ERECTION NOTES

1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303–16; Section 7.10).
2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges " in the ANSI/AISC 303–16; Section 7.10.3).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16; Section 7.14).
4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
   - 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
   - 5.1. welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code – Steel or AWS D1.3 Structural Welding Code – Sheet as applicable, for the processes, positions, and materials involved.
   - 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16, Section 7.15).
10. The Metal Building Provider Field Modifications Policy:
    - 10.1. The Metal Building Provider will only be responsible for the field–modified parts designed and approved by the Metal Building Provider's Customer Service Department.
    - 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
    - 10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field–modified parts performed by third parties.
11. WARNING – SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL–OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

1. The Metal Building Provider Customer, hereafter referred to as the "customer, " obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing. it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303–16; Section 3.3).
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303–16.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil–canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefor is not a cause for rejection.
3. The Metal Building Provider's red–oxide and gray–oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water–holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de–icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
4. All bolts are 1/2" x 1–1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross–section(s) for main frame connections.
5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug–tightened joints in accordance with the specification for Structural Joints Using High–Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug–Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES

Building Code:	North Carolina Building Code 2018
Hot–rolled version:	AISC 360–10
Cold–formed version:	AISI S100–12

GENERAL LOADS

Dead Load:	2.00 psf	(Building A & B)
Roof Collateral Load:	1.00 psf	(Misc.) (Building A & B)
Sprinkler Load:	0.00 psf	
Roof Live Load:	20.00 psf	
Tributary Live Load Reduction:	YES	
Rainfall Intensity:	6.76 in/hr	(5–minute duration 5–year recurrence)

WIND LOAD

Wind Load (3–sec gust) Vult:	119 mph	
Vasd:	92 mph	
V service:	76 mph	
Exposure Factor:	B	
Wind Condition:	Enclosed	(Building A)
Wind Condition:	Partially Enclosed	(Building B)
Internal Pressure Coefficient :	+/– 0.18	(Building A)
Internal Pressure Coefficient :	+/–0.55	(Building B)
Edge Zone Width:	4.00 Ft	(Building A)
Edge Zone Width:	3.00 Ft	(Building B)

SNOW LOAD

Ground Snow Load :	10.00 psf	
Roof Snow Load :	7.00 psf	
Importance Factor:	1.00	
Exposure Factor:	1.00	
Thermal Factor:	1.00	(Building A)
Thermal Factor:	1.20	(Building B)
Slope Factor:	1.00	

Other Loads:
1.Building B is supported by A.

DEFLECTION CRITERIA

Main Frames Horizontal:	H/60	Roof Panels:	L/60
Main Frames Vertical:	L/180	Purlins:	L/180
Bearing Frame Rafter:	L/180	Wall Panels:	L/60
Endwall Columns:	L/180	Girts:	L/90
Wind Frame Horizontal :	H/60		

For components,claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

SEISMIC LOAD

Risk Category:	II – Normal
Seismic Importance Factor :	1.0000
Structural Response Acceleration (Ss):	0.1760
Structural Response Acceleration(S1):	0.0830
Site Class:	D
Design Spectral Response (Sds):	0.1877
Design Spectral Response (Sd1):	0.1328
Seismic Design Category:	C

Building–A

Framing Direction:	Lateral	Longitudinal(G2)
Structural Syst:	'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'	
Response Modification Factor(s) (R):	3.0	1.25
Deflection Amplification Factor(s):	3.0	1.25
Sesimic Response Coefficient(s) (Cs):	0.0626	0.1503
Design Base Shear V :	1.06 Kips	2.24 Kips
Analysis Procedure:	Equivalent Lateral Force	

Building–B

Framing Direction:	Lateral	Longitudinal(G2)
Structural Syst:	'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'	
Response Modification Factor(s) (R):	3.0	3.0
Deflection Amplification Factor(s):	3.0	3.0
Sesimic Response Coefficient(s) (Cs):	0.0626	0.0626
Design Base Shear V :	0.38 Kips	0.21 Kips
Analysis Procedure:	Equivalent Lateral Force	

ROOF PANEL

Profile: Super Span X Gauge: 26 Color: Galvalume Plus
UL580 Class 90: Yes
Clip Type if Standing Seam: NO

WALL PANEL

Profile: Super Span X Gauge: 26 Color: SMP Colonial Red

PRIMARY FRAMING

Built–Up & Hot–Rolled: Gray Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts:	Pre–Galvanized
Girts, Light Gage Columns:	Pre–Galvanized
Light Gage Jambs & Headers:	Pre–Galvanized
Base Angle Finish:	Pre–Galvanized

Hot–Dip Galvanizing conforms to the ASTM A123 specification.
Pre–Galvanized members conform to the ASTM A653, Grade 50, Coating G–90 specification.

APPROVAL SPECIFICATIONS

1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
   - 5.1. Be made in contrasting ink.
   - 5.2. Be legible and unambiguous.
   - 5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE

DWG NO.	ISSUE	DATE	DESCRIPTION
C1	P1	05.29.23	COVER SHEET
F1	0	05.29.23	ANCHOR BOLT PLAN
F2	0	05.29.23	ANCHOR BOLT DETAILS
F3	0	05.29.23	ANCHOR BOLT REACTIONS
F4	0	05.29.23	ANCHOR BOLT REACTIONS
F5	0	05.29.23	ANCHOR BOLT REACTIONS
F6	0	05.29.23	ANCHOR BOLT REACTIONS
P1	P1	05.29.23	RIGID FRAME ELEVATION
P2	P1	05.29.23	RIGID FRAME ELEVATION
P3	P1	05.29.23	RIGID FRAME ELEVATION
W1	P1	05.29.23	PORTAL FRAME ELEVATION
E1	P1	05.29.23	ROOF FRAMING PLAN
E2	P1	05.29.23	ROOF SHEETING PLAN
E3	P1	05.29.23	ENDWALL FRAME & SHEETING ELEVATION
E4	P1	05.29.23	ENDWALL FRAME & SHEETING ELEVATION
E5	P1	05.29.23	SIDEWALL FRAME & SHEETING ELEVATION
E6	P1	05.29.23	SIDEWALL FRAME & SHEETING ELEVATION
E7	P1	05.29.23	SIDEWALL FRAME & SHEETING ELEVATION
E8	P1	05.29.23	BUILDING SECTIONS
D1	P1	05.29.23	STANDARD DETAIL PAGE
D2	P1	05.29.23	STANDARD DETAIL PAGE
D3	P1	05.29.23	STANDARD DETAIL PAGE
D4	P1	05.29.23	STANDARD DETAIL PAGE

TRIM COLOR:

SHADOW GUTTER:	SMP Old Town Gray	GAUGE: 26
SHADOW EAVE:	SMP Old Town Gray	GAUGE: 26
SHADOW RAKE:	SMP Old Town Gray	GAUGE: 26
ACCESSORY:	SMP Old Town Gray	GAUGE: 26
CORNER:	SMP Old Town Gray	GAUGE: 26
DOWNSPOUT:	SMP Old Town Gray	GAUGE: 26
BASE:	SMP Colonial Red	GAUGE: 26

Building A & B
The rigid frame at lines 1&4 are designed as a non–expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.

NORTH CAROLINA PROFESSIONAL ENGINEER 9193 J. WALTER LEWIS
05/31/2023
DN: c=US, o=Virginia, dnQualifier=A01410C0000017D507E13520000513F, cn=James W Lewis
Date: 2023.05.31 16:41:36 -04'00'

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Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

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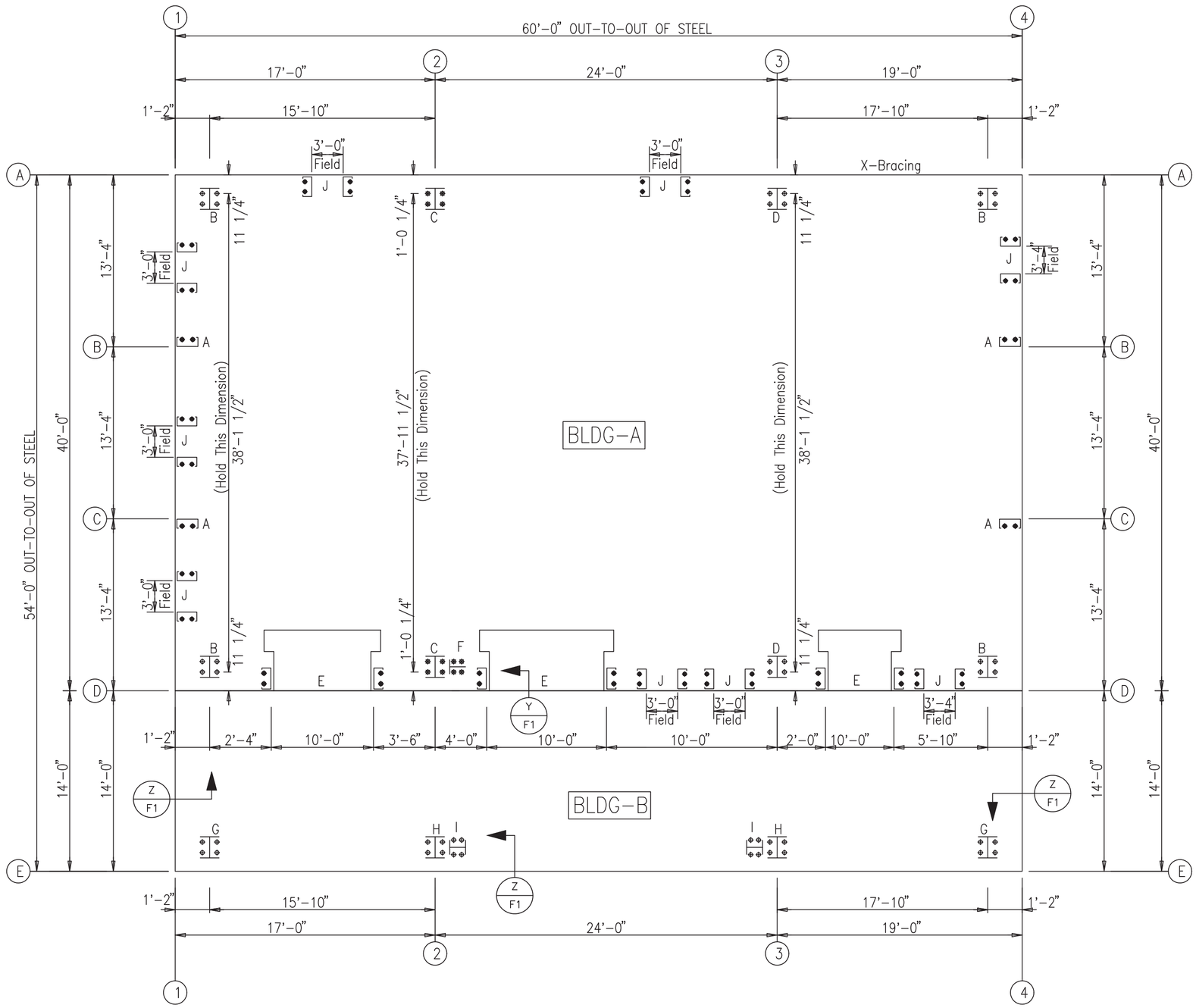
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: COVER SHEET					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017–31607	DWG NO: C1	ISSUE: P1

Dia= 5/8"

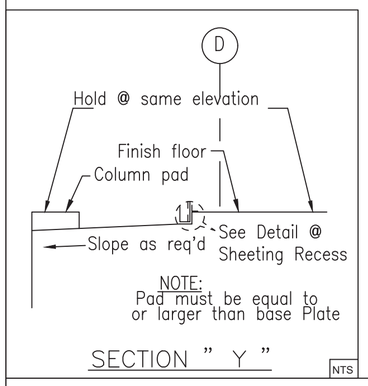
Dia= 3/4"

Dia=1"



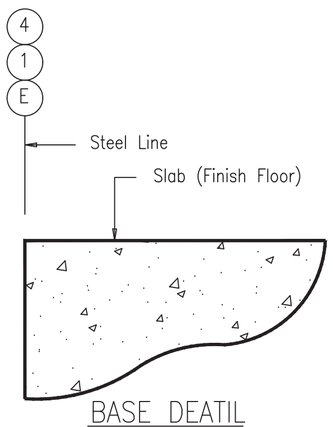
ANCHOR BOLT PLAN

NOTE: All Base Plates @ Finished Floor (U.N.)

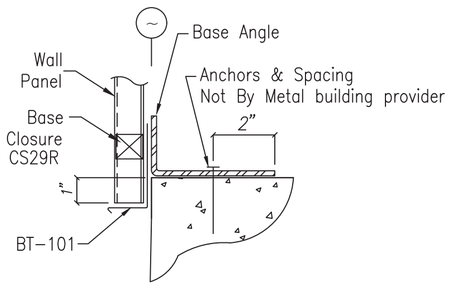


SECTION " Y "

NTS



BASE DEATIL
SECTION-Z



BASE TRIM W/BASE ANGLE CONDITION NO RECESS

NTS

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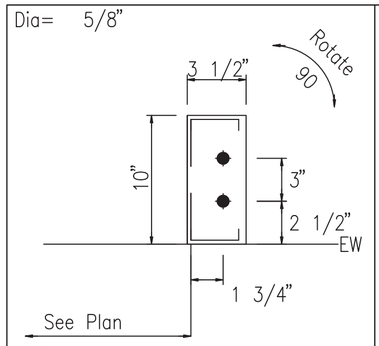
☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

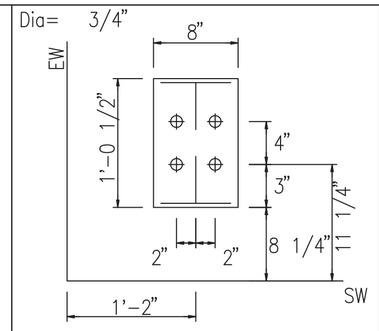
OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
0	05.29.23	FOR ERECTOR INSTALLATION	PND	PNC

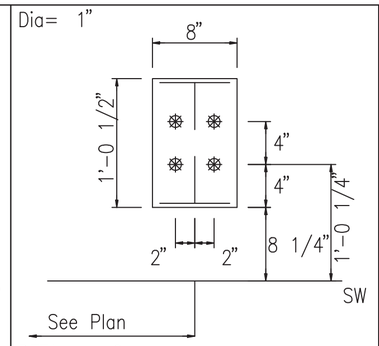
SHEET DESCRIPTION: ANCHOR BOLT PLAN						BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH						CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH							
JOBSITE LOCATION: BENSON, NC 27504						JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607		DWG NO: F1	ISSUE: 0



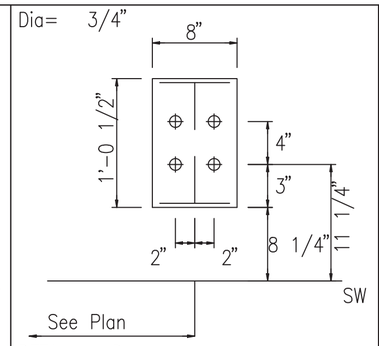
DETAIL A



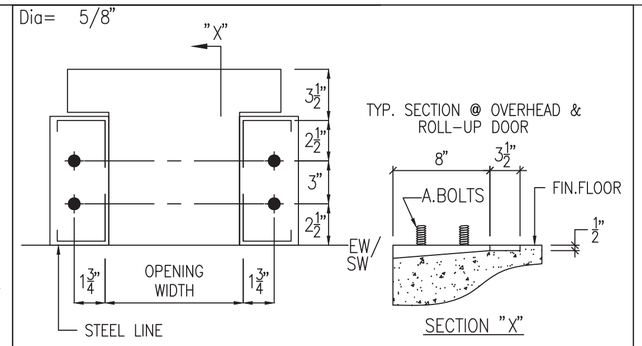
DETAIL B



DETAIL C



DETAIL D



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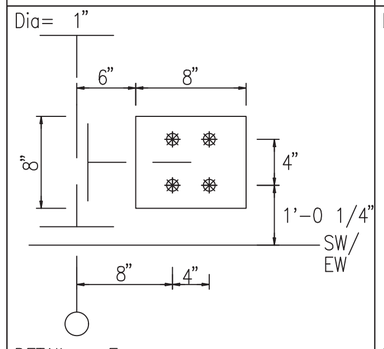
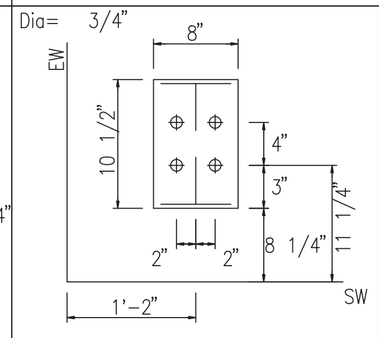
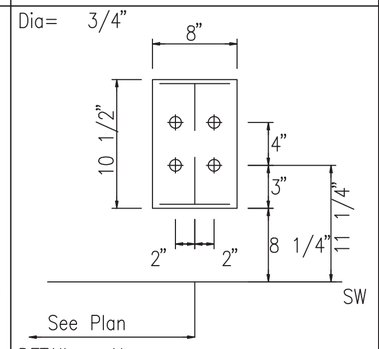


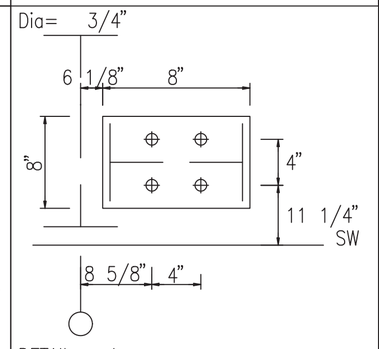
DETAIL F



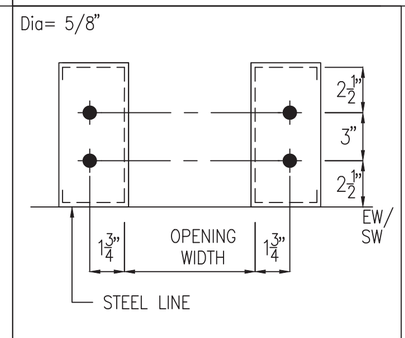
DETAIL G



DETAIL H



DETAIL I



DETAIL J

Dia= 5/8"
Dia= 3/4"
Dia=1"

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NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS
05/31/2023

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OLYMPIA STEEL BUILDINGS.

