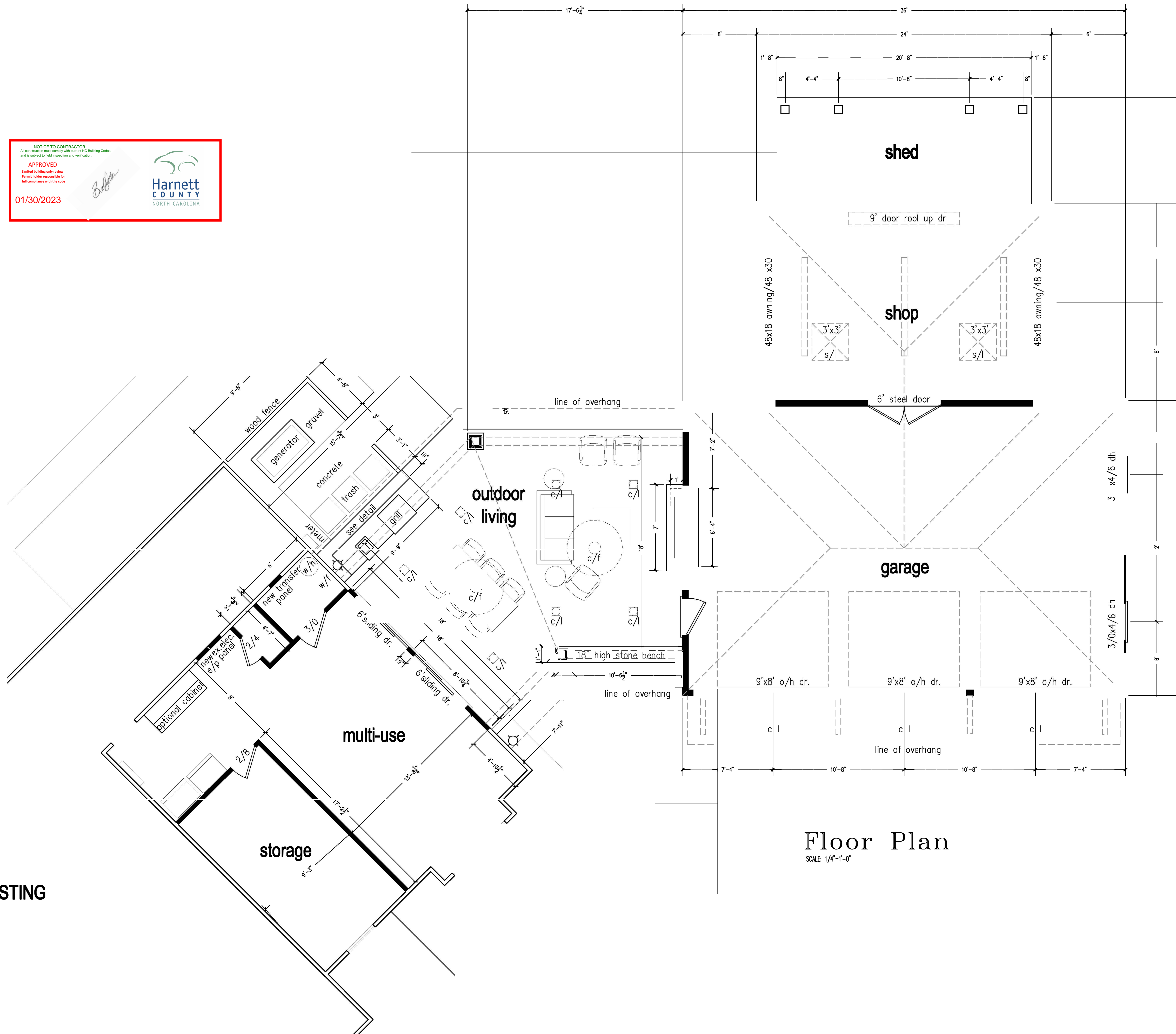
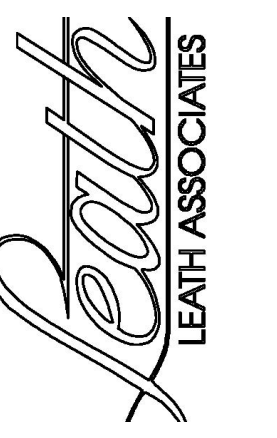




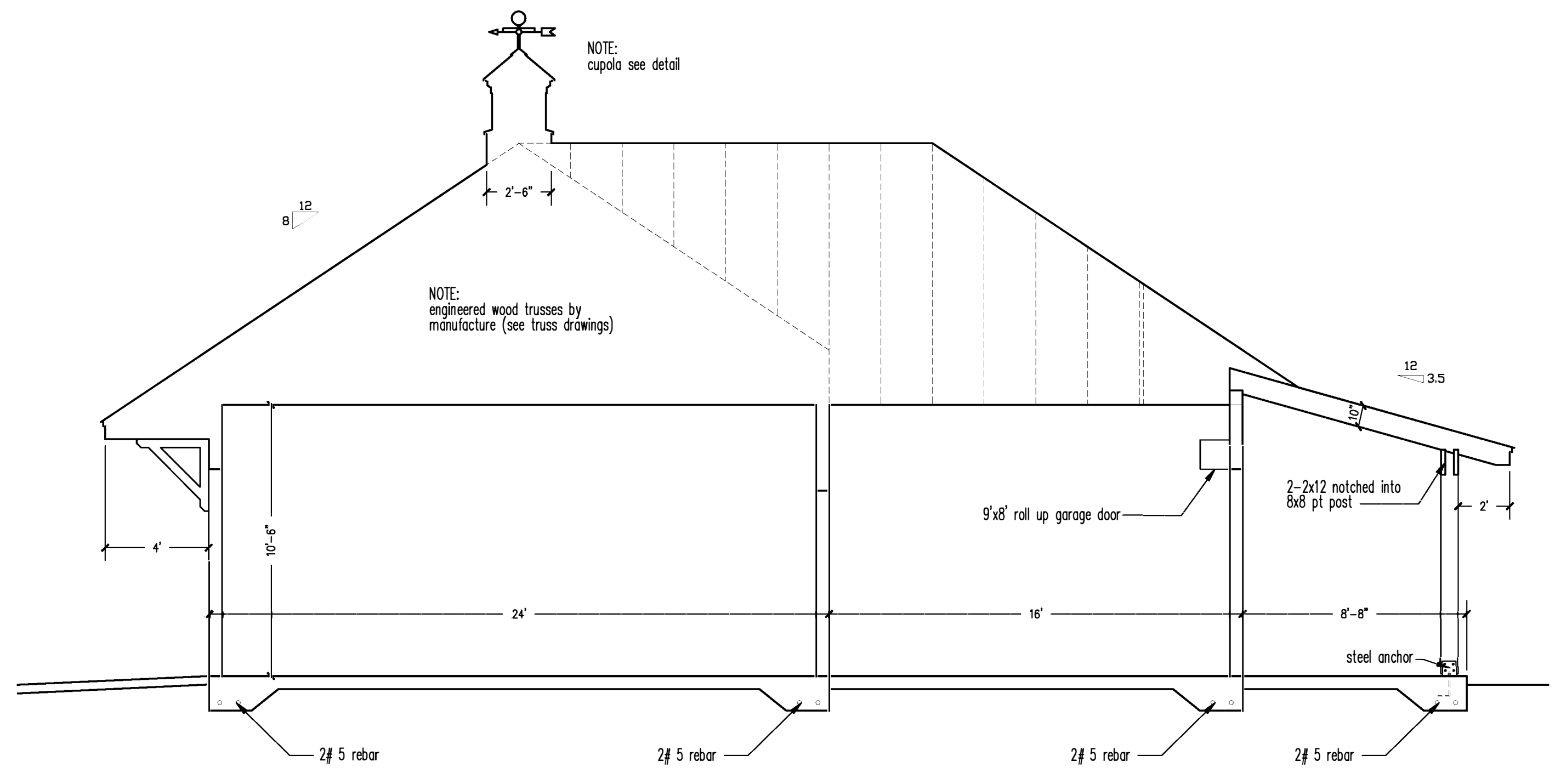
EXISTING



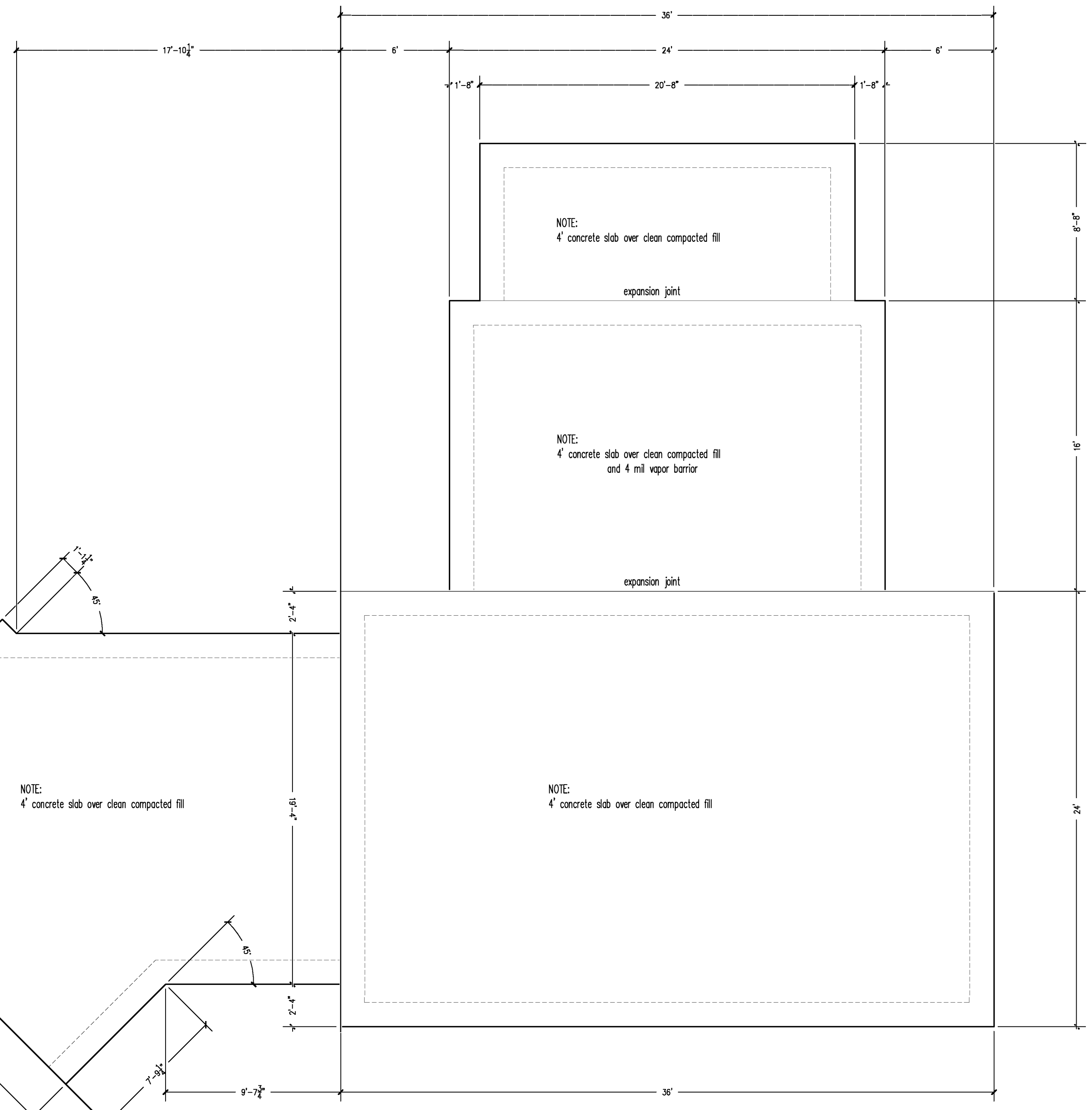
Floor Plan
SCALE: 1/4"=1'-0"



