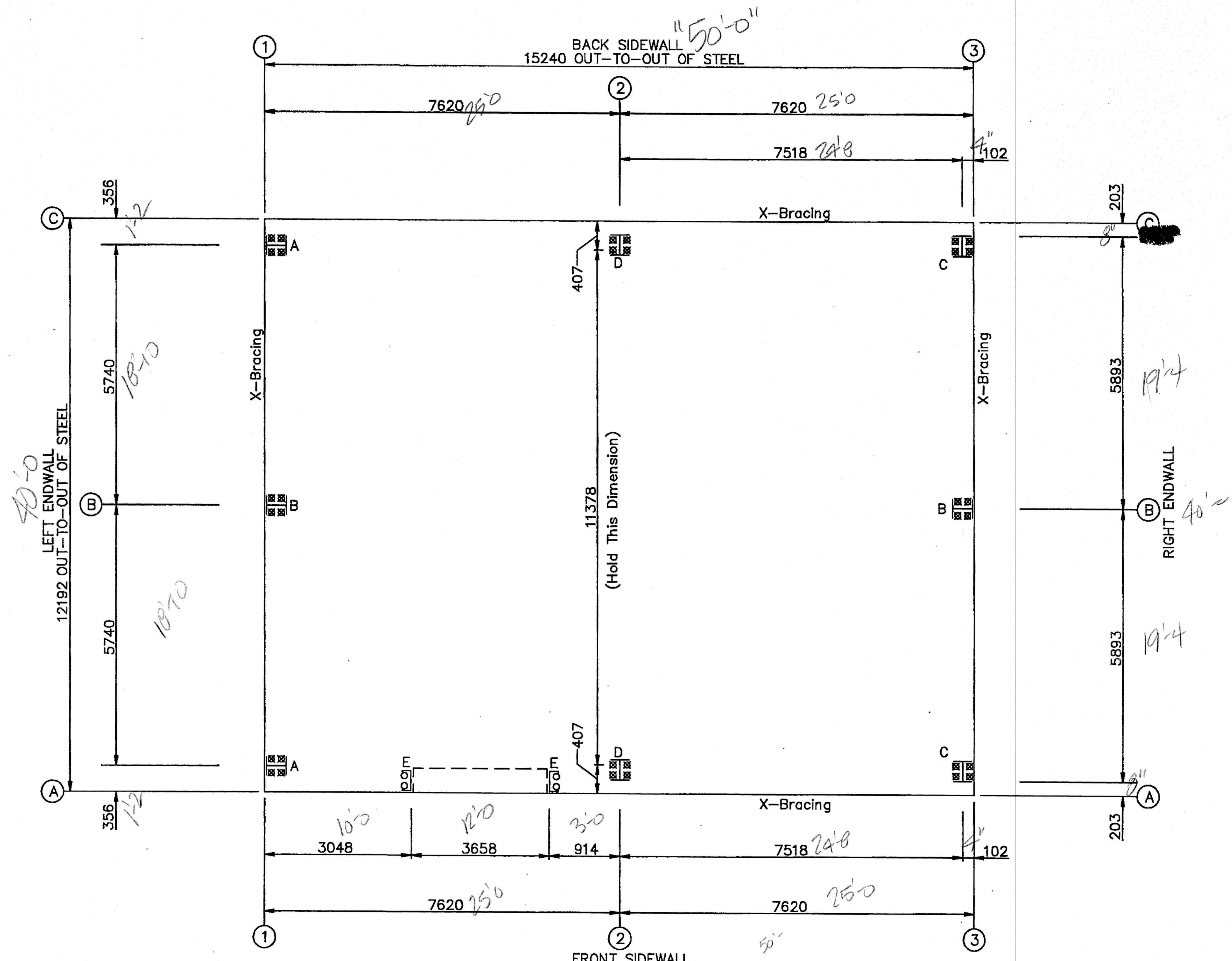


○ Dia=13

⊗ Dia=19

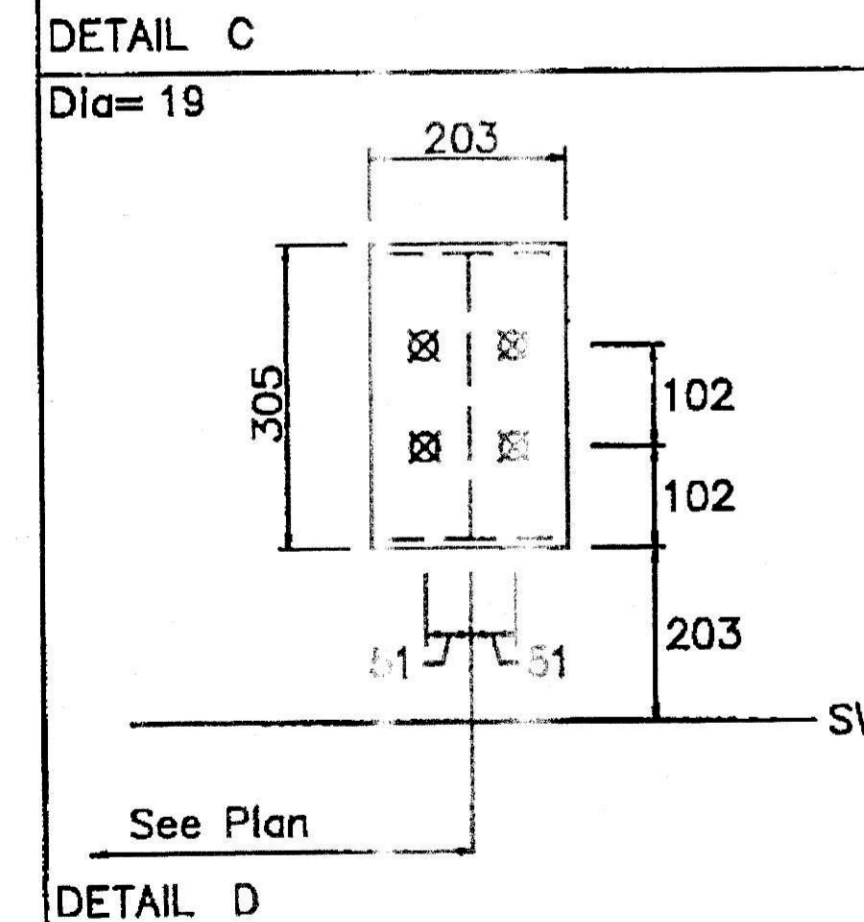
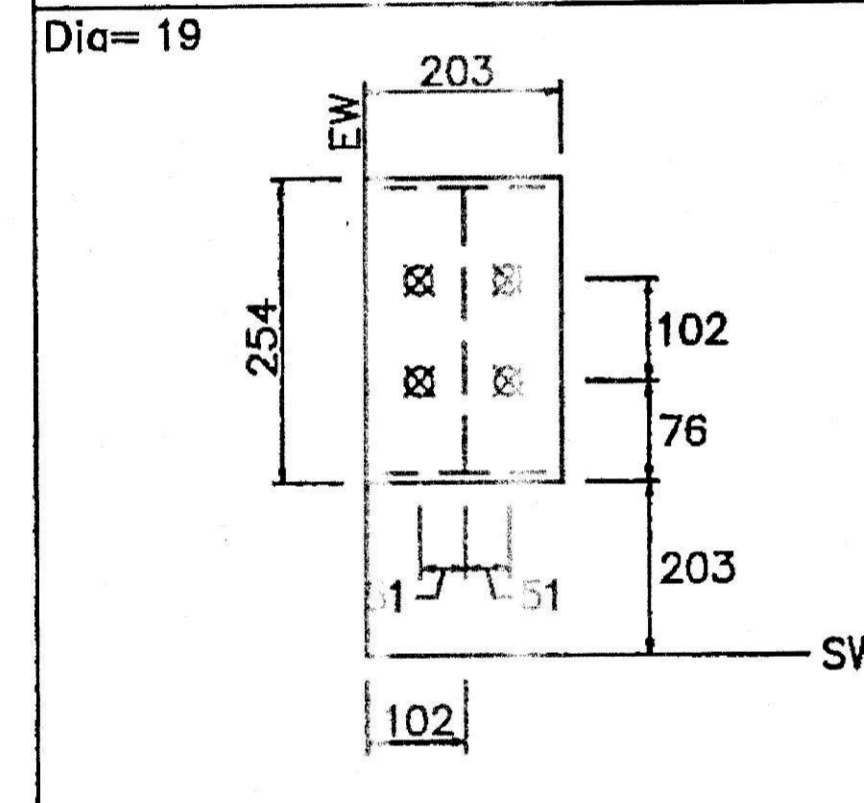
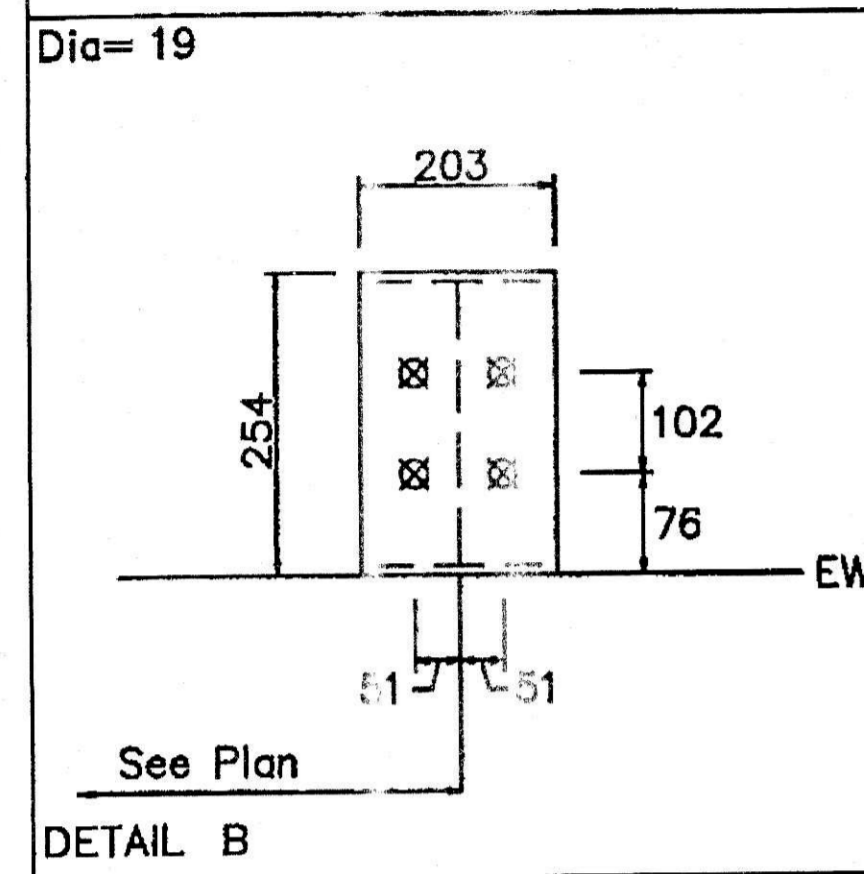
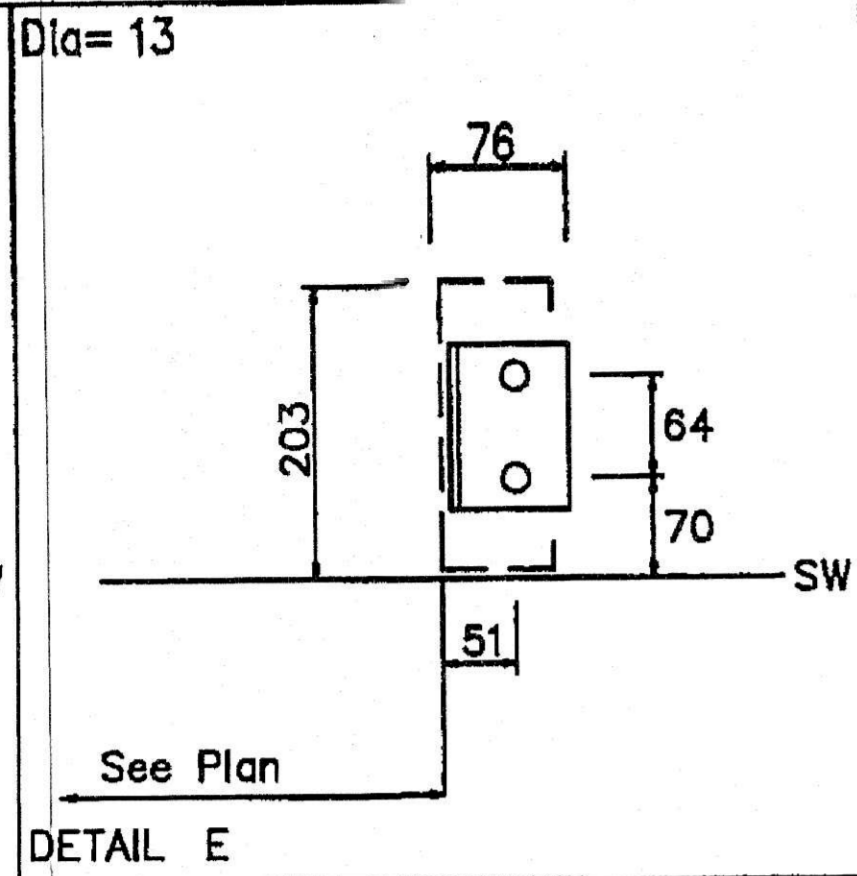
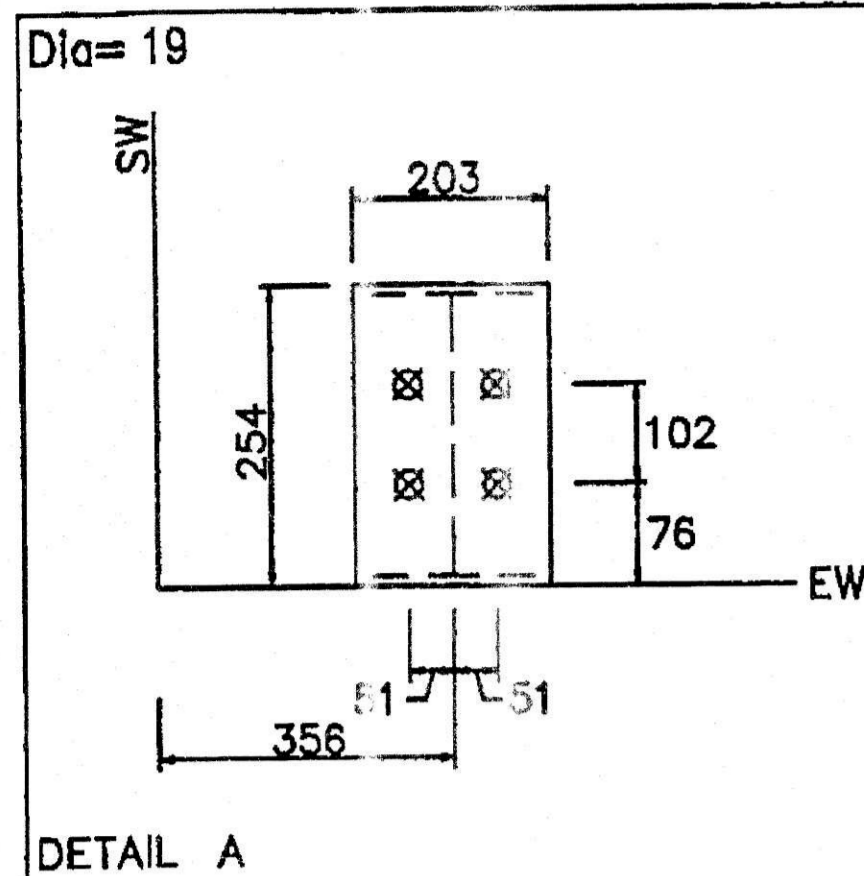


ANCHOR BOLT PLAN  
NOTE: All Base Plates @ 30480 (U.N.)

OLYMPIA STEEL BUILDINGS		Customer:	
City: McKEES ROCKS	State: PA	City:	
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 2/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	
ANCHOR BOLT PLAN			Sht. E1 of 11

GEORGE ROSE





**NOTES FOR REACTIONS**

Building reactions are based on the following building data:

Width (MM)	=	12192
Length (MM)	=	15240
Eave Height (MM)	=	9144 / 9144
Roof Slope	=	8.3:100 / 8.3:100
Dead Load (KPA)	=	0.10
Collateral Load (KPA)	=	0.05
Roof Live Load (KPA)	=	1.0
Roof Snow Load (KPA)	=	1.92
Wind Speed (KMH)	=	145
Wind Code	=	IBC 2006
Wind Exposure	=	C
Closed/Open	=	Closed
Importance - Wind	=	1.00
Importance - Seismic	=	1.00
Seismic Design Category	=	D
Seismic Coeff (Fa*Sa)	=	1.002