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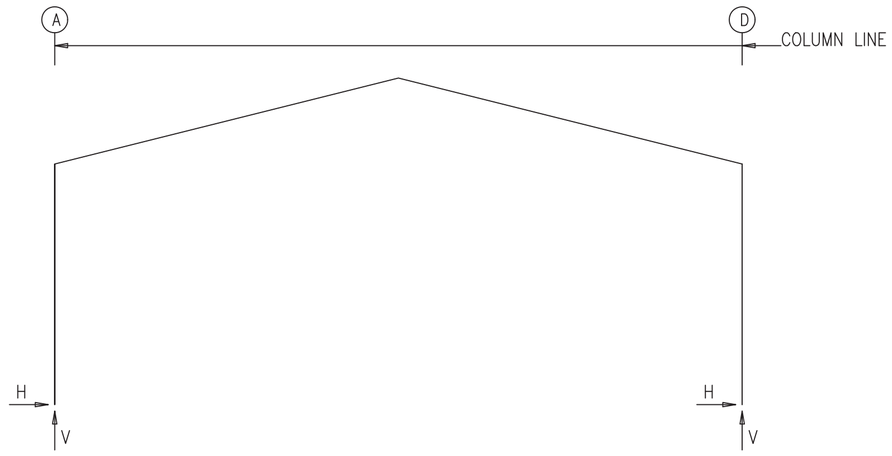
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: F2	ISSUE: 0	

FRAME LINES: 1 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
1*	A	4	1.5	3.0	5	−2.1	−2.9	4	0.750	8.000	12.50	0.375	0.0
		1	1.4	4.3	8	−0.5	−3.6						
1*	D	7	1.9	−2.2	3	−1.8	3.2	4	0.750	8.000	12.50	0.375	0.0
		1	−1.4	5.9	6	1.1	−3.3						

1* Frame lines: 1 4

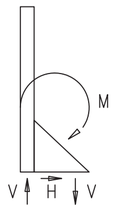
RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
2	A	4	2.6	4.8	5	−3.4	−4.8	4	1.000	8.000	12.50	1.000	0.0
		1	2.2	6.4									
2	D	7	3.3	−3.0	3	−3.1	5.1	4	1.000	8.000	12.50	1.000	0.0
		2	−1.6	30.9	9	−1.9	−36.8						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
3	A	4	2.7	5.1	5	−3.7	−5.1	4	0.750	8.000	12.50	0.375	0.0
		1	2.3	6.9	8	−0.8	−5.5						
3	D	7	3.6	−3.2	3	−3.3	5.4	4	0.750	8.000	12.50	0.375	0.0
		1	−2.3	10.1	6	1.8	−5.6						

FIXED BASE REACTIONS

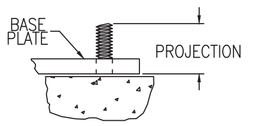


Wall Loc	Wall Line	Col Line	R/L	Load_ID	± Reactions Horz (k)	± Reactions Vert (k)	Moment (f-k)	Anc_Bolt Qty	Anc_Bolt Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick
F_SW	D	2	R	Wind	3.5	55.4	46.2	4	1.00	8.000	8.000	0.625
				Seismic	1.7	27.4	22.9					

GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
1*	A	0.2	0.9	0.1	0.2	1.1	3.2	0.5	1.4	−3.7	−5.8	−0.1	−3.5
1*	D	−0.2	1.1	−0.1	0.3	−1.1	4.6	−0.5	2.0	−1.6	−3.5	2.1	−6.6

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
1*	A	−2.8	−3.4	0.8	−1.1	−1.1	−6.9	−1.6	−6.3	−0.1	−0.1	0.1	0.1
1*	D	−0.2	−1.7	3.4	−4.7	−0.8	−4.3	−1.3	−5.0	−0.2	0.1	0.2	−0.1

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
1*	A	0.0	−0.3	0.7	2.0	0.6	1.7	0.4	0.8
1*	D	0.0	0.0	−0.7	2.0	−0.6	1.0	−0.4	1.4

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3	A	0.4	1.3	0.2	0.4	1.8	5.1	1.0	3.0	−6.5	−9.9	0.1	−6.1
3	D	−0.4	1.7	−0.2	0.6	−1.7	7.7	−1.0	4.2	−3.4	−6.0	3.4	−11.1

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
3	A	−4.7	−4.8	1.9	−1.0	−1.8	−10.5	−2.5	−9.6	−0.2	−0.2	0.2	0.2
3	D	−0.4	−2.0	6.3	−7.1	−2.2	−7.2	−3.0	−8.2	−0.2	0.2	0.2	−0.1

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
3	A	0.0	−0.3	1.5	4.3	1.2	3.6	0.9	1.8
3	D	0.0	0.0	−1.5	4.3	−1.2	2.2	−0.9	3.0

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	A	0.4	1.3	0.1	0.4	1.7	4.7	1.0	2.8	−6.0	−9.2	0.1	−5.7
2	D	−0.4	1.7	−0.1	0.5	−1.6	7.2	−0.9	3.9	−3.1	−5.6	3.1	−10.3

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	A	−4.3	−4.5	1.8	−1.0	−1.7	−8.1	−2.4	−7.3	−0.2	−0.1	0.2	0.1
2	D	−0.4	−1.9	5.9	−6.6	−2.1	−62.1	−2.8	−63.0	−0.2	0.2	0.2	−0.1

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F3UNB_SL_L Horiz	F3UNB_SL_L Vert	F3UNB_SL_R Horiz	F3UNB_SL_R Vert
2	A	0.0	0.0	1.4	4.0	1.1	3.3	0.9	1.7
2	D	0.0	−27.4	−1.4	4.0	−1.1	2.0	−0.9	2.8

1* Frame lines: 1 4

ANCHOR BOLT SUMMARY (GRADE 36)

Qty	Locate	Dia (in)	Type	Proj (in)
● 48	Jamb	5/8"	F1554	2.50
● 8	Endwall	5/8"	F1554	2.50
⊕ 24	Frame	3/4"	F1554	3.00
❄ 8	Frame	1"	F1554	3.50
❄ 4	Fixed Base	1"	F1554	3.50

Col	Wind Press Horz.	Wind Suct Horz.
1D	−0.92	1.02
4D	−0.92	1.02

BLDG−A

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	± Reactions(k) Wind Vert	± Reactions(k) Seismic Horz	± Reactions(k) Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis	Note
L_EW	1								(h)
F_SW	D	2							(g)
R_EW	4								(h)
B_SW	A	4,3	2.7	1.8	0.5	0.3			

(g)Wind column at column line
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh

05/31/2023

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☒ FOR ERECTOR INSTALLATION: Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

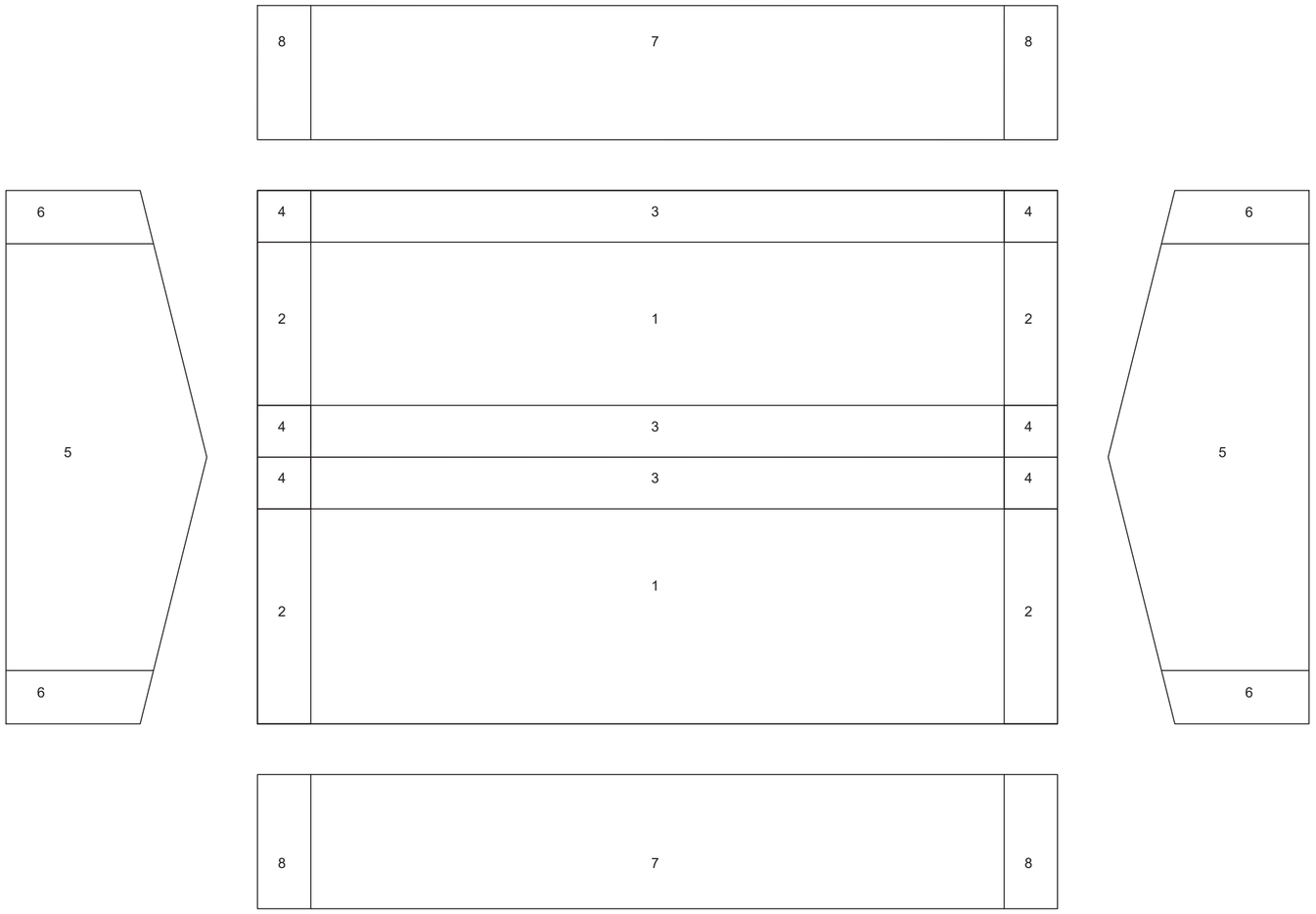
ISSUE	DATE	DESCRIPTION	BY	CHK
0	05.29.23	FOR ERECTOR INSTALLATION	PND	PNC

SHEET DESCRIPTION: ANCHOR BOLT REACTIONS					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017−31607	DWG NO: F3	ISSUE: 0

Components & Cladding

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			16.00	16.00	-21.14	-23.30
2		4.00	16.00	16.00	-29.81	-40.31
3	4.00		16.00	16.00	-29.81	-40.31
4	4.00	4.00	16.00	16.00	-46.93	-59.88
5			20.51	25.45	-22.67	-27.61
6	4.00		20.51	25.45	-24.26	-33.96
7			20.50	25.50	-22.70	-27.60
8	4.00		20.50	25.50	-24.29	-33.95

(+) wind towards surface
(-) wind away from surface



ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz	Seis Long Vert
1	B	0.1	−2.2	2.5	0.0
1	C	0.1	−2.2	2.5	0.0
4	C	0.1	−2.2	2.5	0.0
4	B	0.1	−2.2	2.5	0.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Elev. (in)
1	B	10	1.5	0.0	11	−1.3	0.0	2	0.625	3.500	10.00	0.250	0.0
		12	1.5	0.1									
1	C	10	1.5	0.0	11	−1.3	0.0	2	0.625	3.500	10.00	0.250	0.0
		12	1.5	0.1									
4	C	10	1.5	0.0	11	−1.3	0.0	2	0.625	3.500	10.00	0.250	0.0
		12	1.5	0.1									
4	B	10	1.5	0.0	11	−1.3	0.0	2	0.625	3.500	10.00	0.250	0.0
		12	1.5	0.1									

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Loading conditions are:
   1 Dead+Collateral+Live
   2 Dead+0.6Wind_Long1R
   3 Dead+Collateral+0.75Live+0.45Wind_Left1
   4 Dead+Collateral+0.75Live+0.45Wind_Right2
   5 0.6Dead+0.6Wind_Left1
   6 0.6Dead+0.6Wind_Right1
   7 0.6Dead+0.6Wind_Right2
   8 0.6Dead+0.6Wind_Long1L
   9 0.6Dead+0.6Wind_Long2L
   10 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
   11 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
   12 Dead+0.6Wind_Right2+0.6Wind_Suction

BLDG-A

NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS
05/31/2023

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

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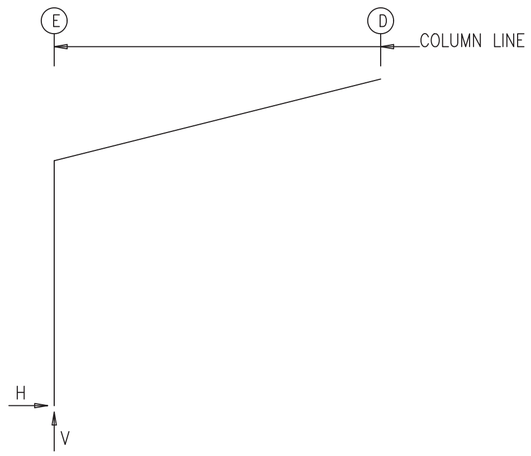
☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
0	05.29.23	FOR ERECTOR INSTALLATION	PND	PNC

SHEET DESCRIPTION: ANCHOR BOLT REACTIONS					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: F4	ISSUE: 0

FRAME LINES: 4 3 2 1



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
4*	E	4	0.6	−1.1	5	−0.7	0.1	4	0.750	8.000	10.50	0.375	0.0
		1	0.0	2.0	6	0.6	−1.5						

4* Frame lines: 4 1

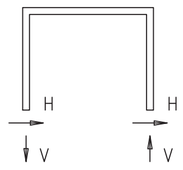
RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
3	E	7	1.3	−2.1	5	−1.4	0.4	4	0.750	8.000	10.50	0.375	0.0
		1	0.1	3.6	6	1.3	−2.8						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
2	E	7	1.2	−2.0	5	−1.3	0.4	4	0.750	8.000	10.50	0.375	0.0
		1	0.1	3.4	6	1.2	−2.6						

PORTAL FRAME REACTIONS



Wall Loc	Wall Line	Col Line	± Reactions Wind(k) Horz	Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick
B_SW	E	2	0.4	0.3	0.1	0.1	4	0.750	8.000	8.000	0.375
B_SW	E	3	0.4	0.3	0.1	0.1	4	0.750	8.000	8.000	0.375

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
4*	E	0.0	0.4	0.0	0.1	0.0	1.5	0.0	0.5	−0.1	−2.5	1.0	−2.2

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	MIN_SNOW Horiz	MIN_SNOW Vert
4*	E	−1.2	−0.3	−0.1	0.0	1.0	−2.8	1.0	−2.1	0.0	0.8

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3	E	0.0	0.6	0.0	0.2	0.1	2.9	0.0	1.1	0.0	−4.6	2.0	−4.3

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	MIN_SNOW Horiz	MIN_SNOW Vert
3	E	−2.3	0.2	−0.4	0.5	2.1	−5.3	2.1	−4.1	0.0	1.6

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	E	0.0	0.5	0.0	0.2	0.1	2.7	0.0	1.0	0.0	−4.3	1.8	−4.0

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	MIN_SNOW Horiz	MIN_SNOW Vert
2	E	−2.1	0.2	−0.4	0.5	2.0	−4.9	2.0	−3.8	0.0	1.5

