

UNINHABITED UTILITY SHED 14' AND 16' WIDE x UP TO 24' LONG

PPTR, PPR, PTR, TR/TRD800, TR/TRD700

STRUCTURAL NOTES:

1. **BUILDING CODE:** 2018 NC BUILDING CODE
2015 IBC & IRC

2. **DESIGN LOADING:**

A. **ROOF LOADS**

- 1. UNIFORM ROOF (LIVE): 20 PSF
 - A. SNOW EXPOSURE FACTOR, C_e : 1.0
 - B. IMPORTANCE FACTOR, I : 1.0
 - C. THERMAL FACTOR, C_t : 1.2
 - D. LUMBER DURATION, C_D : 1.25

2. DEAD LOAD: 10 PSF

B. **WIND LOADS**

- 1. BASIC WIND SPEED (v_{ult}): 120 MPH
- 2. EXPOSURE: C
- 3. INTERNAL PRESSURE COEFFICIENT GC_{pi} : ± 0.18

C. **SEISMIC DESIGN**

- 1. IMPORTANCE FACTOR: 1.0
- 2. SPECTRAL RESPONSE ACCELERATIONS: $S_s = 0.42$
 $S_1 = 0.14$

3. SITE CLASS: D

4. SITE COEFFICIENTS: $S_{DS} = 0.44$
 $S_{D1} = 0.22$

5. SEISMIC DESIGN CATEGORY: C

LUMBER:

- 1. ALL LUMBER SHALL BE SPRUCE PINE FIR #2 GRADE (U.O.N.).
- 2. REFER TO THE TRUSS DESIGN FOR DESIGN INFORMATION.

HEADER NAILING:

HEADER TO STUD - 4-16d END NAIL
DOUBLED HEADER
- 16d @ 16" STAGGERED FACE NAIL

NAILING:

REFER TO SHEET 1 FOR WALL AND
ROOF SHEATHING NAILING.

MAX WALL HEIGHT FOR EACH SHED:

PPTR, PTR, TR/TRD800 - 7'-8 $\frac{1}{4}$ " (92 $\frac{1}{4}$ ")
PPR - 6'-8 $\frac{1}{4}$ " (80 $\frac{1}{4}$ ")

MAX ROOF SLOPE FOR EACH SHED:

PPTR, PPR - 5:12
PTR, TR800 - 4:12

SIDE WALL EDGE NAILING REQUIREMENTS

MARK WALLS BEING USED	END WALL WIDTH	SIDE WALL LENGTH	NAILING (NOTE 4)	MAX. COMB. OPENING (NOTE 3)	MIN TOTAL COMBINED SHEAR WALL MIN 2'-4" WALL SEGMENT
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NO OPENINGS ALONG THE WALL

14'	14'-24'	8d NAILS @ 6" O.C.	N/A	14'
16'	16'-24'	8d NAILS @ 6" O.C.	N/A	14'

MIN 2'-6" RTN WALLS ON EACH END OF WALL OPENING

14'	14'-24'	8d NAILS @ 6" O.C.	UP TO 12'	7'
14'	14'-24'	8d NAILS @ 4" O.C.	UP TO 12'	5'
16'	16'-24'	8d NAILS @ 6" O.C.	UP TO 12'	8'
16'	16'-24'	8d NAILS @ 4" O.C.	UP TO 12'	5'

TABLE NOTES:

- 1. NAILING IS FOR $\frac{3}{8}$ " SMARTSIDE PANEL OR $\frac{3}{8}$ " SMARTSIDE WITH FOIL BACKER.
- 2. NO SINGLE OPENING GREATER THAN 8'-0"
- 3. USE COMMON OR GALVANIZED BOX NAILS.
- 4. FIELD NAILING FOR $\frac{3}{8}$ " SMARTSIDE: 8d @ 12" O.C.

ROOF SHEATHING (7/16" OSB)

WIDTH	LENGTH	FIELD NAILING	EDGE NAILING
14'	14'-24'	8d NAILS @ 12" O.C.	8d NAILS @ 4" O.C.
16'	16'-24'	8d NAILS @ 12" O.C.	8d NAILS @ 4" O.C.

NOTES:

USE 0.113" X 2 $\frac{3}{8}$ " GALVANIZED RING SHANK NAILS OR 8d COMMON.

3/8 SMART SIDE NAILING REQUIREMENTS

END WALL EDGE NAILING REQUIREMENTS

MARK WALLS BEING USED	END WALL WIDTH	SIDE WALL LENGTH	NAILING (NOTE 4)	MAX. COMB. OPENING (NOTE 3)	MIN TOTAL COMBINED SHEAR WALL MIN 2'-4" WALL SEGMENT
-----------------------	----------------	------------------	------------------	-----------------------------	--

NO OPENINGS ALONG THE WALL

14'	14'-24'	8d NAILS @ 6" O.C.	0'	14'
16'	16'-24'	8d NAILS @ 6" O.C.	0'	16'

MIN 2'-6" RTN WALLS ON EACH END OF WALL OPENING

14'	14'-22'	8d NAILS @ 6" O.C.	4'	10'
14'	24'	8d NAILS @ 4" O.C.	4'	10'
14'	14'-18'	8d NAILS @ 6" O.C.	6'	8'
14'	20'-24'	8d NAILS @ 4" O.C.	6'	8'
14'	14'-20'	8d NAILS @ 4" O.C.	8'	6'
14'	22'-24'	8d NAILS @ 3" O.C.	8'	6'
16'	16'-24'	8d NAILS @ 6" O.C.	4'	12'
16'	16'-20'	8d NAILS @ 6" O.C.	6'	10'
16'	22'-24'	8d NAILS @ 4" O.C.	6'	10'
16'	16'	8d NAILS @ 6" O.C.	8'	8'
16'	18'-22'	8d NAILS @ 4" O.C.	8'	8'

TUFF SHED
Storage Buildings & Garages



TUFF SHED, MFG. FACILITIES

Order #: _____
Customer: _____
Site Address: _____
Building Size: WIDTH - LENGTH - HEIGHT - SQ. FT. AREA

P.O. # _____
Drawn By: TB
Date: 1/19/23
Checked By: _____
Date: _____
Scale: N.T.S.