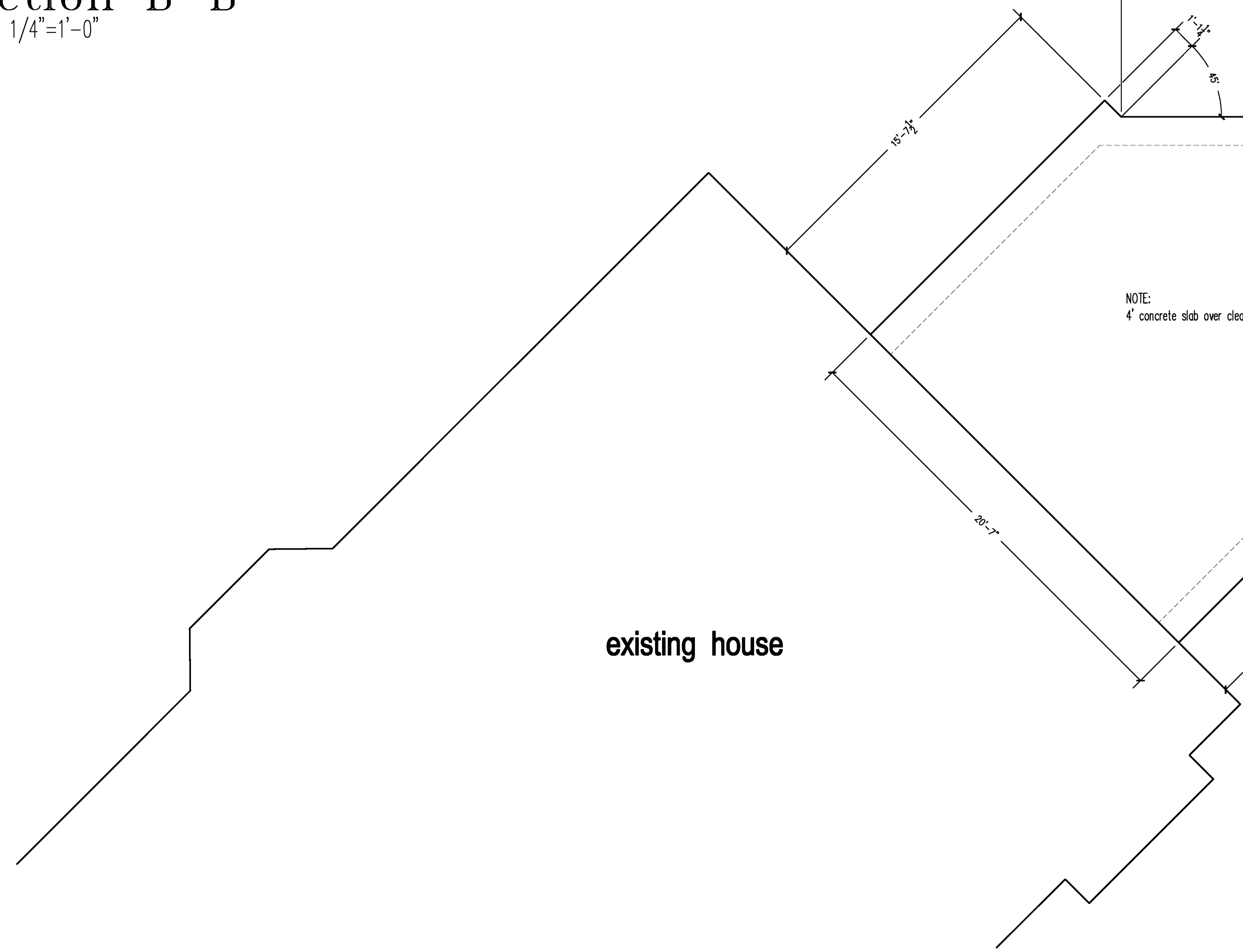
PROJECT NO.	HAMLET
DRAWN BY:	RWL
DATE:	12/17/21
REVISIONS:	



Section B-B
SCALE: 1/4"=1'-0"

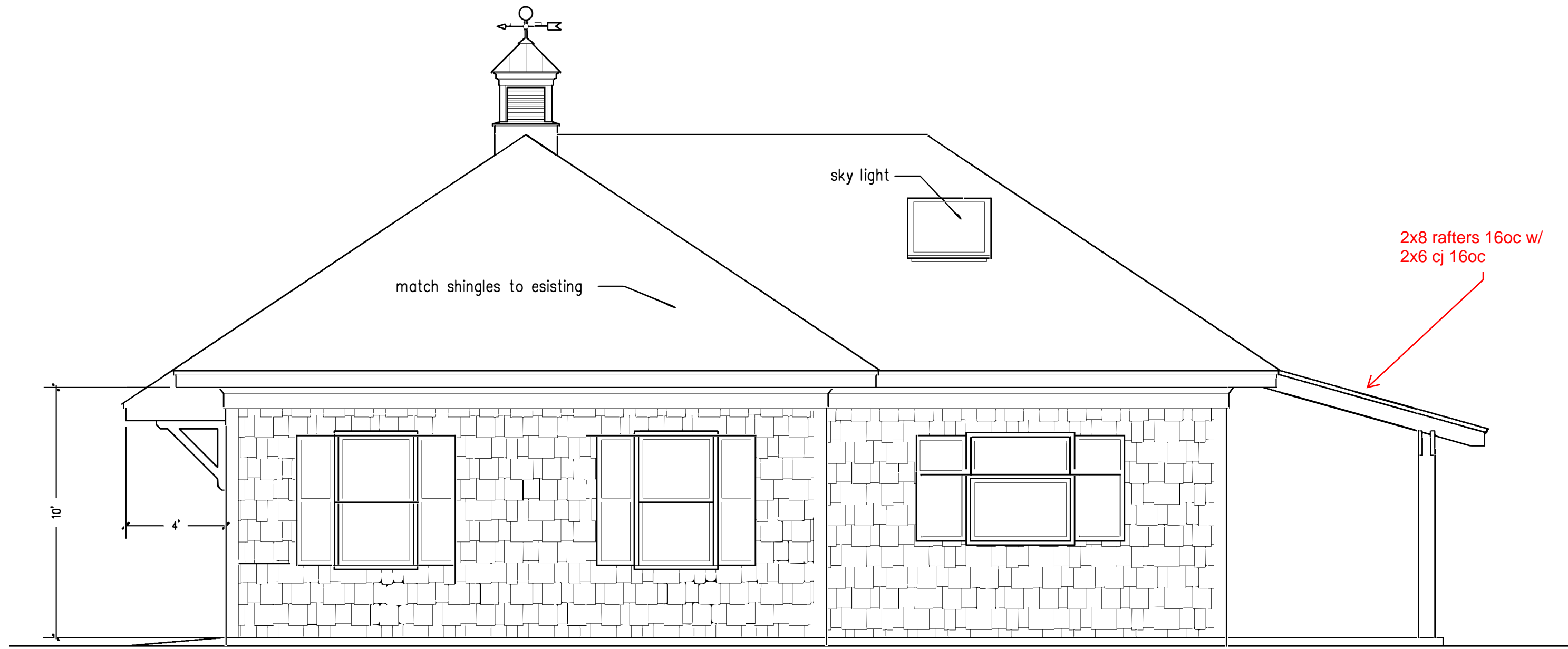


Foundation Plan
SCALE: 1/4"=1'-0"