Load Combinations

DL+CL+(LL or SL)
DL+CL+W
DL+CL+0.75WL+0.75(LL or SL)
DL+0.75(0.75EIS)+0.75(LL or SL)
0.6DL+W
0.6DL+0.75EIS

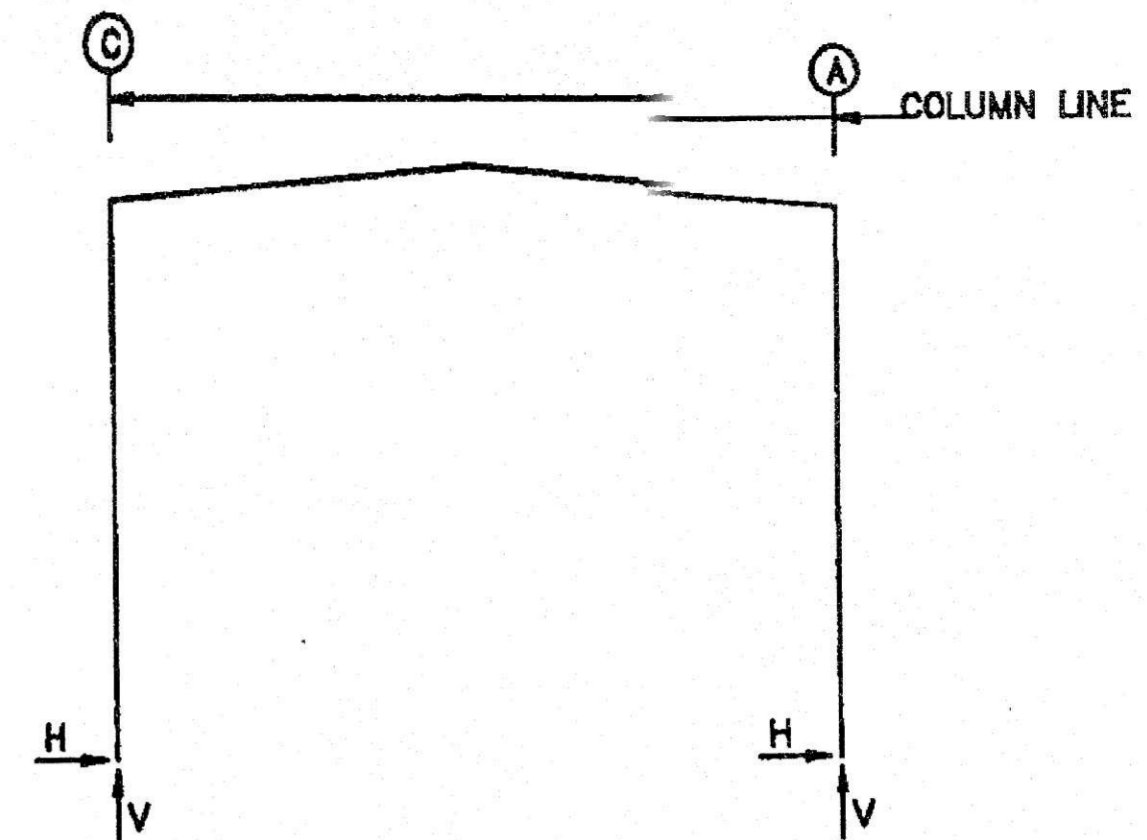
**GENERAL NOTES**

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
- THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
- THE SPECIFIED ANCHOR BOLT DIAMETER ASSUMES ASTM A307. ANCHOR BOLT MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER.
- ANCHOR BOLTS TO BE SUPPLIED BY OTHERS. ANCHOR BOLT EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION ENGINEER.
- ANCHOR BOLT PROJECTION ABOVE CONCRETE FINISHED SURFACE TO BE 76 mm UNLESS OTHERWISE NOTED BY FOUNDATION DESIGNER.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 3 mm IN ELEVATION AND LOCATION.
- THE ANCHOR BOLT LOCATIONS PROVIDED BY THE METAL BUILDING MANUFACTURER MAY NOT SATISFY ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF FOUNDATION DESIGN. IT IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER TO MAKE SURE THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED IN THE FOUNDATION DESIGN.
- MINOR FIELD WORK OF STRUCTURAL SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.
- THIS DRAWING IS NOT TO SCALE.

**BUILDING BRACING REACTIONS**

Wall Loc	Col Line	± Reactions (kN)			Panel Shear (N/m)
		Wind	Seismic	Wind	
L_EW	1 C,B	12.2	18.9	8.6	13.3
F_SW	A 2,3	19.6	22.0	15.8	17.7
R_EW	3 B,C	12.2	18.9	8.6	13.3
B_SW	C 3,2	19.6	22.0	15.8	17.7

FRAME LINES: 2



**RIGID FRAME: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate Width (mm)	Base Plate Length (mm)	Base Plate Thick (mm)	Grout (mm)
2	C	4	19	203	305	13	0
2	A	4	19	203	305	13	0

**RIGID FRAME: BASIC COLUMN REACTIONS (kN)**

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	C	1.1	11.1	0.4	2.8	7.1	55.6	14.2	111.2	-24.8	-57.0	23.7	-11.2
2	A	-1.1	11.1	-0.4	2.8	-7.1	55.6	-14.2	111.3	-23.7	-11.2	24.8	-57.0

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	C	-33.4	-39.7	15.0	6.1	13.7	-63.8	5.0	-48.5	-11.8	-17.8	11.8	17.8
2	A	-15.0	6.1	33.4	-39.7	-13.7	-63.8	-5.0	-48.5	-11.8	17.8	11.8	-17.8

Frame Line	Column Line	Seismic_Long		F1UNB_SL_L		F1UNB_SL_R	
		Horiz	Vert	Horiz	Vert	Horiz	Vert
2	C	0.0	-46.1	10.9	110.1	11.2	59.0
2	A	0.0	-46.1	-11.2	59.0	-10.9	110.1

**ENDWALL COLUMN: BASIC COLUMN REACTIONS (kN)**

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	C	3.1	0.4	8.9	17.7	12.2	-27.3	0.0	14.8	12.2	-27.3	0.0	14.8	0.0	14.8	0.0	14.8	0.0	14.8
1	B	6.3	1.4	27.2	54.4	0.0	-0.9	12.2	-39.7	0.0	-0.9	12.2	-39.7	0.0	-0.9	12.2	-39.7	0.0	-0.9
1	A	3.1	0.4	8.9	17.7	0.0	-5.1	0.0	-8.4	0.0	-5.1	0.0	-8.4	0.0	-5.1	0.0	-8.4	0.0	-5.1

Frm Line	Col Line	Wind Suct		Wind Long1		Wind Long2		Sels_Left		Sels_Right		E1UNB_SL_L		E1UNB_SL_R	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	C	11.0	-6.7	-3.9	22.3	-34.0	0.0	35.9	0.1	23.1	0.0	3.0	0.0	3.0	
1	B	21.3	-20.4	-12.0	0.0	34.5	22.3	-36.4	0.0	42.5	0.1	42.1	0.0	42.1	
1	A	11.0	-6.7	-3.9	0.0	-0.5	0.0	0.5	0.0	2.7	0.0	23.3	0.0	23.3	

Frm Line	Col Line	Dead		Collat		Live		Snow		Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
3	A	3.1	0.4	8.9	17.7	0.0	-8.4	0.0	-5.1	0.0	-8.4	0.0	-5.1	0.0	-8.4	0.0	-5.1	0.0	-8.4
3	B	6.3	1.4	27.2	54.4	12.2	-39.7	0.0	-0.9	12.2	-39.7	0.0	-0.9	12.2	-39.7	0.0	-0.9	12.2	-39.7
3	C	3.7	0.4	8.9	17.7	0.0	14.8	12.2	-27.3	0.0	14.8	12.2	-27.3	0.0	14.8	12.2	-27.3	0.0	14.8

Frm Line	Col Line	Wind Suct		Wind Long1		Wind Long2		Sels_Left		Sels_Right		E2UNB_SL_L		E2UNB_SL_R	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
3	A	11.0	-6.7	-3.9	0.0	0.5	0.0	-0.5	0.0	23.3	0.0	2.7	0.0	2.7	
3	B	21.3	-20.4	-12.0	22.3	-36.4	0.0	34.5	0.1	42.1	0.0	42.5	0.0	42.5	
3	C	11.0	-6.7	-3.9	0.0	35.9	22.3	-34.0	0.0	3.0	0.1	23.1	0.0	23.1	

**ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate Width (mm)	Base Plate Length (mm)	Base Plate Thick (mm)	Grout (mm)
1	C	4	19	203	254	13	0
1	B	4	19	203	254	13	0
1	A	4	19	203	254	13	0
3	A	4	19	203	254	13	0
3	B	4	19	203	254	13	0
3	C	4	19	203	254	13	0

**LOAD CASES**

- WIND\_L1 = WIND LOAD FROM LEFT CASE 1
- WIND\_R1 = WIND LOAD FROM RIGHT CASE 1
- WIND\_LN1 = LONGITUDINAL WIND CASE 1
- SEISMIC\_L = SEISMIC LOAD FROM LEFT
- SEISMIC\_R = SEISMIC LOAD FROM RIGHT
- SEISMICLN = LONGITUDINAL SEISMIC LOAD
- F1UNB\_SL\_L = FRAME 1 UNBALANCED SNOW LEFT SIDE
- F1UNB\_SL\_R = FRAME 1 UNBALANCED SNOW RIGHT SIDE
- F1CRANE 1 = FRAME 1 CRANE LOAD IN POSITION 1
- DRIFT = SNOW DRIFT LOAD
- SLIDE = SLIDE SNOW LOAD