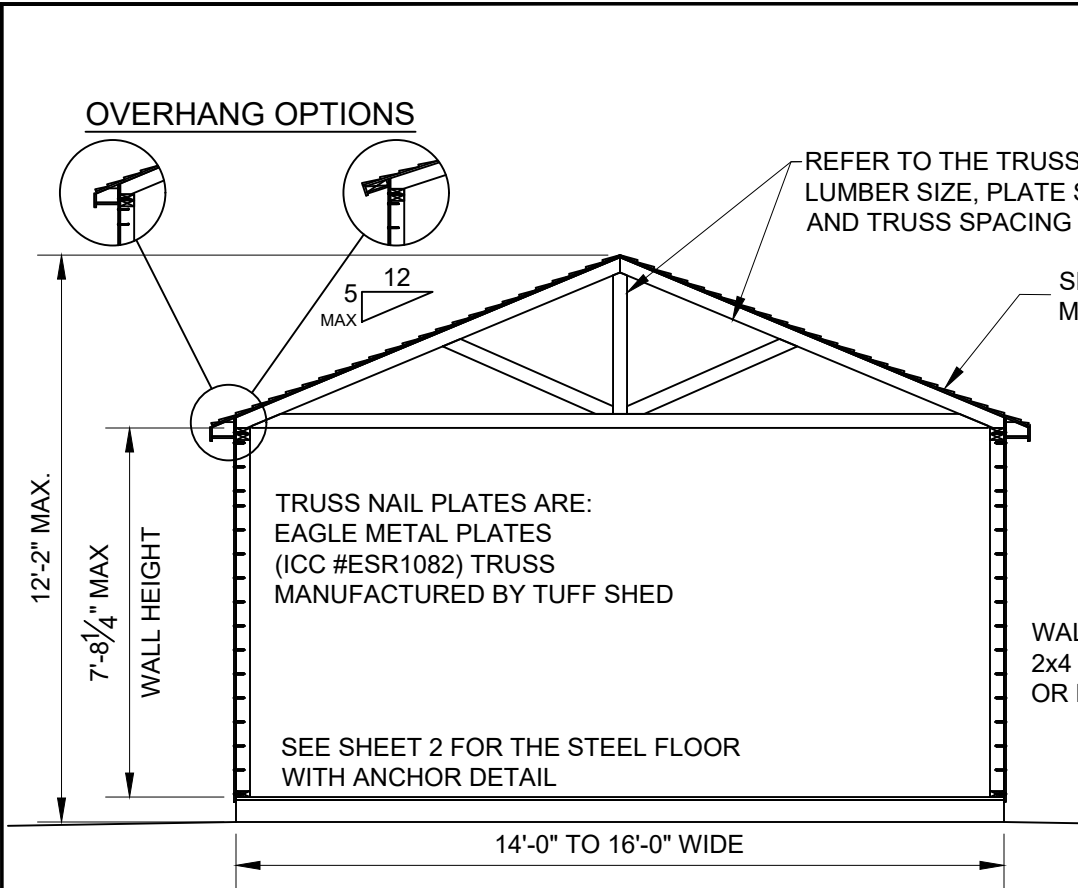
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ENGINEERING DEPARTMENT

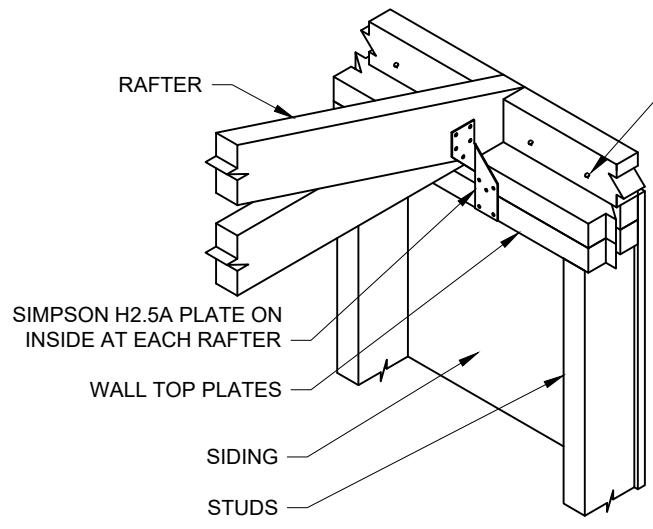
RICHARD J. WILLS, P.E.
RWILLS@TUFFSHED.COM
1777 S. HARRISON STREET
DENVER, COLORADO 80210
(303) 753-8833 EXT. 96315