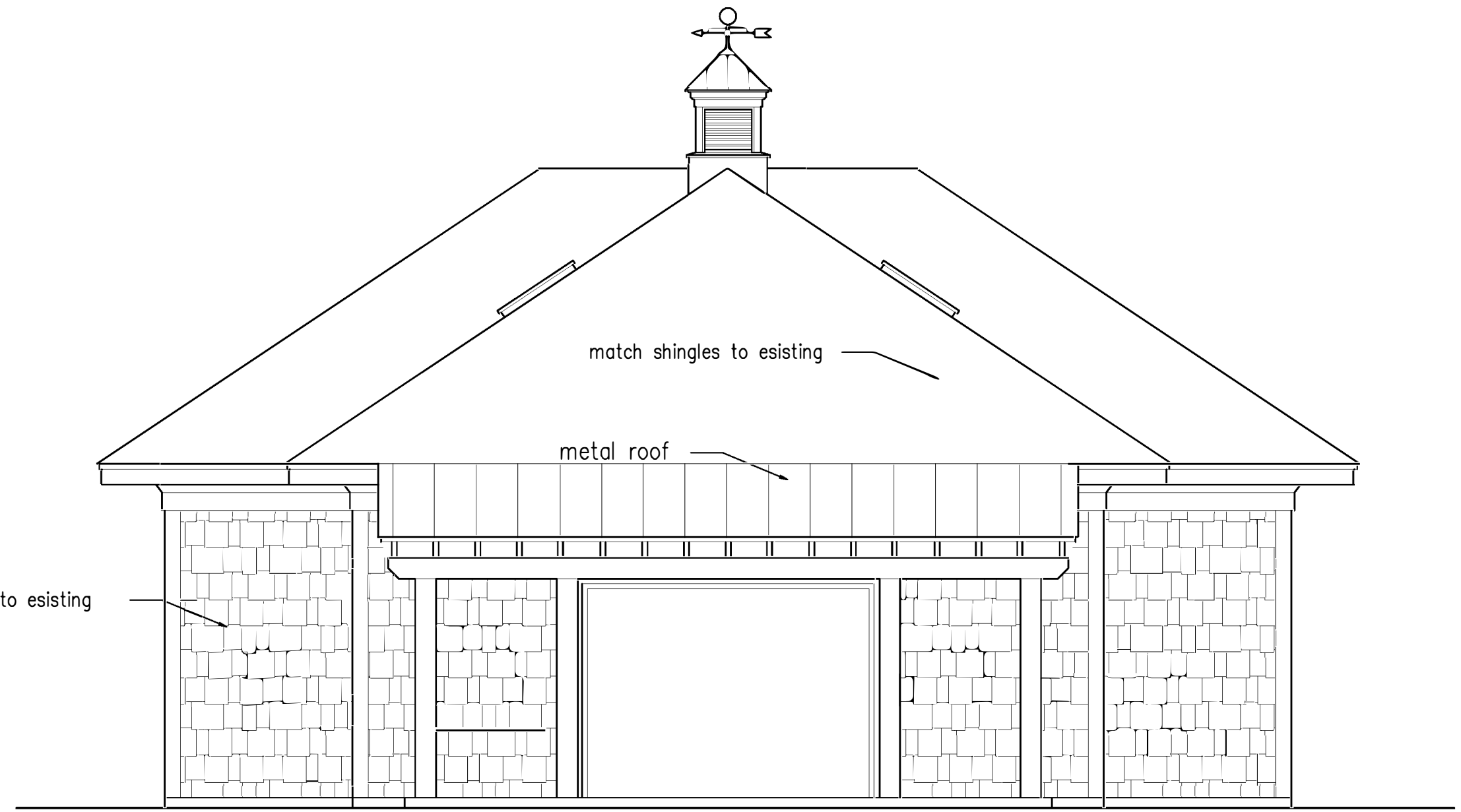


existing house

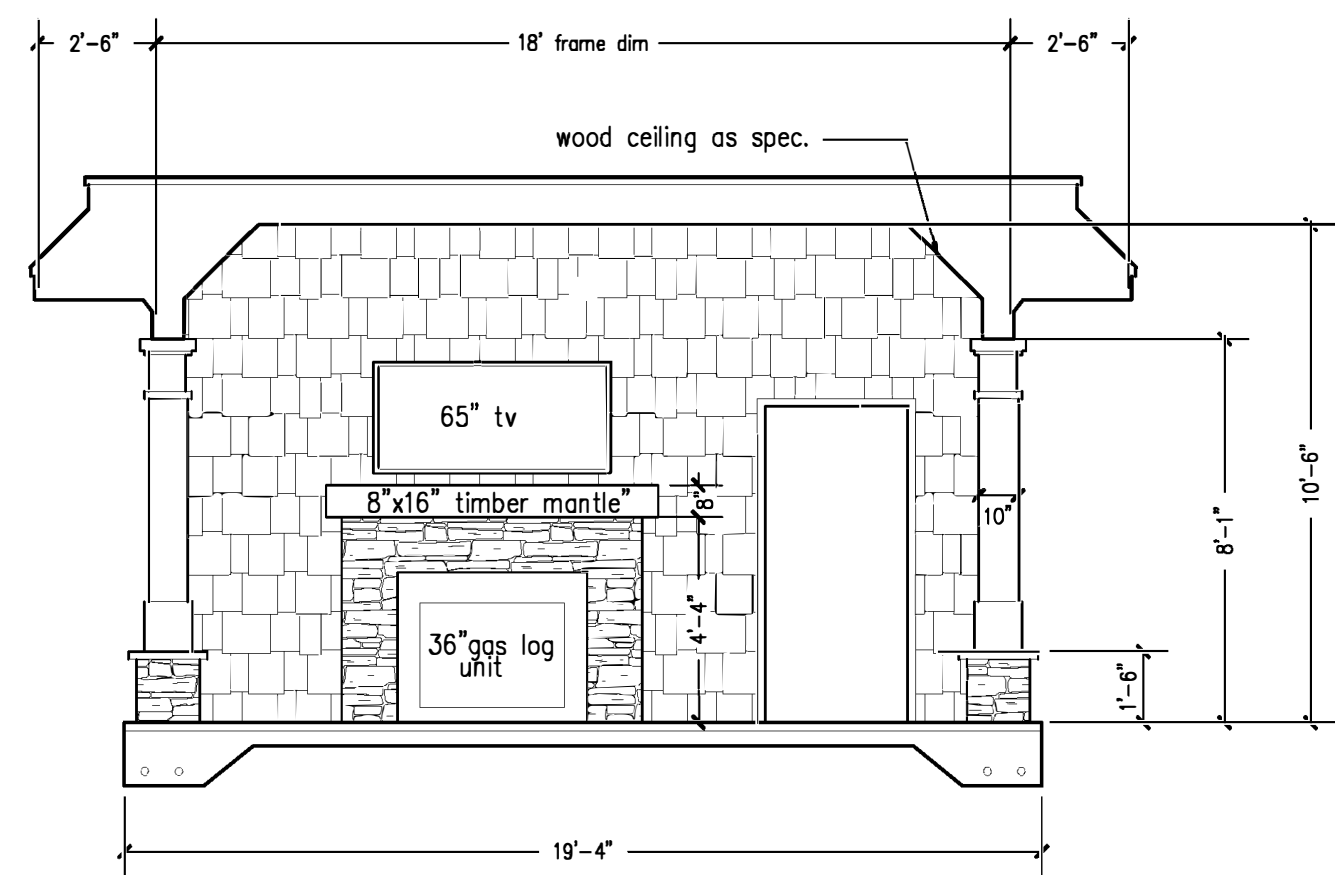
PROJECT NO.:	HAMLET
DRAWN BY:	RWL
DATE:	12/17/21
REVISIONS:	



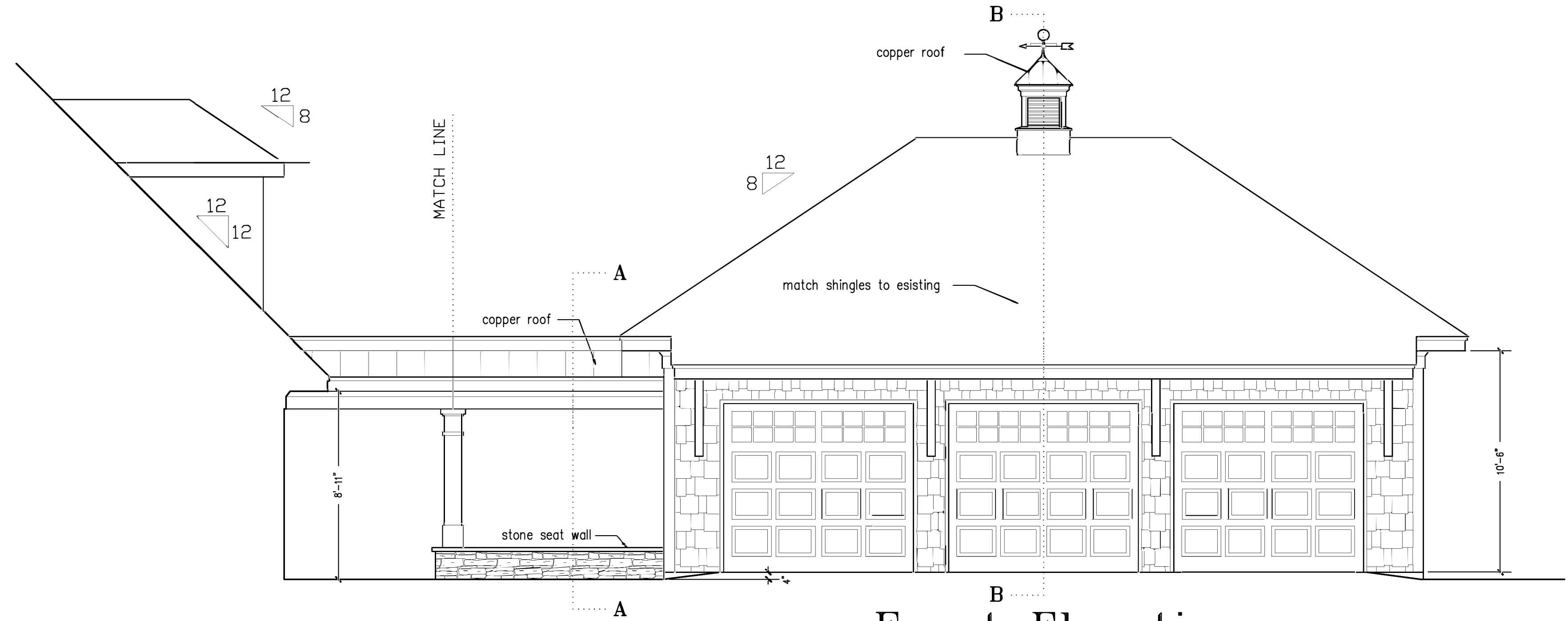
Side Elevation
SCALE: 1/4"=1'-0"



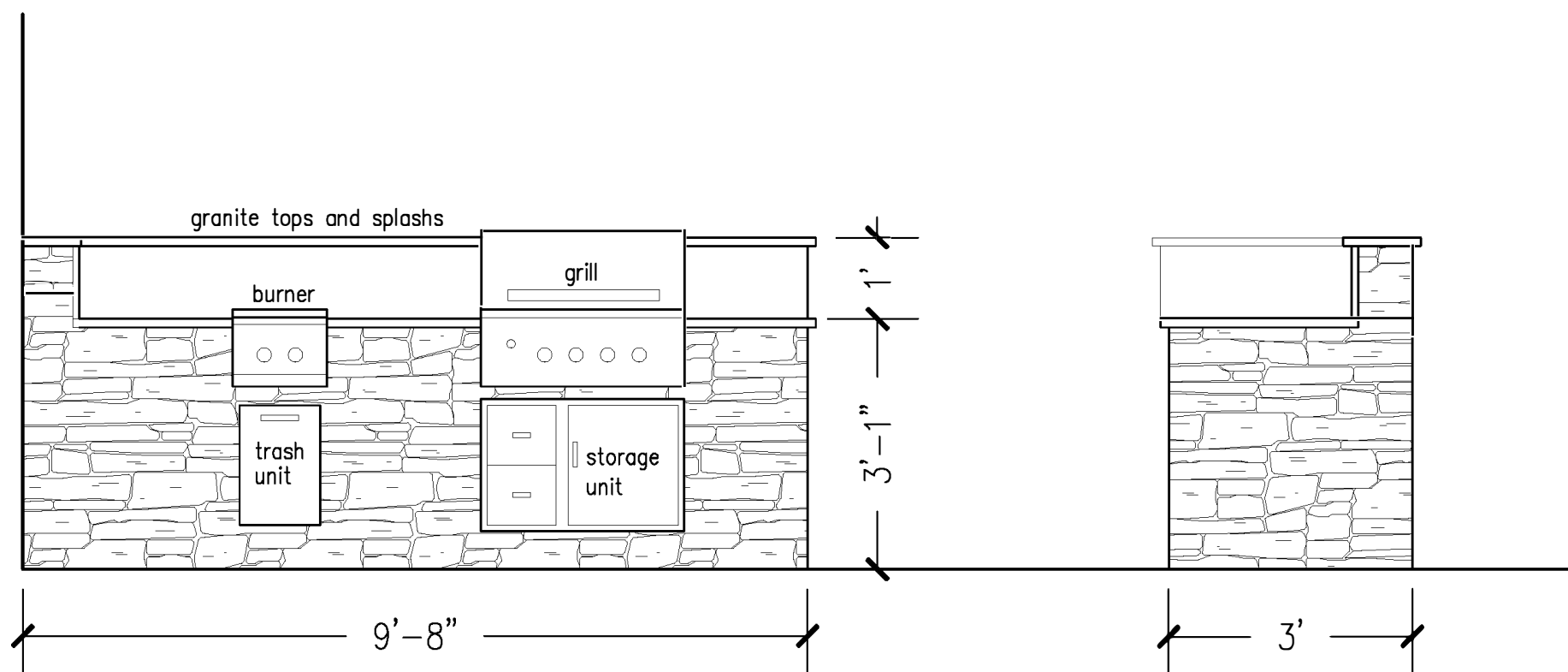
Rear Elevation
SCALE: 1/4"=1'-0"



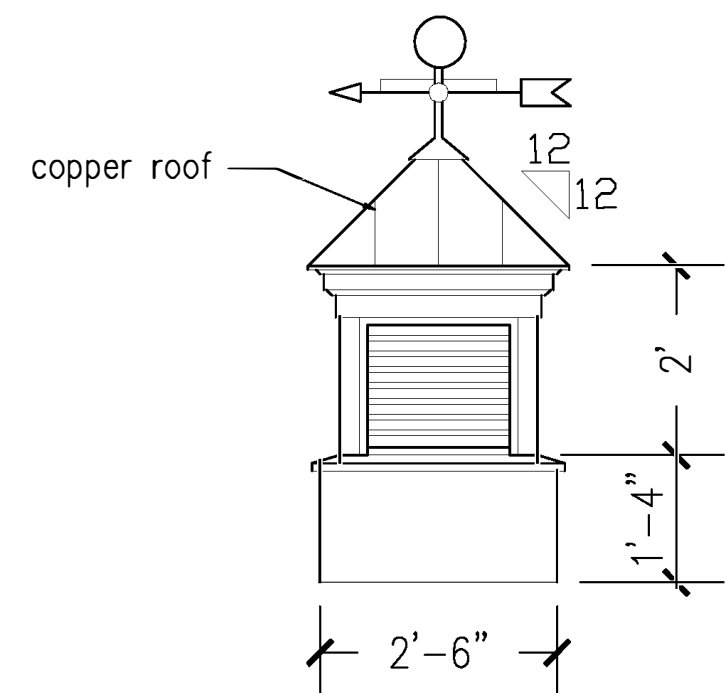
Section A-A
SCALE: 1/4"=1'-0"



