOLDOWN DEVICE USED	NO	NO	NO	NO			
WIND							
TABULATED WIND BRACING AMOUNT	8.0375 FT	8.0375 FT	3.9125 FT	3.9125 FT			
EXPOSURE HEIGHT FACTOR	1	1	1	1			
EAVE-TO-RIDGE HEIGHT FACTOR	0.76	0.76	0.76	0.76			
WIND WALL HEIGHT FACTOR	1.1	1.1	1.1	1.1			
NUMBER OF BWL FACTOR	1	1	1	1			
HOLDOWN FACTOR	1	1	1	1			
BLOCKED JOINT FACTOR	1	1	1	1			
GYPSUM ON INSIDE FACTOR	1.4	1	1.4	1.4			
WIND GB CONSTRUCTION FACTOR	1	1	1	1			
REQUIRED WIND BRACING AMOUNT	9.41 FT	6.72 FT	4.58 FT	4.58 FT			
RESULTS							
LENGTH OF WALL BRACING REQUIRED	9.41 FT	6.72 FT	4.58 FT	4.58 FT			
LENGTH OF WALL BRACING PROVIDED	12.0 FT	6.75 FT	24.0 FT	17.0 FT			

KEY
A BRACING METHOD (SEE $\binom{6}{D1}$)
4.0 - LENGTH OF QUALIFIED BRACING

WALL BRACING NOTES

WALL PANELS. SEE MAIN FLOOR PLANS FOR EXACT DIMENSIONS. WALL BRACING SHALL BE PROVIDED BY CONTINUOUSLY SHEATHED BRACED WALL PANELS (CS-WSP) PER R602.10 OF THE 2015 IRC AS ALLOWED UNDER R602.10(4) OF THE NCRC. ALL SURFACES OF EXTERIOR WALLS ARE TO BE COMPLETELY SHEATHED INCLUDING INFILL AREAS ABOVE AND BELOW WINDOWS AND DOORS.

FOR CONTINUOUSLY SHEATHED WALLS, THE PANELS SHOWN ON THIS PLAN ARE ONLY INTENDED TO SHOW THAT THE PLAN MEETS THE MINIMUM PANEL REQUIREMENTS. ALL SURFACES OF CONTINUOUSLY SHEATHED WALL LINES ARE TO BE COMPLETELY SHEATHED INCLUDING INFILL AREAS ABOVE AND BELOW WINDOWS AND DOORS. THE REQUIRED LENGTH OF BRACING IN TABLES R602.10.3(1) AND R602.10.3(3) HAS BEEN MULTIPLIED BY THE APPROPRIATE ADJUSTMENT FACTOR IN TABLES R602.10.3(2) AND R602.10.3(4), RESPECTIVELY FOR WALLS WHERE A REQUIRED INTERIOR FINISH HAS BEEN OMITTED.

THIS PLAN IS INTENDED TO SHOW THE MINIMUM LOCATION OF BRACED





FOUNDATION PLAN SCALE: 1/4 in = 1 ft

22'-10"







ELEVATIONS & CROSS SECTIONS ARE DERIVED FROM A 3D MODEL OF THE PLAN WHICH MAY INCLUDE UNNECESSARY LINES AND MINOR DISCREPANCIES. PLEASE DEFER TO FLOOR PLANS AND INDICATED DETAILS



RIGHT ELEVATION SCALE: 1/4 in = 1 ft







VINYL SIDING

REAR ELEVATION SCALE: 1/4 in = 1 ft