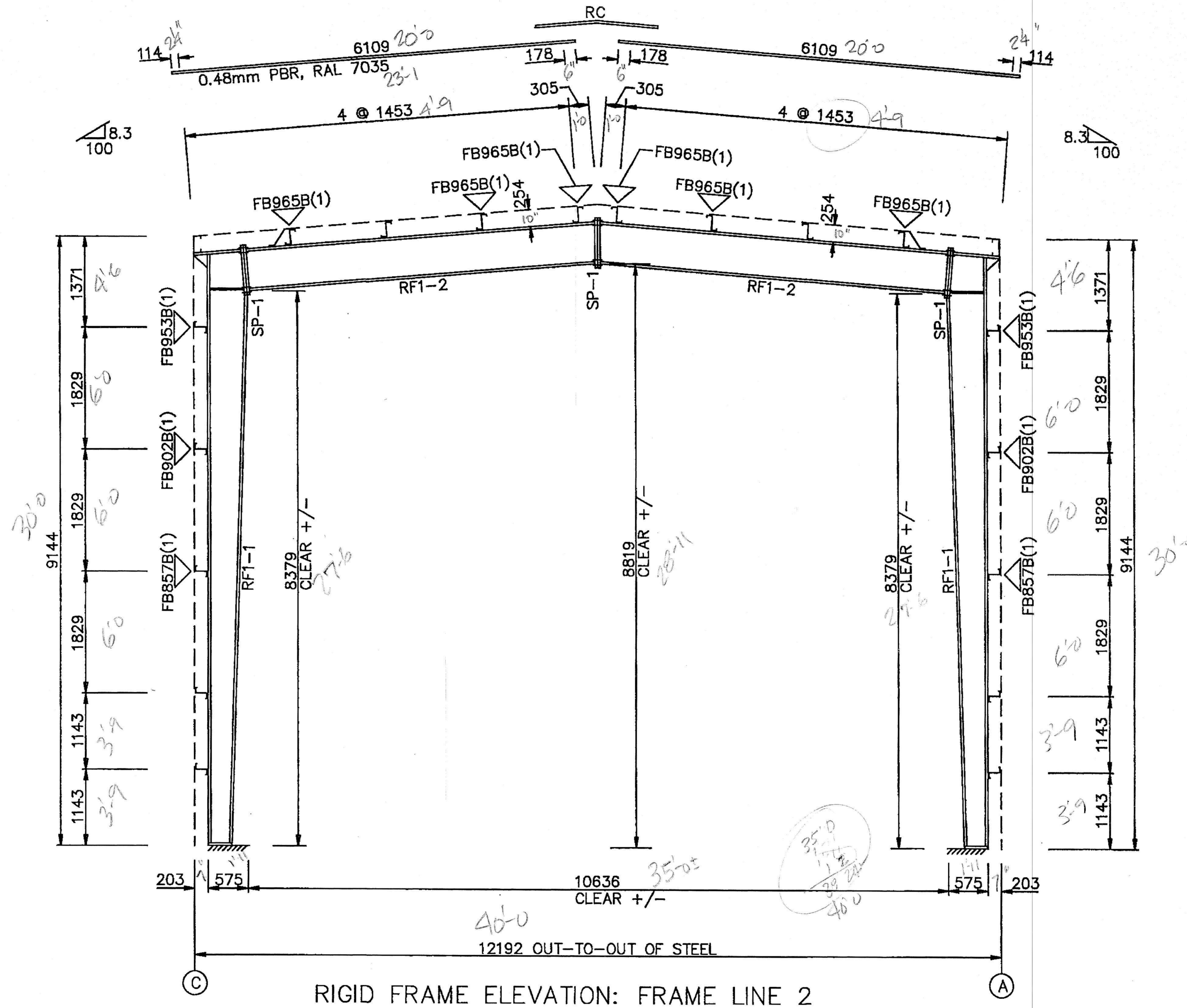
OLYMPIA STEEL BUILDINGS		Customer:	
City: McKEES ROCKS		State: PA	City:
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 2/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	
ANCHOR BOLT DETAILS & REACTIONS			Sht. E2 of 11



SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325T	19	64

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF1-1	279	559	4	8272	152.4 x 6.4 x 8885	152.4 x 9.5 x 8277		
	559	504	5.6	656	152.4 x 7.9 x 716			
RF1-2	559	559	4.8	5344	152.4 x 7.9 x 5344	152.4 x 7.9 x 5297		

▽ FLANGE BRACES: Both Sides (U.N.)  
 FBxxB(1): xx=length(mm)  
 B - L2X2X1/8



RIGID FRAME ELEVATION: FRAME LINE 2

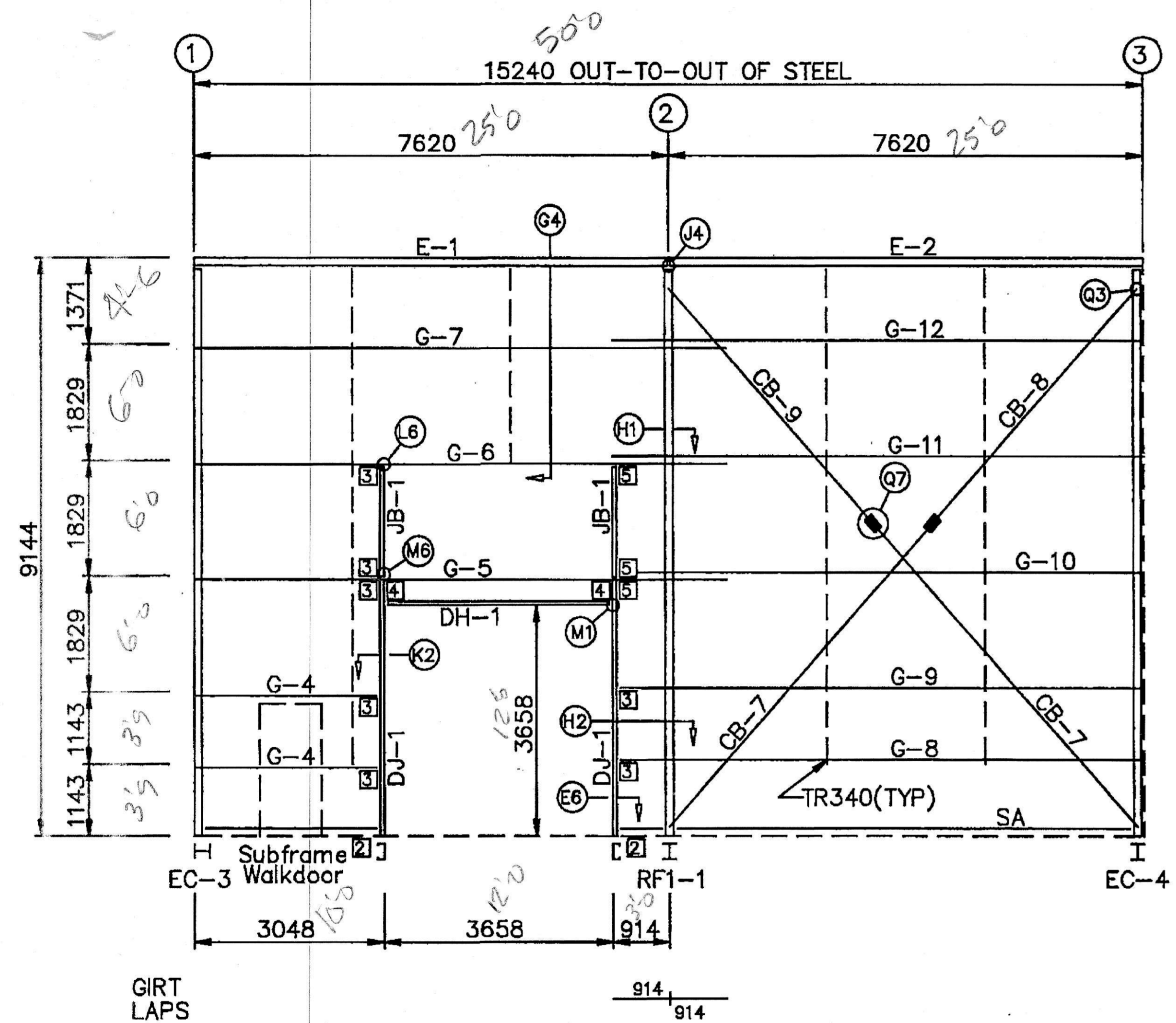
**GENERAL NOTES:**  
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

OLYMPIA STEEL BUILDINGS		Customer:	
City: McKEES ROCKS	State: PA	City:	
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 2/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	
RIGID FRAME ELEVATION			Sht. E3 of 11