TITLE
GENERAL NOTES
NAILING REQUIREMENTS
120 MPH, EXP. C

DRAWING NO.
NC-PPTR-TR800-02
REV. LEVEL 01
SHEET **1**
PAGE 1 OF 4

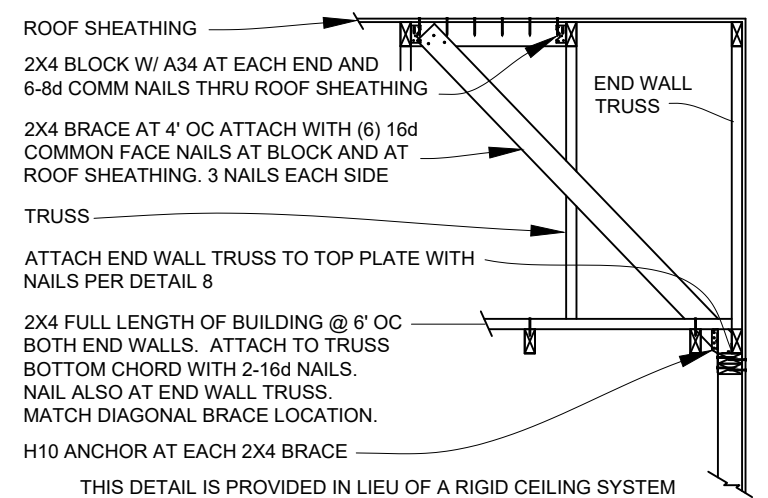


1 BUILDING SECTION
SCALE: N.T.S.

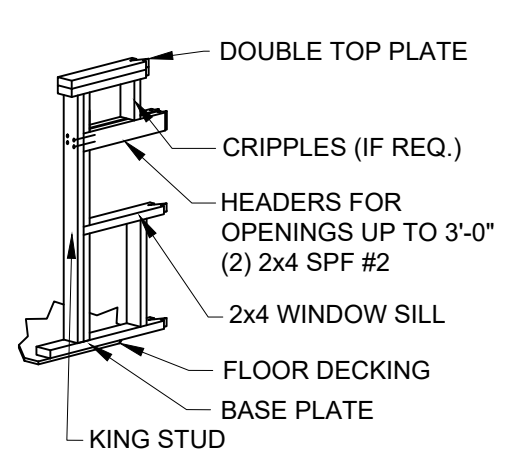


SHED RAFTER H2.5A ATTACHMENT

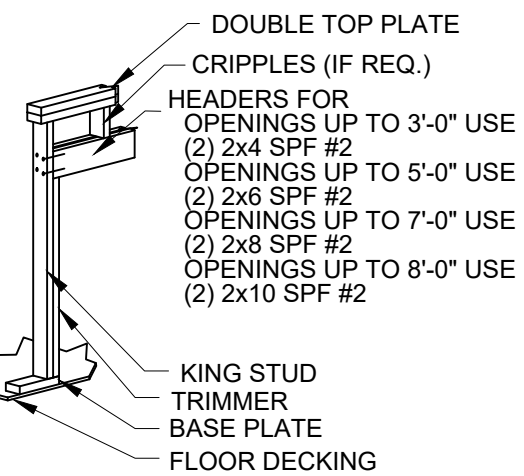
4 TRUSS TO WALL CONNECTION DETAIL
SCALE: N.T.S.



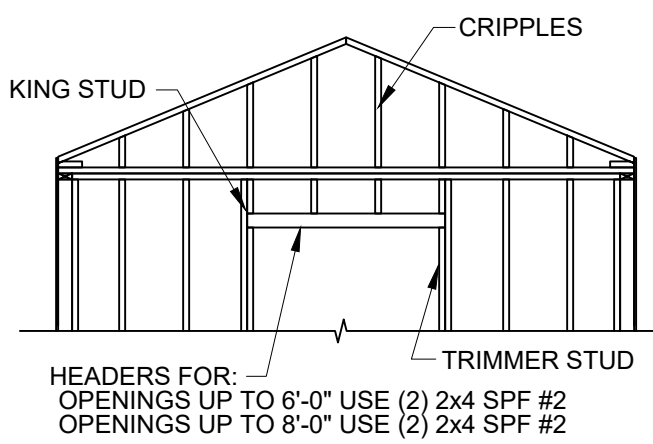
5 END WALL BRACE AND TRUSS BOTTOM CHORD BRACE
SCALE: N.T.S.



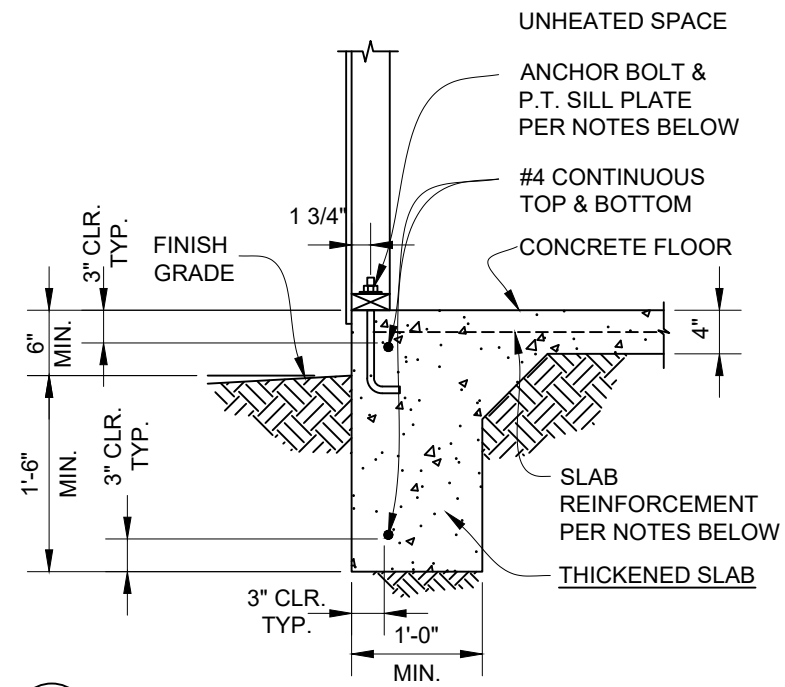
2A WINDOW HEADER DETAIL FOR SIDE WALLS
SCALE: N.T.S.



2B DOOR HEADER DETAIL FOR SIDE WALLS
SCALE: N.T.S.