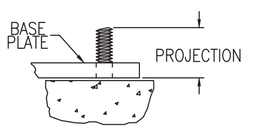
4* Frame lines: 4 1

BLDG−B

GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

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DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: F5	ISSUE: 0

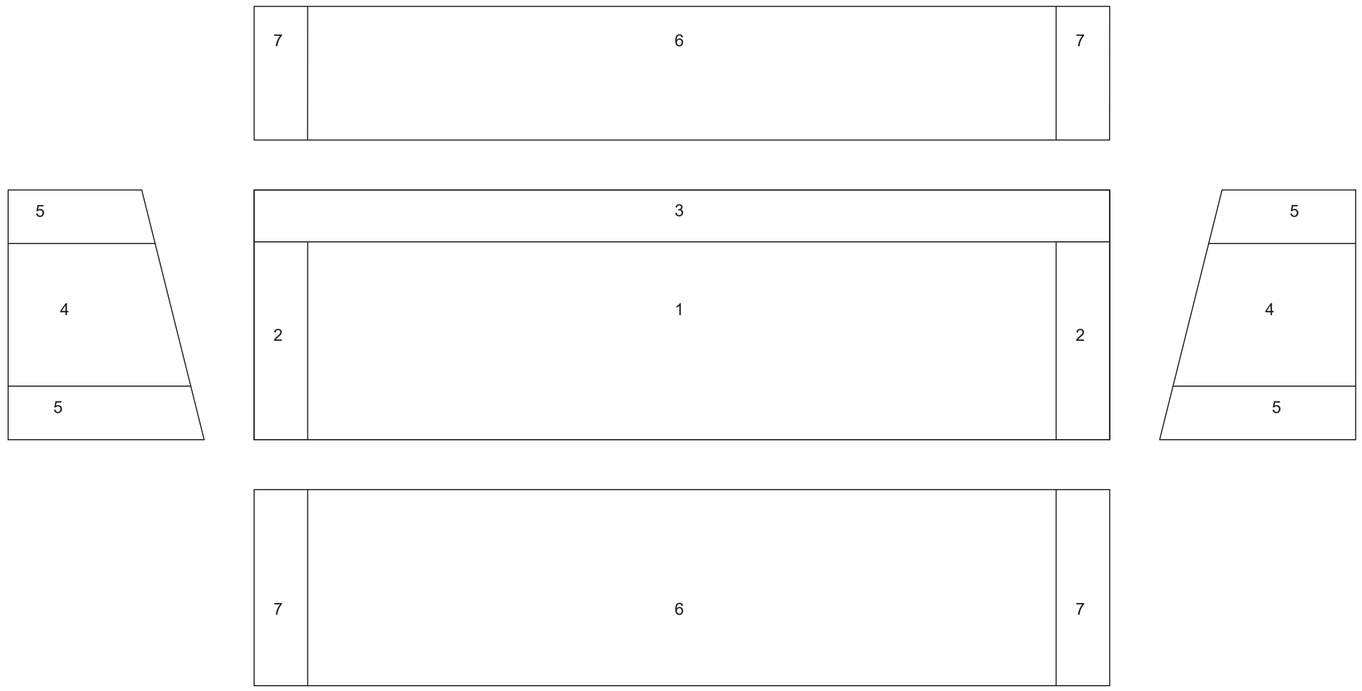
NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 9193 J. WALTER LEWIS

05/31/2023

Components & Cladding

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			18.33	20.49	-35.59	-39.91
2		3.00	18.33	20.49	-37.73	-46.30
3	3.00		18.33	20.49	-37.73	-46.30
4			28.49	33.43	-30.65	-35.59
5	3.00		28.49	33.43	-32.18	-42.00
6			28.50	33.40	-30.70	-35.60
7	3.00		28.50	33.40	-32.24	-42.01

(+) wind towards surface
(-) wind away from surface



NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Loading conditions are:
   1 Dead+Collateral+Live
   2 Dead+Collateral+0.75Snow+0.45Wind_Long2L+0.75Slide_Snow
   3 0.6Dead+0.6Wind_Left1
   4 0.6Dead+0.6Wind_Right1
   5 0.6Dead+0.6Wind_Left2
   6 0.6Dead+0.6Wind_Long1R
   7 0.6Dead+0.6Wind_Long2R

ANCHOR BOLT SUMMARY (GRADE 36)

Qty	Locate	Dia (in)	Type	Proj (in)
⊕ 16	Frame	3/4"	F1554	3.00
⊕ 8	Portal Frame	3/4"	F1554	3.00

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	4								(h)
F_SW	D								(f)
R_EW	1								(h)
B_SW	E	3,2							(a)

(a)Wind bent in bay
(f)Bracing loads are applied to adjacent building
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh

BLDG-B

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OLYMPIA STEEL BUILDINGS.

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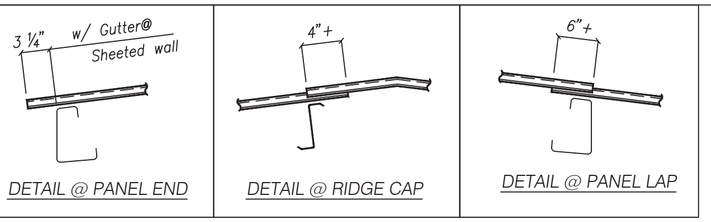
0	05.29.23	FOR ERECTOR INSTALLATION	PND	PNC

SHEET DESCRIPTION: ANCHOR BOLT REACTIONS					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: F6	ISSUE: 0

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-9 1/4"
SP-2	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-1"
SP-3	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	11 1/2"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	1'-0 1/2"
BP-2	8"	3/8"	10 1/2"



DETAIL @ PANEL END

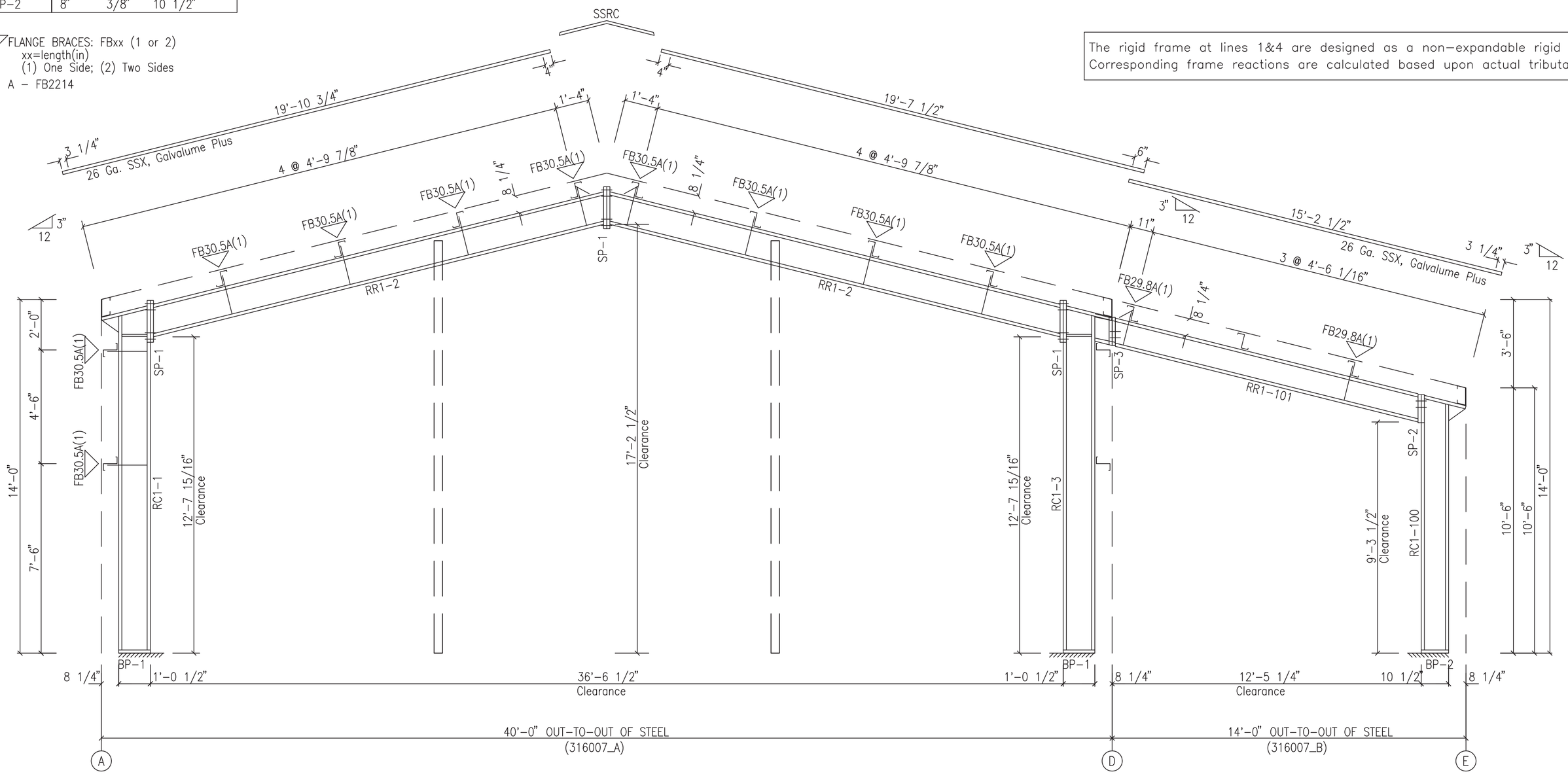
DETAIL @ RIDGE CAP

DETAIL @ PANEL LAP

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC1-1	12.0/12.0	0.135	5 x 1/4" 6 x 1/4"	5 x 1/4"
RR1-2	12.0/12.0	0.135	5 x 1/4"	5 x 1/4"
RC1-3	12.0/12.0	0.135	6 x 1/4" 6 x 1/4"	6 x 1/4"
RC1-100	10.0/10.0	0.135	5 x 1/4" 6 x 1/4"	5 x 1/4"
RR1-101	10.0/10.0	0.135	5 x 1/4"	5 x 1/4"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – FB2214

The rigid frame at lines 1&4 are designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.

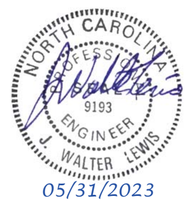


RIGID FRAME ELEVATION: FRAME LINE 1 4

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.



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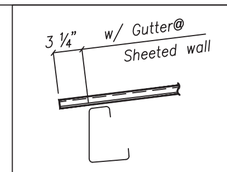
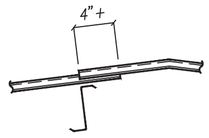
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: P1	ISSUE: P1

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	1'-9 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-9 1/4"
SP-3	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-1"
SP-4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	11 1/2"

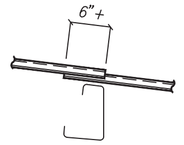
BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1"	1'-0 1/2"
BP-2	8"	3/8"	10 1/2"



DETAIL @ PANEL END



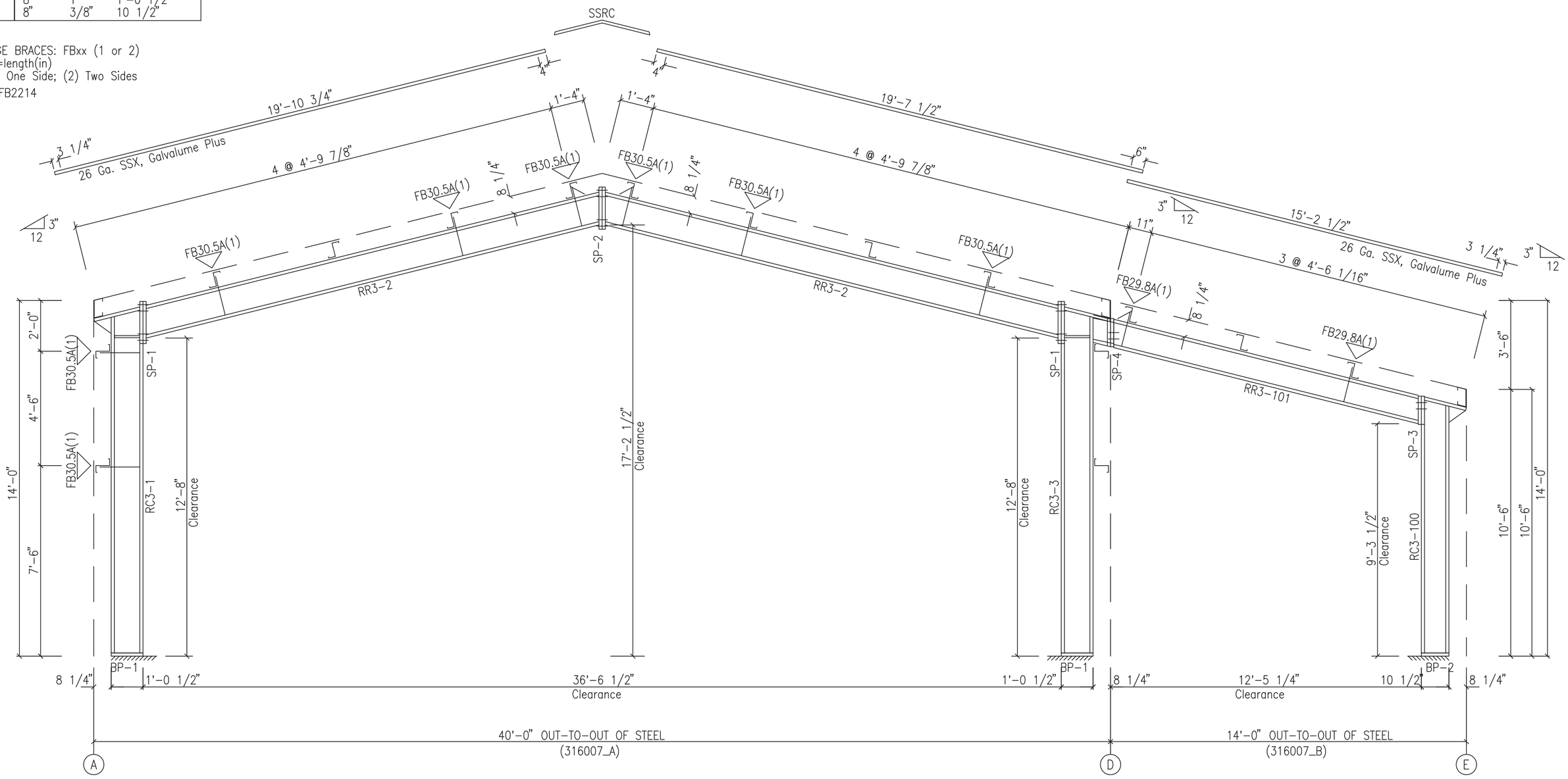
DETAIL @ RIDGE CAP



DETAIL @ PANEL LAP

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC3-1	12.0/12.0	0.135	5 x 1/4"	5 x 1/4"
	12.0/12.0	0.164	6 x 1/4"	
RR3-2	12.0/12.0	0.135	5 x 1/4"	5 x 1/4"
RC3-3	12.0/12.0	0.164	6 x 1/4"	6 x 1/4"
	12.0/12.0	0.135	6 x 1/4"	
RC3-100	10.0/10.0	0.135	5 x 1/4"	5 x 1/4"
			6 x 1/4"	
RR3-101	10.0/10.0	0.135	5 x 1/4"	5 x 1/4"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – FB2214



RIGID FRAME ELEVATION: FRAME LINE 2

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS
05/31/2023

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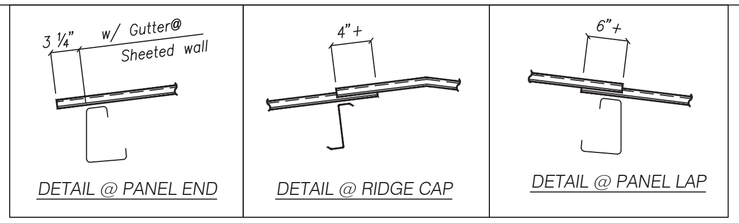
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: P2	ISSUE: P1

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	1'-9 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-9 1/4"
SP-3	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-1"
SP-4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	11 1/2"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	1'-0 1/2"
BP-2	8"	3/8"	10 1/2"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – FB2214