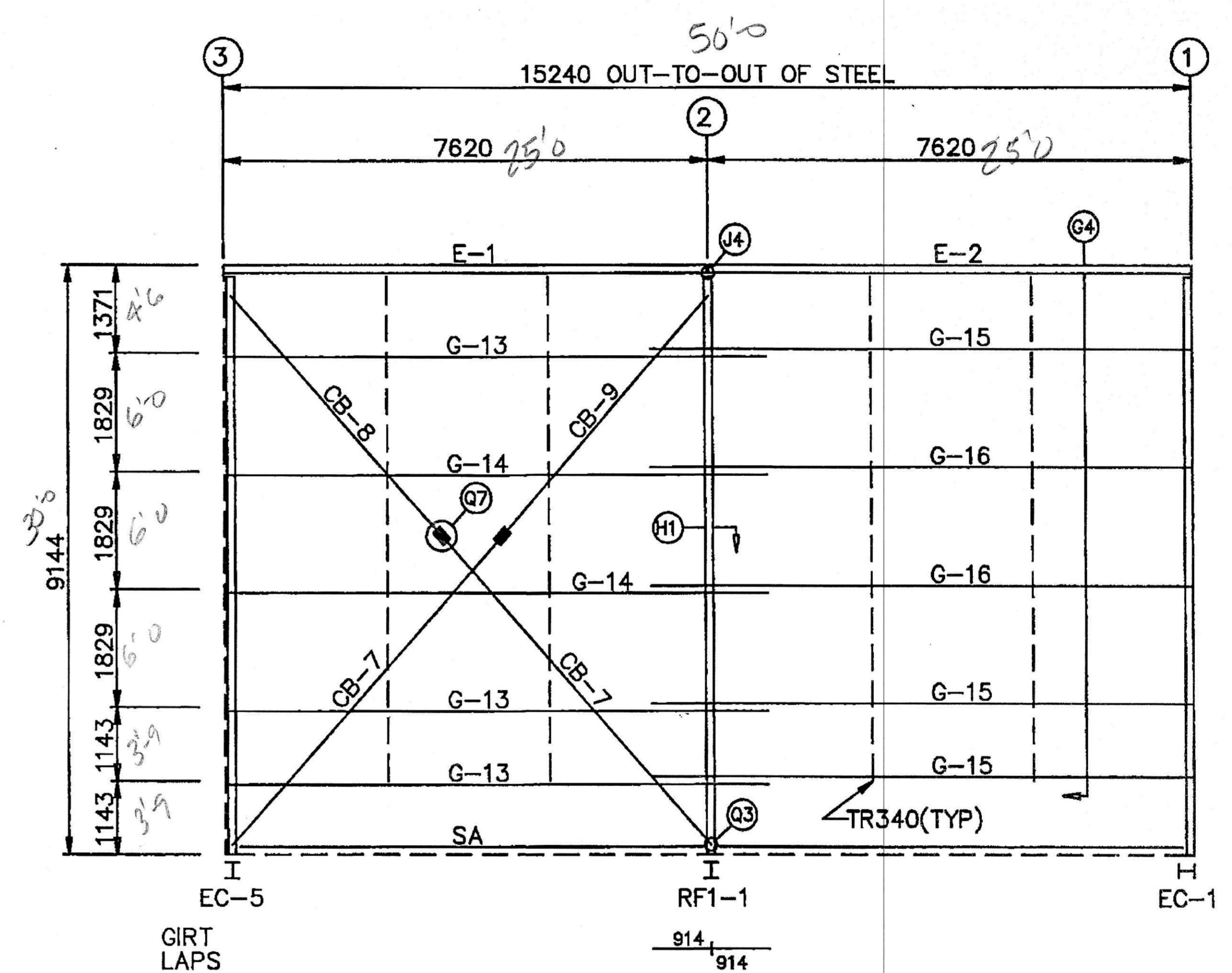


MEMBER TABLE FRAME LINE A & C		
MARK	PART	LENGTH
DJ-1	8x25C16	4001
DH-1	8x25C16	3645
E-1	1030E12	7601
E-2	1030E12	7601
G-4	8x25Z16	2921
G-5	8x25Z13	8522
G-6	8x25Z15	8522
G-7	8x25Z16	8522
G-8	8x25Z16	8407
G-9	8x25Z15	8407
G-10	8x25Z15	8522
G-11	8x25Z15	8522
G-12	8x25Z16	8522
G-13	8x25Z16	8522
G-14	8x25Z15	8522
G-15	8x25Z16	8522
G-16	8x25Z15	8522
JB-1	8x25C15	1600
CB-7	0.75_ROD	6096
CB-8	0.75_ROD	5334
CB-9	0.75_ROD	5512

CONNECTION PLATES FRAME LINE A & C	
ID	MARK/PART
2	CLO02
3	CLO20
4	CLO25
5	CLO93



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE C

**GENERAL NOTES:**  
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

OLYMPIA STEEL BUILDINGS		Customer	
City: McKEES ROCKS	State: PA	City:	
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 2/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	
SIDEWALL FRAMING			Sht. E4 of 11



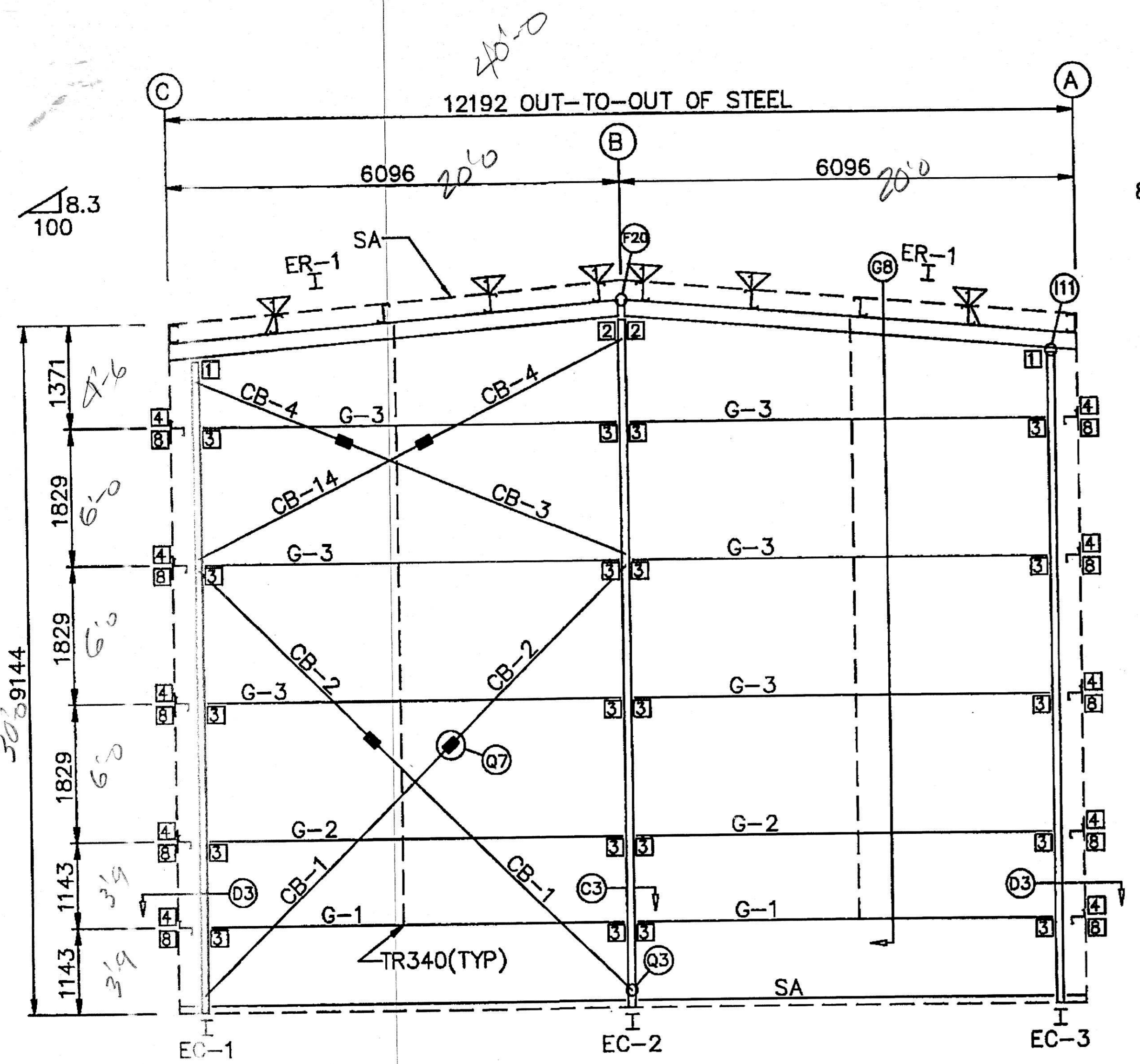
BOLT TABLE FRAME LINE 1 & 3			
LOCATION	QUAN	TYPE DIA	LENGTH
ER-1/ER-1	6	A325T16	57
ER-2/ER-2	6	A325T16	57
EC-2/Raf	8	A325T13	51
EC-1&EC-3/Raf	8	A325T16	57
EC-4&EC-5/Raf	4	A325T16	57

MEMBER TABLE FRAME LINE 1 & 3		
MARK	PART	LENGTH
EC-1	W10541	8638
EC-2	W10641	9117
EC-3	W10541	8638
EC-4	W10541	8672
EC-5	W10841	8672
ER-1	W10541	6117
ER-2	W10541	6117
G-1	8x25Z16	5512
G-2	8x25Z15	5512
G-3	8x25Z14	5512
CB-1	0.75_ROD	6096
CB-2	0.75_ROD	2159
CB-3	0.63_ROD	5740
CB-4	0.63_ROD	610
CB-5	0.75_ROD	2083
CB-6	0.63_ROD	5842
CB-13	0.63_ROD	5639
CB-14	0.63_ROD	5918

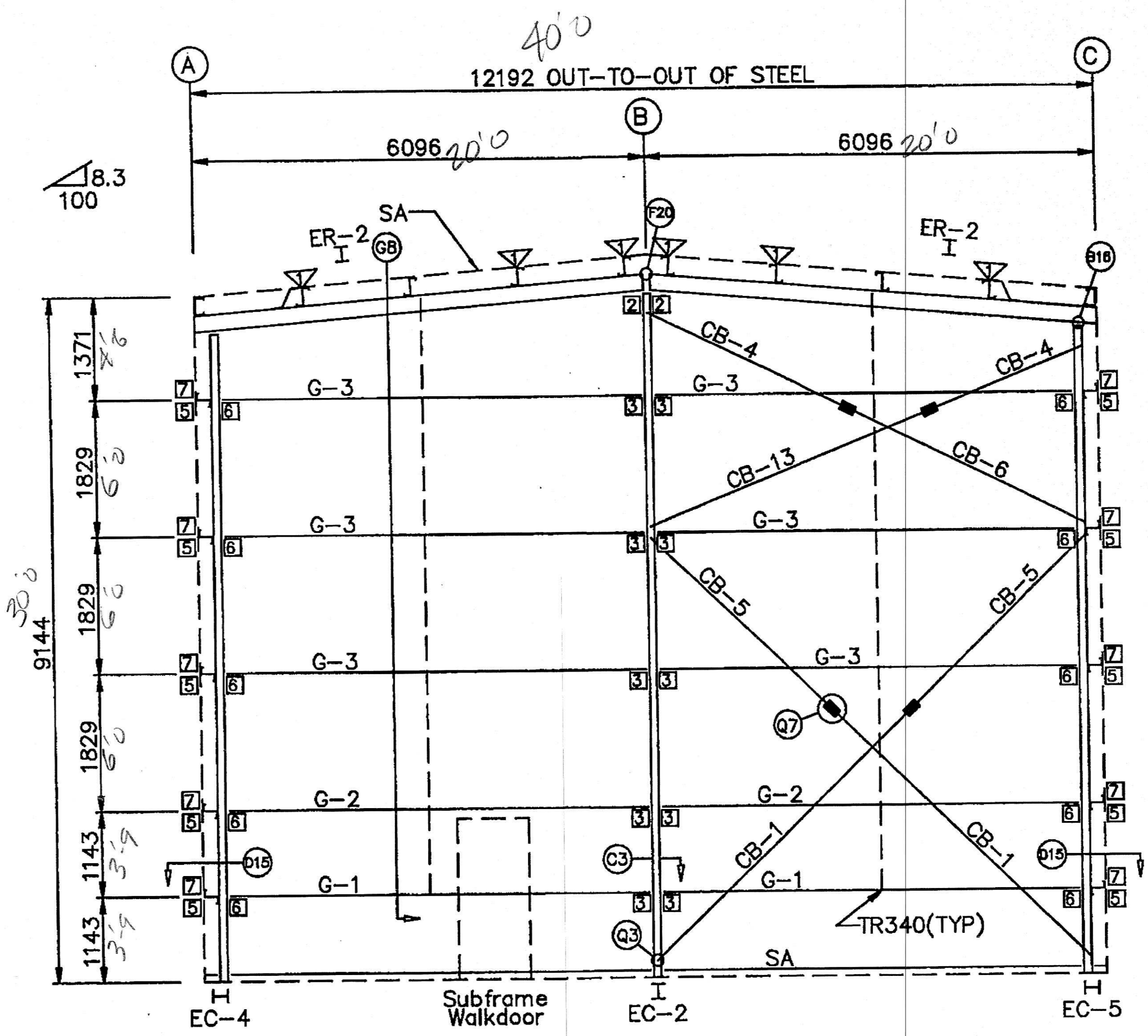
FLANGE BRACE TABLE FRAME LINE 1 & 3		
VID MARK	LENGTH	
1	FB756	756