3 HEADER DETAIL FOR END WALLS
SCALE: N.T.S.



6 OPTIONAL CONCRETE FOUNDATION DETAIL
SCALE: N.T.S.

- CONTINUOUS FOOTING NOTES**
- TOP OF CONCRETE TO BE 6" MIN. ABOVE GRADE. SLAB REINFORCEMENT SHALL BE WWF 6X6 W10xW10 PER ASTM A185. LOCATE AT MID-DEPTH OF SLAB.
 - ALL FOOTING FORMS SHALL BE INSPECTED FOR SIZE AND REINFORCING BEFORE POURING CONCRETE.
 - FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL, COMPETENT SOIL, OR PROPERLY COMPACTED STRUCTURAL FILL. ALLOWABLE SOIL BEARING PRESSURE IS 1500 PSF AT 12" BELOW GRADE.
 - CONCRETE: MINIMUM 28 DAY COMPRESSIVE STRENGTH, $f_c = 2500$ PSI.
 - REINFORCING STEEL: A615, GRADE 40 OR GRADE 60. ALL REINFORCING STEEL SHOWN TO BE CONTINUOUS MAY BE LAPPED A MINIMUM OF 38 BAR DIAMETERS OR 24".
 - SEISMIC DESIGN CATEGORY: C
 - ATTACH PRESSURE TREATED SILL PLATE TO THE FOOTING USING 1/2" DIA X 10" LONG 'L' BOLTS WITH NUTS AND WASHERS.
 - ANCHOR BOLTS SHALL BE EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SHALL BE SPACED NOT MORE THAN 6" OC.
 - THERE SHALL BE A MINIMUM OF 2 BOLTS PER SILL PLATE PIECE WITH 1 BOLT LOCATED NOT MORE THAN 12" NOR LESS THAN 7 BOLT DIAMETERS FROM EACH END OF EACH PIECE.
- NOTE: FOR BUILDINGS 20' AND LONGER OR ANY BUILDING DESIGNED AS A 3-SIDED DIAPHRAGM, ADD SIMPSON SSTB16 ANCHORS AND HDU2 HOLD DOWNS AT EACH CORNER OF THE END WALLS. FOLLOW MANUFACTURER'S INSTALLATION INSTRUCTIONS.



Order #. _____
Customer: _____
Site Address: _____
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Checked By: _____
Date: _____
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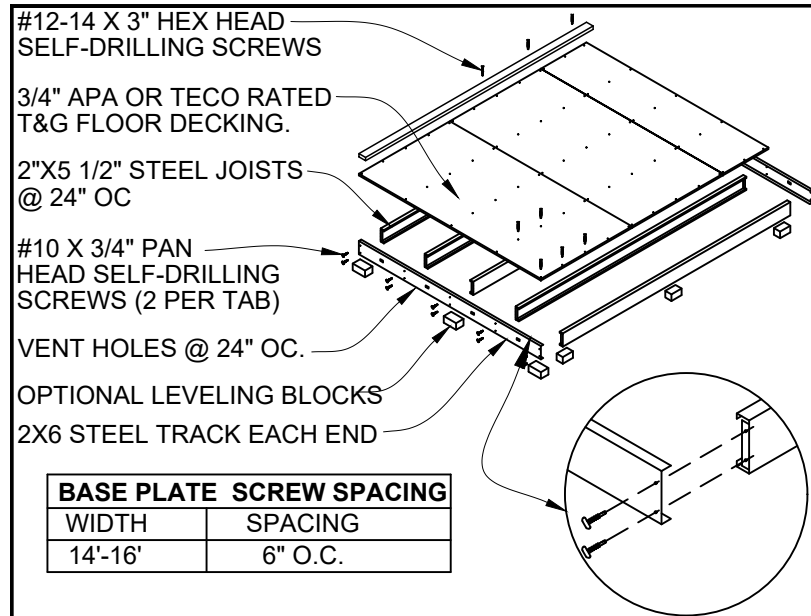
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TUFF SHED, INC.
ENGINEERING DEPARTMENT

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1777 S. HARRISON STREET
DENVER, COLORADO 80210
(303) 753-8833 EXT. 96315

TITLE
GENERAL NOTES
& TABLES
120 MPH, EXP. C

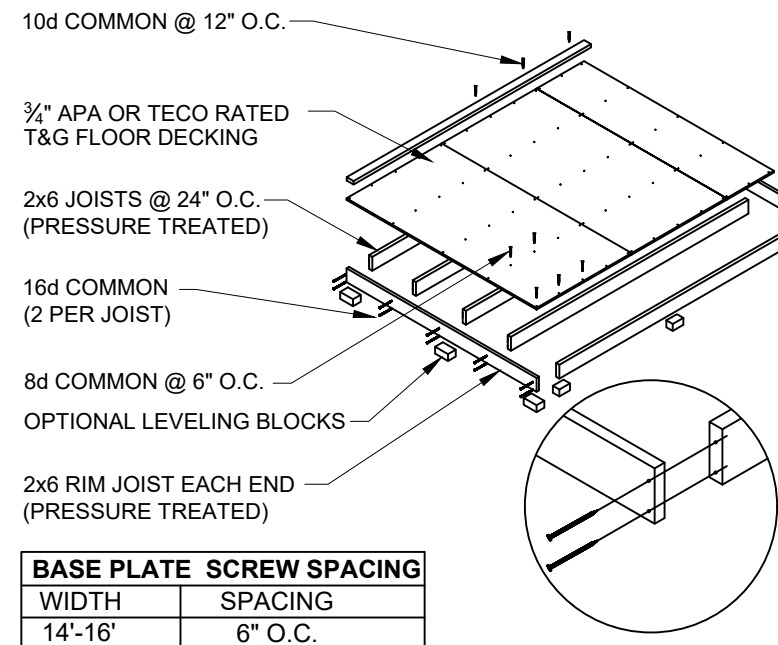
DRAWING NO. NC-PPTR-TR800-02
REV. LEVEL 01
SHEET 2
PAGE 2 OF 4



BASE PLATE SCREW SPACING	
WIDTH	SPACING
14'-16'	6" O.C.

