

τ 919 773-1200 = ¢ 919 773-9658 250 Shipwash Drive, Suite 104 = Garner = North Carolina = 27529 www.tyndallengineering.com

June 5, 2023

Mike Campbell 25 Burton Street Walton, NY 13856 Email: soup0528@gmail.com

Reference: Engineering Services 7934 NC-210 Bunnlevel, NC 28323 TE&D Project No.: 2301-020499



To Whom It May Concern;

As requested by the client, a representative of Tyndall Engineering & Design, PA (TE&D) was onsite to inspect and observe the existing footing and foundation. We understand the previous home was severely damaged by a fire (as well as the subsequent fire suppression) and was completely removed from the lot. We inspected and observed the following:

- 1) Analysis of the existing soil underlying the existing remaining footing.
- 2) Observe the materials/condition of the existing remaining foundation.

The following conclusions and recommendations were noted:

- The underlying soils were visually observed, qualitatively probed, and subjected to Dynamic Cone Penetrometer (DCP) testing in multiple locations at depths to 2'-0" below existing grade. Hand augers were also advanced to depths of 2'-0" below existing grade. The existing soils were found to equal or be in excess of the minimum 2000 psf bearing capacity required by the 2018 North Carolina Residential Building Code. Based on our observations, analysis, and the results of our field-testing program, the underlying soils are structurally adequate to support the anticipated loading conditions of the existing footing.
- 2) We visually observed the foundation as consisting of 8" x 16" CMU walls at the interior and exterior and 16" x 16" CMU piers at the porches. The foundation was observed to be supported by concrete footings. The existing foundation was visually observed and inspected for damage as well as subjected to non-destructive (Schmidt rebound hammer) testing at the garage slab. The concrete at the garage slab was found to equal or be in excess of the minimum 2500 psi compressive strength required by the 2018 North Carolina Residential Building Code. Based on our observations, analysis, and the results of our field-testing, the existing foundation is to be repaired/enhanced per the following:
 - At multiple locations, we observed severe deterioration of the existing mortar joints. Based on our observations and analysis, we recommend the deteriorated mortar joints be removed and repointed with mortar as needed.



- b. At the front porch piers, we observed several piers as being damaged at the top course. Based on our observations and analysis, we recommend the damaged piers be repaired as needed by removing the topmost course and replacing it with new 16" x 16" CMU blocks.
- c. We also recommend the remaining sill plates be removed and replaced with new pressure treated 2 x 8 sill plates. New sill plates are to be installed so that the existing anchor bolts are within 1'-0" of plate splices. If this cannot be achieved, or the existing anchor bolts are damaged during removal, the sill plate may be fastened to the existing foundation with 1/2" x 8" Simpson Titen HD screws or 1/2" x 12" threaded rods with nuts and washers. The rods are to be embedded a minimum 10" into the turned down footing with Hilti HIT-HY 200 Epoxy per the manufacturer's specifications. Screws and/or rods are to be installed with spacing no greater than 6'-0" o.c. and within 12" of plate splices.

We appreciate being able to assist you during this phase of the project. If you need further assistance or require additional information, please do not hesitate to contact us.

Sincerely, Tyndall Engineering & Design

Anna 5

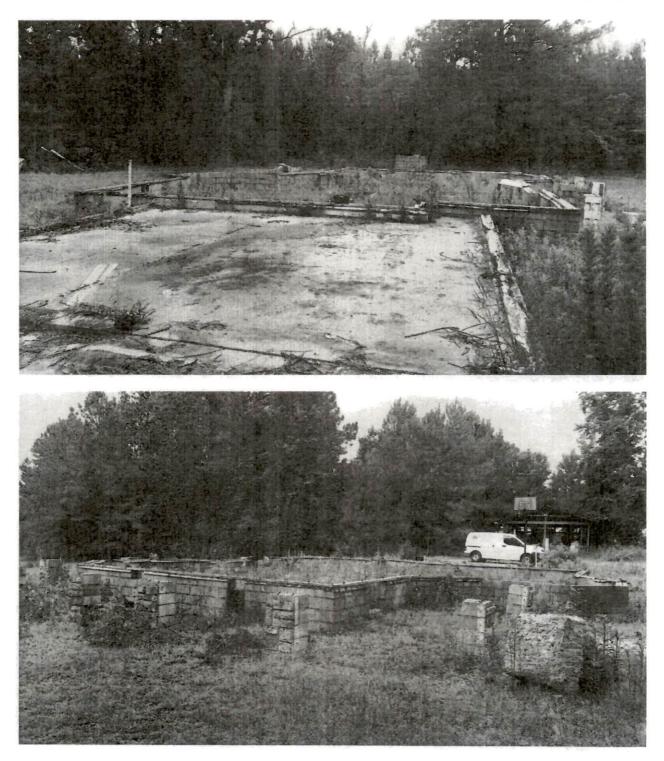
Tripp Amoš PT III | 2301-020499

Prentice Tyndall Jr., P.E.





