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STRUCTURAL NOTES

GENERAL

1. DESIGN CODE DATA

2018 INTERNATIONAL BUILDING CODE ASCE 7-16: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. AISC 360-05: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS

2. DESIGN LOADS

ROOF DEAD LOADS: 10 PSF ROOF LIVE LOADS: 20 PSF

3. WIND DESIGN CRITERIA

WIND LOAD: 150 MPH 3 SECOND GUST NOMINAL DESIGN WIND SPEED: 116 MPH INTERNAL PRESSURE COEFFICIENT: 0.0 (OPEN) RISK CATEGORY 1 BUILDING (THIS STRUCTURE IS DESIGNED AS AN UNINHABITABLE STRUCTURE AND SHALL NOT BE CONVERTED TO A HABITABLE WITHOUT PRIOR APPROVAL FROM ENGINEER OF RECORD) EXPOSURE CATEGORY B BASE VELOCITY PRESSURE: 34,3 PSF

4. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON OR EXISTING STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.

	COMPONE	NTS AND CLADDIN	G	
ROOFS	DESIGN PRESSURE (ULT)		DESIGN PRESSURE (ASD)	
TRIBUTARY AREA 10 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
ZONE 1 max	33.4	30.7	20.0	18.4
ZONE 2 max	51.6	47.6	30.9	28.5
ZONE 3 max	66.8	61.5	40.1	36.9
TRIBUTARY AREA 100 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
ZONE 1 max	33.4	30,7	20.0	18.4
ZONE 2 max	33.4	47.6	20.0	28.5
ZONE 3 max	33.4	47.6	20.0	28.5
WALLS	DESIGN PRESSURE		DESIGN PRESSURE	
TRIBUTARY AREA 10 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
ZONE 4	34.3	37.7	20.6	22.6
ZONE 5	34.3	48.0	20.6	28.8
TRIBUTARY AREA 100 SF	POSITIVE (PSF)	NEGATIVE (PSF)	POSITIVE (PSF)	NEGATIVE (PSF)
ZONE 4	28,2	31.7	16.9	19.0
ZONE 5	28.2	35.9	16.9	21.5

BUILDING RISK CATEGORY 1 - AGRICULTURAL STORAGE STRUCTURES INTENDED ONLY FOR INCIDENTAL HUMAN OCCUPANCY, OR A DETACHED ONE & TWO FAMILY DWELLING ASSIGNED SDC A, B, OR C, OR WITH A Ss, IS LESS THAN 0.4g.

5. SNOW LOAD (S)

19.00psf = Roof Snow Load 25psf = Ground Snow Load

Thermal Factor = 1.00 Snow Exposure Factor = 1.00 Snow Importance Factor = 0.80

6. SEISMIC LOAD (E)

Equivalent Lateral Force Procedure

0.35 = 0.2s Short Period Spectral Response Acceleration S(s)

0.12 = 1.0s Spectral Response Acceleration S(1)

Site Classification =D

Seismic Importance Factor =1.00

Seismic Design Category = C

Seismic Design Short Period Acceleration, Sds = 0.355

Seismic Design 1 Sec Period Acceleration, SD1 = 1.189

Cantilever Columns for Timber Frames, R=1.5 Transverse & Longitudinal,

Analysis Procedure: Under Section 1613 of the IBC 2018,

Earthquake Loads are not required

to be considered for a Risk Category 1 Structure.

7.THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.

8.THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF CONSTRUCTION AND ALL JOB SITE SAFETY.

9.VERIFY ALL DIMENSIONS PRIOR TO THE START OF CONSTRUCTION - DO NOT SCALE DRAWINGS.