1 STEEL SHED BASE DETAIL
SCALE: N.T.S.

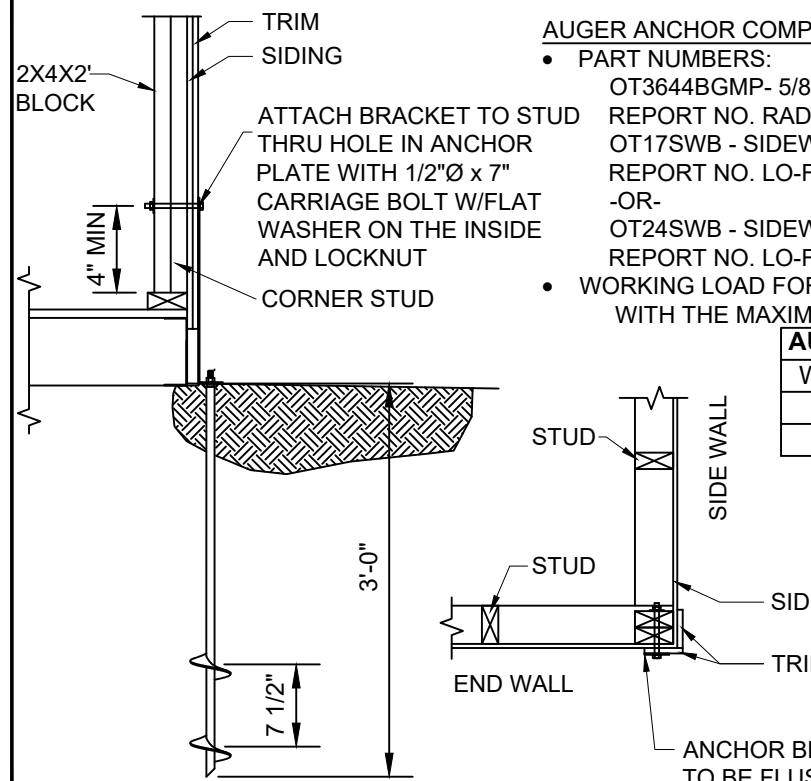
- STEEL SHED FOUNDATION:**
550T125-054 - 16 GAUGE STEEL TRACKS G140 ZINC COATED
550S137-054 - 16 GAUGE STEEL JOISTS G140 ZINC COATED @ 24" O.C.
(SUPPLIER: ALLIED STUDCO (JOIST: 550S137-054 / TRACK: 550T125-054) ICC ER-4943P.
- 3/4" APA OR TECO RATED TONGUE AND GROOVE FLOOR DECKING. 24" MAX PANEL SPAN. STAGGER PANEL LAYOUT.
- FASTEN FLOOR DECKING TO JOIST & TRACKS USING #8 x 1-5/8" ZINC PLATED SCREWS @ 12" O.C. NO BLOCKING REQUIRED. ALL EDGES SHALL LIE ON FLOOR JOISTS. STAGGER PANEL LAYOUT PER APA CONDITION 1.
- FASTEN SOLE PLATE THROUGH FLOOR DECKING INTO JOISTS OR TRACKS WITH #12-14 X 3" GALVANIZED SELF-DRILLING SCREWS @ 12" O.C.
- ALLOWABLE FLOOR LIVE LOAD: 75 PSF FOR STEEL JOISTS CONTINUOUSLY SUPPORTED. 50 PSF FOR JOISTS ON BLOCKS AS SHOWN.
- USE OPTIONAL CONCRETE BLOCKS AS REQUIRED TO LEVEL BUILDING:
SUGGESTED SIZES: 2X8 x 16", 4" x 8" x 16", OR 8" x 8" x 16".
BLOCKS UNDER JOISTS SPACED @ 8'-0" O.C. MAXIMUM.
BLOCKS UNDER TRACK SPACED @ 4'-0" O.C. MAXIMUM.



BASE PLATE SCREW SPACING	
WIDTH	SPACING
14'-16'	6" O.C.

2 OPTIONAL WOOD SHED BASE DETAIL
SCALE: N.T.S.

- WOOD SHED FOUNDATION:**
2x6 #2 PRESSURE TREATED HEM FIR RIM JOISTS
2x6 #2 PRESSURE TREATED HEM FIR JOISTS @ 24" O.C.
- 3/4" APA OR TECO RATED TONGUE AND GROOVE FLOOR DECKING. 24" MAX PANEL SPAN. NO BLOCKING REQUIRED. ALL EDGES SHALL LIE ON FLOOR JOISTS. STAGGER PANEL LAYOUT PER APA CONDITION 1.
NAIL PLYWOOD TO JOISTS AND RIM JOISTS:
BORDER: 8d COMMON SPACED @ 6" O.C.
EDGE: 8d COMMON SPACED @ 6" O.C.
FIELD: 8d COMMON SPACED @ 12" O.C.
- FASTEN SOLE PLATE THROUGH FLOOR DECKING INTO JOISTS OR RIM JOISTS WITH 10d COMMON SPACED @ 12" O.C.
- ALLOWABLE FLOOR LIVE LOAD: 40 PSF
- USE OPTIONAL CONCRETE BLOCKS AS REQUIRED TO LEVEL BUILDING:
SUGGESTED SIZES: 2" x 8" x 16", 4" x 8" x 16", OR 8" x 8" x 16".
BLOCKS UNDER JOISTS SPACED @ 8'-0" O.C. MAXIMUM.
BLOCKS UNDER RIM JOISTS SPACED @ 4'-0" O.C. MAXIMUM.