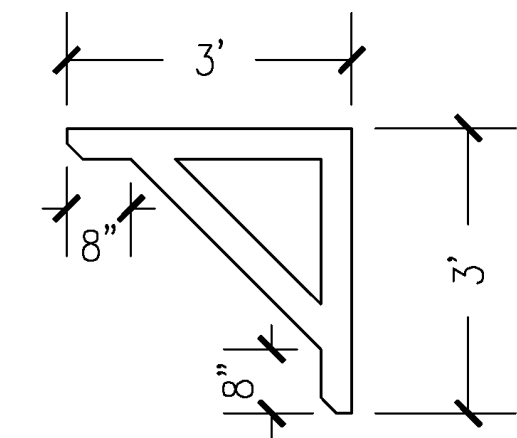
Front Elevation
SCALE: 1/4"=1'-0"



Kitchen Detail
SCALE: 1/2"=1'-0"



Cupola Detail
SCALE: 1/2"=1'-0"



Bracket Detail
SCALE: 1/2"=1'-0"



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444

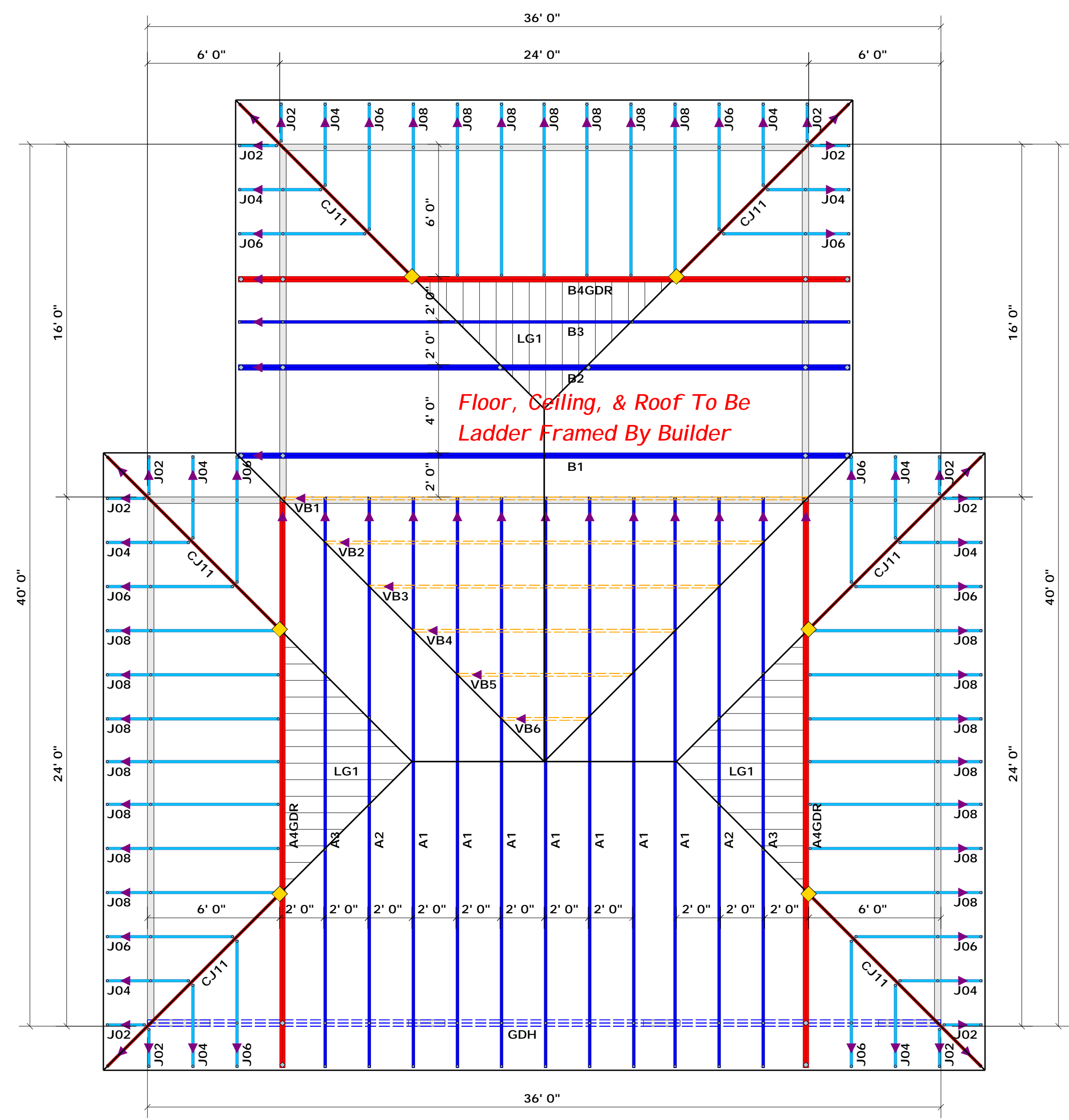
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Curtis Quick
 Curtis Quick

LOAD CHART FOR JACK STUDS

(BASED ON TABLES ROU511.1 & 12)

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADERS/GIRDERS		NUMBER OF JACK STUDS REQUIRED @ EA END OF JOIST HEADS	
END REACTION (IP TO)	REQ'D STUDS FOR JOIST HEADS	END REACTION (IP TO)	REQ'D STUDS FOR JOIST HEADS
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		



Floor, Ceiling, & Roof To Be Ladder Framed By Builder

▲ = Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

HANGER LEGEND

	= USP HJC26 / Hip Hanger
--	--------------------------

Truss Placement Plan
 SCALE: 1/4" = 1'

Beam Legend

PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH	36' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

BUILDER	Wellco Contractors	CITY / CO.	Lillington / Harnett
JOB NAME	Geszler-Hamlet Job	ADDRESS	960 Cummings Rd.
PLAN	Plan	MODEL	Model
SEAL DATE	Seal Date	DATE REV.	12/12/22
QUOTE #	Quote #	DRAWN BY	Curtis Quick
JOB #	J1222-6117	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com.



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Curtis Quick
 Curtis Quick

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

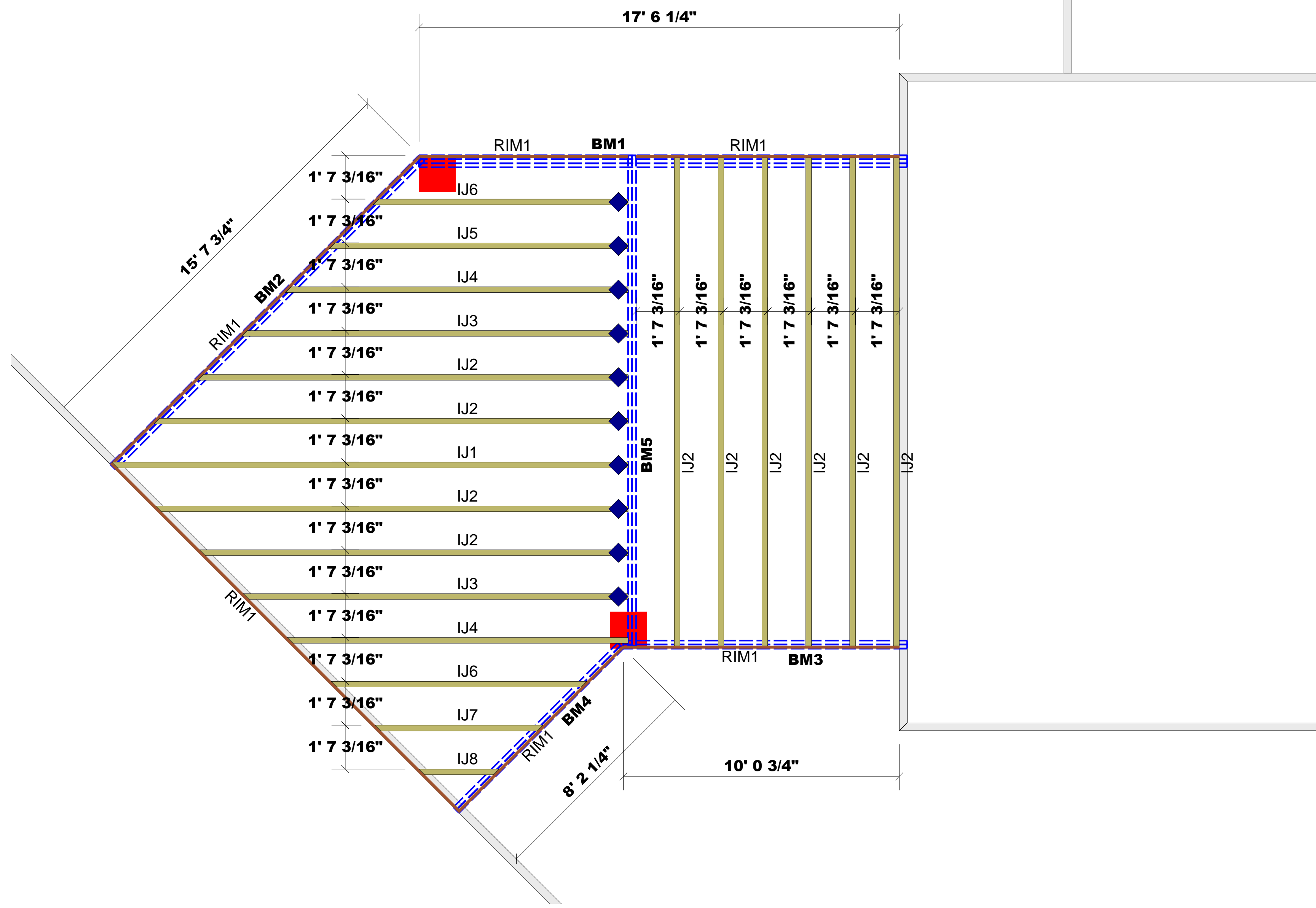
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ. D. STUDS FOR (1) PL. HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (1) PL. HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (1) PL. HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

Wellco Contractors	Lillington / Harnett
Job Name	960 Cummings Rd.
Plan	Model
Seal Date	12/12/22
Quote #	Curtis Quick
Job #	Lenny Norris

BUILDER	Wellco Contractors
JOB NAME	Geszler-Hamlet Job
PLAN	Plan
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J1222-6117

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



I-Joist Legend

PlotID	Length	Product	Plies	Net Qty	Fab Type
IJ1	20' 0"	16" NI-60	1	1	MFD
IJ2	18' 0"	16" NI-60	1	10	MFD
IJ3	16' 0"	16" NI-60	1	2	MFD
IJ4	14' 0"	16" NI-60	1	2	MFD
IJ5	12' 0"	16" NI-60	1	1	MFD
IJ6	10' 0"	16" NI-60	1	2	MFD
IJ7	8' 0"	16" NI-60	1	1	MFD
IJ8	4' 0"	16" NI-60	1	1	MFD
RIM1	12' 0"	1 1/8" x 16" Rim Board	1	6	FF
		Web Stiffeners (16" NI-60)	1	20	Other