CONNECTION PLATES FRAME LINE 1 & 3	
ID	MARK/PART
1	CL079
2	CL083
3	CL020
4	CL050
5	CL051
6	CL028
7	CL052
8	CL027

ROD NOTES:  
FIELD SLOT GIRTS FOR  
ROD TO PASS-THRU.



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 3

GENERAL NOTES:  
MINOR FIELD WORK OF STRUCTURAL, SECONDARY  
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO  
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED  
A NORMAL PART OF METAL BUILDING ERECTION.  
WE WILL NOT HONOR BACKCHARGES FOR MINOR  
FIELD WORK.

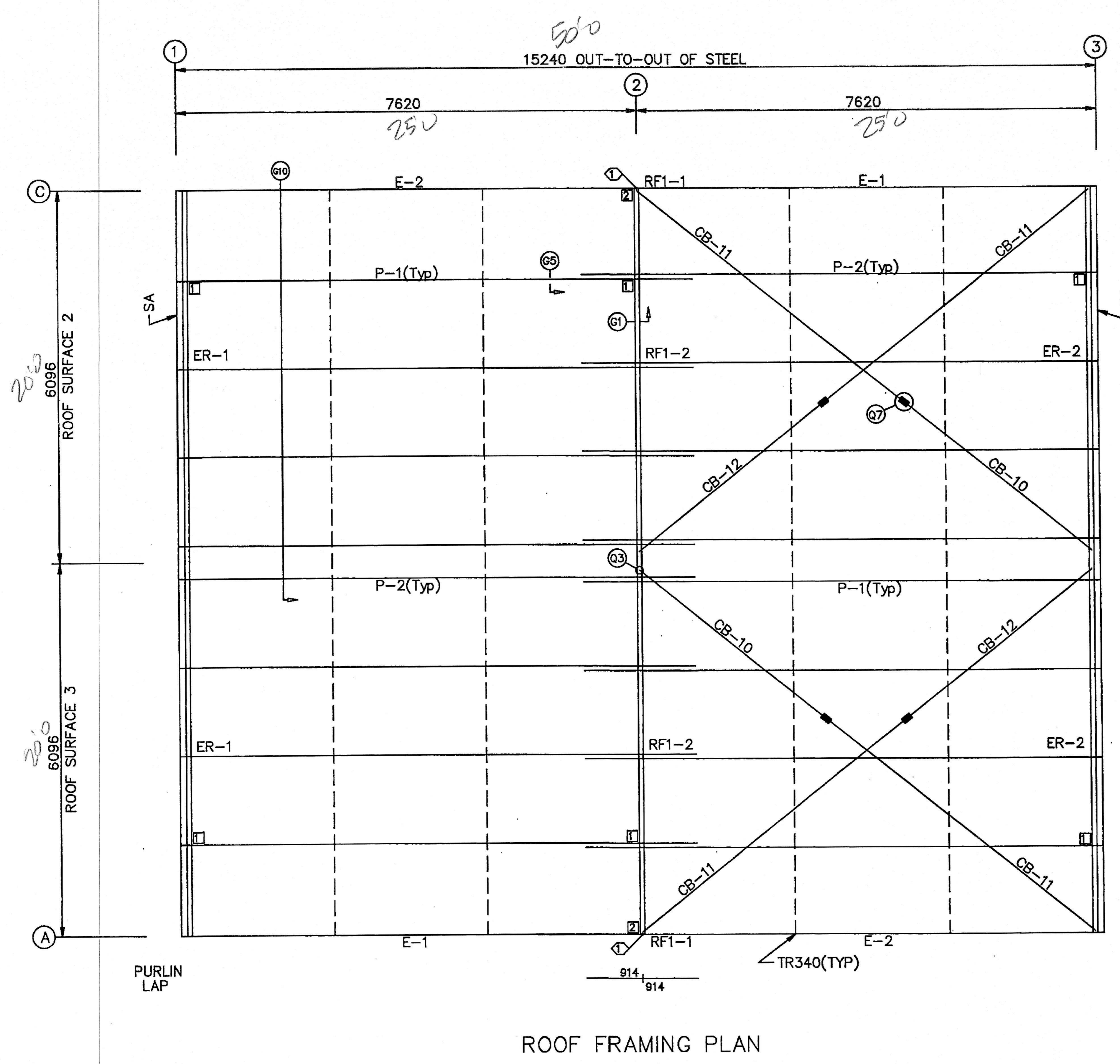
OLYMPIA STEEL BUILDINGS		Customer:	
City: McKEES ROCKS	State: PA	City:	
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 2/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	



SPECIAL BOLTS				
ROOF PLAN				
Ø ID	QUAN	TYPE	DIA	LENGTH WASH
1	6	A325T	13	51 0

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	10P25Z12	8522
P-2	10P25Z12	8522
E-1	10x30E12	7601
E-2	10x30E12	7601
CB-10	0.50_ROD	3429
CB-11	0.50_ROD	6096
CB-12	0.50_ROD	3556

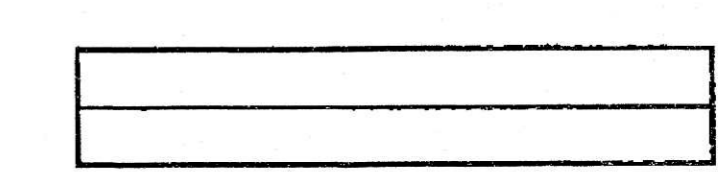
CONNECTION PLATES	
ROOF PLAN	
Ø ID	MARK/PART
1	CL156
2	CL154



ROOF FRAMING PLAN

6109 (17)

RC  
17



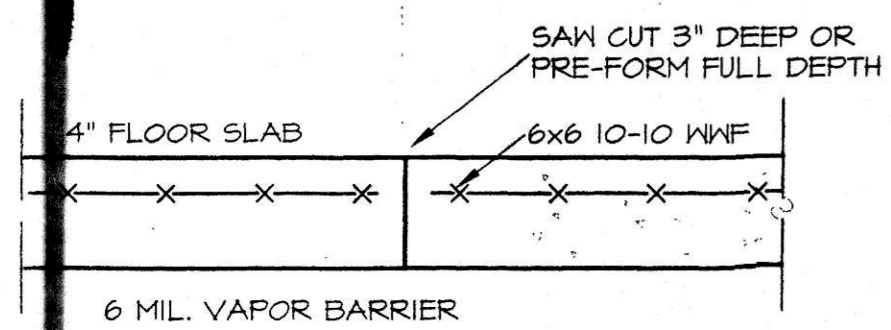
3048 mm LOW PROFILE RIDGE VENT  
(FOUR REQ'D) (FIELD LOCATE AT RIDGE)  
ERECTOR NOTE:  
(50) #14x22mm S.D. SCREWS,  
(8) OUTSIDE CLOSURES AND 1524mm  
OF MASTIC (RS100)  
PROVIDED FOR EACH VENT.