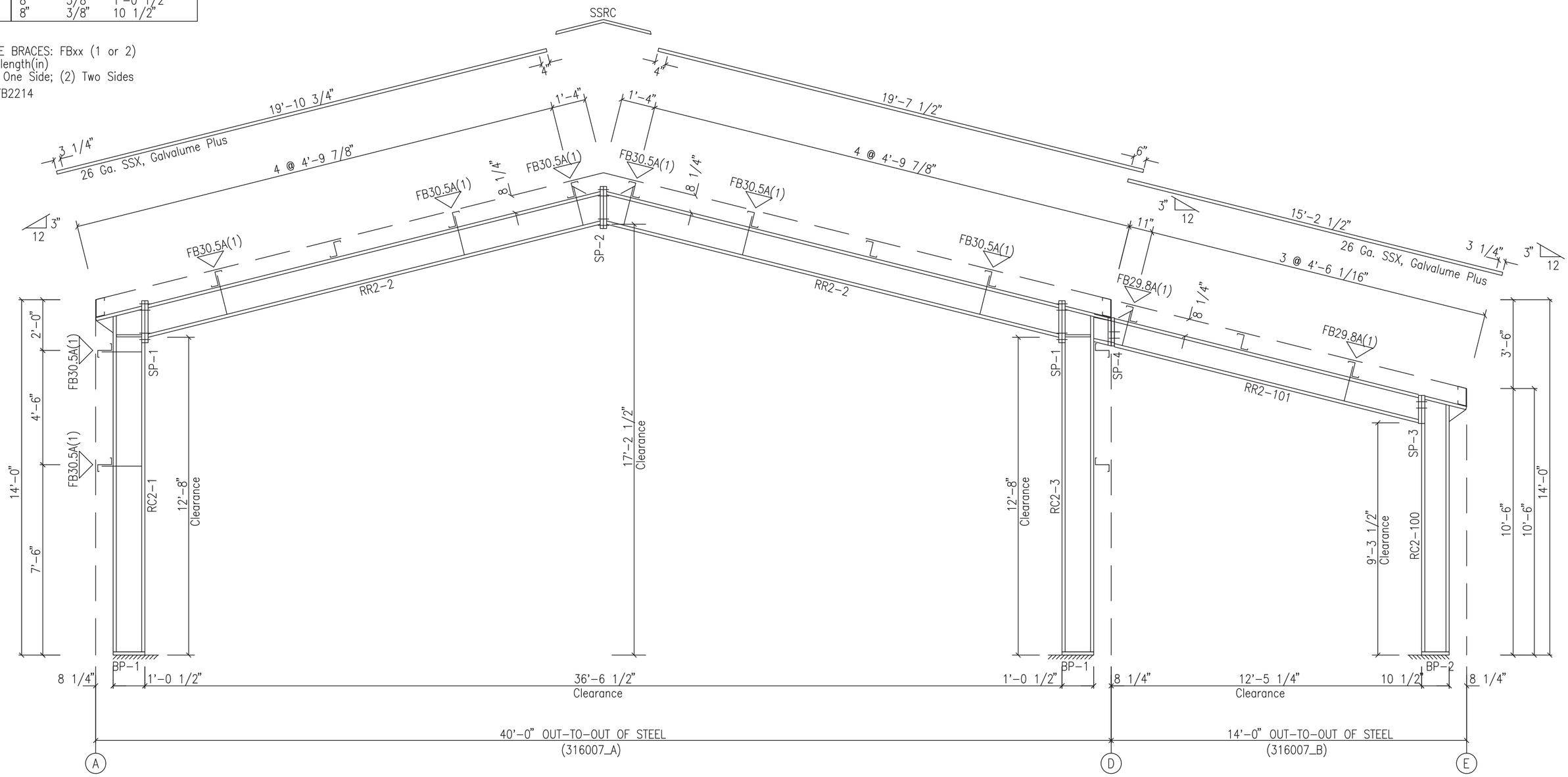


DETAIL @ PANEL END

DETAIL @ RIDGE CAP

DETAIL @ PANEL LAP

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC2-1	12.0/12.0	0.135	5 x 1/4"	5 x 1/4"
	12.0/12.0	0.164	6 x 1/4"	
RR2-2	12.0/12.0	0.135	5 x 1/4"	5 x 1/4"
RC2-3	12.0/12.0	0.164	6 x 1/4"	6 x 1/4"
	12.0/12.0	0.135	6 x 1/4"	
RC2-100	10.0/10.0	0.135	5 x 1/4"	5 x 1/4"
			6 x 1/4"	
RR2-101	10.0/10.0	0.135	5 x 1/4"	5 x 1/4"



RIGID FRAME ELEVATION: FRAME LINE 3

BOLT TIGHTENING (Snug–Tight)

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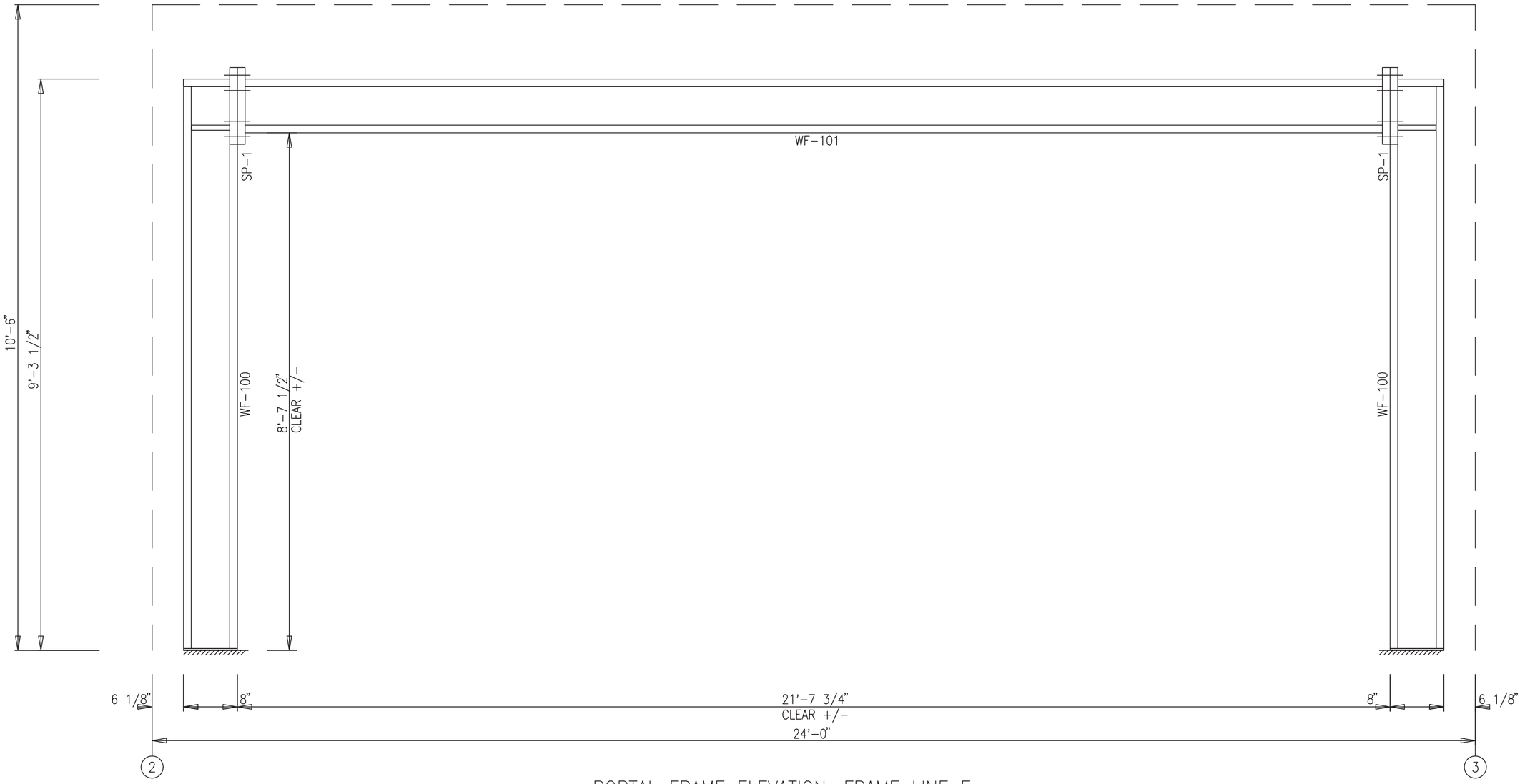
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017–31607	DWG NO: P3	ISSUE: P1

SPLICE BOLTS									
Splice Mark	Quan Top/	Bot	-----Bolt----- Type	Dia	Length	Width	Thick	Length	
SP- 1	4	4	A325	0.750	1.75	6"	3/8"	1'-4 1/4"	

MEMBER SIZE TABLE	
MARK	MEMBER
WF-101	W8641
WF-100	W8541



PORTAL FRAME ELEVATION: FRAME LINE E

BOLT TIGHTENING (Snug-Tight)

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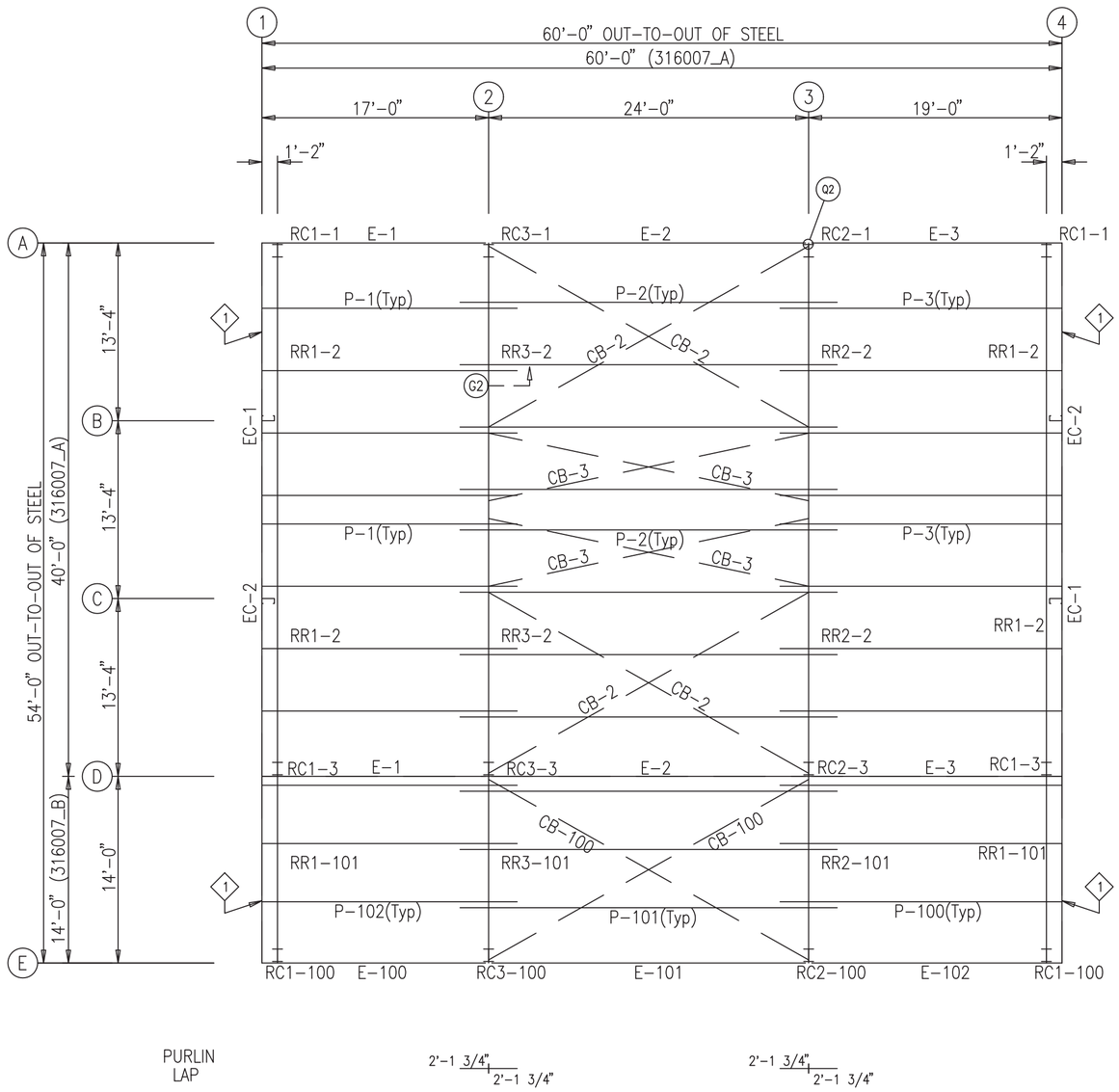
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: W1	ISSUE: P1

MEMBER TABLE ROOF PLAN	
MARK	PART
P-1	8X25Z16
P-2	8X25Z16
P-3	8X25Z16
E-1	8ES143
E-2	8ES143
E-3	8ES143
CB-2	0.25_CBL
CB-3	0.25_CBL
P-100	8X25Z16
P-101	8X25Z16
P-102	8X25Z16
E-100	8ES143
E-101	8ES143
E-102	8ES143
CB-100	0.25_CBL

ANGLE TABLE ROOF PLAN		
◇ID	MARK	LENGTH
1	RA2000	20'-0"



PURLIN LAP

2'-1 3/4" | 2'-1 3/4"

2'-1 3/4" | 2'-1 3/4"

ROOF FRAMING PLAN

UL580, CLASS 90 CONST. NUMBER 167

05/31/2023

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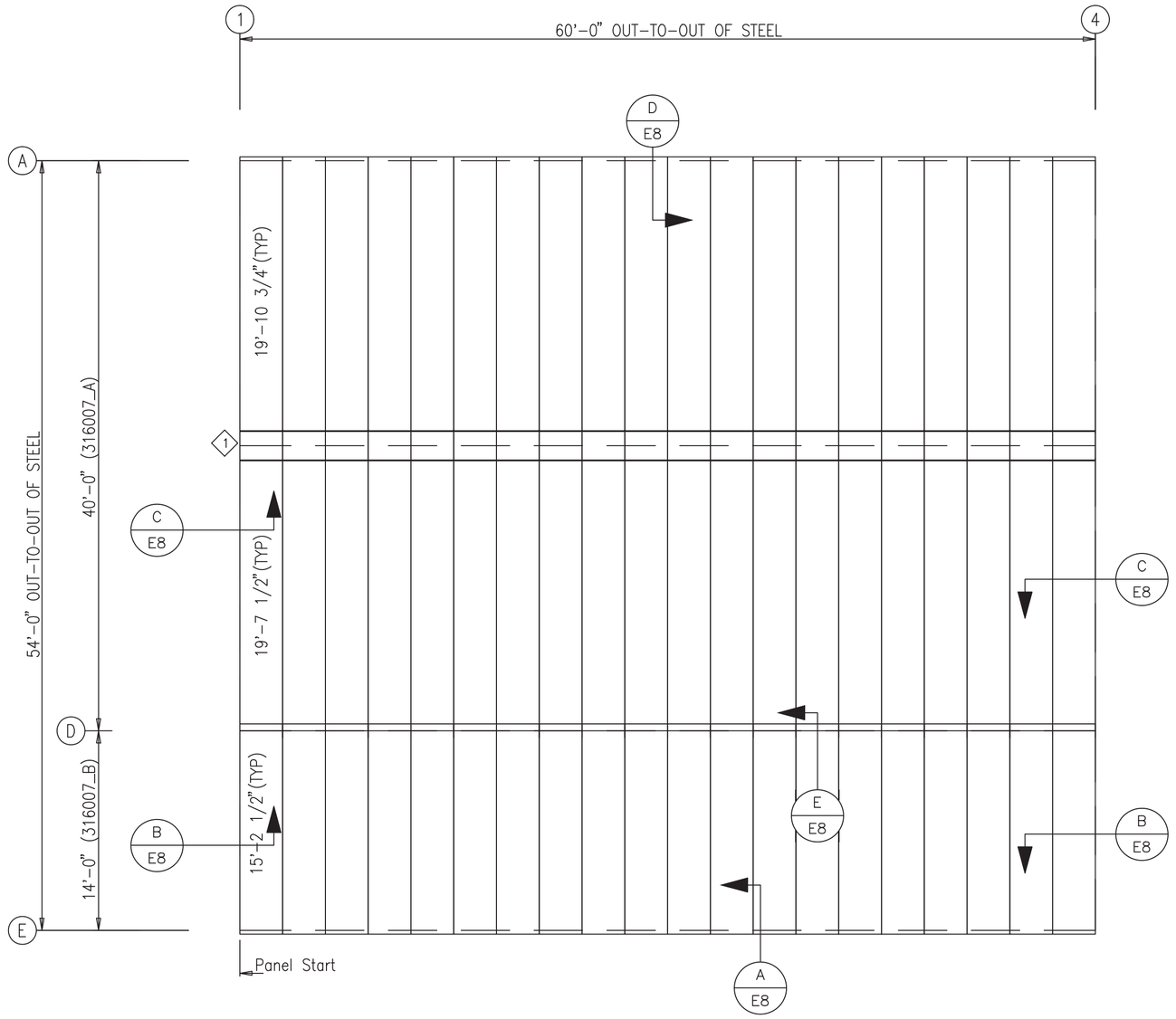
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: E1	ISSUE: P1

ROOF SHEETING TRIM TABLE		
◇ID	PART	LENGTH
1	SSRC30	3'-0"



ROOF SHEETING PLAN

PANELS: 26 Ga. SSX – Galvalume Plus

NORTH CAROLINA PROFESSIONAL ENGINEER J. WALTER LEWIS 9193

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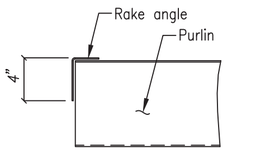
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: E2	ISSUE: P1