HANGER LEGEND

	= USP IHF2516 / I-Joist Hanger
--	--------------------------------

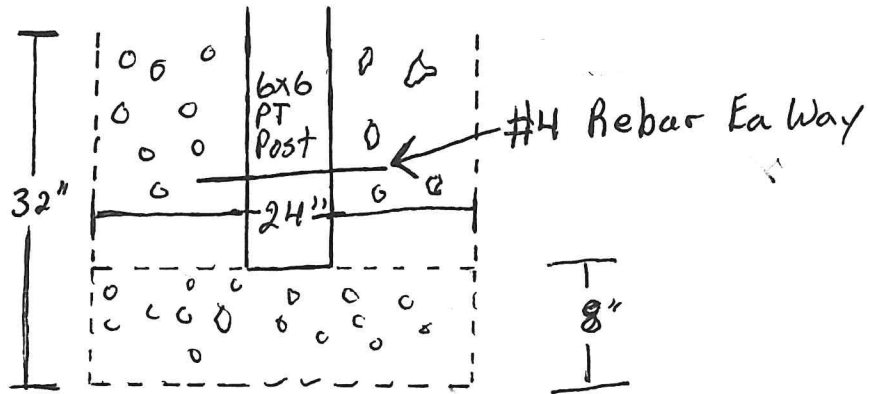
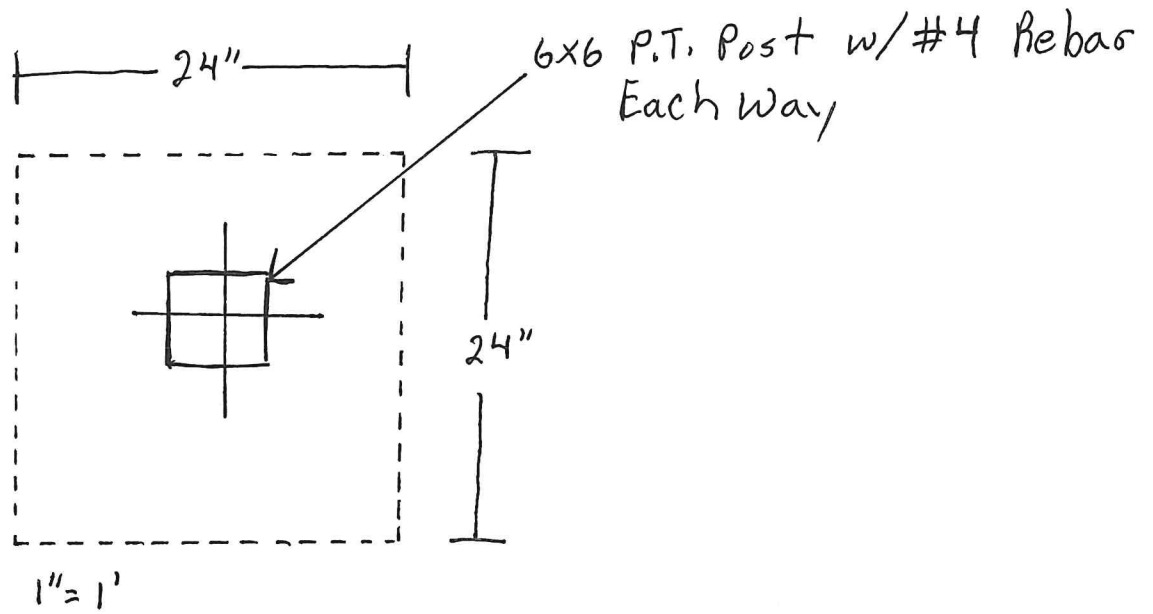
Beam Legend

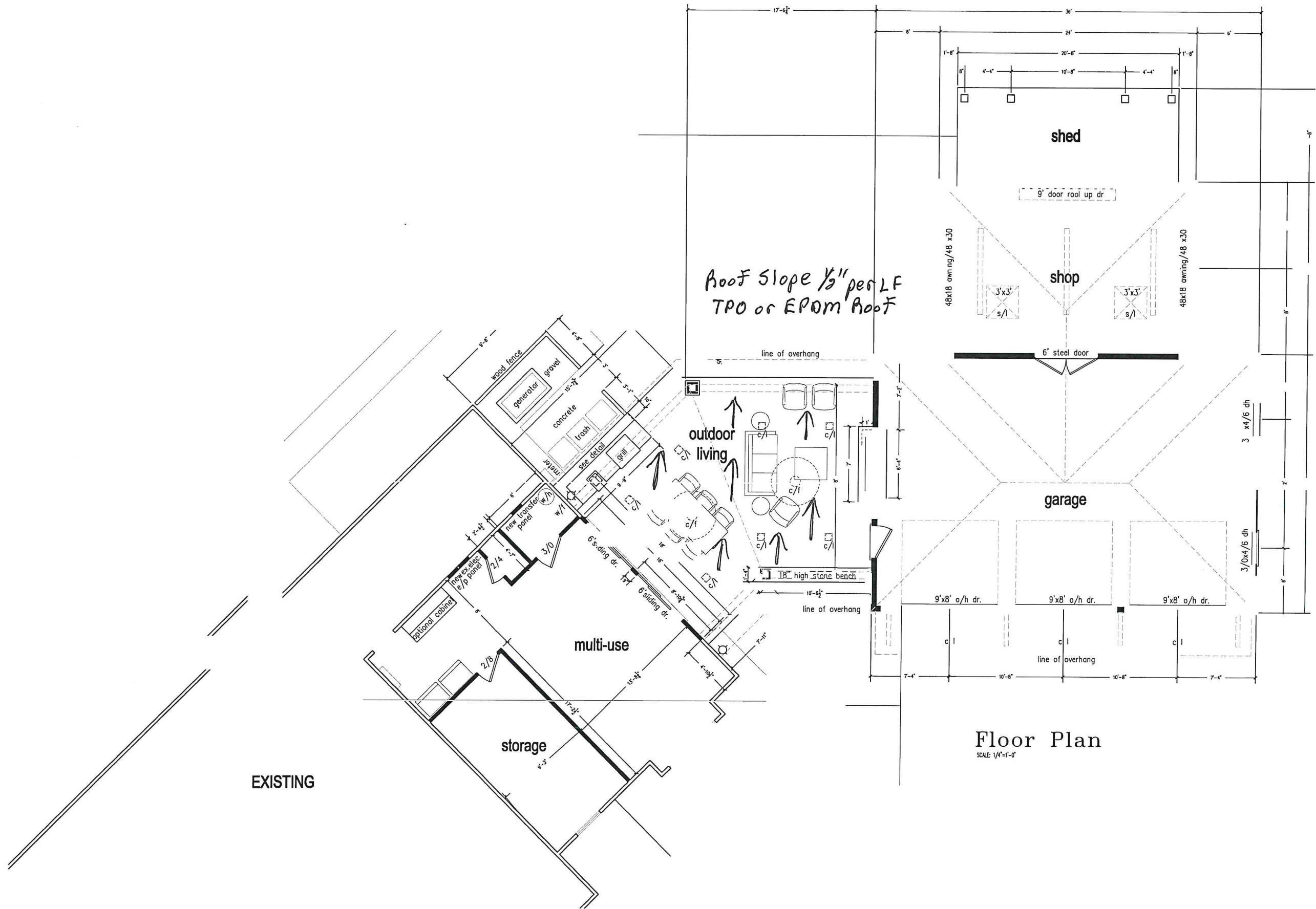
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM5	18' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM1	18' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2	16' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM4	9' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF

Truss Placement Plan
 SCALE: 3/8" = 1'

960 Cummings Rd.

6x6 Post Footing Detail



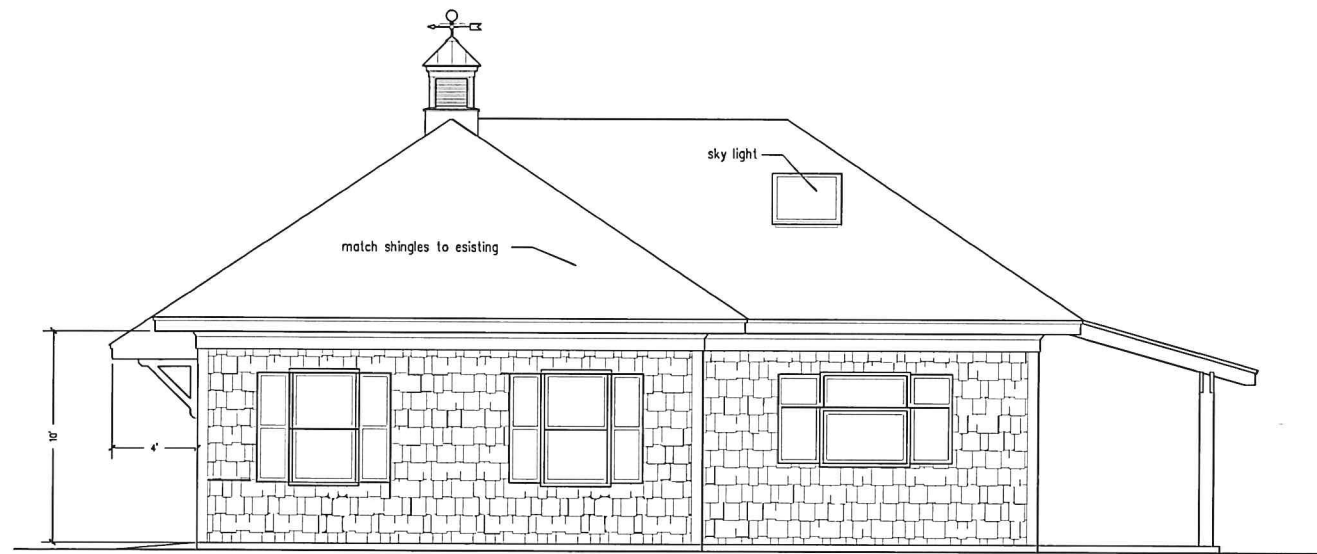


Floor Plan
SCALE: 1/4"=1'-0"

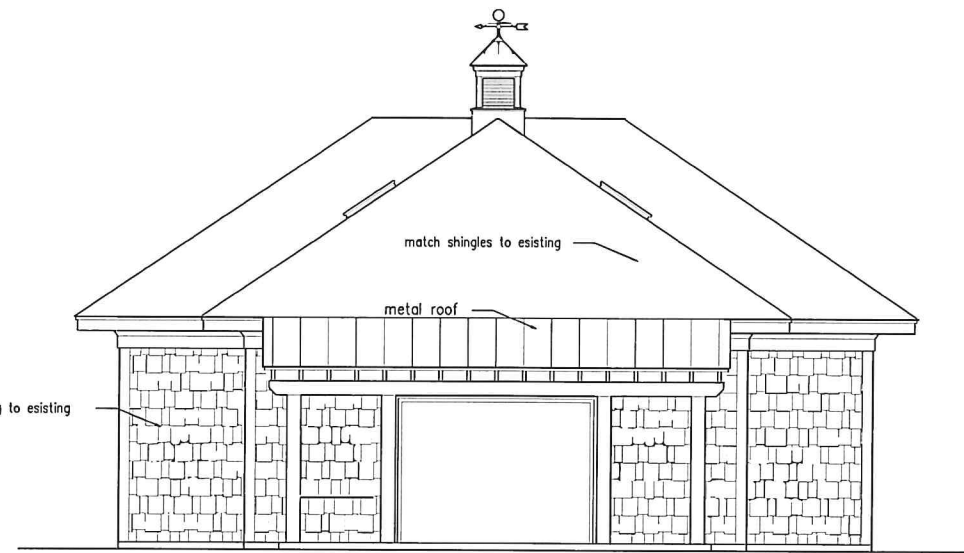


PROJECT N	HAMLET
DRAWN BY:	RWL
DATE:	12/17/21
REVISIONS:	