ROOF SHEETING  
PANELS: 0.48mm PBR  
COLOR: RAL 7035

**GENERAL NOTES:**  
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

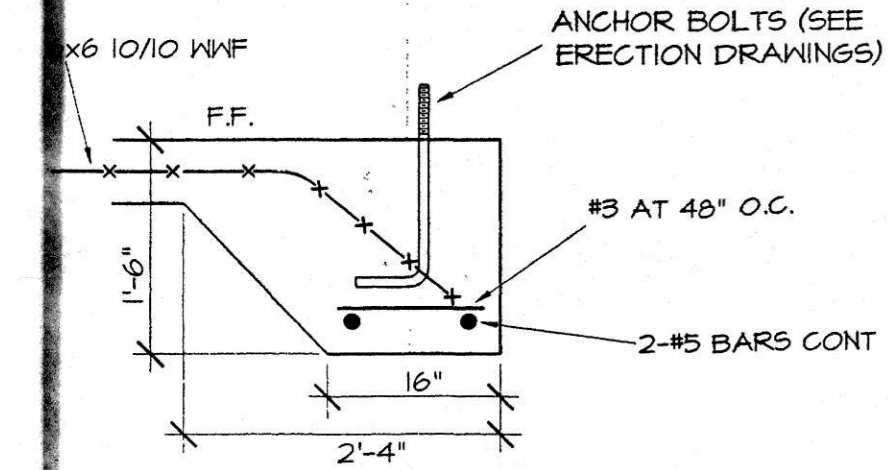
OLYMPIA STEEL BUILDINGS		Customer:	
City: McKEES ROCKS	State: PA	City:	
Designer: DAZ	Date: 1/23/14	Drafter: DZ	Date: 1/23/14
Detailer: HH	Date: 1/4/14	Office: PA	Job No.: 17611
Checker:	Date:	Office:	
ROOF FRAMING & SHEETING			Sht. E6 of 11





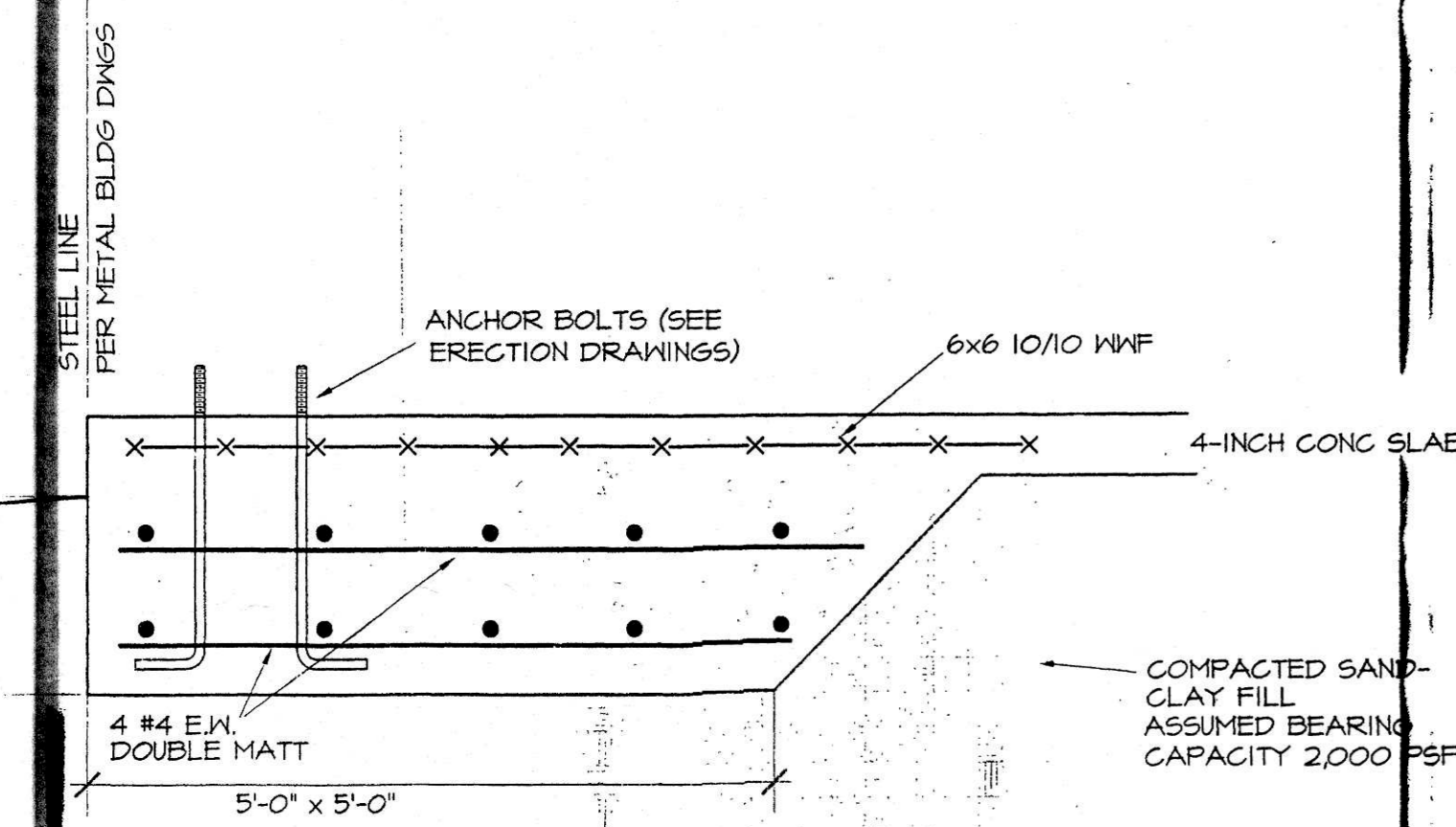
**CONSTRUCTION JOINT**

NO SCALE



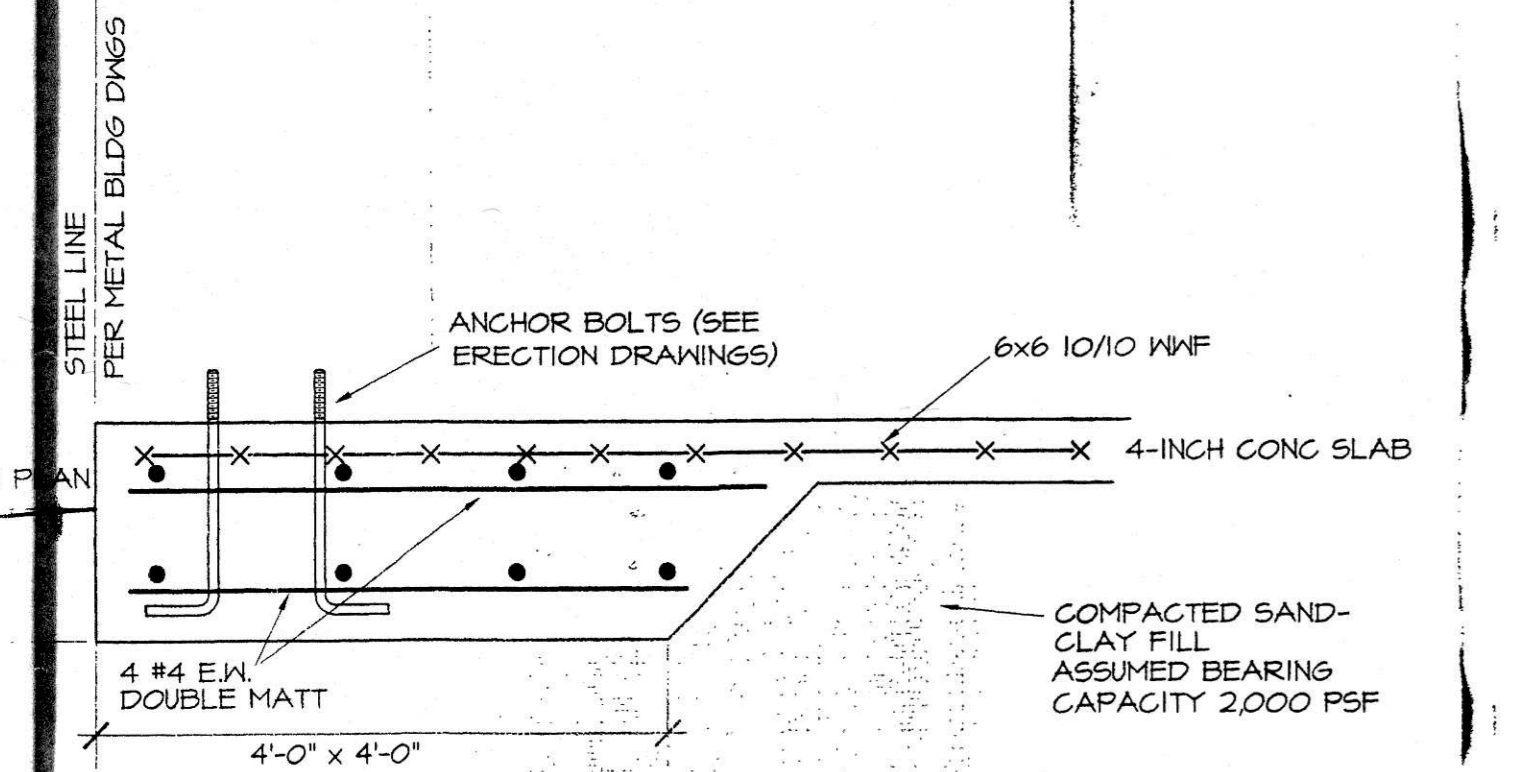
**PERIMETER FOOTING DETAIL**

3/4" = 1' - 0"



**FOOTING DETAIL B**

3/4" = 1' - 0"



**FOOTING DETAIL A**

3/4" = 1' - 0"

**GENERAL CONDITIONS**

THE GENERAL CONTRACTOR SHALL MAKE ADEQUATE SANITARY PROVISIONS. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR JOB SAFETY AND COMPLIANCE WITH THE REQUIREMENTS OF THE OCCUPATIONAL SAFETY AND HEALTH ACT AS IT MAY REGARD ANY PHASE OF THE WORK ON THIS PROJECT.