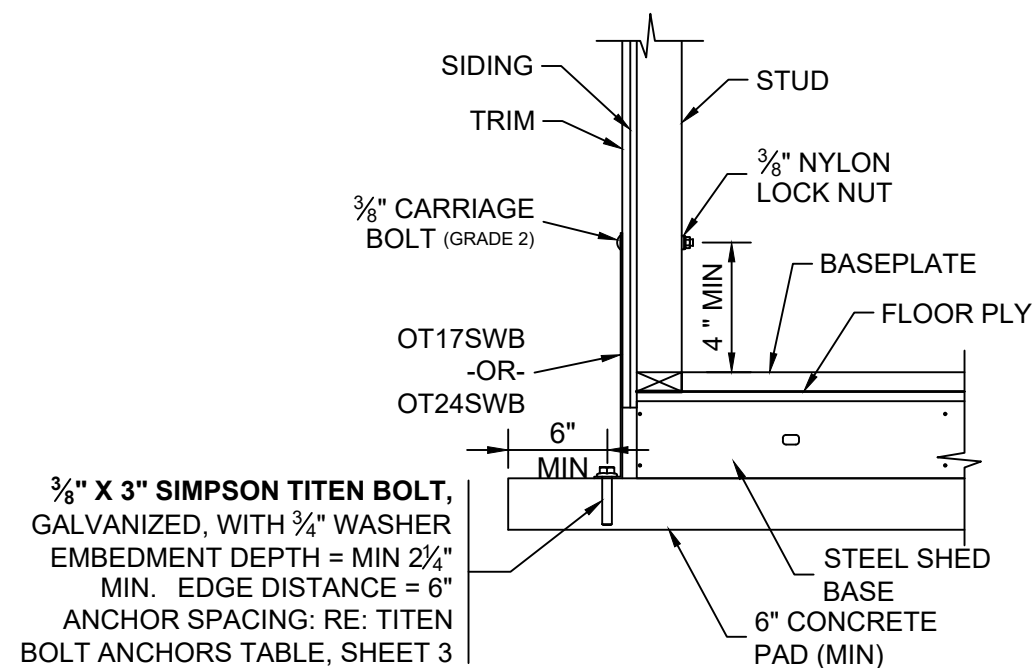
AUGER ANCHOR COMPONENTS BY OLIVER TECHNOLOGIES

- PART NUMBERS:**
OT3644BGMP- 5/8" X 36" (36" IMBED) GALVANIZED AUGER REPORT NO. RAD-3060
OT17SWB - SIDEWALL BRACKET FOR USE WITH THRU BOLTS REPORT NO. LO-FJ90129-A
-OR-
OT24SWB - SIDEWALL BRACKET FOR USE WITH THRU BOLTS REPORT NO. LO-FJ90129-B
- WORKING LOAD FOR ANCHOR SYSTEM IS 3,150 LBS WITH THE MAXIMUM LOAD OF 4,725 LBS

AUGER ANCHORS		
WIDTH	LENGTH	# OF ANCHORS
14'	14'-24'	6 ANCHORS
16'	16'-20'	6 ANCHORS

6 ANCHORS - PROVIDE (1) AT EA. CORNER OF THE BUILDING AND (1) AT MID-LENGTH OF EACH SIDEWALL.

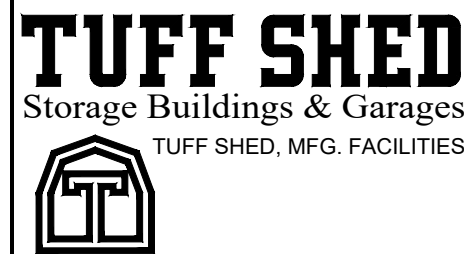
3 AUGER ANCHOR DETAIL
SCALE: N.T.S.



4 SIDEWALL BRACKET DETAIL
SCALE: N.T.S.

TITEN HD ANCHOR BOLTS (INTO CONCRETE) RE: DETAIL 4 SHEET 3		
WIDTH	LENGTH	QTY
14'	14'-24'	8
16'	16'-18'	8
16'	20'-24'	10

- NOTES:**
- ANCHORS TO BE SIMPSON TITEN HD ANCHORS. ANCHORS MAY BE GALVANIZED OR STAINLESS STEEL.
 - PROVIDE (1) ANCHOR AT EA. CORNER OF THE BUILDING. THE REMAINING ANCHORS EQUALLY SPACED ALONG THE LENGTH OF THE BUILDING. (1/2 THE REMAINING ANCHORS ON EA. LENGTH SIDE EQUALLY SPACED).



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(303) 753-8833 EXT. 96315

TITLE
GENERAL DETAILS

120 MPH, EXP. C

DRAWING NO.
NC-PPTR-TR800-02

REV. LEVEL 01

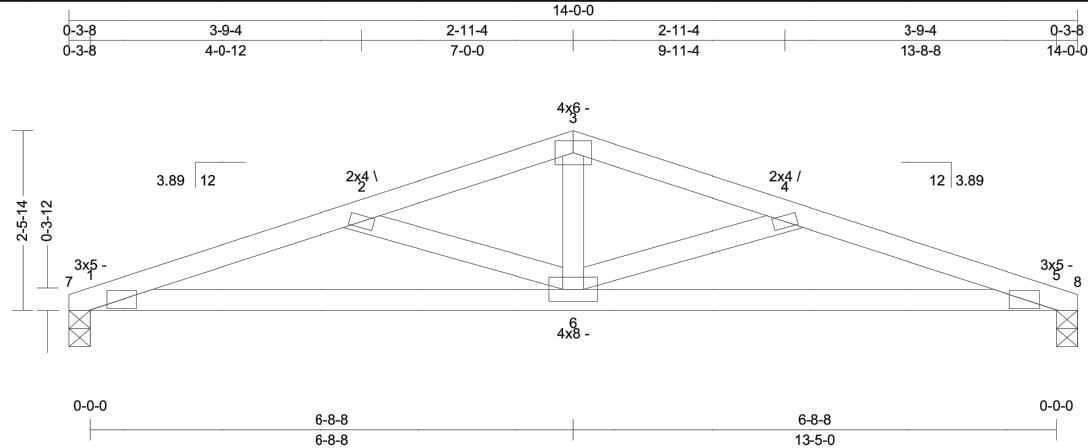
SHEET **3**

PAGE 3 OF 4

Tuff Shed
1777 S Harrison St
Suite 600
Denver, CO 80210

TrussT01-14X 4_12
Job: NCSTANDARDS
Design: B
Date: 01/20/23 09:13:16
Page: 1 of 1