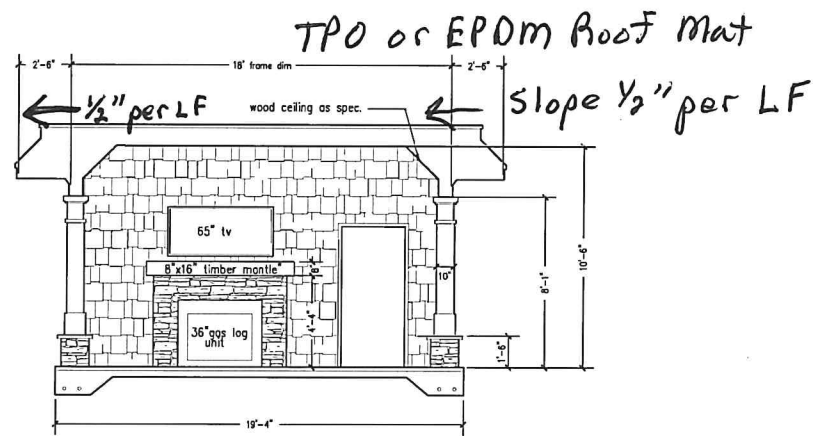
GESZLER-HAMLET
960 CUMMINGS ROAD
LILLINGTON N C



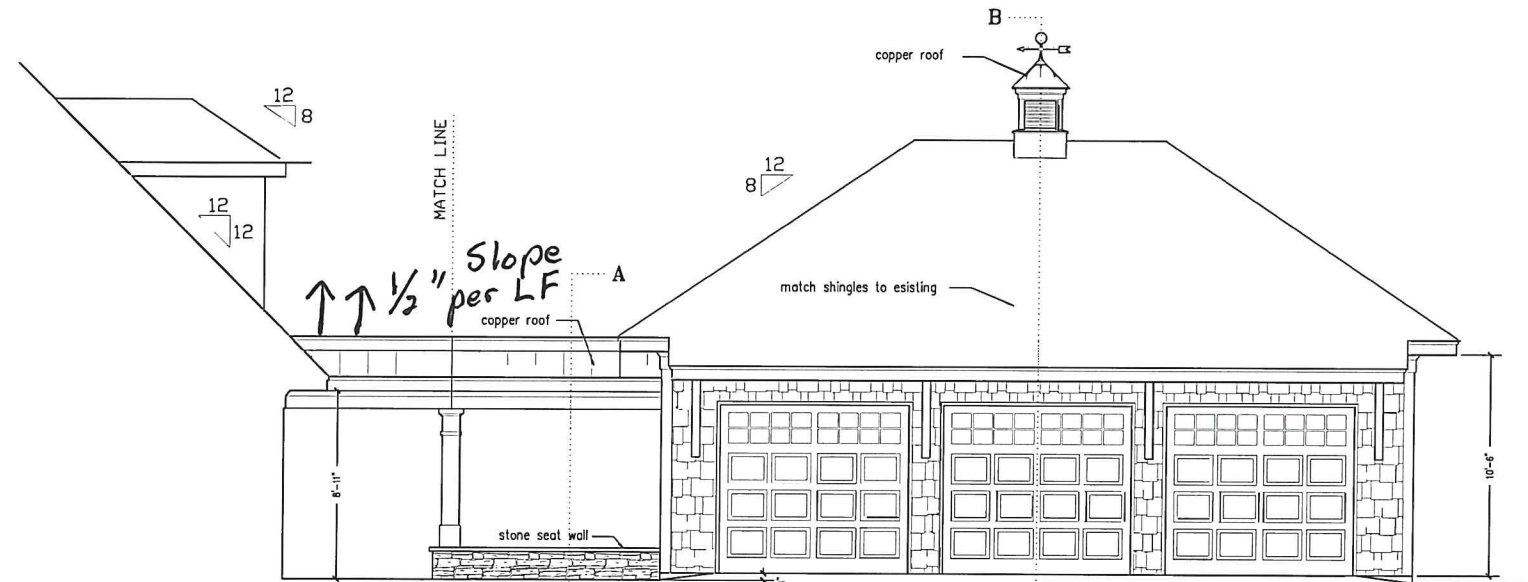
Side Elevation
SCALE: 1/4"=1'-0"



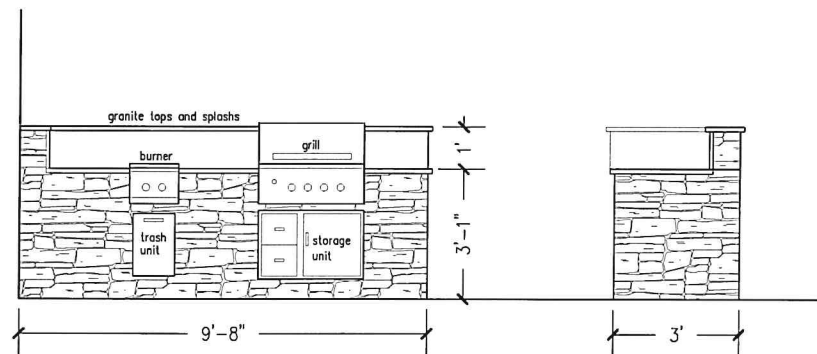
Rear Elevation
SCALE: 1/4"=1'-0"



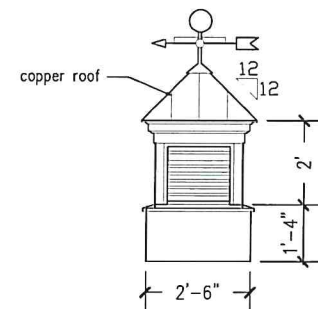
Section A-A
SCALE: 1/4"=1'-0"



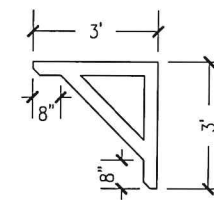
Front Elevation
SCALE: 1/4"=1'-0"



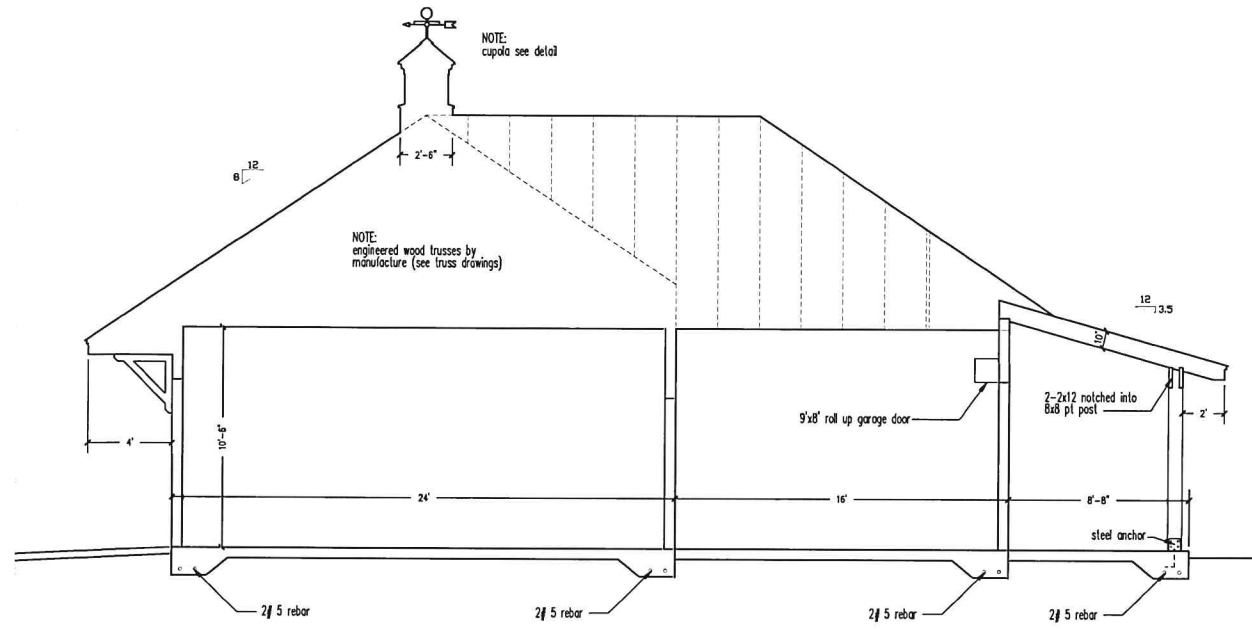
Kitchen Detail
SCALE: 1/2"=1'-0"



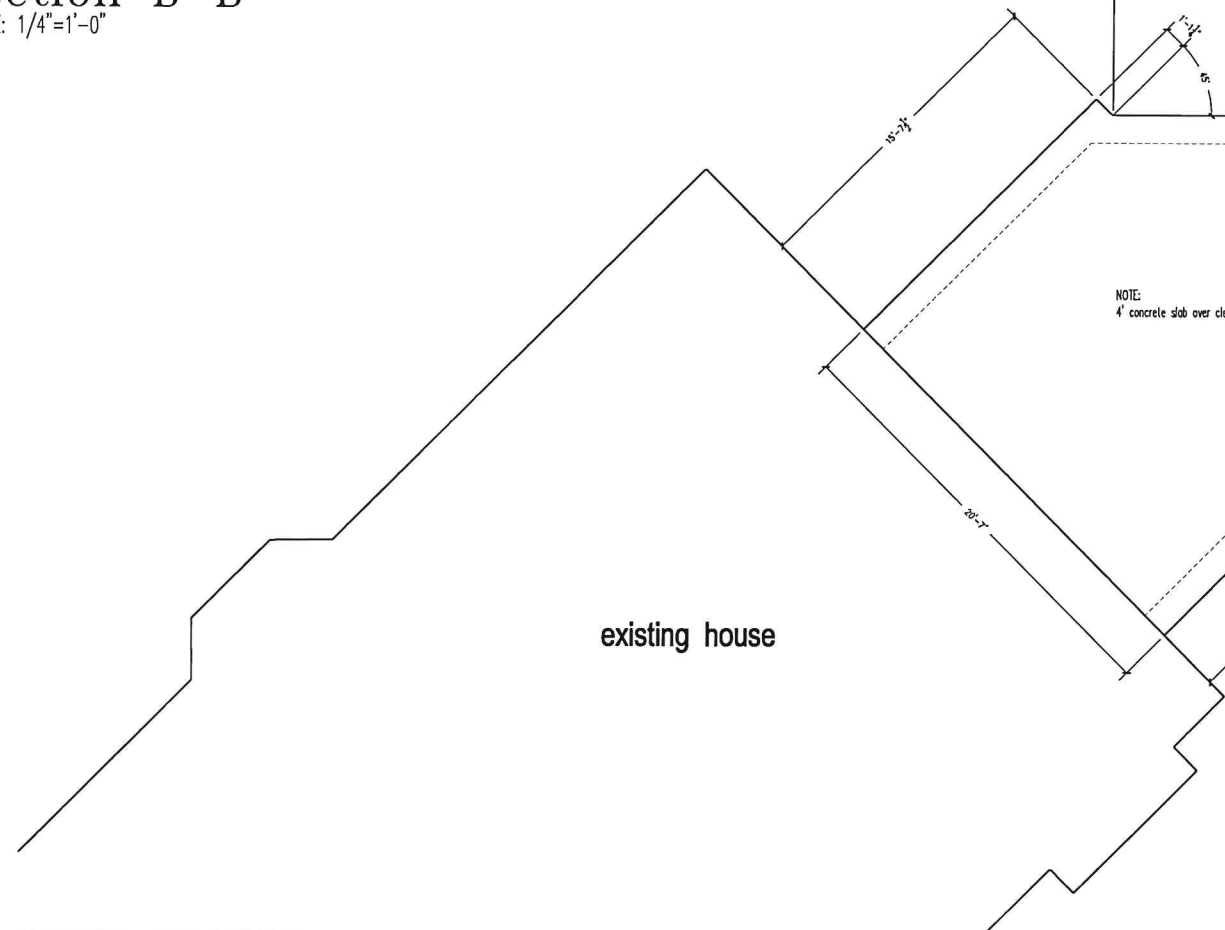
Cupola Detail
SCALE: 1/2"=1'-0"