Detail at Rake Angle

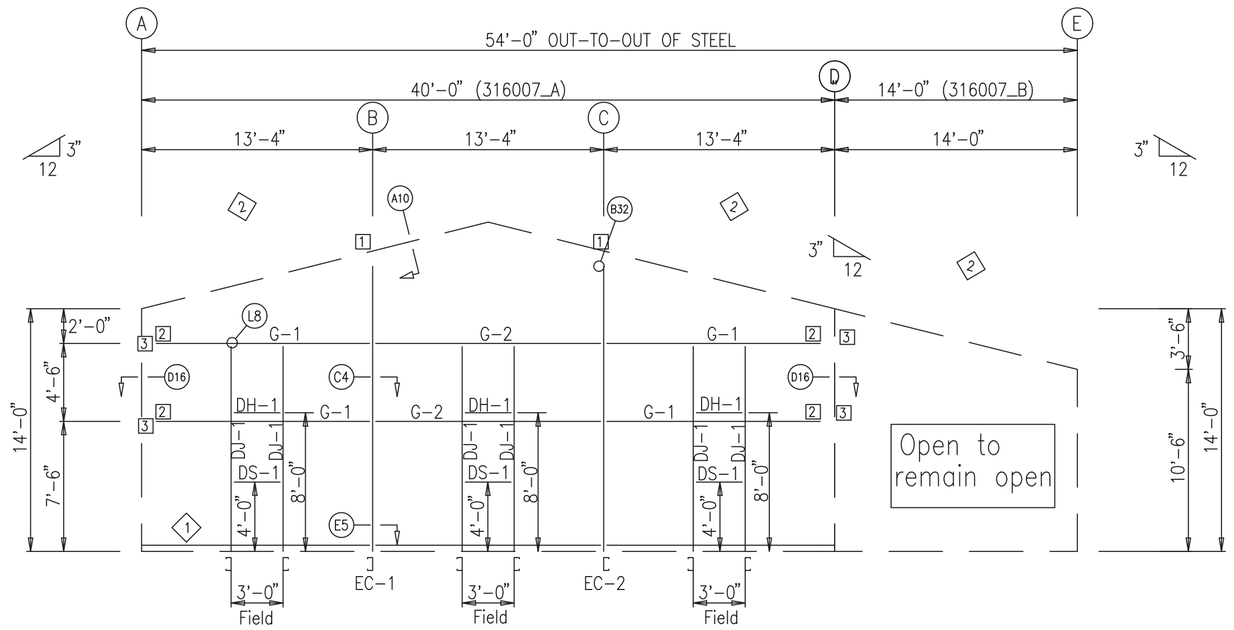
BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	4	A325	5/8"	1 1/2"

TRIM TABLE FRAME LINE 1		
◇ID	MARK	LENGTH
3	BT-101	10'-3"
4	CT-102	14'-4"
5	RT-101	10'-3"
6	RT-101	15'-3"
7	SPB_	
8	MT-116B	4'-4"
9	JT-101	4'-4"
10	MT-116B	3'-4"
11	HT-101	3'-4"
12	MT-114	3'-4"
13	SCB	
14	CF-102	15'-3"

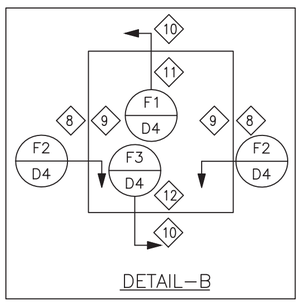
MEMBER TABLE FRAME LINE 1	
MARK	PART
EC-1	10M35C14
EC-2	10M35C14
DJ-1	8M25C14
DH-1	8M25C14
DS-1	8M25C14
G-1	8X25Z16
G-2	8X25Z16

ANGLE TABLE FRAME LINE 1	
◇ID	MARK
1	BB2000
2	RA2000

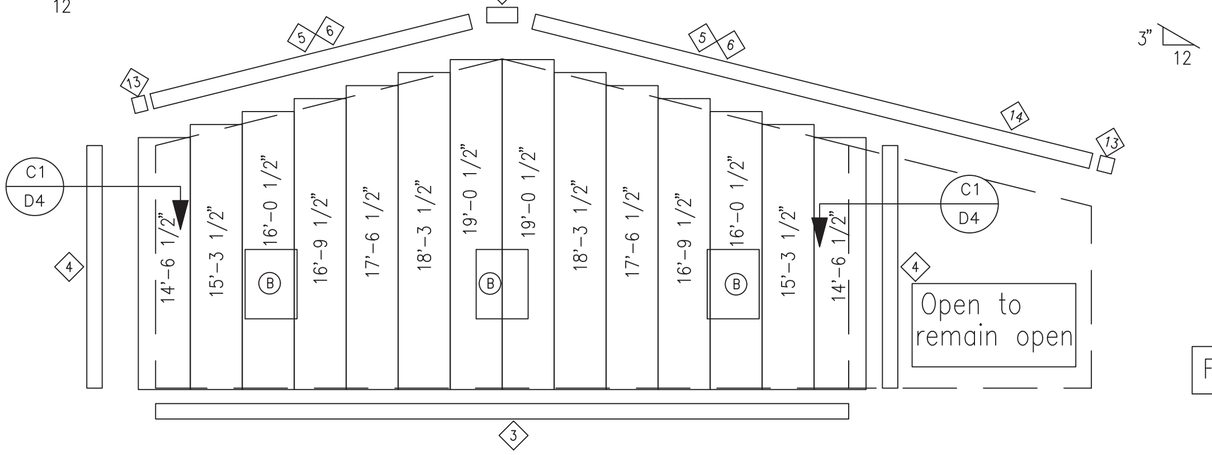
CONNECTION PLATES FRAME LINE 1	
□ID	MARK/PART
1	AD411
2	AK244
3	SC5



ENDWALL FRAMING: FRAME LINE 1



DETAIL-B



FIELD BEVEL CUT PANELS AS REQUIRED

FIELD CUT PANELS AS REQUIRED

ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX – SMP Colonial Red

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

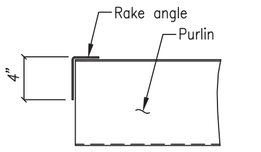
OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

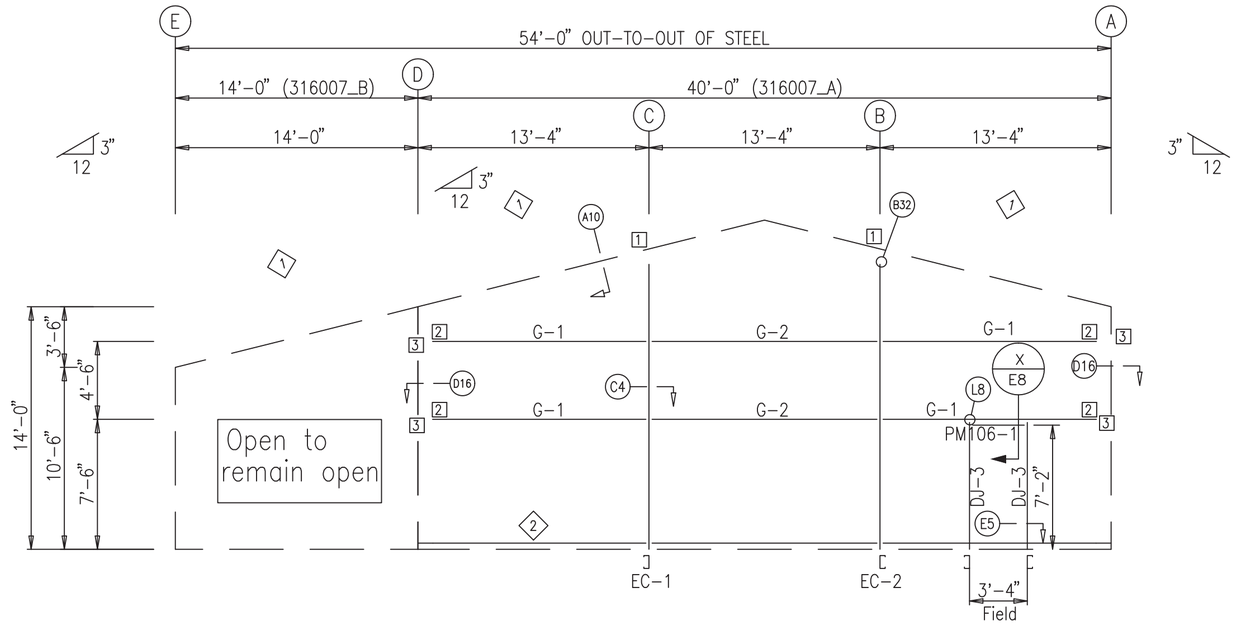
SHEET DESCRIPTION: ENDWALL FRAME & SHEETING ELEVATION					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: E3	ISSUE: P1

NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS

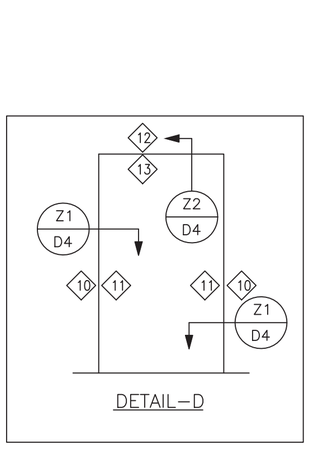
05/31/2023

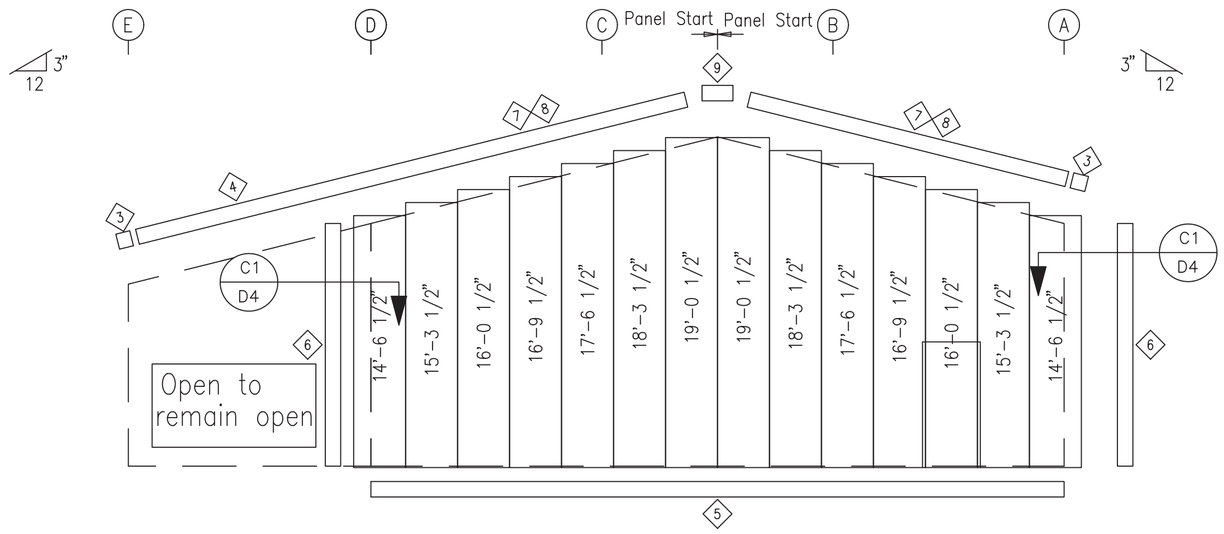


Detail at Rake Angle



ENDWALL FRAMING: FRAME LINE 4





FIELD BEVEL CUT PANELS AS REQUIRED

FIELD CUT PANELS AS REQUIRED

ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Ga. SSX – SMP Colonial Red

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	4	A325	5/8"	1 1/2"

TRIM TABLE FRAME LINE 4		
◇ID	MARK	LENGTH
3	SCB	
4	CF–102	15'–3"
5	BT–101	10'–3"
6	CT–102	14'–4"
7	RT–101	10'–3"
8	RT–101	15'–3"
9	SPB_	
10	MT–116B	7'–6"
11	JT–101	7'–6"
12	MT–116B	3'–8"
13	HT–101	3'–8"

MEMBER TABLE FRAME LINE 4	
MARK	PART
EC–1	10M35C14
EC–2	10M35C14
DJ–3	8M25C14
PM106–1	PM106
G–1	8X25Z16
G–2	8X25Z16

ANGLE TABLE FRAME LINE 4		
◇ID	MARK	LENGTH
1	RA2000	20'–0"
2	BB2000G	20'–0"

CONNECTION PLATES FRAME LINE 4	
□ID	MARK/PART
1	AD411
2	AK244
3	SC5

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 4'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot–rolled or built–up members must be pre–drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self–tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1–1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1–3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.
14. Sub–jambs for overhead doors, if required, is not furnished by Metal Building Provider

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: ENDWALL FRAME & SHEETING ELEVATION					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017–31607	DWG NO: E4	ISSUE: P1

NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS

05/31/2023

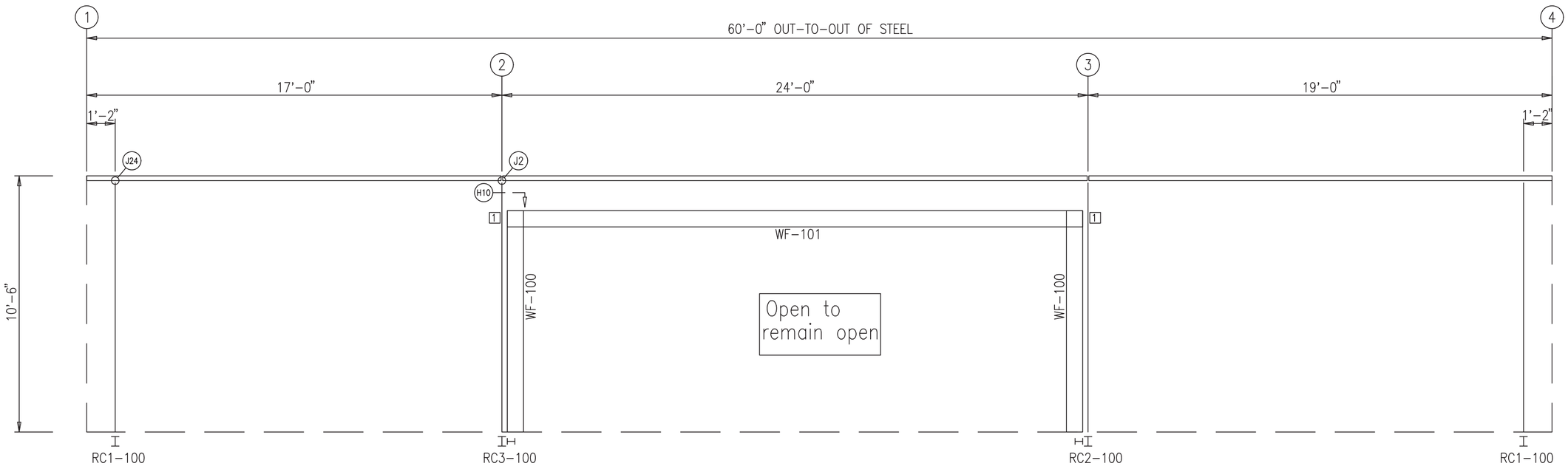
DOWNSPOUT LOCATIONS

BOLT TABLE FRAME LINE E				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-100 – WF-101	8	A325	3/4"	1 3/4"
WF-100 – RC3-100	8	A325	5/8"	1 3/4"
WF-100 – RC2-100	8	A325	5/8"	1 3/4"

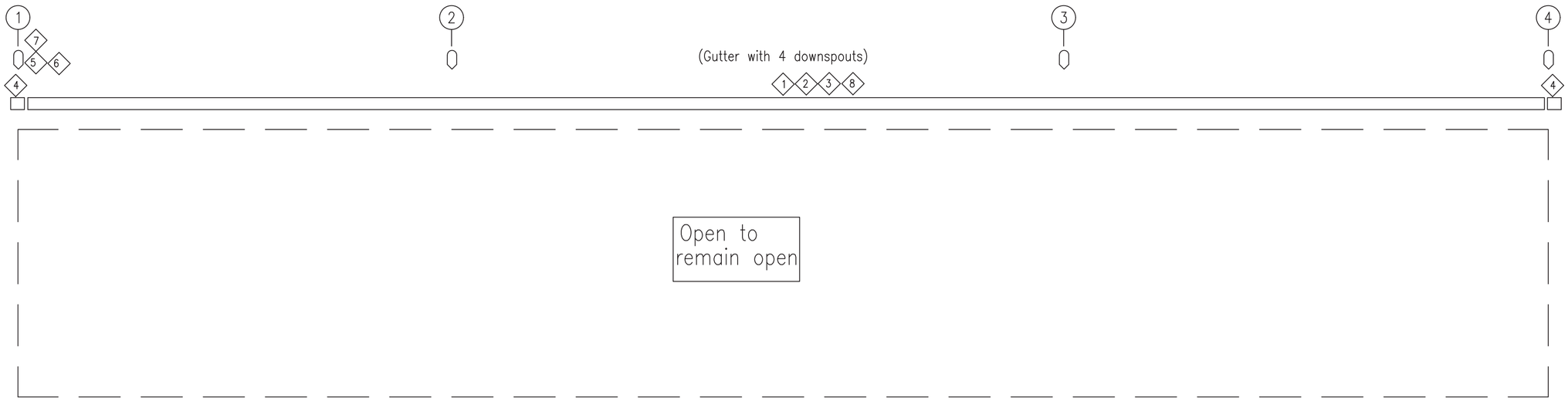
TRIM TABLE – THIS WALL ONLY FRAME LINE –E		
◇ID	PART	LENGTH
1	GC-103	10'-3"
2	GS-121	
3	GU-123	20'-3"
4	EC-123	
5	DS-101	11'-8"
6	DS-105	
7	DS-125	
8	CF-102	20'-3"