SPAN 13-5-0	PITCH 3.89/12	QTY 10	OHL 0-3-8	OHR 0-3-8	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 41 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IBC 2015/	TC: 0.47 (4-5)	Vert TL: 0.18 in	L/898	(5-6)	L/180
TCDL: 10	TPI 1-2014	BC: 0.67 (5-6)	Vert LL: 0.09 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.13 (3-6)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	694 lbs		-135 lbs	-324 lbs	-324 lbs	-17 lbs
8	1	3.5 in	1.50 in	694 lbs		-135 lbs	-324 lbs	-324 lbs	

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-11-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 120 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 14 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Unbalanced roof live loads have not been considered.
- Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4
TC	12 0.465 -1,774 lbs	3-4 0.197 -1,299 lbs		
BC	56 0.671 1,678 lbs (-726 lbs)	6-1 0.671 1,678 lbs (-726 lbs)		
Web	26 0.105 -512 lbs	3-6 0.131 532 lbs (-156 lbs)	4-6 0.105 -512 lbs	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 0% (Cq = 1.00).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Listed wind uplift reactions based on MWFRS & C&C loading.

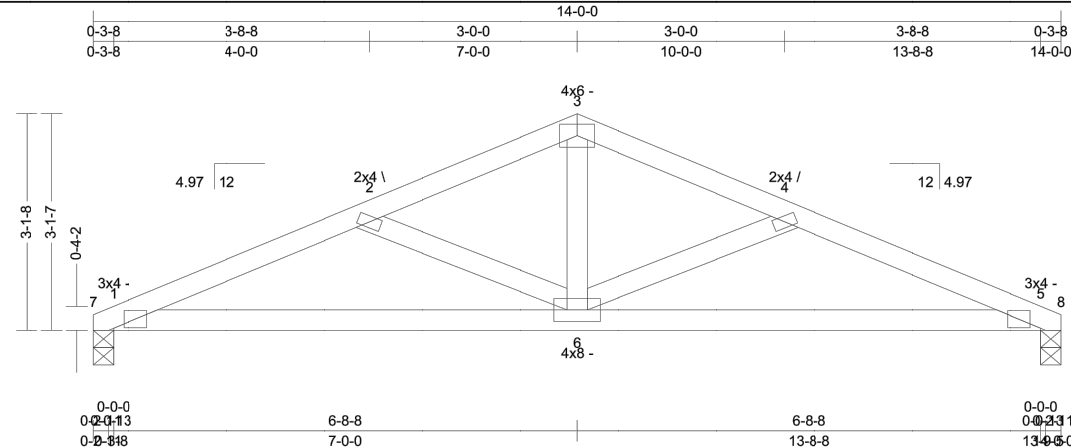
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software V5.6.395
Eagle Metal Products

TUFF SHED
1777 SOUTH HARRISON STREET
SUITE 600
DENVER, CO 80210

TrussT01-14X 5_12
Job: NCSTANDARDS
Design: B
Date: 01/20/23 09:13:37
Page: 1 of 1

SPAN 13-5-0	PITCH 4.97/12	QTY 9	OHL 0-3-8	OHR 0-3-8	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 42 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IBC 2015/	TC: 0.44 (4-5)	Vert TL: 0.18 in	L/914	(5-6)	L/180
TCDL: 10	TPI 1-2014	BC: 0.55 (5-6)	Vert LL: 0.07 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.13 (3-6)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	700 lbs		-131 lbs	-316 lbs	-316 lbs	3 lbs
8	1	3.5 in	1.50 in	700 lbs		-131 lbs	-316 lbs	-316 lbs	

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 4-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 120 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 14 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Unbalanced roof live loads have not been considered.
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply.

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-0-3-8	0-0-0	Down	Proj	10 psf	10 psf	24 in
Top	13-5-0	13-8-8	Down	Proj	10 psf	10 psf	24 in

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

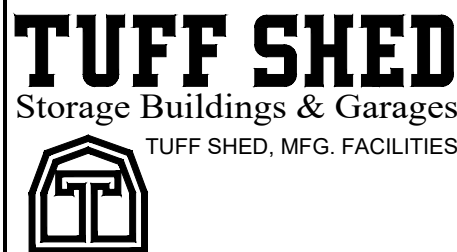
Member	Force 1	Force 2	Force 3	Force 4
TC	12 0.444 -1,437 lbs	3-4 0.183 -1,060 lbs		
BC	56 0.553 1,312 lbs (-537 lbs)	6-1 0.553 1,312 lbs (-537 lbs)		
Web	26 0.093 -421 lbs	3-6 0.132 539 lbs (-172 lbs)	4-6 0.093 -421 lbs	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 0% (Cq = 1.00).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.50 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products



Order #. _____
Customer: _____
Site Address: _____
Building Size: WIDTH - LENGTH - HEIGHT - SQ. FT. AREA _____

P.O. # _____
Drawn By: TB
Date: 1/19/23
Checked By: _____
Date: _____
Scale: N.T.S.