FOOTING AND FOUNDATION WALL

3,000 PSI @ 28 DAYS

SLAB ON GRADE

3,000 PSI @ 28 DAYS

ALL OTHER CIP CONCRETE CONCRETE REINFORCING STEEL

3,000 PSI @ 28 DAYS 60 KSI, ASTM A615

11. FOUNDATION

- 1. ALLOWABLE SOIL BEARING CAPACITY = 2,000 PSF FOR FOOTINGS (PRESUMPTIVE)
- 2. GRADE AREAS IN ACCORDANCE WITH ELEVATIONS AND GRADES SHOWN ON THE SITE DRAWINGS AND AS REQUIRED FOR DRAINAGE.
- 3. ALL SLAB ON GRADE AREAS SHALL BE PROOF ROLLED. ALL SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH COMPACTABLE FILL.
- 4. SLAB ON GRADE TO BE CONSTRUCTED ON A MINIMUM OF 6" OF COMPACTED GRANULAR FILL
- 5. ALL FILL MATERIAL USED IN GRADING OPERATIONS SHALL CONSIST OF EARTH, WHICH IS FREE OF DEBRIS. BOULDERS OR ORGANIC MATERIAL. FILL SHALL BE PLACED IN MAXIMUM OF 12" LIFTS AND COMPACTED TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY.
- 6. ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR COMPACTED FILL HAVING A MINIMUM ALLOWABLE BEARING CAPACITY AS INDICATED ABOVE.
- 7. THE ENGINEER SHALL BE NOTIFIED IF ACTUAL FIELD CONDITIONS DO NOT MEET BEARING REQUIREMENTS OR, IF QUESTIONABLE SOIL CONDITIONS ARE DISCOVERED INCLUDING BUT NOT LIMITED TO PEAT AND OTHER HIGH ORGANIC SOILS.
- 8. ANY FOUNDATION UNDER THE BASE FLOOD ELEVATION SHALL COMPLY W/ R322.2.2 OF THE FLORIDA BUILDING CODE & WILL PROVIDE FLOOD VENTS TO MEET THESE REQUIREMENTS.

ANGLES, PLATES, AND CHANNELS 36 KSI, ASTM A36

SQUARE AND RECTANGULAR HSS 46 KSI, ASTM A500 GRADE B.

BOLTS @ RIDGE (PEAK):

(2) GRADE A5, 5/8" W/ A325 BOLTS

BOLTS @ TRUSS TO POST CONN.

(4) FLAT & (2) LOCK WASHERS (2) 5/8" X 12" CARRIAGE BOLTS &

(4) 2"x2"x1/8" FLAT & LOCK **WASHERS**

WELDING ELECTRODES

E70XX

(ALL WELDS ARE FULL PENETRATION WELDS U.N.O.)

13. WOOD

SYP NO. 2 OR BETTER 2x8 AND SMALLER MINIMUM DESIGN VALUES

> 1,500 PSI Fb

Ft 825 PSI

Fv 175 PSI

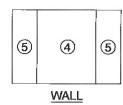
Fc⊥ 565 PSI

FcII 1,600 PSI

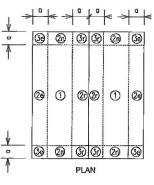
Ε 1,600,000 PSI

Emin 580,000 PSI

WIND ZONES



*3'-0" EDGE DISTANCE



Gable and Flat Roofs 7° < Ø < 45°

6x6, 8X8 OR 10X10 WD POST TREATED SOUTHERN YELLOW PINE (SYP) MINIMUM DESIGN VALUES

1,350 PSI Fb

E 1,500,000 PSI

14. ROOF & WALL SHEATHING

ROOF METAL PANELS SHALL BE 26 GA. x 3'-0" WIDE w/ 5 RIBS (RECOMMENDED) OR 29 GA, x 3'-0" WIDE w/ 5 RIBS





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12'-50' HSS Tube Truss Drawings for: Blackwater Truss Systems 8736 Hwy 87 N. Milton, FL 32570	REV. DESCRIPTION		
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GENERAL NOTES