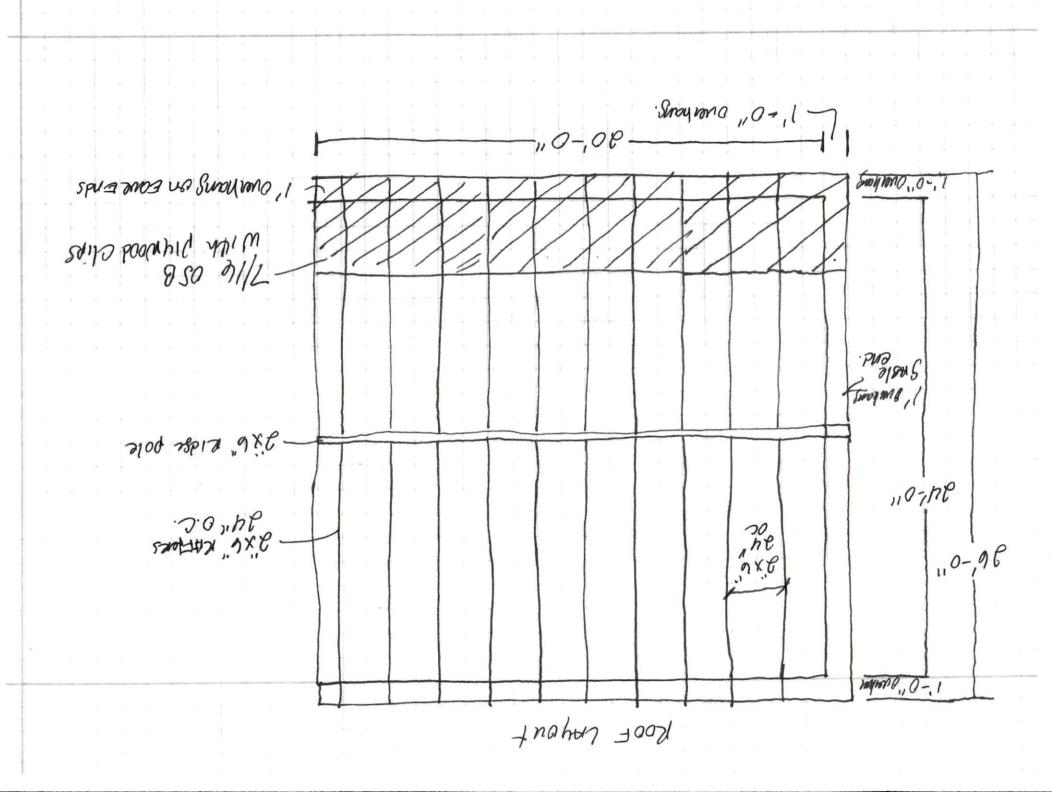
7 919 773-1200 = \$ 919 773-9658 250 Shipwash Drive, Suite 104 = Garner = North Carolina = 27529 www.tyndallengineering.com