MEMBER TABLE FRAME LINE E	
MARK	PART
WF-100	W8541
WF-101	W8641

CONNECTION PLATES FRAME LINE E	
□ID	MARK/PART
1	AK508



SIDEWALL FRAMING: FRAME LINE E



SIDEWALL TRIM: FRAME LINE E

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: SIDEWALL FRAME & SHEETING ELEVATION					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: E5	ISSUE: P1

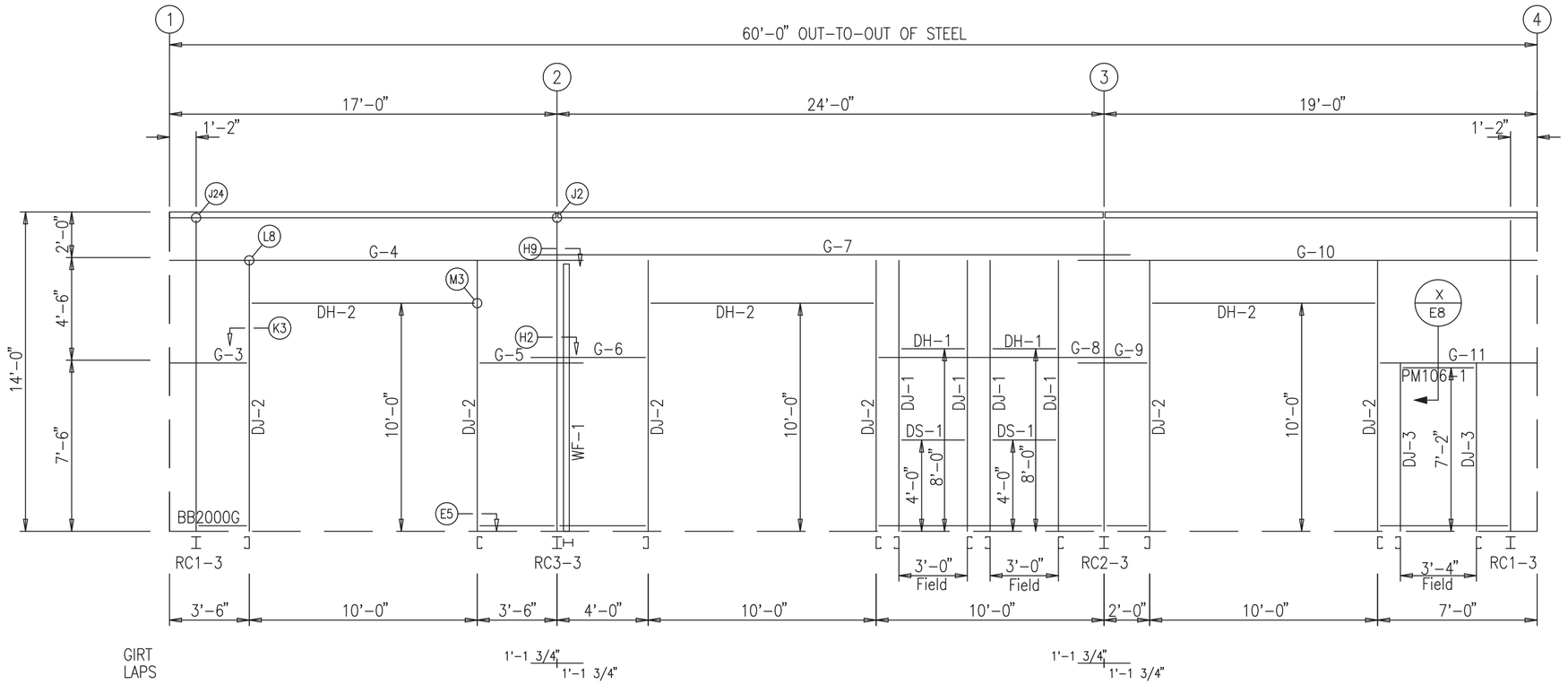
NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS

05/31/2023

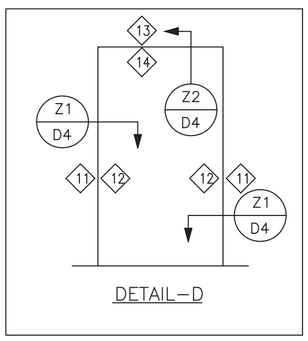
BOLT TABLE FRAME LINE D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – RC3-3	12	A325	5/8"	1 3/4"

TRIM TABLE – THIS WALL ONLY FRAME LINE –D		
◇ID	PART	LENGTH
1	ET-803	20'-3"
2	MT-116B	10'-4"
3	JT-101	10'-4"
5	HT-101	10'-4"
6	MT-116B	4'-4"
7	JT-101	4'-4"
8	MT-116B	3'-4"
9	HT-101	3'-4"
10	MT-114	3'-4"
11	MT-116B	7'-6"
12	JT-101	7'-6"
13	MT-116B	3'-8"
14	HT-101	3'-8"

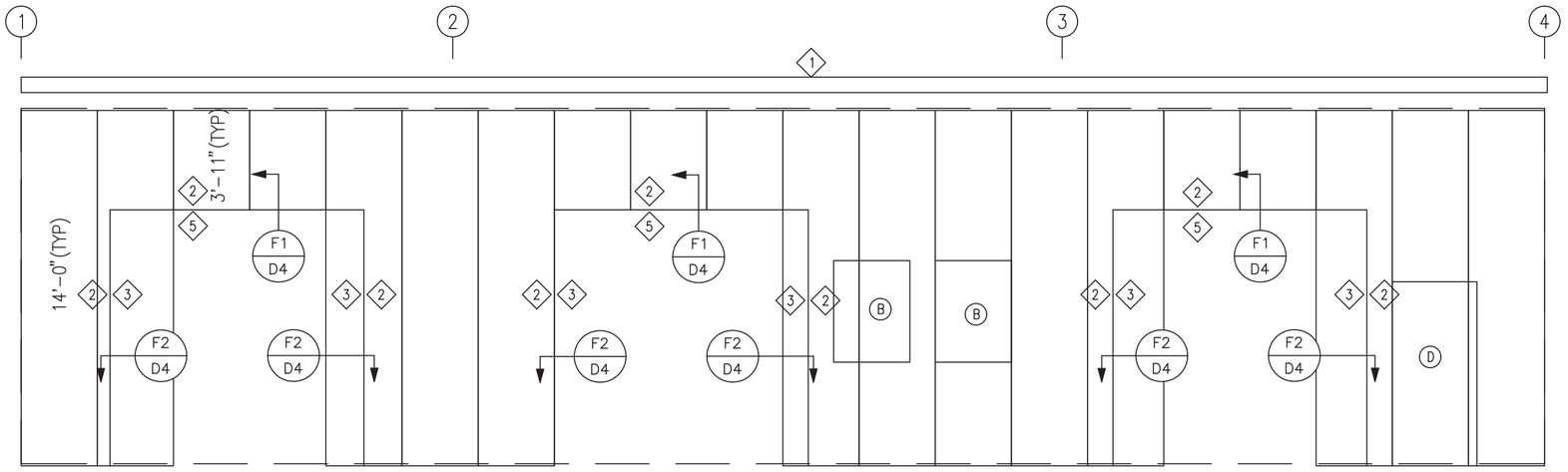
MEMBER TABLE FRAME LINE D	
MARK	PART
WF-1	W10541
DJ-1	8M25C14
DJ-2	8M35C14
DJ-3	8M25C14
DH-1	8M25C14
DH-2	8M25C14
PM106-1	PM106
DS-1	8M25C14
G-3	8X25Z16
G-4	8X25Z14
G-5	8X25Z16
G-6	8X25Z16
G-7	8X25Z14
G-8	8X25Z16
G-9	8X25Z16
G-10	8X25Z14
G-11	8X25Z16

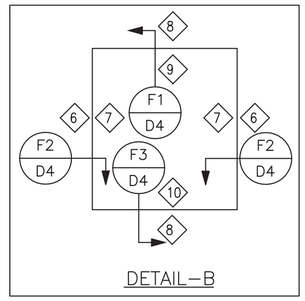


SIDEWALL FRAMING: FRAME LINE D



DETAIL–D





DETAIL–B

SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Ga. SSX – SMP Colonial Red

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
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GENERAL FRAMING NOTES

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3. Flange braces are marked by their length in decimal inches.
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7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
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Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

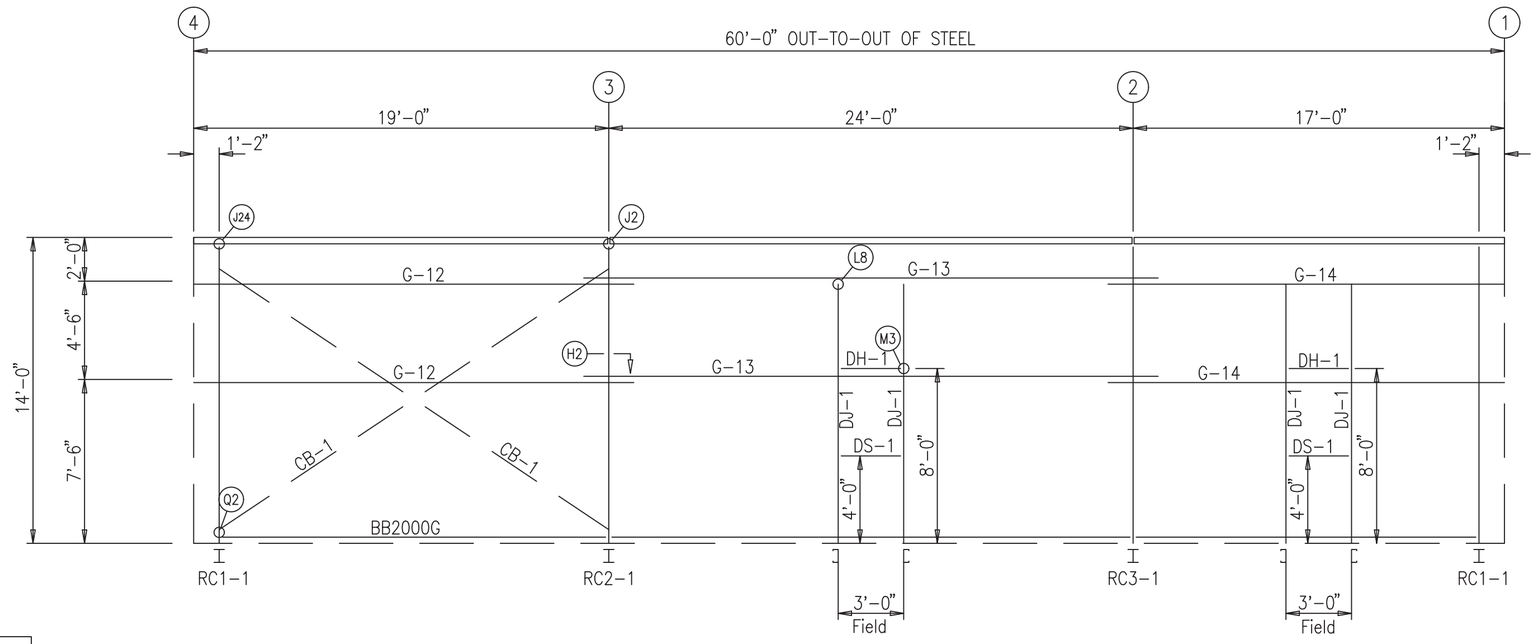
The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

SHEET DESCRIPTION: SIDEWALL FRAME & SHEETING ELEVATION					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: E6	ISSUE: P1

NORTH CAROLINA PROFESSIONAL ENGINEER J. WALTER LEWIS 9193

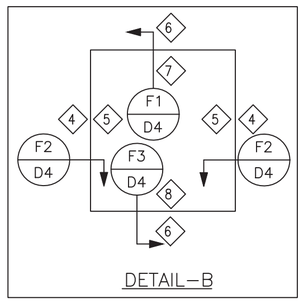
05/31/2023

DOWNSPOUT LOCATIONS



TRIM TABLE – THIS WALL ONLY FRAME LINE –A		
◇ID	PART	LENGTH
1	BT–101	10'–3"
2	GS–121	
3	GU–123	20'–3"
4	MT–116B	4'–4"
5	JT–101	4'–4"
6	MT–116B	3'–4"
7	HT–101	3'–4"
8	MT–114	3'–4"
9	GC–103	10'–3"
10	EC–123	
11	DS–101	13'–8"
12	DS–105	

MEMBER TABLE FRAME LINE A	
MARK	PART
DJ–1	8M25C14
DH–1	8M25C14
DS–1	8M25C14
G–12	8X25Z16
G–13	8X25Z16
G–14	8X25Z16
CB–1	0.25_CBL



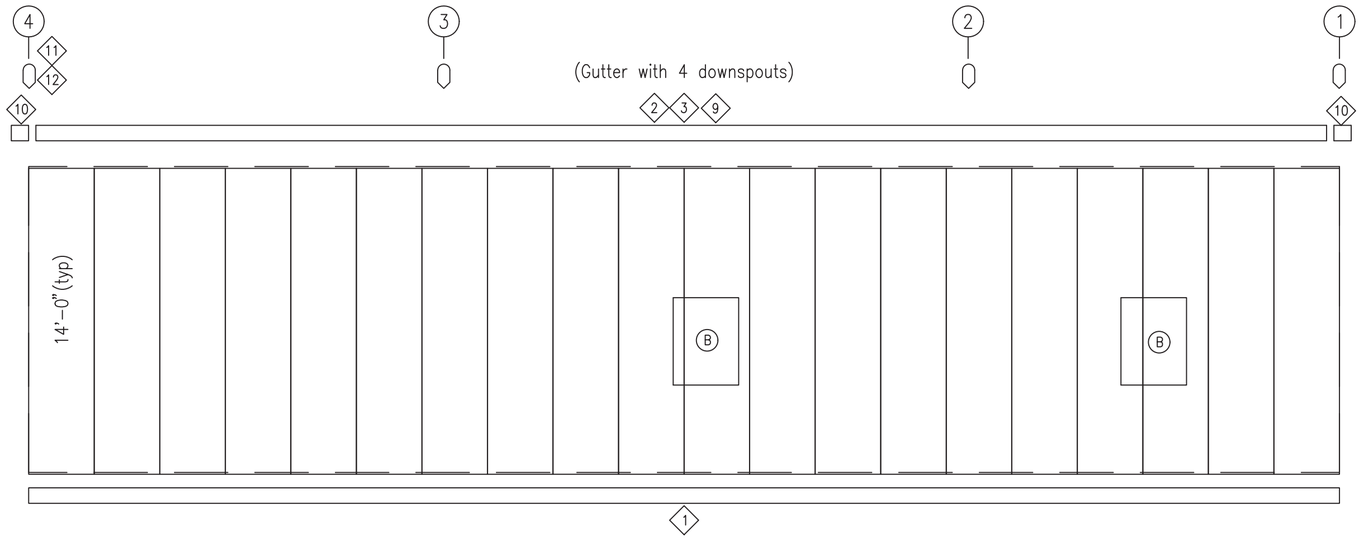
DETAIL–B

GIRT LAPS

1'–1 3/4" | 1'–1 3/4"

1'–1 3/4" | 1'–1 3/4"

SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. SSX – SMP Colonial Red

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 4'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot–rolled or built–up members must be pre–drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self–tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1–1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1–3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.
14. Sub–jambs for overhead doors, if required, is not furnished by Metal Building Provider

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: SIDEWALL FRAME & SHEETING ELEVATION					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017–31607	DWG NO: E7	ISSUE: P1