SHEET TITLE

MKW

8/17/2022

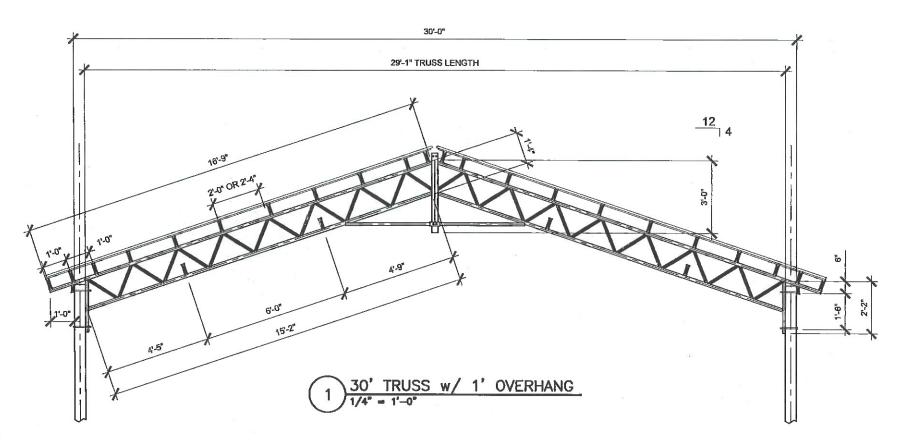
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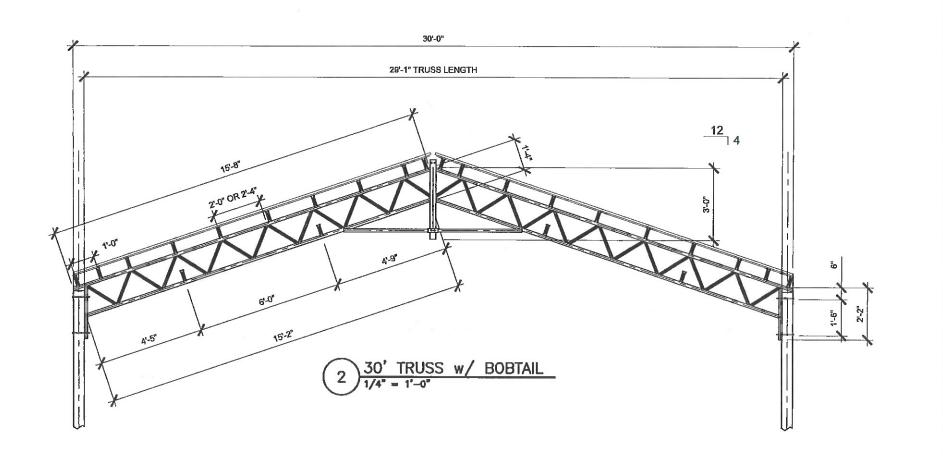
TOP CHORD SHALL BE HSS1-1/2x1-1/2x14 GA.

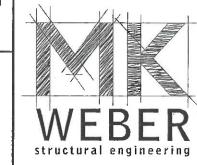
BOTTOM CHORD SHALL BE HSS1-1/2x1-1/2x14 GA.

DIAGONAL WEB MEMBERS SHALL BE HSS1x1x14 GA.

VERTICAL ENDS SHALL BE L1-1/2x1-1/2x3/16

RIDGE COLLAR-TIE SHALL BE L1-1/2x1-1/2x11 GA.





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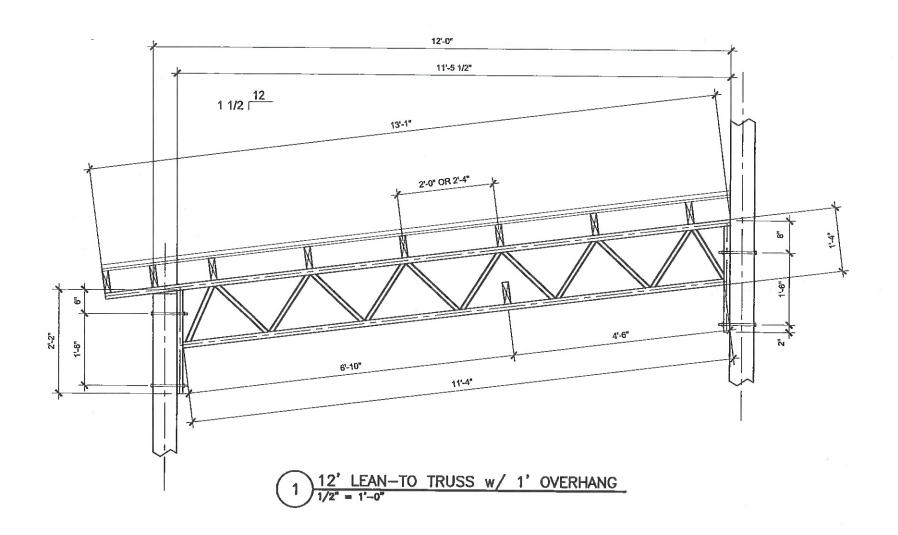
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JOB NUMBER:	22007-40
DRAWN BY:	DAW
CHECKED BY:	MKW
PLOT DATE:	8/17/2022
SHEET	TITLE

30' TRUSS PLAN

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1" FROM ENDS AND/OR CENTERED RELATIVE TO ENDS.

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BOTTOM CHORD SHALL BE HSS1-1/2x1-1/2x14GA. TUBE

DIAGONAL WEB MEMBERS SHALL BE HSS1x1x14GA. TUBE.

VERTICAL ENDS SHALL BE L1-1/2x1-1/2x3/16

NOTE:
MAXIMUM STEP IN ROOF IS 14"
BETWEEN LEAN-TO TRUSS &
MAIN STRUCTURE DUE TO
SNOW DRIFT REQUIREMENTS

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