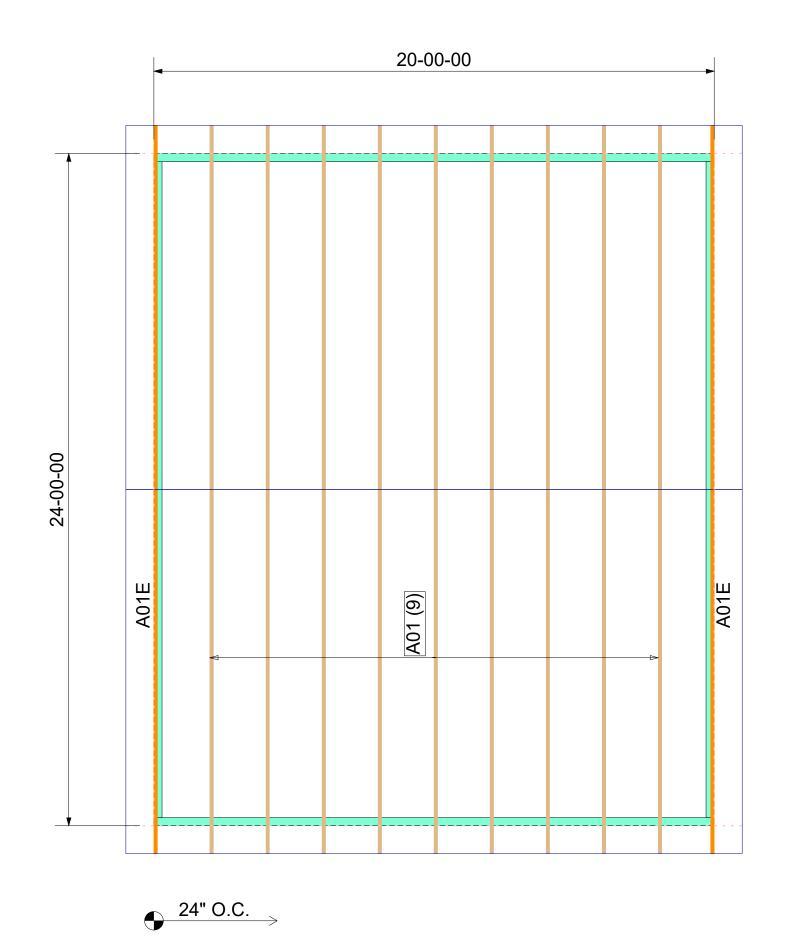


Site of Inspection

romanda SECOL/S/L PAROUSUI NEW BRIDGE BUHERNER JOOD BROADD 1 1 man door , OEX, he Smopul H SUTS WHASIXA UD 8#75 Suntand Surysixa 34, ×30, 94549E posoday Hds Grow Crown 210 + 100 100 00 30, 00 st/13 biter DEO ATO DELOID BOSS FIRDDODDD (90000 9999 (9000099 1591 = 7248 1-918 = that BURNCEVEL, NUC 26323 ,50 = tom SOLP ON MEBL ,011 = +4617 0.800 989010 : 0:0 south the property mand substitution to a 000.8108-98-9850:00 SUPLE = 3 per Block. GL

Double, Topp)ate 2 TACK Studs on EACH Side top cripples 2"x 8" Bofed 2×8" Boxled hogder HEPDER Ack Stud JACK-Stud King -STUC. Kingstud 10-0 OPANIAS U. Noon 2'24" dar caning -1600 End 6 4010 Acomidate Below Capples z 1-10 WIDE GACAGE DOOR 1/ 10 2" 24" Sigle bottom plate on top of Sill de 136 4 Windows @ 34" x 49" Double Hurg. Hegders will be 37" wide To Accomidate SACK Studs. will book @ 36" Header Will Be 39" To Accomidate JACK studs. HENder 2'X 6" Ridge Pole 1"xb" 0.24" oc 14 Raties × 4 2x6 FACION 2"×4" Collor +1 tiE 5 Actual = 13 - 74 @ 24" From Bottom of Ridge. Hurrichne/ truss ties Double Top plake 2'24' Hoor Studs 12" Ovieling w/v.nyl SOSFIT. 7/10 050 -Single Bottom plane on TOP of Sill plate. 2XB' Pressure treated Sill Plate. Anchored using Existing Anchors. FOAM SILL SEAL 811 Existing Anchor Botts. in BRISting Foundation Concrete







NC 1-919-787-8787 / 1-800-473-8787 Fax 1-919-783-0617

VA 1-757-833-5300 / 1-800-868-8787 Fax 1-757-833-5400

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the entire truss support structure including, but not limited to headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult "Bracing of wood trusses" available from the Truss Plate Institute, 583 D'Onifrio Drive; Madison, WI 53179

TRUSS TO BEARING DESIGN **RESPONSIBILITY**.

Truss to bearing connections if shown on this layout are suggested by Truswood based solely on the uplift reactions and considerations for the truss component. All trus to bearing connections must be specified or approved by the Building Designer to adequately transfer all loads to the building system and foundation. Consult hardware manufacturer's specifications for all installation requirements.

GENERAL NOTES: 1.) REFER TO INDIVIDUAL TRUSS DRAWINGS FOR ADDITIONAL

.....WALL U.N.O. 3.) DIMENSIONAL VERIFICATION IS THE RESPONSIBILITY OF THESITE CONTRACTOR AND /OR ARCHITECT. 4.) ALL INTERIOR HEADERS TO BE DROPPED EXCEPT AS NOTED. 5.) ALL TRUSSES MUST BE SPACED AT A MAXIMUM OF 24" OCUNLESS OTHERWISE NOTED. 6.) "DO NOT CUT, DRILL, OR ALTER ANY TRUSS WITHOUT THEWRITTEN CONSENT FROM A REGISTERED ENGINEER.