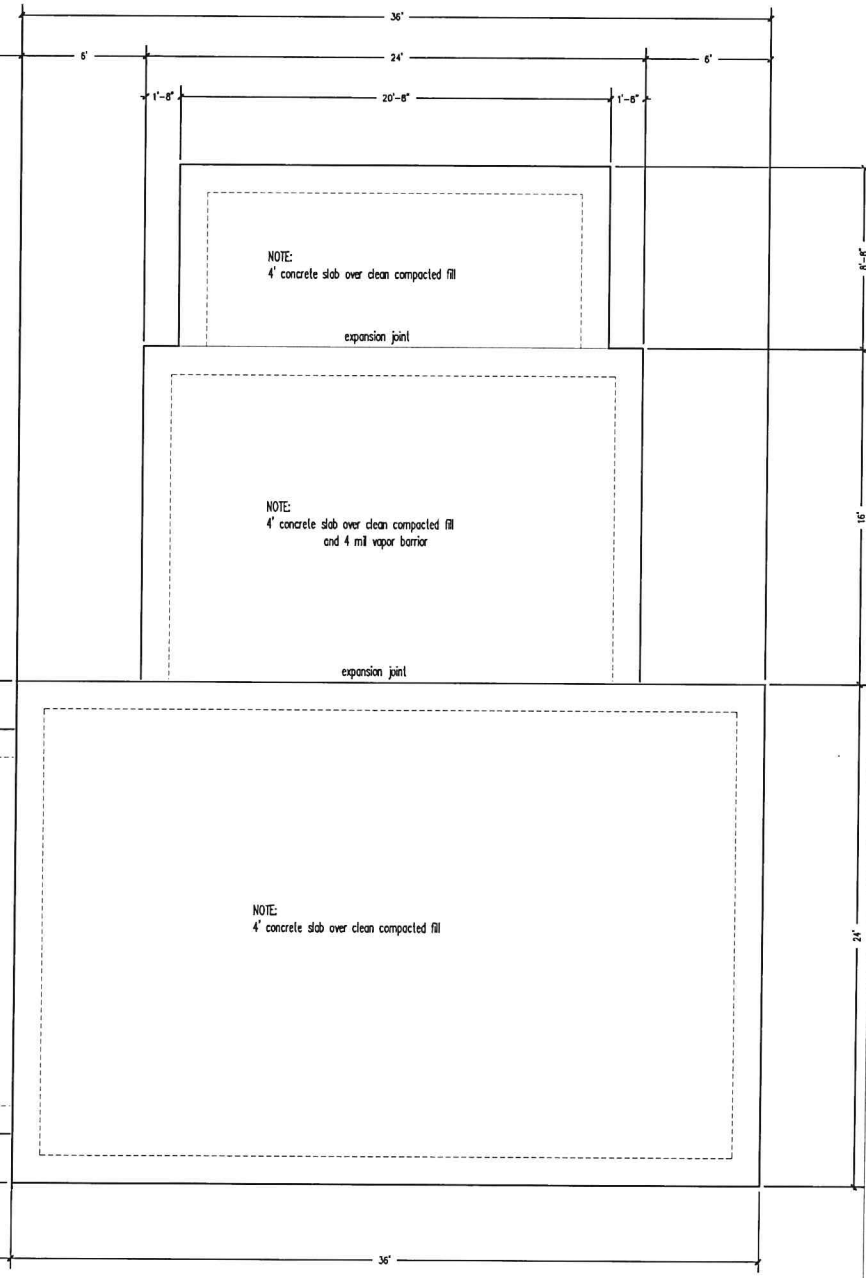
Bracket Detail
SCALE: 1/2"=1'-0"



Section B-B
SCALE: 1/4"=1'-0"



Foundation Plan
SCALE: 1/4"=1'-0"



DESIGN CONSULTANTS
717 HAY STREET FAYETTEVILLE NC 28301

Leath
LEATH ASSOCIATES

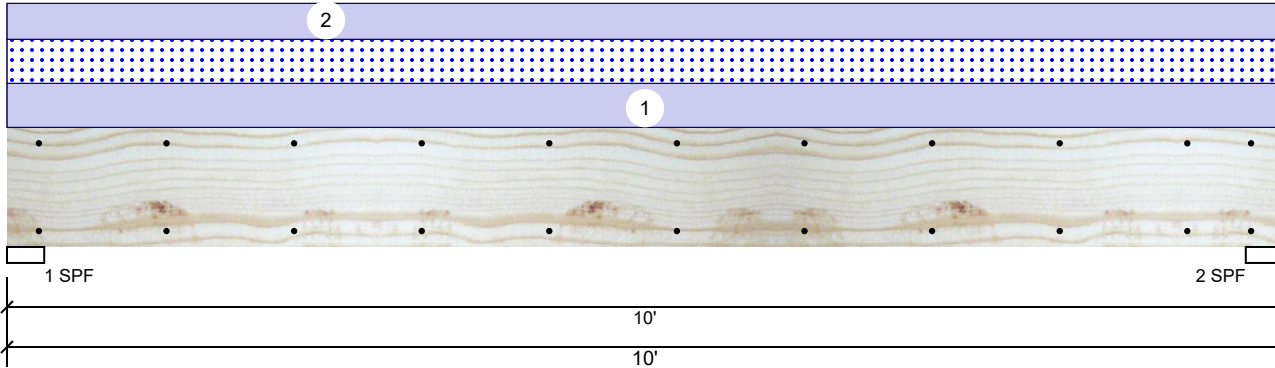
PROJECT NO:	HAMLET
DRAWN BY:	RWL
DATE:	12/17/21
REVISIONS:	

GESZLER-HAMLET
960 CUMMINGS ROAD
LILLINGTON N C

SHEET NO:
3

Shop Door S-P-F #2 2.000" X 12.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1397	530	0	0
2	Vertical	0	1397	530	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	43%	1397 / 530	1927	L	D+S
2 - SPF	3.500"	Vert	43%	1397 / 530	1927	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4385 ft-lb	5'	5306 ft-lb	0.826 (83%)	D+S	L
Unbraced	4385 ft-lb	5'	4387 ft-lb	1.000 (100%)	D+S	L
Shear	1453 lb	1'2 3/4"	3493 lb	0.416 (42%)	D+S	L
LL Defl inch	0.040 (L/2886)	5'	0.239 (L/480)	0.166 (17%)	S	L
TL Defl inch	0.144 (L/794)	5'	0.318 (L/360)	0.453 (45%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 6'8 15/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	106 PLF	0 PLF	106 PLF	0 PLF	0 PLF	J08
2	Tie-In Far	0-0-0 to 10-0-0	4-4-0	Top	40 PSF	0 PSF	0 PSF	0 PSF	0 PSF	Shed
2	Tie-In Near	0-0-0 to 10-0-0	0-0-0	Top	40 PSF	0 PSF	0 PSF	0 PSF	0 PSF	Shed

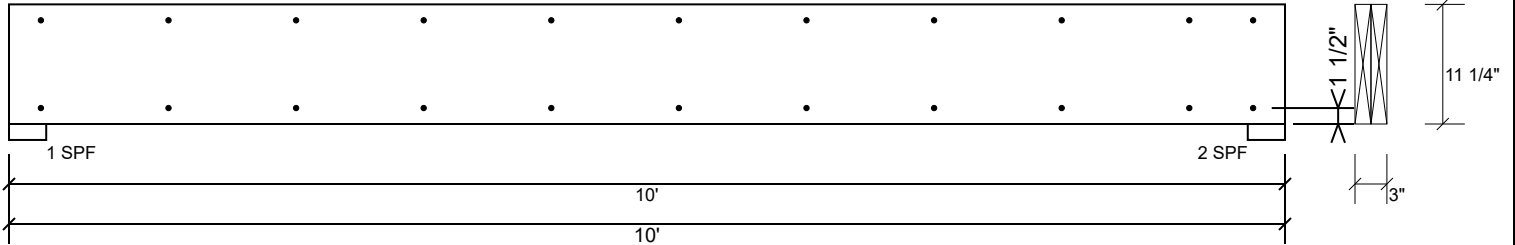
Manufacturer Info

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 11/3/2024

Shop Door S-P-F #2 2.000" X 12.000" 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	157.4 PLF
Yield Limit per Fastener	78.7 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
--------------------------	--



This design is valid until 11/3/2024

Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	/ /	ORDER #	J1222-6117
ORDER DATE	12/12/22	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000006558
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Jason Wellons	INVOICE #	
COUNTY	Harnett	TERMS	Net 10 Days
SUPERINTENDANT	Jason Wellons	SALES REP	Lenny Norris
JOBSITE PHONE #	(910) 263-0276	SALES AREA	Curtis Quick

SOLD TO	Wellco Contractors, Inc. PO Box 766 Spring Lake, NC 28390 (910) 436-3131	JOB NAME: Geszler-Hamlet Job	LOT #	SUBDIV:
	Wellco Contractors 960 Cummings Rd. Lillington, NC	MODEL:	TAG:	JOB CATEGORY: WCall - Will Call
SHIPP TO		DELIVERY INSTRUCTIONS:		
		SPECIAL INSTRUCTIONS:		
PLAN SEAL DATE:				
BY DATE				

BUILDING DEPARTMENT	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	/ /
Roof Order	END CUT	RETURN				LAYOUT	CQ 12/12/22
	PLUMB		GABLE STUDS	24 IN. OC	JOBSITE 1	JOBSITE 1	CUTTING CQ 12/12/22

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
20.0,10.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS			
		PLY	TOP			BOT	TOP	BOT	LEFT		RIGHT		
	7	8.00	0.00	COMMON A1	25-10-08 25-10-08	2 X 6	2 X 6			Joint 8 1108.6 lbs. -61.5 lbs.	Joint 12 965.4 lbs. -40.7 lbs.		
	2	8.00	0.00	HIP A2	25-10-08 25-10-08	2 X 6	2 X 6			Joint 7 1097.7 lbs. -54.1 lbs.	Joint 10 951.0 lbs. -33.6 lbs.		
	2	8.00	0.00	HIP A3	25-10-08 25-10-08	2 X 6	2 X 6			Joint 6 1108.6 lbs. -44.6 lbs.	Joint 10 940.2 lbs. -23.5 lbs.		
	2 2 Ply	8.00	0.00	HIP GIRDER A4GDR	25-10-08 25-10-08	2 X 6	2 X 6			Joint 7 1744.2 lbs. -514.5 lbs.	Joint 12 1590.8 lbs. -501.5 lbs.		
	1 2 Ply	8.00	0.00	COMMON B1	27-09-00 27-09-00	2 X 6	2 X 6			Joint 8 1110.0 lbs. -62.4 lbs.	Joint 12 1110.0 lbs. -62.4 lbs.		
	1 2 Ply	8.00	0.00	HIP B2	27-09-00 27-09-00	2 X 6	2 X 6			Joint 7 1665.0 lbs. -83.0 lbs.	Joint 10 1665.0 lbs. -83.0 lbs.		
	1	8.00	0.00	HIP B3	27-09-00 27-09-00	2 X 6	2 X 6			Joint 7 1110.0 lbs. -45.8 lbs.	Joint 11 1110.0 lbs. -45.8 lbs.		
	1 2 Ply	8.00	0.00	HIP GIRDER B4GDR	27-09-00 27-09-00	2 X 6	2 X 6			Joint 1 148.1 lbs. -4.8 lbs.	Joint 3 148.1 lbs. -4.8 lbs.	Joint 8 1777.7 lbs. -519.6 lbs.	Joint 12 1776.9 lbs. -519.9 lbs.
	6	5.66	0.00	DIAGONAL HIP CJ11	11-00-02 11-00-02	2 X 6	2 X 6			Joint 6 249.9 lbs. -221.2 lbs.	Joint 7 509.9 lbs. -205.7 lbs.		
	12	8.00	0.00	JACK-OPEN J02	01-09-07 01-09-07	2 X 4	2 X 6			Joint 1 70.5 lbs. 8.6 lbs.	Joint 2 59.1 lbs. -34.3 lbs.	Joint 3 35.2 lbs. 10.6 lbs.	

Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	/ /	ORDER #	J1222-6117
ORDER DATE	12/12/22	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000006558
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Jason Wellons	INVOICE #	
COUNTY	Harnett	TERMS	Net 10 Days
SUPERINTENDANT	Jason Wellons	SALES REP	Lenny Norris
JOBSITE PHONE #	(910) 263-0276	SALES AREA	Curtis Quick

S O L D T O	Wellco Contractors, Inc. PO Box 766 Spring Lake, NC 28390 (910) 436-3131	JOB NAME: Geszler-Hamlet Job	LOT #	SUBDIV:
	Wellco Contractors 960 Cummings Rd. Lillington, NC	MODEL:	TAG:	JOB CATEGORY: WCall - Will Call
S H I P T O	DELIVERY INSTRUCTIONS:			
	SPECIAL INSTRUCTIONS:			
PLAN SEAL DATE:				
BY DATE				

BUILDING DEPARTMENT	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	/ /
Roof Order	END CUT	RETURN				LAYOUT	CQ 12/12/22
	PLUMB		GABLE STUDS	24 IN. OC	JOBSITE 1	JOBSITE 1	CUTTING CQ 12/12/22

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
20.0,10.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS				
		PLY	TOP			BOT	TOP	BOT	LEFT	RIGHT	Joint 1	Joint 3	Joint 4	Joint 5
	12	8.00	0.00	JACK-OPEN J04	03-09-07 03-09-07	2 X 6	2 X 6			Joint 1 117.4 lbs. -16.4 lbs.	Joint 3 10.5 lbs. -39.2 lbs.	Joint 4 6.6 lbs. -52.4 lbs.	Joint 5 405.3 lbs. 17.2 lbs.	Joint 19 140.0 lbs. 2.6 lbs.
	12	8.00	0.00	JACK-OPEN J06	05-09-07 05-09-07	2 X 6	2 X 6			Joint 3 102.5 lbs. -71.9 lbs.	Joint 4 53.3 lbs. -4.6 lbs.	Joint 5 353.9 lbs. 27.3 lbs.		
	21	8.00	0.00	JACK-OPEN J08	07-10-08 07-10-08	2 X 6	2 X 6			Joint 3 170.9 lbs. -102.7 lbs.	Joint 4 100.1 lbs. 3.4 lbs.	Joint 5 421.5 lbs. 32.1 lbs.		
	3	14.42	0.00	GABLE LG1	11-10-05 11-10-05	2 X 4	2 X 4			Joint 1 143.5 lbs. -72.6 lbs.	Joint 7 130.0 lbs. -49.0 lbs.	Joint 8 198.9 lbs. -112.7 lbs.	Joint 9 208.8 lbs. -114.5 lbs.	Joint 10 125.7 lbs. 27.2 lbs.
	1	8.00	0.00	VALLEY VB1	23-09-05 23-09-05	2 X 4	2 X 4			Joint 1 156.3 lbs. -21.8 lbs.	Joint 7 127.0 lbs. 14.0 lbs.	Joint 8 337.6 lbs. -95.0 lbs.	Joint 9 455.4 lbs. -103.6 lbs.	Joint 10 459.0 lbs. 62.6 lbs.
	1	8.00	0.00	VALLEY VB2	19-09-05 19-09-05	2 X 4	2 X 4			Joint 1 83.3 lbs. -58.6 lbs.	Joint 7 57.0 lbs. -28.8 lbs.	Joint 8 260.9 lbs. -73.6 lbs.	Joint 9 462.3 lbs. -105.5 lbs.	Joint 11 444.3 lbs. 57.1 lbs.
	1	8.00	0.00	VALLEY VB3	15-09-05 15-09-05	2 X 4	2 X 4			Joint 1 132.4 lbs. -4.6 lbs.	Joint 5 124.3 lbs. 8.8 lbs.	Joint 6 376.1 lbs. -106.4 lbs.	Joint 7 248.0 lbs. 53.0 lbs.	Joint 8 376.3 lbs. -106.6 lbs.
	1	8.00	0.00	VALLEY VB4	11-09-05 11-09-05	2 X 4	2 X 4			Joint 1 54.9 lbs. -39.7 lbs.	Joint 5 37.6 lbs. -22.3 lbs.	Joint 6 314.7 lbs. -94.0 lbs.	Joint 7 263.5 lbs. 34.4 lbs.	Joint 8 315.0 lbs. -94.2 lbs.
	1	8.00	0.00	VALLEY VB5	07-09-05 07-09-05	2 X 4	2 X 4			Joint 1 149.6 lbs. -23.6 lbs.	Joint 3 149.7 lbs. -29.0 lbs.	Joint 4 251.2 lbs. 18.0 lbs.		
	1	8.00	0.00	VALLEY VB6	03-09-05 03-09-05	2 X 4	2 X 4			Joint 1 115.3 lbs. -6.1 lbs.	Joint 3 115.3 lbs. -6.1 lbs.			

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
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Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	/ /	ORDER #	J1222-6117
ORDER DATE	12/12/22	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000006558
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Jason Wellons	INVOICE #	
COUNTY	Harnett	TERMS	Net 10 Days
SUPERINTENDANT	Jason Wellons	SALES REP	Lenny Norris
JOBSITE PHONE #	(910) 263-0276	SALES AREA	Curtis Quick

SOLD TO	Wellco Contractors, Inc. PO Box 766 Spring Lake, NC 28390 (910) 436-3131	JOB NAME: Geszler-Hamlet Job		LOT #	SUBDIV:
	Wellco Contractors 960 Cummings Rd. Lillington, NC	MODEL:	TAG:	JOB CATEGORY: WCall - Will Call	
SHIP TO	DELIVERY INSTRUCTIONS:				
	SPECIAL INSTRUCTIONS:				
PLAN SEAL DATE:					
BY DATE					

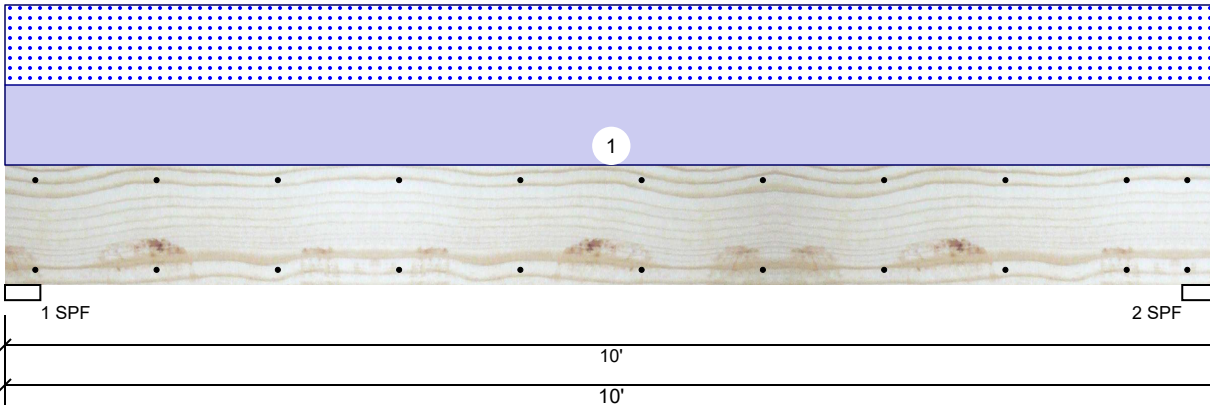
BUILDING DEPARTMENT	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	/ /
Roof Order	END CUT	RETURN				LAYOUT	CQ 12/12/22
	PLUMB		GABLE STUDS	24 IN. OC	JOBSITE 1	CUTTING	CQ 12/12/22

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
6	Hangers, USP	HJC26			SIMPSON (THJU26)
2	LVL Beams (Sized)	LVL, 1-3/4" x 11-7/8" (S)	36-00-00		GDH

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2226	2180	0	0
2	Vertical	0	2226	2180	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	85%	2226 / 2180	4406	L	D+S
2 - SPF	3.500"	Vert	85%	2226 / 2180	4406	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10029 ft-lb	5'	22897 ft-lb	0.438 (44%)	D+S	L
Unbraced	10029 ft-lb	5'	10030 ft-lb	1.000 (100%)	D+S	L
Shear	3286 lb	8'8 5/8"	10197 lb	0.322 (32%)	D+S	L
LL Defl inch	0.097 (L/1180)	5'	0.239 (L/480)	0.407 (41%)	S	L
TL Defl inch	0.196 (L/584)	5'	0.318 (L/360)	0.616 (62%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 9'2" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	436 PLF	0 PLF	436 PLF	0 PLF	0 PLF	"A" Trusses
	Self Weight				9 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

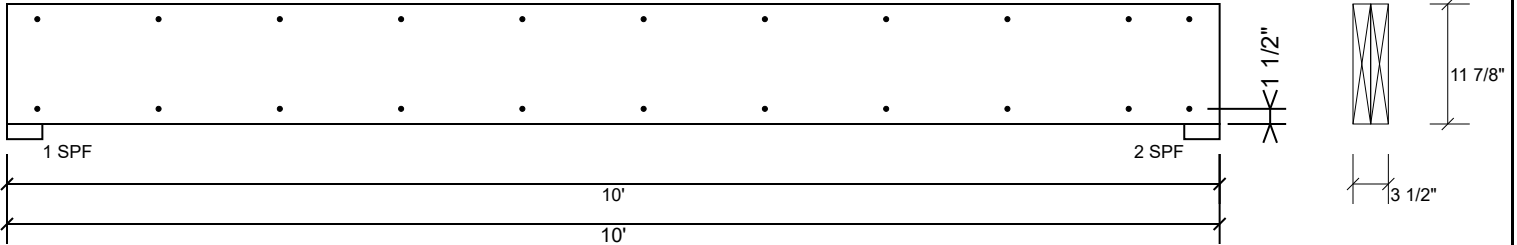
Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

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