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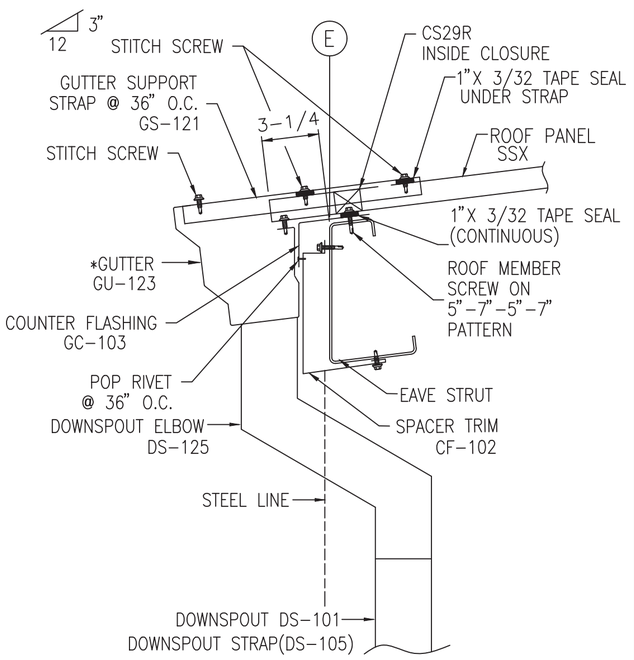
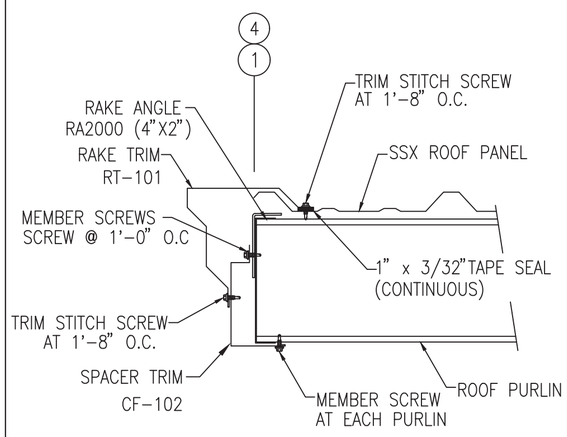
05/31/2023



*LAP GUTTER 2" w/ (2) CONTINUOUS BEADS OF CAULK

SECTION "A"



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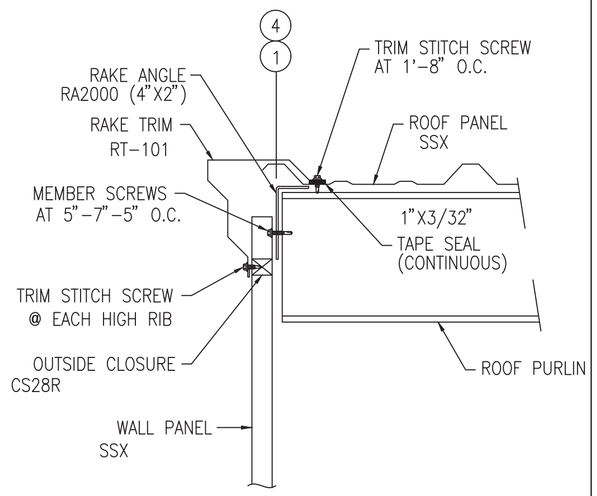

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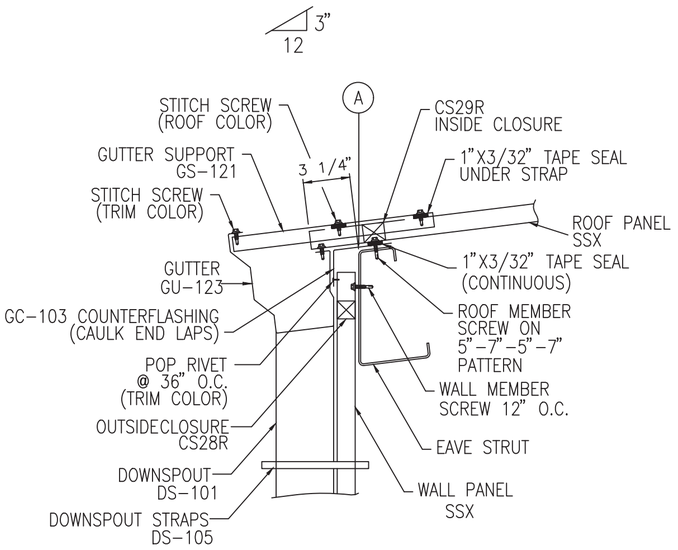
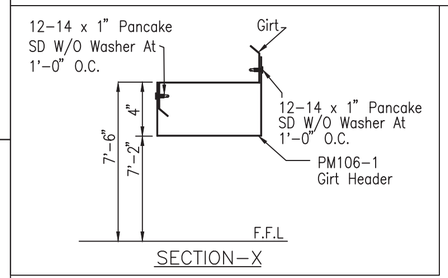
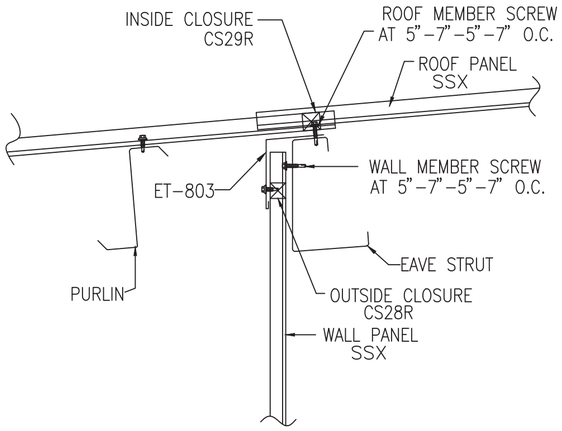


*LAP GUTTER 2" w/ (2) CONTINUOUS BEADS OF CAULK

SECTION "D"



SECTION-X



SECTION "E"

05/31/2023

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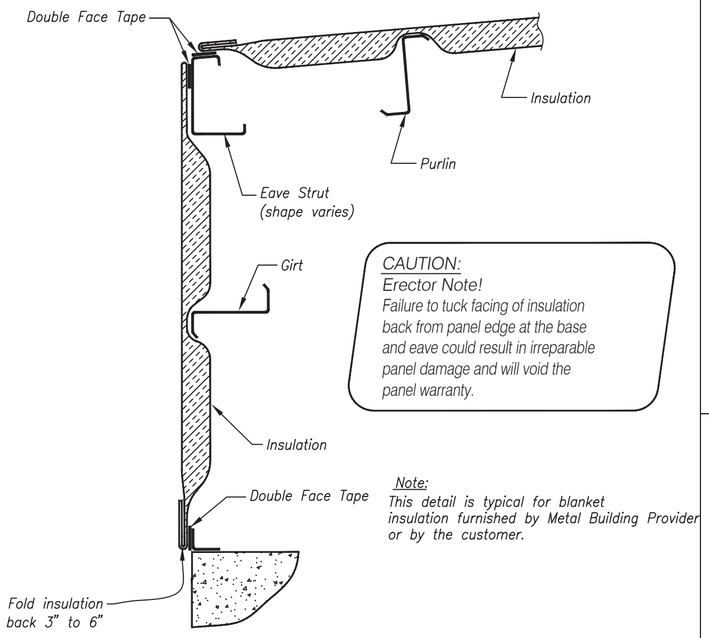
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: BUILDING SECTIONS	BLDG SIZE: VARIES
CUSTOMER: JOE CREECH	CUSTOMER LOCATION: BENSON, NC 27504
PROJECT REFERENCE: JOE CREECH	
JOBSITE LOCATION: BENSON, NC 27504	JOBSITE COUNTY: JOHNSTON

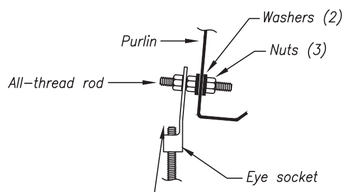
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
PND	PNC	05.29.23	MAH	11017-31607	E8	P1

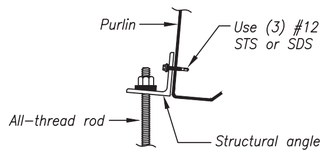


CAUTION:
Erector Note!
Failure to tuck facing of insulation back from panel edge at the base and eave could result in irreparable panel damage and will void the panel warranty.

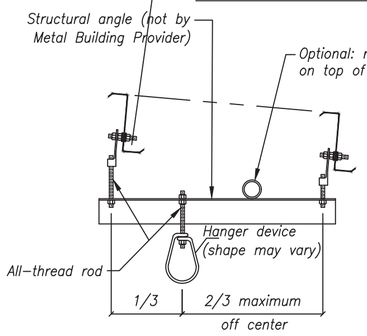
Note:
This detail is typical for blanket insulation furnished by Metal Building Provider or by the customer.

STANDARD DETAIL @ BLANKET INSULATION

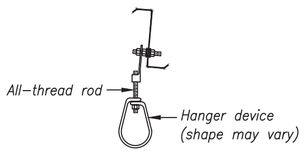




ACCEPTABLE CONNECTION METHODS

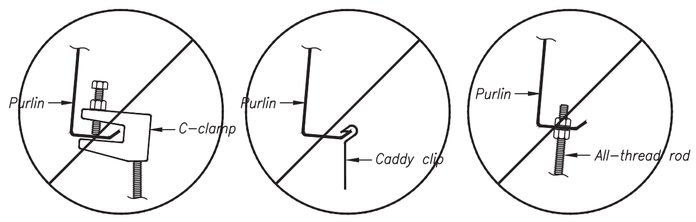


NOTE:
Metal Building Provider only provides the roof purlin. All other material and hardware is by others.



TRAPEZE HANGER ~ 4" MAX. PIPE

HANGER ~ 2-1/2" MAX. PIPE

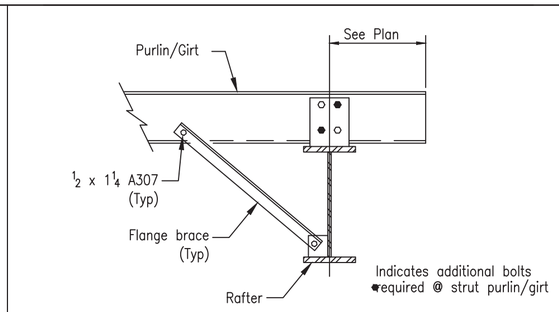


Flange C-Clamp is not an acceptable connection

Caddy clip attached to lip is not an acceptable connection

Connection through the flange is not acceptable

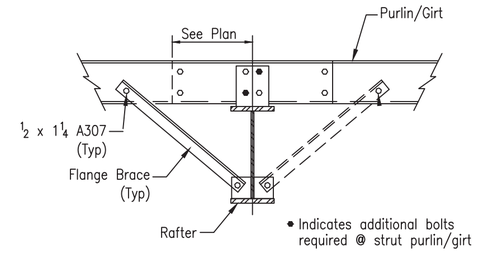
ACCEPTABLE CONNECTIONS FOR ALL COLLATERAL LOADS FOR HANGER ATTACHMENT



(6) 1/2" x 1-1/4 A-307 required at standard laps.
(8) 1/2" x 1-1/4 Required at strut members.
(A strut member is a purlin or girt at the bracing points. The struts continue across the length of the building.)
See E-drawing for exceptions to size and quantity of bolts.

END DETAIL

Note: Flange braces may be on one or both sides. See frame cross-section for locations.



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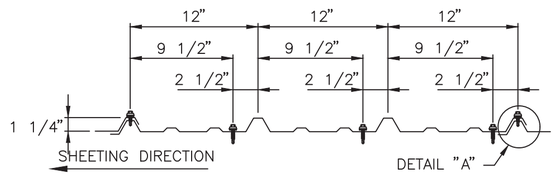
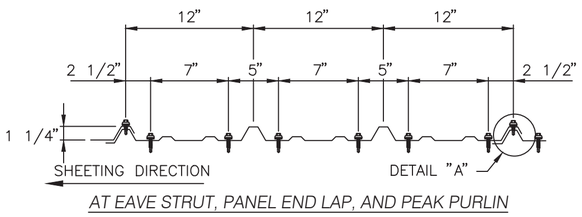
(A strut member is a purlin or girt at the bracing points. The struts continue across the length of the building.)
See E-drawing for exceptions to size and quantity of bolts.

LAP DETAIL

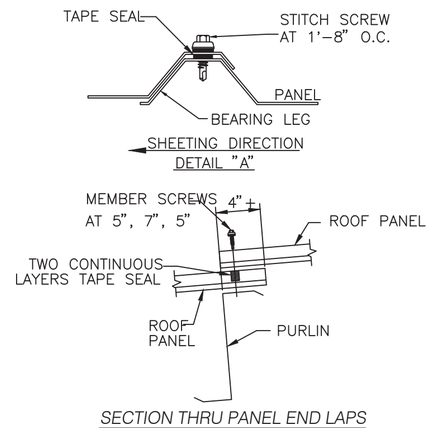


ALL ROOF MEMBERS EXCEPT AS NOTED BELOW



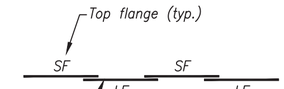
AT EAVE STRUT, PANEL END LAP, AND PEAK PURLIN

FASTENER LOCATION FOR "SUPER SPAN X" ROOF PANEL

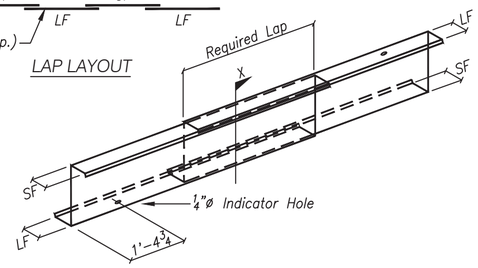


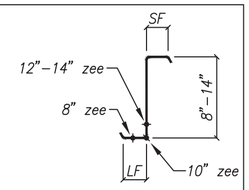
SECTION THRU PANEL END LAPS

Notes:
1. Rotate each zee as required to alternate between long and short flanges.
2. The 1/4"ø hole 1'-4 3/4 from each end is located nearest the long flange.
3. Lap connection bolts are omitted for clarity.

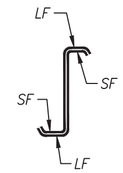


LAP LAYOUT





1/4" INDICATOR HOLE LOCATION

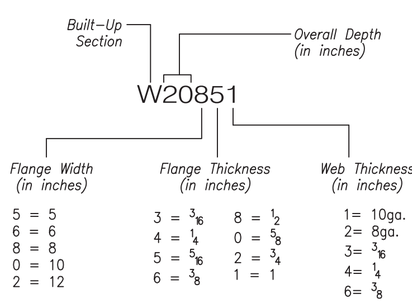


SECTION "X"

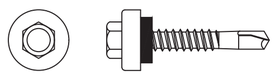
LF = Long Flange
SF = Short Flange

ZEE LAP DETAIL @ THE LONG AND SHORT FLANGES

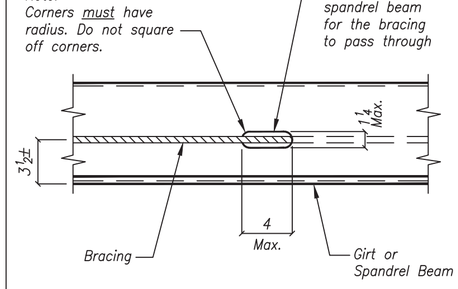
BUILT-UP SECTION LEGEND



Description: 12-14 x 1 1/4 Hex Head Undercut (#3 Long Pilot Point Long-Life Zinc Die Cast Head) (#12 x 1 1/4 Long-Life S.D.S.)
Seating Torque: 30 to 60 in-lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre-Drill: None

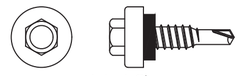


Actual Size

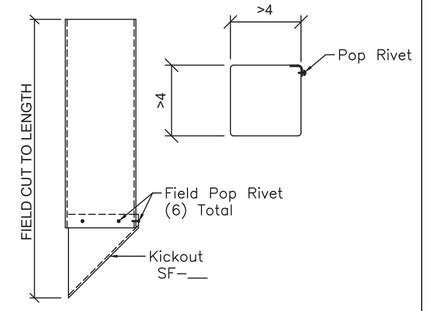


DETAIL @ FIELD LOCATED BRACING SLOT

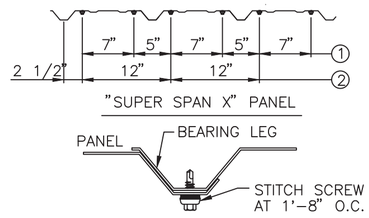
Description: 1/4-14 x 7/8 Hex Head Undercut (#1 Point Self-Drilling Lap Long-Life Zinc Die Cast Head) (#14 x 7/8 Long-Life Lap-Tek S.D.S.)
Seating Torque: 30 to 60 in-lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre-Drill: None



Actual Size



DOWNSPOUT w/SCUPPER
Field Install Scupper If Req'd



FASTENER LOCATION FOR WALL PANEL (SUPER SPAN X)

NORTH CAROLINA PROFESSIONAL ENGINEER 9193 J. WALTER LEWIS
05/31/2023

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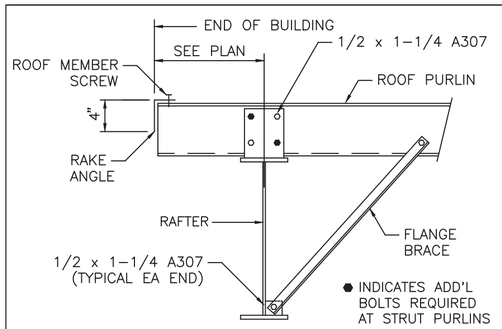
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

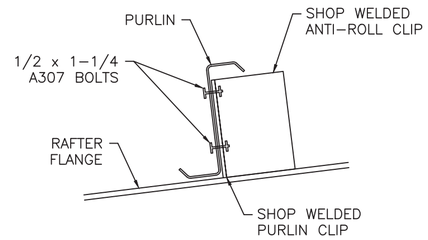
SHEET DESCRIPTION: STANDARD DETAIL PAGE					BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH					CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH						
JOBSITE LOCATION: BENSON, NC 27504					JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607	DWG NO: D1	ISSUE: P1



(2) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(4) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

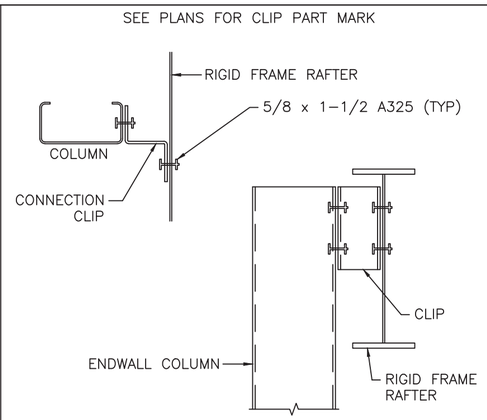
A STRUT PURLIN IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

A10 ROOF PURLIN CONNECTION AT MAIN FRAME ENDWALL

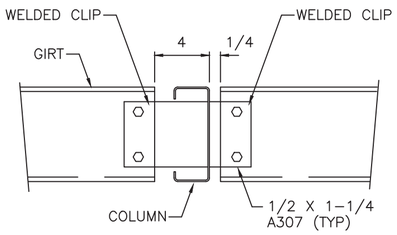


SOME PURLIN CLIPS WILL HAVE AN ADDITIONAL ANTI-ROLL CLIP ATTACHED. THE QUANTITY AND SPACING OF THESE CLIPS IS DETERMINED BY THE DESIGN FOR EACH SPECIFIC BUILDING.