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TUFF SHED, INC.
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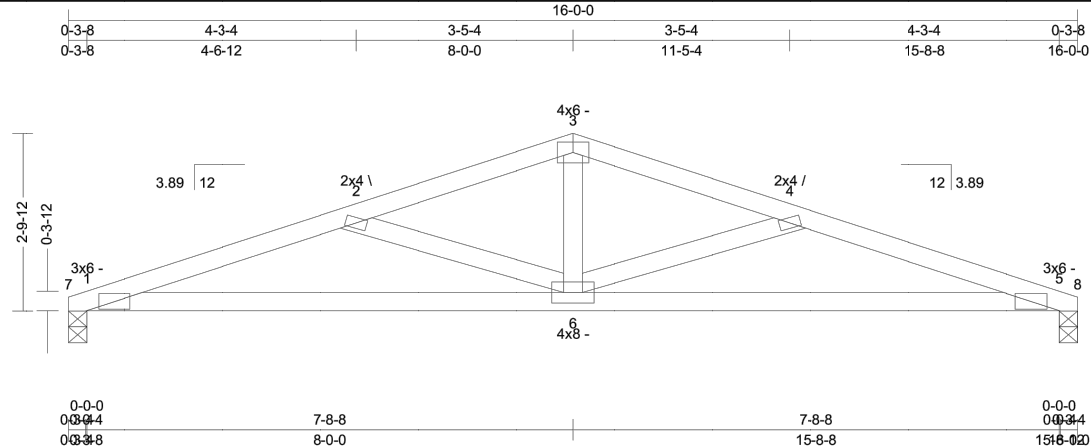
TITLE
TRUSS DETAILS
120 MPH, EXP. C

DRAWING NO.
NC-PPTR-TR800-02
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TUFF SHED
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SUITE 600
DENVER, CO 80210

TrussT01-16X-4 12
Job: NC STANDARDS
Design: BK
Date: 01/20/23 09:13:49
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SPAN 15-5-0 PITCH 3.89/12 QTY 10 OHL 0-3-8 OHR 0-3-8 CANT L 0-0-0 CANT R 0-0-0 PLYS 1 SPACING 24 in WGT/PLY 47 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IBC 2015/	TC: 0.56 (1-2)	Vert TL: 0.32 in	L/579	(5-6)	L/180
TCDL: 10	TP1 1-2014	BC: 0.78 (6-1)	Vert LL: 0.13 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.16 (4-6)	Horz TL: 0.09 in		8	
BCDL: 10	Lumber D.O.L.: 115%					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	800 lbs		-139 lbs	-326 lbs	-326 lbs	-16 lbs
8	1	3.5 in	1.50 in	800 lbs		-139 lbs	-326 lbs	-326 lbs	

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-10-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 120 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 16 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply.

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-0-3-8	0-0-0	Down	Proj	10 psf	10 psf	24 in
Top	15-5-0	15-8-8	Down	Proj	10 psf	10 psf	24 in

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5
TC	12 0.564 -2,038 lbs	3-4 0.240 -1,481 lbs			
BC	56 0.775 1,926 lbs (-733 lbs)	6-1 0.775 1,926 lbs (-733 lbs)			
Web	26 0.160 -620 lbs	3-6 0.150 610 lbs (443 lbs)	4-6 0.160 -620 lbs		

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 0% (Cq = 1.00).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.50 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

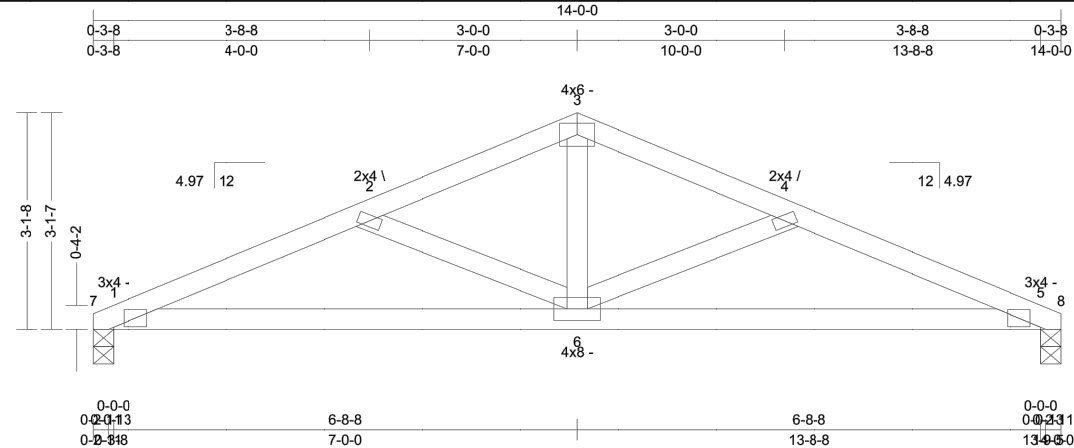
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TrussT01-14X 5 12
Job: NC STANDARDS
Design: FB
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SPAN 13-5-0 PITCH 4.97/12 QTY 9 OHL 0-3-8 OHR 0-3-8 CANT L 0-0-0 CANT R 0-0-0 PLYS 1 SPACING 24 in WGT/PLY 42 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 30	Bldg Code: IBC 2015/	TC: 0.44 (4-5)	Vert TL: 0.18 in	L/914	(5-6)	L/180
TCDL: 10	TP1 1-2014	BC: 0.55 (5-6)	Vert LL: 0.07 in	L/999	(5-6)	L/240
BCLL: 0	Rep Mbr: Yes	Web: 0.13 (3-6)	Horz TL: 0.06 in		8	
BCDL: 10	Lumber D.O.L.: 115%					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	700 lbs		-131 lbs	-316 lbs	-316 lbs	3 lbs
8	1	3.5 in	1.50 in	700 lbs		-131 lbs	-316 lbs	-316 lbs	

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 4-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 120 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 14 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Unbalanced roof live loads have not been considered.
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply.

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	-0-3-8	0-0-0	Down	Proj	10 psf	10 psf	24 in
Top	13-5-0	13-8-8	Down	Proj	10 psf	10 psf	24 in

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5
TC	12 0.444 -1,437 lbs	3-4 0.183 -1,060 lbs			
BC	56 0.553 1,312 lbs (-537 lbs)	6-1 0.553 1,312 lbs (-537 lbs)			
Web	26 0.093 -421 lbs	3-6 0.132 539 lbs (472 lbs)	4-6 0.093 -421 lbs		

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 0% (Cq = 1.00).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.50 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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TITLE
TRUSS DETAILS

120 MPH, EXP. C

DRAWING NO.
NC-PPTR-TR800-02

REV. LEVEL 01

SHEET 4

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28 MAR 2023