**CONCRETE WORK**

1. ALL CONCRETE FOR THE PROJECT SHALL BE "READY MIX" AND SHALL COMPLY WITH ASTM C-94.
2. ALL SECTIONS OF THE CONCRETE WORK SHALL COMPLY WITH ALL ASTM AND ACI REQUIREMENTS.
3. FORM WORK - ALL FORMS TO BE CAREFULLY BUILT AND SECURED IN PLACE IN SUCH A MANNER AS TO HAVE SUFFICIENT STRENGTH TO CARRY THE DEAD WEIGHT OF THE CONSTRUCTION AS A LIQUID, WITHOUT DEFLECTION OR VIBRATION. FORMS TO BE BUILT TIGHT, TRUE TO POSITION AND DIRECTION, THOROUGHLY BRACED, WIRED AND SPIKED OR OTHERWISE FASTENED TOGETHER.
4. CONCRETE - MINIMUM OF 3,000 P.S.I. COMPRESSIVE STRENGTH AT 28 DAYS, MINIMUM OF FIVE SACKS OF CEMENT PER CUBIC YARD OF CONCRETE, MAXIMUM OF 4" SLUMP.
5. FINISHING - IN ACCORDANCE WITH THE LATEST A.C.I. CODE. PLUMB, LEVEL, TRUE IN LINE, FREE OF HONEYCOMB. BUILDING SLAB SHALL HAVE A HARD STEEL TROWEL FINISH. WALKS SHALL HAVE BROOMED FINISH, AND EXPANSION JOINTS AT APPROXIMATELY 50' O.C. AND DUMMY JOINTS AS SHOWN ON THE SITE PLAN.
6. REMOVAL OF FORMS - FORMS SHALL BE CAREFULLY REMOVED SO AS NOT TO IMPAIR THE FACE OF THE CONCRETE. IMMEDIATELY AFTER THE FORMS ARE REMOVED ALL DAMAGE OF IMPERFECT WORK SHALL BE PATCHED IN A NEAT AND WORKMANLIKE MANNER, OR IF BADLY DAMAGED, IN THE OPINION OF THE OWNER, THE WORK SHALL BE REBUILT. THE MINIMUM TIME BEFORE ANY FORMS CAN BE REMOVED IS SEVEN (7) DAYS FOR SUCH MEMBERS AS ARE SUBJECT TO BENDING STRESSES, SUCH AS SLABS.
7. CURING - USE MEMBRANE CURING METHOD. USE MF6. RATE, SPRAY IMMEDIATELY FOLLOWING FINISHING. PROTECT FROM FREEZING WEATHER. CURE A TOTAL OF 28 DAYS USING A.C.I. METHODS.

**REINFORCING STEEL**

ALL REINFORCING STEEL SHALL BE DEFORMED STEEL BARS CONFORMING TO A.S.T.M. A615, GRADE 60. ALL REINFORCING STEEL SHALL BE MANUFACTURED, DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. 315R, 318R AND A.C.I. SP 66. WELDED WIRE FABRIC SHALL CONFORM TO A.S.T.M. A185, IN AS LONG A LENGTH AS IS PRACTICAL. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST ONE GRID WIDTH PLUS 2". REINFORCEMENT SHALL BE BENT COLD AND SHALL NOT BE WELDED.

SPLICES: REINFORCEMENT IN CONCRETE AND MASONRY SHALL HAVE LAP LENGTHS AS FOLLOWS, UNLESS OTHERWISE SPECIFIED ON DRAWINGS:

BAR SIZE:	IN CONCRETE:	IN MASONRY:
#3	1'-6"	2'-0"
#4	2'-0"	2'-6"
#5	2'-6"	3'-0"

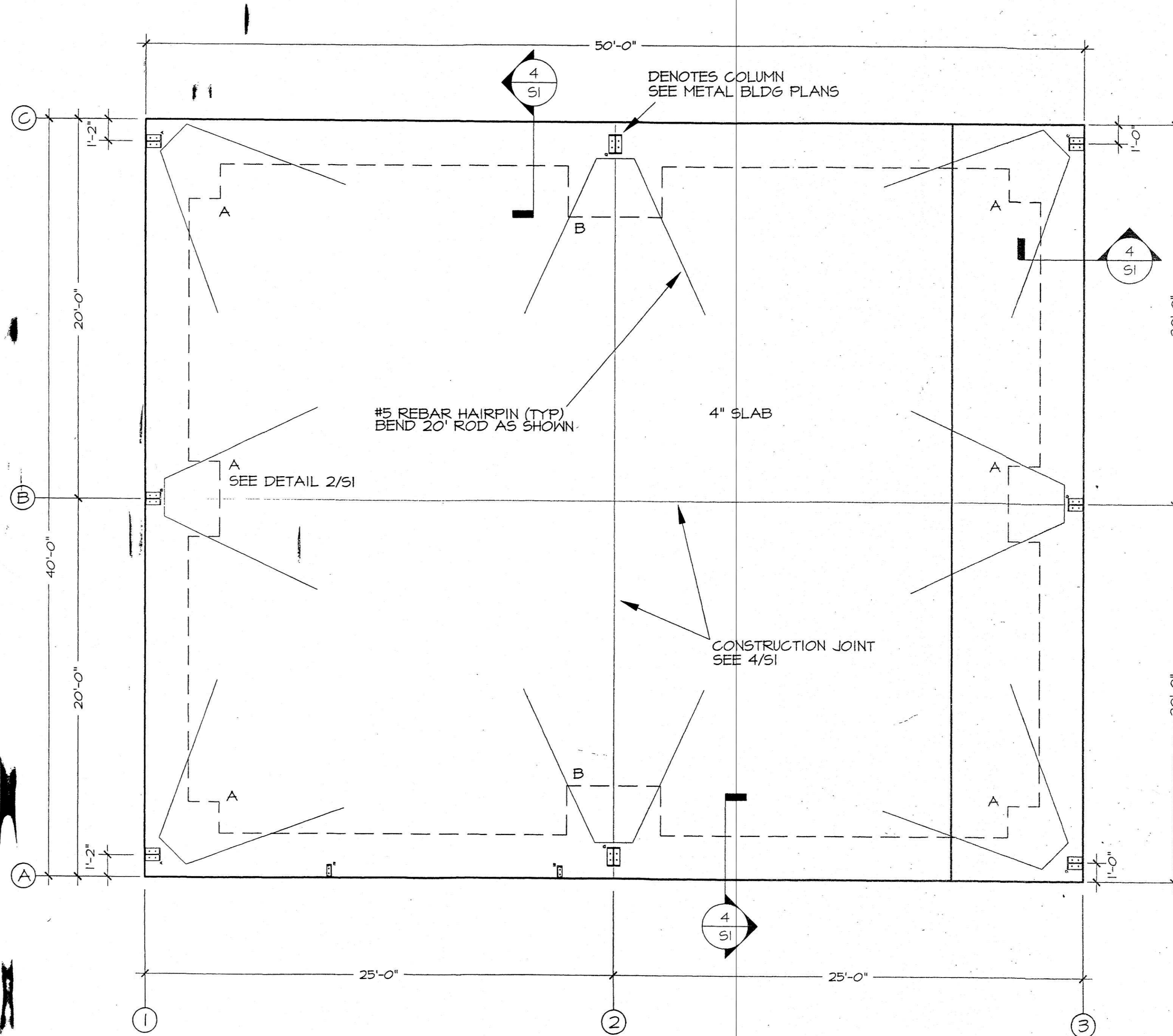
PLACEMENT: REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED BY CONCRETE, METAL, OR OTHER APPROVED CHAIRS, SPACERS OR TIES, AND SECURED AGAINST DISPLACEMENT DURING CONCRETE OR GROUT PLACEMENT.

EXCEPT WHERE OTHERWISE NOTED, REINFORCEMENT SHALL HAVE CONCRETE COVER AS FOLLOWS:

CONCRETE DEPOSITED AGAINST EARTH	3"
FORMED CONCRETE AGAINST EARTH	2"
EXTERIOR FACES OF WALLS	1"
TO TOP OF SLABS-ON-GRADE	3/4"

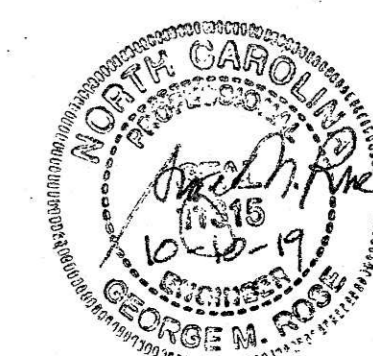
ALL SCALES, LOOSE RUST, GREASE OR DIRT SHALL BE REMOVED FROM THE REINFORCING BEFORE IT IS PLACED. PROVIDE #4 "HAIRPIN" AS SHOWN ON THE SLAB PLAN VIEW. ANCHOR BOLTS SHALL BE (A-307T) HIGH STRENGTH.

**SOIL TREATMENT**  
ADMINISTRATION AS ACCEPTABLE.



**SLAB PLAN**

1/4" = 1' - 0"



REVISIONS


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**METAL BUILDING FOR GEORGE YEOMAN**  
LILLINGTON, NC  
205 TOM MYERS ROAD  
SLAB PLAN AND DETAILS

DATE: OCT 2019  
DRAWN BY: GMR  
CHECKED: GMR  
SCALE: NOTED

SHEET NO.  
**S1**