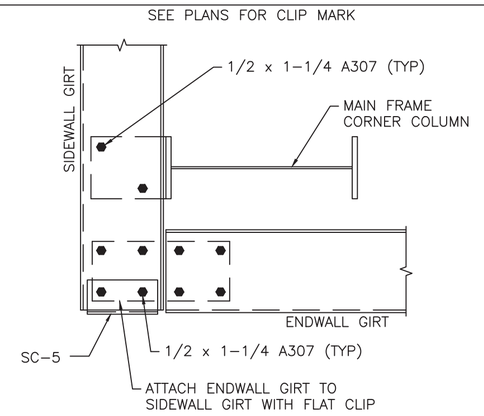
ANTI DETAIL AT ANTI-ROLL CLIP



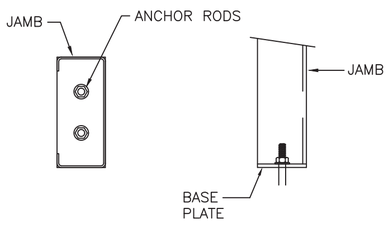
B32 CEE COLUMN TO RIGID FRAME RAFTER



C4 GIRT TO COLUMN

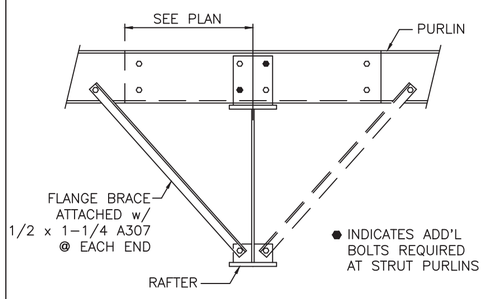


D16 GIRT CONNECTION AT CORNER SW & EW GIRTS AT SAME ELEVATION



– ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.

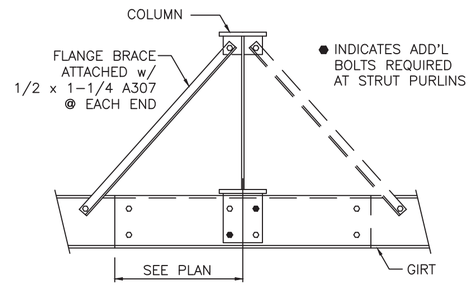
E5 BASE PLATE FOR DOOR JAMB



(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTIONS TO SIZE & QTY OF BOLTS.

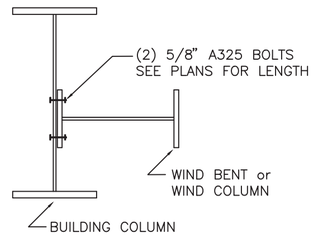
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



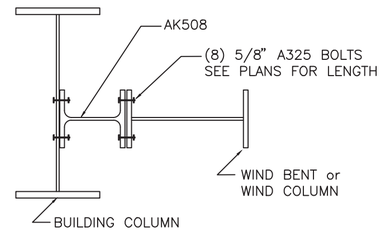
(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

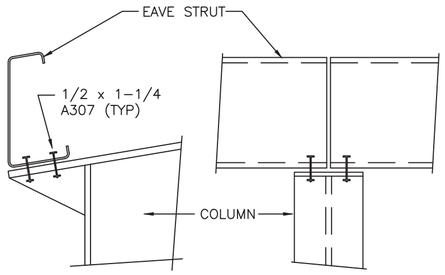
H2 WALL GIRT TO FRAME COLUMN



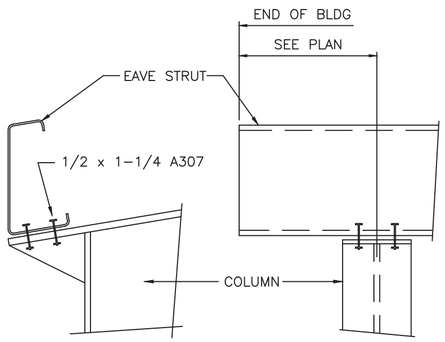
H9 WIND BENT OR WIND COLUMN TO BUILDING COLUMN



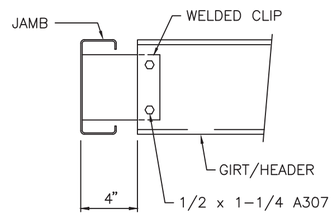
H10 WIND BENT OR WIND COLUMN TO BUILDING COLUMN



J2 EAVE STRUT TO RIGID FRAME



J24 EAVE STRUT TO RIGID FRAME



K3 WALL GIRT TO DOOR JAMB

NORTH CAROLINA PROFESSIONAL 9193 ENGINEER J. WALTER LEWIS
05/31/2023

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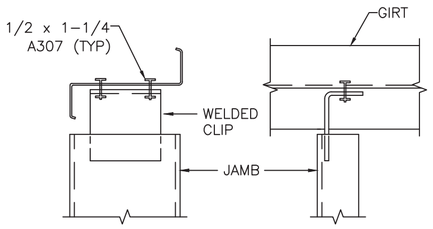
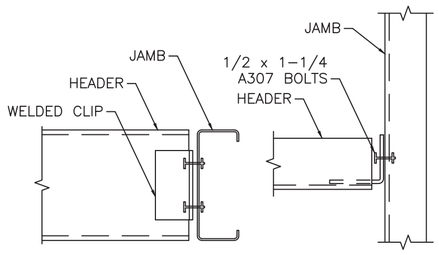
OLYMPIA STEEL BUILDINGS.

ISSUE	DATE	DESCRIPTION	BY	CHK
P1	05.29.23	FOR CONSTRUCTION PERMIT	PND	PNC

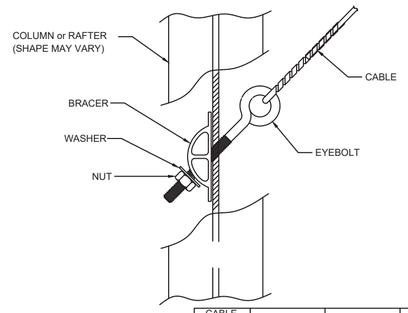
SHEET DESCRIPTION: STANDARD DETAIL PAGE						BLDG SIZE: VARIES	
CUSTOMER: JOE CREECH						CUSTOMER LOCATION: BENSON, NC 27504	
PROJECT REFERENCE: JOE CREECH							
JOBSITE LOCATION: BENSON, NC 27504						JOBSITE COUNTY: JOHNSTON	
DWN: PND	CHK: PNC	DATE: 05.29.23	ENG: MAH	JOB NO: 11017-31607		DWG NO: D2	ISSUE: P1



L8 DOOR JAMB TO WALL GIRT

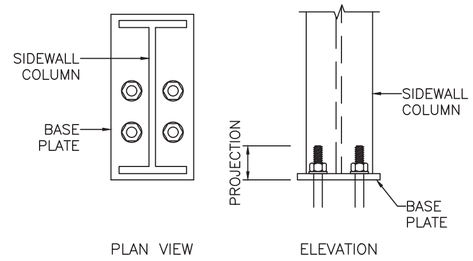


M3 HEADER TO CEE JAMB



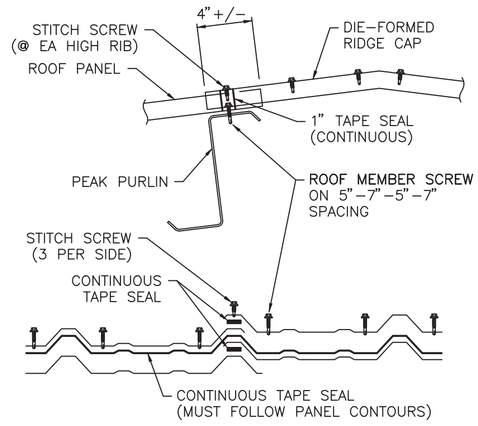
CABLE SIZE	BRACER	WASHER	NUT
1/4"	BRACER #1	F844 1/2"	A563 1/2"
5/16"	BRACER #1	F844 5/8"	A563 5/8"
3/8"	BRACER #2	F844 3/4"	A563 3/4"
1/2"	BRACER #2	F844 7/8"	A563 7/8"

Q2 DIAGONAL CABLE BRACING INSTALLATION



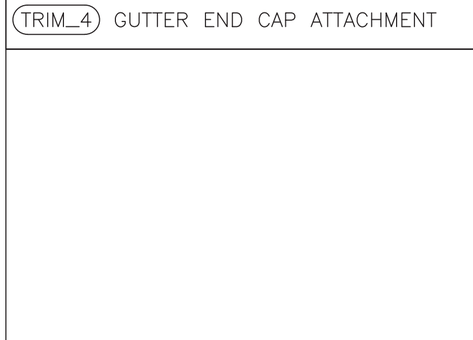
- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

R2 ANCHOR RODS AT SIDEWALL COLUMN



SCREW_8 TYPICAL DIE-FORMED RIDGECAP ENDLAP - SUPER SPAN X ROOF

SEE PLANS FOR PART MARKS

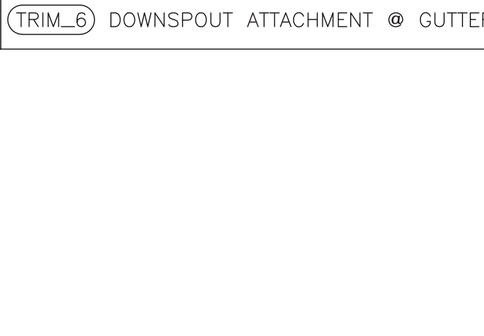


MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL AND THERE ARE NO BREAKS IN THE BEADS OF CAULK

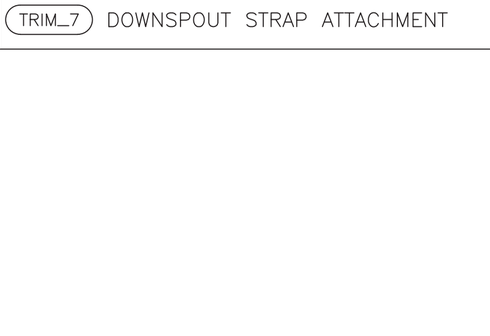
TRIM_4 GUTTER END CAP ATTACHMENT

MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL

TRIM_5 CORNER BOX ATTACHMENT

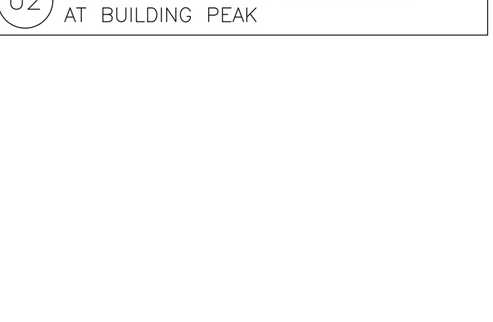


TRIM_6 DOWNSPOUT ATTACHMENT @ GUTTER



ATTACH ONE DS-105 STRAP AT THE BASE OF THE DOWNSPOUT THEN APPROXIMATELY 5'-0" CENTERS (8'-0" MAX.)

TRIM_7 DOWNSPOUT STRAP ATTACHMENT



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK

NORTH CAROLINA PROFESSIONAL ENGINEER J. WALTER LEWIS 9193

05/31/2023

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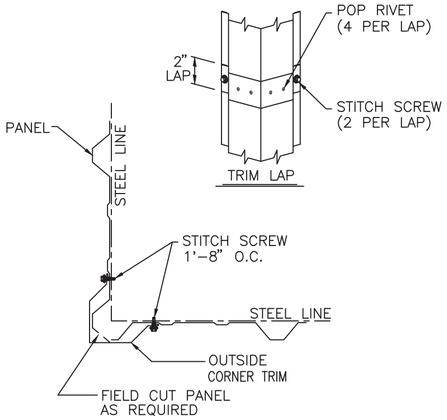
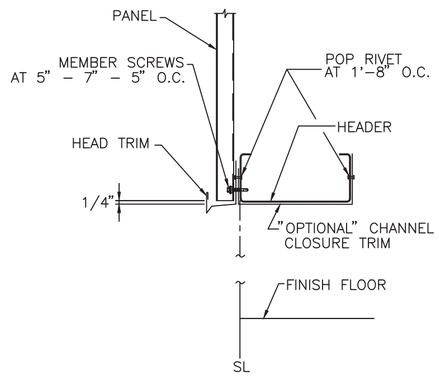
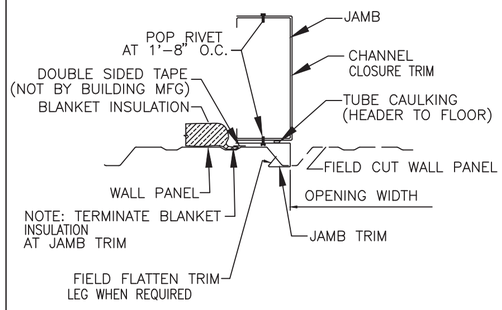


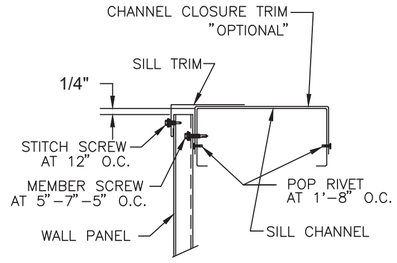
C1 CORNER TRIM INSTALLATION



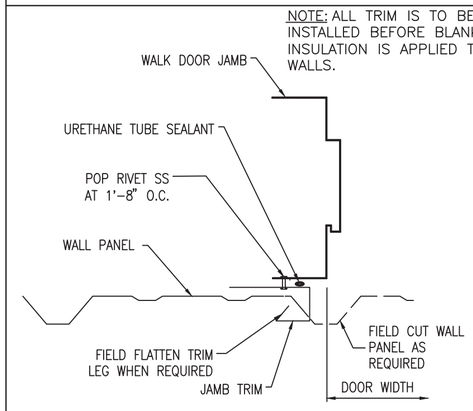
F1 HEAD TRIM INSTALLATION (SUPER SPAN X)



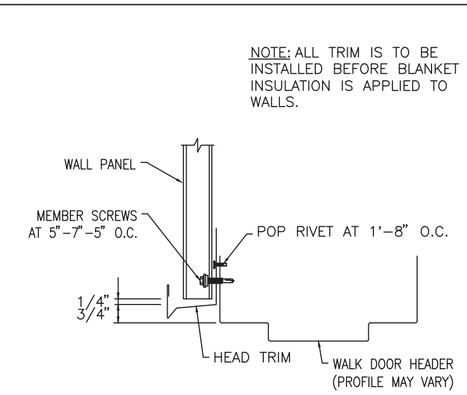
F2 JAMB TRIM INSTALLATION (SUPER SPAN X)



F3 SILL TRIM INSTALLATION (SUPER SPAN X)



Z1 WALK DOOR STD JAMB TRIM (SUPER SPAN X)



Z2 WALK DOOR STD HEAD TRIM (SUPER SPAN X)

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The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

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NORTH CAROLINA PROFESSIONAL ENGINEER 9193 J. WALTER LEWIS

05/31/2023